<u>GENERAL</u> . THESE DRAWINGS A						J.
FOR USE ONLY ON		D INSTRUMENT	S OF SERVICE	OF HOHB	ACH-LEWIN, INC.	
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	FOUNDATION IS BA REPARED BY: <u>ROC</u> : 20-1808			SEOTECHN	ICAL	D.
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SACKED TO A UNIFORM FINISH. YES - PLASTIC OR METAL (NON-ALUMINUM) CONDUITS MAY BE EMBEDDED IN THE SLAB ROVIDED THAT THE FOLLOWING CRITERIA ARE MET: NO PIPES OR CONDUITS, OTHER THAN ELECTRICAL, SHALL BE EMBEDDED IN STRUCTURAL CONCRETE THE MAXIMUM CONDUIT SIZE SHALL BE 1 3/4 INCH OUTSIDE DIAMETER CONDUITS TO BE LIMITED TO TEN (10) CONDUITS EVERY TEN (10) FEET 4. LOCATE CONDUITS WITHIN THE MIDDLE THIRD OF THE SLAB THICKNESS 5. PROVIDE A MINIMUM OF SIX (6) INCHES CLEAR SPACING BETWEEN ADJACENT CONDUIT 6. NO CONDUIT SHALL BE LOCATED WITHIN COLUMN DROP CAPS OR BETWEEN STUD RAILS OR WITHIN A THREE (3) FOOT RADIUS OF A COLUMN. 7. AVOID INTERSECTING CONDUITS WHERE POSSIBLE. INTERSECTING CONDUITS ARE TO BE BE INSTALLED PERPENDICULAR TO EACH OTHER. NO MORE THAN TWO (2) CONDUITS PER OINTERSECTION. DO NOT INTERSECT CONDUIT AT POST-TENSIONING TENDON OR DO REINFORCEMENT INTERSECTIONS. 8. IF DENSER AMOUNTS OF CONDUIT OCCURS, CONTACT THE ENGINEER FOR ASSISTANCE. SEE DETAILS FOR ALLOWABLE CONDUIT GROUPS AND LAYOUTS 9. DO NOT COIL EXCESS CONDUITS IN SLAB 10. IN PARKING GARAGE SLABS, EMBEDDED ELECTRICAL CONDUIT SHALL BE LIMITED TO SERVICE LIGHTING NETRATIONS - PENETRATIONS SHALL NOT BE PERMITTED IN BEAMS OR DROP CAPS XCEPT AS SHOWN IN P/T DRAWINGS OR TYPICAL DETAILS. ERTS - ALL INSERTS AND SLEEVES SHALL BE CAST IN PLACE WHENEVER POSSIBLE. ALLED AND POWER-DRIVEN FASTENERS WILL BE PERMITTED ONLY WHEN IT CAN BE OWN THAT THE INSERTS WILL NOT SPALL THE CONCRETE AND ARE LOCATED TO AVOID E TENDONS AND ANCHORAGES. THE CONTRACTOR MUST LOCATE TENDONS ON THE RFACE SLAB. DNSTRUCTION JOINTS: 1. CONSTRUCTION JOINTS SHOWN MAY BE PROVIDED AT CONTRACTORS OPTION. ANY PROPOSED CONSTRUCTION JOINTS NOT SHOWN MUST BE SUBMITTED TO THE DESIGN PROFESSIONAL OF RECORD FOR APPROVAL. 2. ROUGHENED CONSTRUCTION JOINTS (R.C.J.): WHERE NOTED ON DRAWINGS R.C.J. ROUGHEN JOINT TO MINIMUM 1/4 INCH AMPLITUDE. ERIOR SLAB ON GRADE: 1. DO NOT ALLOW WATER TO COLLECT ON OR AROUND BUILDING PAD. 2. INITIAL CURING: INITIAL CURING SHALL IMMEDIATELY FOLLOW THE FINISHING OPERATION. CONCRETE SHALL BE KEPT CONTINUOUSLY MOIST AT LEAST OVERNIGHT. 3. FINAL CURING: IMMEDIATELY FOLLOWING THE INITIAL CURING AND BEFORE THE CONCRETE HAS DRIED, SLABS TO BE CONTINUOUSLY CURED FOR 7 DAYS BY WET COVERING OR MOISTURE RETAINING COVERING TO REDUCE THE LIKELIHOOD OF SHRINKAGE OR CRACKING. LIQUID MEMBRANE CURING COMPOUNDS SHALL NOT BE PERMITTED (WITHOUT OWNER'S WRITTEN APPROVAL). 4. INTERIOR SLABS SHALL RECEIVE A LIGHT BROOM FINISH U.O.N. TOLERANCE SHALL BE 1/8" IN 10'-0". EDGES SHALL BE SMOOTH TROWELED. _ CONC. TO BE REINFORCED UNLESS SPECIFICALLY MARKED "NOT REINFORCED" POR BARRIER: 1. 15 MIL ASTM E-1745 CLASS A, TYP. U.O.N. DTCRETE NOTES OTCRETE SHALL CONFORM TO THE REQUIREMENTS FOR REINFORCED CONCRETE AND OVISIONS OF ACI 506. THE MIXTURE PROPORTIONS SHALL BE PER ACI 506R-16 SECTION 2.5. OVIDE PRECONSTRUCTION TESTS PER CBC §1908.5 FOR ALL BARS GREATER THAN NO.5 R CLEARANCES: FOR NO. 5 A MINIMUM CLEARANCE BETWEEN PARALLEL REINFORCEMENT RS OF 2 1/2-IN. FOR LARGER BARS, A MINIMUM CLEARANCE BETWEEN PARALLEL BARS UAL TO SIX DIAMETERS OF BARS SHALL BE USED. AT THE TWO CURTAINS OF STEEL NDITIONS, THE CURTAIN NEARER THE NOZZLE SHALL HAVE A MINIMUM SPACING EQUAL TO 12 IR DIAMETERS AND THE BACK CURTAIN SHALL HAVE A MINIMUM SPACING OF SIX BAR AMETERS. DOUBLING BARS MAY BE REQUIRED IF TIGHTER SPACING IS SPECIFIED ON PLANS WEVER MINIMUM PARALLEL SPACING WOULD BE STILL BE REQUIRED. P SPLICES OF REINFORCING BARS SHALL UTILIZE THE NONCONTACT LAP SPLICE METHOD WITH MINIMUM CLEARANCE OF 2 INCHES (51 mm) BETWEEN BARS. THE USE OF CONTACT LAP LICES NECESSARY FOR SUPPORT OF THE REINFORCING IS PERMITTED WHEN APPROVED BY E BUILDING OFFICIAL, BASED ON SATISFACTORY PRECONSTRUCTION TESTS THAT SHOW THAT PEQUATE ENCASEMENT OF THE BARS WILL BE ACHIEVED, AND PROVIDED THAT THE SPLICE IS LIENTED SO THAT A PLANE THROUGH THE CENTER OF THE SPLICED BARS IS PERPENDICULAR THE SURFACE OF THE SHOTCRETE. EN REQUIRED BY THE BUILDING OFFICIAL A TEST PANEL SHALL BE SHOT, CURED, CORED OR WN, EXAMINED AND TESTED PRIOR TO COMMENCEMENT OF THE PROJECT. THE SAMPLE PANEL ALL BE REPRESENTATIVE OF THE PROJECT AND SIMULATE JOB CONDITIONS AS CLOSELY AS SSIBLE. THE PANEL THICKNESS AND REINFORCING SHALL REPRODUCE THE THICKEST AND OST CONGESTED AREA SPECIFIED IN THE STRUCTURAL DESIGN. IT SHALL BE SHOT AT THE ME ANGLE, USING THE SAME NOZZLEMAN AND WITH THE SAME CONCRETE MIX DESIGN THAT LL BE USED ON THE PROJECT. THE EQUIPMENT USED IN PRECONSTRUCTION TESTING SHALL BE SAME EQUIPMENT USED IN THE WORK REQUIRING SUCH TESTING, UNLESS SUBSTITUTE UIPMENT IS APPROVED BY THE BUILDING OFFICIAL. THE TEST PANEL SHALL HAVE MINIMUM MENSIONS OF 36-IN. X 36-IN.

- OTCRETE. REBOUND SHALL NOT BE USED AS AGGREGATE.
- DRE THAN 30 MINUTES.
- OTCRETE THAT EXHIBITS SAGS, SLOUGHS, SEGREGATION, HONEYCOMBING, SAND POCKETS COTHER OBVIOUS DEFECTS SHALL BE REMOVED AND SHALL NOT BE REUSED.
- RING THE CURING PERIOD SHOTCRETE SHALL BE MAINTAINED ABOVE 40-DEGREES RING COMPOUND.
- IAL CURING SHALL CONTINUE FOR SEVEN DAYS AFTER SHOTCRETING.
- RENGTH TESTS FOR SHOTCRETE SHALL BE MADE BY AN APPROVED AGENCY ON SPECIMENS NCH-DIAMETER CORES.
- KEN AT LEAST ONCE EACH SHIFT, BUT NOT LESS THAN ONE FOR EACH 50 CUBIC YARDS OF OTCRETE.
- ECIFIED CONCRETE , WITH NO SINGLE CORE LESS THAN THAT AMOUNT.
- INFORCING STEEL
- INFORCING STEEL GUALL REAG FOLLOWS.

REINF.	TYPE
BARS/TIES/SPIRALS	ASTM A615, GRADE 60, U.O.N.
WELDED REINF.	ASTM A706, GRADE 60 OR 80 AS NOTED
TIE AND SPIRAL WIRE REINF.	ASTM A1064, GRADE 60
WELDED WIRE REINF.	ASTM A1064, GRADE 60
REINF. USE	TYPE
SLAB-ON-GRADE	ASTM A615, GRADE 60
FOUNDATIONS	ASTM A615, GRADE 80
CONC. COLUMNS	ASTM A706, GRADE 80
POST-TENSIONED SLABS	ASTM A615, GRADE 80
CONVENTIONAL SLABS/BEAMS	ASTM A615, GRADE 80
CONC. SHEAR WALLS *	ASTM A706 OR ASTM A615, GRADE 60
	SED ON MILL TESTS DOES NOT EXCEED FY B' THE ACTUAL TENSILE STRENGTH TO THE ACTU, 5.

- RITING BY ENGINEER OF RECORD.
- E. WELDING TO CONFORM TO AWS D1.4

1. ALL REINFORCING BARS, ANCHOR BOLTS, AND ALL OTHER CONC. INSERTS SHALL BE WELL SECURED IN POSITION PRIOR TO PLACING CONCRETE.

2. CHAMFER ALL CORNERS OF CONCRETE TO PREVENT DAMAGE 3. CONSTRUCTION TOLERANCE SHALL COMPLY TO ACI 117.

CONCRETE.

4. CONCRETE SHALL BE PLACED IN A CONTINUOUS OPERATION BETWEEN PREDETERMINED CONSTRUCTION JOINTS. 5. USE VIBRATORS TO CONSOLIDATE CONCRETE. DO NOT USE VIBRATORS TO MOVE

6. CONCRETE SHALL BE CONTINUOUSLY CURED FOR 7 DAYS AFTER PLACEMENT IN ANY APPROVED MANNER. FOOTINGS ARE EXEMPTED FROM THIS REQUIREMENT. 7. PATCHING OF CONCRETE: ALL INSERT HOLES AND OTHER IMPERFECTIONS ON THE SURFACES OF THE CONCRETE SHALL BE FILLED WITH GROUT, BRUSHED AND

REBOUND OR ACCUMULATED LOOSE AGGREGATE SHALL BE REMOVED FROM THE RFACES TO BE COVERED PRIOR TO PLACING THE INITIAL OR ANY SUCCEEDING LAYERS OF

CEPT WHERE PERMITTED HEREIN, UNFINISHED WORK SHALL NOT BE ALLOWED TO STAND FOR

HRENHEIT AND IN MOIST CONDITION. INITIAL CURING SHALL BE KEPT CONTINUOUSLY MOIST FOR HOURS AFTER SHOTCRETING IS COMPLETE OR SHALL BE SEALED WITH AN APPROVED

AT ARE REPRESENTATIVE OF THE WORK AND WHICH HAVE BEEN WATER SOAKED FOR AT AST 24 HOURS PRIOR TO TESTING. SPECIMEN SHALL CONSIST OF NOT LESS THAN

ECIMENS SHALL BE TAKEN FROM IN-PLACE WORK OR FROM TEST PANELS, AND SHALL BE

E AVERAGE COMPRESSIVE STRENGTH OF THREE CORES FROM THE IN-PLACE WORK OR A IGLE TEST PANEL SHALL EQUAL OR EXCEED 85% THE COMPRESSIVE STRENGTH OF THE

INFORCING STEEL SHALL BE PLACED IN ACCORDANCE WITH ACI 315 AND ACI 318.

MORE THAN IAL YIELD

) NOT FIELD BEND OR STRAIGHTEN IN ANY MANNER THAT WILL DAMAGE REINFORCING. OVIDE SPLICES IN REINFORCING ONLY WHERE SHOWN ON DRAWINGS OR APPROVED IN

1. CONC. CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH . 2. CONC. EXPOSED TO EARTH OR WEATHER WALL PANELS, SLABS, JOISTS . . OTHER MEMBERS 1 1/2" 3. CONC. NOT EXPOSED TO WEATHER NOR IN CONTACT WITH GROUND: . . 3/4" SLABS, WALLS, AND JOISTS BEAMS AND COLUMNS: PRIMARY REINF. . . 1 1/2" TIES, STIRRUPS, SPIRALS . B. REFERENCE STANDARDS - POST-TENSIONED CONCRETE SHALL CONFORM TO ALL REQUIREMENTS OF THE FOLLOWING DOCUMENTS, EXCEPT AS MODIFIED BELOW:

A. MINIMUM REINF. COVER FOR CAST-IN-PLACE CONCRETE (PRESTRESSED):

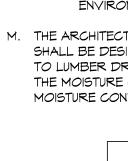
• <u>POST-TENSIONED</u> (SEE CONCRETE NOTES)

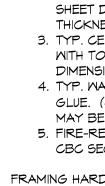
ACI 301 "STANDARD SPECIFICATION FOR STRUCTURAL CONCRETE" ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" PTI "POST TENSIONING MANUAL" PTI "SPECIFICATION FOR UNBONDED SINGLE STRAND TENDONS" PTI "FIELD PROCEDURES MANUAL FOR UNBONDED SINGLE STRAND TENDONS"

- C. FIELD FOREMAN THE FIELD FOREMAN RESPONSIBLE FOR THE PLACEMENT OF ALL POST-TENSIONING SHALL HAVE A MINIMUM OF THREE (3) YEARS EXPERIENCE IN THIS CAPACITY FOR THIS TYPE OF CONSTRUCTION.
- D. P/T STRAND QUALITY ONE SAMPLE OF EACH REEL OR HEAT SHALL BE TESTED BY ANAPPROVED LABORATORY. TEST RESULTS OR MILL CERTIFICATES SHALL BE SUBMITTED TO THE ENGINEER BEFORE STRESSING OF TENDONS. POST-TENSIONING TENDONS SHALL BE LOW-RELAXATION QUALITY, AND SHALL CONFORM TO THE FOLLOWING: SEVEN WIRE STRAND ASTM DESIGNATION . . A-416 1/2 " DIAMETER STRAND AREA . . . 0.153 IN SQ. ULTIMATE STRENGTH 270 KSI
- E. P/T HARDWARE QUALITY ALL ANCHORAGES, COUPLERS AND MISCELLANEOUS HARDWARE SHALL CONFORM TO THE REQUIREMENTS OF AN ENCAPSULATED SYSTEM PER THE PTI "SPECIFICATION."
- F. P/T SHEATHING QUALITY UNBONDED STRANDS SHALL BE ENCASED IN SLIPPAGE SHEATHING THAT SHALL CONSIST OF A 50 MIL. THICK WATERPROOF CONTINUOUSLY EXTRUDED SEAMLESS PLASTIC TUBING CAPABLE OF PREVENTING THE PENETRATION OF MOISTURE AND CEMENT PASTE, AND WILL CONTAIN A RUST-INHIBITING GREASE COATING. TEARS IN THE SHEATHING SHALL BE REPAIRED TO RESTORE THE WATER TIGHTNESS OF THE SHEATHING.
- G. SHOP DRAWINGS THE CONTRACTOR SHALL SUBMIT SHOP DRAWINGS STAMPED BY AN ENGINEER LICENSED IN THE STATE OF CALIFORNIA SHOWING TENDON LAYOUT, DEAD-END AND STRESS-END LOCATIONS AND TENDON SUPPORT LAYOUTS, WITH DETAILS NECESSARY FOR INSTALLATION FOR THE ENGINEER'S REVIEW.
- H. TENDON PLACEMENT CARE SHALL BE TAKEN THAT TENDONS ARE LOCATED AND HELD IN THEIR DESIGNED POSITIONS. TOLERANCES FOR THE LOCATION OF THE PRESTRESSING STEEL SHALL NOT BE MORE THAN ± 1/4" VERTICALLY, EXCEPT AS NOTED OR REVIEWED BY THE ENGINEER. ACCESS TO STRESSING ENDS SHALL BE MAINTAINED WHERE SHOWN.
- TENDON ADJUSTMENTS SLIGHT DEVIATIONS IN THE HORIZONTAL SPACING OF THE SLAB TENDONS WILL BE PERMITTED WHEN REQUIRED TO AVOID OPENINGS, INSERTS, AND DOWELS, WHICH ARE SPECIFICALLY LOCATED. WHERE LOCATIONS OF TENDONS SEEM TO INTERFERE WITH EACH OTHER, ONE TENDON MAY BE MOVED HORIZONTALLY IN ORDER TO AVOID THE INTERFERENCE.
- J. TENDON BUNDLES THE MAXIMUM ALLOWABLE NUMBER OF STRANDS PER BUNDLE IS FIVE (5). K. TWISTING - TWISTING OR ENTWINING OF INDIVIDUAL TENDONS WITHIN A BUNDLE OR A BEAM
- SHALL NOT BE PERMITTED. PROFILES - PROFILES SHALL CONFORM TO CONTROLLING POINTS SHOWN ON THE DRAWINGS AND SHOULD BE IN AN APPROXIMATE PARABOLIC DRAPE BETWEEN SUPPORTS, UNLESS OTHERWISE NOTED. LOW POINTS ARE AT MIDSPAN UNLESS OTHERWISE NOTED. ALL DIMENSIONS SHOWING THE VERTICAL LOCATION OF PRESTRESSING TENDONS ARE TO THE CENTER OF
- M. MINIMUM CHAIRING TENDONS SHALL BE SECURED TO A SUFFICIENT NUMBER OF POSITIONING DEVICES TO ENSURE CORRECT LOCATION DURING AND AFTER THE PLACING OF THE CONCRETE, AND SHALL BE SUPPORTED AT A MAXIMUM OF 4'-O" ON CENTER, CHAIRS GREATER. THAN 2.5" IN SIZE SHALL BE STAPLED TO THE FORMWORK. CONDUITS MUST NOT INTERRUPT NOR BE TIED TO POST-TENSIONING TENDONS.
- N. BLOCKOUTS ALL POCKETS OR BLOCKOUTS REQUIRED FOR ANCHORAGE SHALL BE ADEQUATELY REINFORCED SO AS NOT TO DECREASE THE STRENGTH OF THE STRUCTURE. ALL POCKETS SHOULD BE WATERPROOFED TO ELIMINATE WATER LEAKAGE THROUGH OR INTO THE POCKET.
- O. CHLORIDES GROUT OR CONCRETE CONTAINING CHLORIDES SHALL NOT BE USED.

GRAVITY OF THE STRAND (CGS) UNLESS OTHERWISE NOTED.

- P. PUMPED CONCRETE IF CONCRETE IS PLACED BY THE PUMP METHOD, THEN HORSES SHALL BE PROVIDED TO SUPPORT THE HOSE. THE HOSE SHALL NOT BE ALLOWED TO RIDE ON THE TENDONS. CONCRETE CONSOLIDATION - THE CONTRACTOR SHALL TAKE PRECAUTIONS TO ASSURE COMPLETE CONSOLIDATION AND DENSIFICATION OF CONCRETE BEHIND ALL POST-TENSIONING ANCHORAGES.
- Q. CONCRETE STRENGTH AT STRESSING AT TRANSFER OF PRESTRESS, CONCRETE SHALL BE 3,000 PSI MINIMUM FOR ANY GIVEN CYLINDER COMPRESSION TEST.
- R. TENDON STRESSES SHALL CONFORM TO THE FOLLOWING: JACKING STRESS . . 216 KSI ANCHORAGE STRESS IMMEDIATELY AFTER PRESTRESS TRANSFER 189 KSI
- 5. EFFECTIVE FORCE FINAL EFFECTIVE FORCE SHALL BE 26.8 KIPS (175 KSI) PER LOW-RELAXATION TENDON, WHEN TENDON LENGTH IS LESS THAN 100 FEET, FOR VARIANCE FROM THIS VALUE, CONTRACTOR SHALL PROVIDE FRICTION AND LONG-TERM LOSS CALCULATIONS FOR THE ENGINEERS REVIEW.
- T. TENDON STRESSING TENSIONING SHALL BE DONE BY JACKING UNDER IMMEDIATE CONTROL OF A PERSON EXPERIENCED IN THIS TYPE OF WORK. CONTINUOUS INSPECTION AND RECORDING OF ELONGATIONS IS REQUIRED DURING ALL STRESSING OPERATIONS.
- U. SCHEDULE STRESSING SHALL BE PERFORMED WITHIN 4 DAYS OF CONCRETE PLACEMENT UNLESS PRIOR APPROVAL HAS BEEN RECEIVED FROM THE ENGINEER.
- V. CALIBRATIONS THE RAM AND ATTENDANT GAUGE USED SHALL HAVE BEEN CALIBRATED WITHIN SIXTY (60) DAYS OF THEIR USE.
- W. STRESSING SEQUENCE UNIFORMLY DISTRIBUTED TENDONS AND TEMPERATURE TENDONS SHALL BE STRESSED BEFORE CONCENTRATED BEAM STRIP (BANDED) TENDONS, AND SLAB TENDONS SHALL BE STRESSED BEFORE BEAM TENDONS.
- X. ELONGATIONS INDIVIDUAL TENDON FIELD READINGS OF ELONGATIONS AND/ OR STRESSING FORCES SHALL NOT VARY BY MORE THAN +/- 7% FROM CALCULATED REQUIRED VALUES SHOWN ON THE SHOP DRAWINGS. IF THE MEASURED ELONGATIONS VARY FROM CALCULATED VALUES BY MORE THAN +/- 7%, THE CONTRACTOR SHALL PROVIDE FRICTION CALCULATIONS AND/ OR OTHER JUSTIFICATION TO THE SATISFACTION OF THE ENGINEER.
- Y. MEMBER FORCES THE POST-TENSIONED FORCE PROVIDED IN THE FIELD FOR EACH STRUCTURAL MEMBER SHALL NOT BE LESS THAN THE VALUES NOTED ON THE STRUCTURAL DRAWINGS. IN THIS CONTEXT, STRUCTURAL MEMBERS ARE BEAMS OR SLABS, SLABS, WHETHER WITH BANDED OR DISTRIBUTED TENDONS, EACH SERVING THEIR RESPECTIVE TRIBUTARY.
- TENDON ENDS DO NOT CUT OFF TENDON ENDS UNTIL THE ENTIRE FLOOR SYSTEM HAS BEEN SATISFACTORILY STRESSED AND THE ENGINEER'S REVIEW IS OBTAINED. TENDON LENGTH PROTRUDING BEYOND WEDGES AFTER CUTTING SHALL BE 3/4 INCH ± 1/4 INCH. THE STRESSING END ANCHORS AND WEDGES SHALL BE SPRAY PAINTED WITH RUST-OLEUM OR A SIMILAR COATING FOR CORROSION PROTECTION. INSTALL LOCKING TRANSLUCENT GREASE CAPS WITHIN THE FOLLOWING 24-HOUR PERIOD.
- AA. GROUTING OF STRESSING POCKETS STRESSING POCKETS SHALL BE FILLED WITH NON-SHRINK GROUT AFTER STRESSING, PAINTING & GREASE-CAPPING TO STOP MOISTURE PENETRATION. SUBMIT SPECIFICATION FOR PROPOSED BONDING AGENT AND GROUT TO ENGINEER FOR REVIEW PRIOR TO USE.
- BB. DE-SHORING SLABS OR BEAMS MAY BE DE-SHORED WHEN ALL TENDONS HAVE BEEN SATISFACTORILY STRESSED AND THE ENGINEER'S REVIEW IS OBTAINED, UNLESS SHORING IS REQUIRED TO CARRY FLOORS ON ABOVE LEVELS, OR SHORING IS REQUIRED TO SUPPORT STRUCTURE AT CLOSURE POURS.





NAILS: COMMO
BOLTS: ASTM A

F.	PROVIDE LATE OF BEARING.
G.	LAG SCREMS I

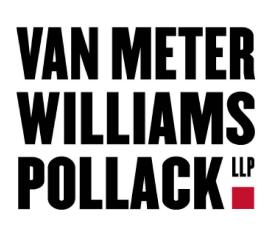
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THREADED PC

Η.	GLUE-LAMINAT
	1. 24F∨4
	BEAMS
	2. INDUSTR
	3 CAMBE

	4. FABRIC, 5. MATERI,
١.	MICRO-LAMS 2

•	CONCRETE MASONRY			
A.	CONCRETE MASONRY TO BE SUPPI PER SECTION 2104	LIED PER 2019 CE	BC SECTION 2105	AND PLACED
В.	ASSEMBLY STRENGTH f'm = 1500 p	si AT 28 DAYS.		
C.	UNITS: MEDIUM WEIGHT 2 CELL BLO BLOCKS SHALL NOT EXCEED .0659			SHRINKAGE OF
D.	MORTAR: ASTM C270, TYPE M.			
E.	GROUT: ASTM C476. COMPRESSIVE ASSEMBLY STRENGTH. ALL CELLS			AIN SPECIFIED
F.	USE LOW LIFT CONSTRUCTION WITH			
G.	GROUTING IS ACCEPTABLE IF APPR ALL MASONRY TO BE REINFORCED			
ц	REINFORCED'. SEE PLAN FOR LOCATIONS OF VER		IOINTS HORIZOI	
Π.	AND LINTEL REINFORCING SHALL E JOINTS.			
١.	ALL CELLS, SHALL BE GROUTED SO PLACE BEFORE GROUTING STARTS		G STEEL SHALL	BE SECURED IN
J.	MASONRY BUILDING WALLS HAVE E	BEEN DESIGNED T		
	SPANS FROM FLOOR TO ROOF AN STRUCTURE AND THE COMPLETION RESISTANCE TO WIND AND SEISMIC	OF ALL MASONR	Y WALLS FOR ST	ABILITY AND FOR
	RESPONSIBLE FOR PROVIDING ALL FOR CONSTRUCTIONS LOADS, FOR	L NECESSARY TEN STABILITY AND F	1PORARY BRAC FOR RESISTANCE	ING AS REQUIRED TO WIND AND
	SEISMIC FORCES UNTIL THE ENTIRE NOT RELY ON ANY MOMENT RESIST			
•	<u>MOOD</u> FRAMING LUMBER - DOUGLAS FIR	JON		
м.	 JOISTS AND RAFTERS: <u>NO.</u> POSTS, BEAMS, AND HEADE 	2		
	3. STUDS, PLATES: <u>NO. 2</u> 4. BLOCKS, LIGHT FRAMING A	ND MISC: STUD GI		
	 ALL LUMBER IN CONTACT W TREATED. ALL LUMBER SHALL HAVE A 			
	INSTALLATION. 7. FIRE-RETARDANT-TREATED	FRAMING SHALL	COMPLY WITH	
в	CBC SECTION 2303.2. S.A.I SHEATHING:	J. FOR FIRE RATIN	NG REQUIREMEN	IS AND LOCATIONS.
- -	1. TYP. ROOF SHEATHING AND EXPOSURE 1. (4 PLY MIN.)			
	2. TYP. FLOOR SHEATHING (PL MIN. WITH TONGUE AND GRO SHEET DIMENSION FOR PAN	OVE EDGES GLU	ED TO SUPPORT	, U.O.N. MINIMUM
	SHEET DIMENSION FOR PAN THICKNESS AND SHEAR STR 3. TYP. CEILING SHEATHING: <u>1/</u>	ENGTH MAY BE U	SED IN LIEU PLYM	NOOD.
	WITH TONGUE AND GROOVE DIMENSION FOR PANEL SHA	EDGES GLUED T LL BE 24".	O SUPPORT, U.O.	N. MINIMUM SHEET
	4. TYP. WALL SHEATHING: <u>15/3</u> GLUE. (4 PLY MIN.) ALT. OS MAY BE USED IN LIEU OF PL	B WITH EQUIVALE	NT THICKNESS AN	ND SHEAR STRENGTH
	5. FIRE-RETARDANT-TREATED CBC SECTION 2303.2. S.A.I	SHEATHING SHAL	L COMPLY WITH	
C.	FRAMING HARDWARE: AS MANUFAC			
D	SIMPSON DESIGNATIONS USED.	NAILING TO CONF	ORM TO CBC T	ABLE 2304 10 1 11 0 N
	BOLTS: ASTM A307. PROVIDE WAS			
F.	PROVIDE LATERAL SUPPORT FOR OF BEARING.	BEAMS, JOISTS A	ND RAFTERS AT	ENDS AND POINTS
	LAG SCREWS PER ANSI/ASME STAN			
G.				
G.	AND DEPTH AS SHANK AND THEN I THREADED PORTIONS.			DIAMETER FOR
	AND DEPTH AS SHANK AND THEN I THREADED PORTIONS. GLUE-LAMINATED BEAMS: 1. 24FV4 FOR SIMPLE SPANS	RILL HOLE 60%	- 70% OF SHANK	
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STRUCTURAL SHEET INDEX SHEET # SHEET NAME S1.0 STRUCTURAL GENERAL NOTES STRUCTURAL GENERAL NOTES S1.1 S1.2 STRUCTURAL GENERAL NOTES S2.11 LEVEL 1 - FOUNDATION PLAN 52.12A LEVEL 2 PODIUM - REINFORCEMENT PLAN LEVEL 2 PODIUM - P-T PLAN 52.12B LEVEL 2 PODIUM - FRAMING PLAN 52.120 52.13 LEVEL 3 - FLOOR FRAMING PLAN 52.14 LEVEL 4 - FLOOR FRAMING PLAN ROOF LEVEL - ROOF FRAMING PLAN S2.15 ELEVATION - NORTH AND SOUTH 53.01 ELEVATION - EAST AND WEST 53.02 S3.11 CONCRETE SHEAR WALL ELEVATIONS 53.12 CONCRETE SHEAR WALL ELEVATIONS 53.13 CONCRETE SHEAR WALL ELEVATIONS S4.11 UNIT FRAMING PLANS ENLARGED PLAN AND SECTIONS - STAIR CENTRAL 54.21 ENLARGED PLAN AND SECTIONS - STAIR WEST 54.22 54.23 ENLARGED PLAN AND SECTIONS - STAIR EAST 54.24 ENLARGED PLAN - ELEVATOR ENLARGED PLAN AND SECTIONS - CANOPY S4.25 S5.1 TYPICAL CONCRETE DETAILS S5.2 TYPICAL CONCRETE DETAILS S5.3 TYPICAL CONCRETE DETAILS S5.4 CONCRETE DETAILS S5.5 CONCRETE DETAILS S5.6 CONCRETE DETAILS S5.7 CONCRETE DETAILS S5.8 CONCRETE DETAILS 559 CONCRETE DETAILS S6.1 TYPICAL CMU DETAILS S7.1 TYPICAL STEEL DETAILS 57.2 TYPICAL STEEL DETAILS 57.3 TYPICAL STEEL DETAILS TYPICAL WOOD FRAMING DETAILS 58.1 58.2 TYPICAL WOOD FRAMING DETAILS 58.3 TYPICAL WOOD FRAMING DETAILS 58.4 TYPICAL WOOD FRAMING DETAILS S8.5 TYPICAL WOOD FRAMING DETAILS 58.6 TYPICAL WOOD FRAMING DETAILS 58.7 TYPICAL WOOD FRAMING DETAILS 58.8 TYPICAL WOOD FRAMING DETAILS 58.9 EARTHBOUND SEISMIC HOLDOWN SYSTEM 58.10 EARTHBOUND SEISMIC HOLDOWN SYSTEM 59.1 TYPICAL LIGHT GAUGE DETAILS S9.2 TYPICAL LIGHT GAUGE DETAILS



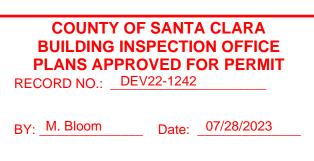
ARCHITECTURE | URBAN DESIGN SAN FRANCISCO | DENVER | MINNEAPOLIS 333 Bryant Street, Suite 300, San Francisco, CA 94107 T 415.974.5352

- **BKF-SAN JOSE** 1730 N. FIRST ST. STE 600 SAN JOSE, CA 95112
- LANDSCAPE ARCHITECT PLURAL STUDIO 2742 17TH STREET SAN FRANCISCO, CA 94110
- STRUCTURAL ENGINEER HOHBACH-LEWIN INC 250 SHERIDAN AVE STE 100 PALO ALTO, CA 94306
- <u>MEP ENGINEER</u> EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- SUSTAINABILITY/ENERGY **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521
- JOINT TRENCH/DRY UTILITY MILLENIUM DESIGN PO BOX 737 ALAMO, CA, 94507



→ HOHBACH-LEWIN, INC. STRUCTURAL & CIVIL ENGINEERS 260 Sheridan Avenue, Suite 150 Palo Alto, CA 94306 650) 617-5930





HARD COPY OF THESE STAMPED PLANS MUST BE ON THE SITE FOR INSPECTIONS

ID	DATE	NAME
1	11/11/2022	PERMIT SET-CONV
Α	12/16/2022	BID SET
В	03/20/2023	BID ADDENDUM
2	03/20/2023	PLAN CHECK RESPONSE 2
3	05/12/2023	PLAN CHECK RESPONSE 3

Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	
mercy HOUSING	
abode	
Communities MERCY HOUSING/ ABODE COMMUNITIES	
STRUCTURAL	
GENERAL NOTES	
JOB #: 1925 SCALE: As indicated	 # 14515
SCALE: AS Indicated	 4BACH-LEWIN # 14515

 B. U.O.N. STEEL SHALL BE AS FOLLOWS: I. NUDE FLANCE SHAPES: SATM ARR2 I. NUTHER SHAPES AND PLATE: SATM ARR2 (S. ASTM ARR2 (S. AST		ITEEL ITRUCTURAL STEEL TO BE SUPPLIED DETAILED, FABRICATED AND ERECTED IN ACCORDANCE NITH A.I.S.C. SPECIFICATIONS.		1.	ELLULAI BLOCK:	ASTM D			
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NIMM, REQUIRED DY STESSE NCREAS FILLS TO A S.C. MINIM SIZE DAED ON SPECA LARGE STATUS STEL VOT SCENNS FRE SWALL ON NOTIFY RECLARCE DEVICES TO A S.C. MINIM SIZE DAED ON STEL VOT SCENNS FRE SWALL ON SPECA THE CERVICE ON CONTROL OF MALES FROM ON THE CRM STEL VOT SCENNS FRE SWALL DE HOT DY INC GALVANCED UCA. NOTIFY RECLARD SPECATION OF STALL DE HOT DY INC GALVANCED UCA. NUMESONT PART TO MEET REQUIREMENTS OF ASTY ETR. AND MERK SWALL DE HOT DY INC GALVANCED UCA. NUMESONT PART TO MEET REGUIREMENTS OF ASTY ETR. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT SCENT PART TO MEET REGUIREMENTS OF ASTY ETR. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT SCENT PART TO MEET REGUIREMENTS OF ASTY ETR. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT SCENT PART TO MEET REGUIREMENTS OF ASTY ETR. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT SCENT PART TO MEET REGUIREMENTS OF THE STEL PART. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT SCENT PART TO MEET REGUIREMENTS OF THE STEL PART. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT DY INC CONTROL TO THE TO MEET REGUIREMENTS OF THE STEL PART. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT SCENT PART TO MEET REGUIREMENTS OF THE STEL PART. AND MERK SWALL DE HOT DY INC GALVANCED UCA. DUAT DY INC CONTROL TO STE MERK AND MERK SWALL DE HOT DY INC CONTROL TO STEL AND MERK SWALL				GIVEN I	n manuf	ACTURE	ER'S RECOMI	IENDAT	10
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ALL STEEL DROGED TO NEATHER SHALL BE HOT OP 2NG SALVANEED JOX. NON SIRING SRCUT. TSGO DI COMPRESS YE STRENGT, NON METALL CONFORMING TO SHALL DE TER COMPRESS TO BALL. DE NEL STARE STRENGT, NON METALL CONFORMING TO SHALL DE TER COMPRESS YE STRENGT, NON METALL CONFORMING TO MEDISON TO METAL STORE SHALL BE HER DIAL STRENGT, NON METALL CONFORMING TO MEDISON TO METAL STORE SHALL BE HER DIAL STRENGT, NON METALL CONFORMING TO MEDISON TO METAL STRENGT HER COMPRESS YE STRENGT, NON METAL DE TER COMPRESS TO THE STLES STRENGT, STRENGT, NON METAL DE TER COMPRESS TO THE STLES STRENGT, NON METAL DE TER COMPRESS TO THE STLES STRENGT, STRENGT, NON METAL DE TER COMPRESS TO THE STLES STRENGT, THE STRENGT, NON METAL DO THE AND THE VARIANCE DE MONE MALL DE TER COMPRESS TO THE STLES STRENGT, STREN				THE LO	CATIONS	5 SHOWN	N ON THE DR	AMINGS	·.
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INTRODUCTOR INTEGRED AND SET CONFECTORY AMERICAN SET THE COLLECTORY STELL ISED FOR STIDD. TRACK, BLOCKING, GASETS, BRACE STRAPS, ETC. COLD FORM STELL STID FOR STIDD. TRACK, BLOCKING, GASETS, BRACE STRAPS, ETC. MILL MOST THE RESURPENTS OF THE STELL STID MANUNAL TIMERS ASSOCIATION (SSMA) ESC. SAILL MEET THE RESURPENTS OF THE STELL STID MANUNAL TIMERS ASSOCIATION (SSMA) ESC. MILL MOST LESTID FRANKS SHALL CONFORM TO THE FOLLOWING. SM MIL AND LENTER-STATUSED SCIENCE MILL MOSTEL STID FRANKS SHALL CONFORM TO THE FOLLOWING. SM MIL AND LENTER-STATUSED SCIENCE MILL MOSTEL STID FRANKS SHALL CONFORM TO THE FOLLOWING. SM MIL AND LENTER-STATUSED SCIENCE MILL STELL STIDS, JOST I TRACK SHALL HAVE A LEBELE LEAGEL, STAMP OR ENDROSENENT, AT ANOMING OF COL, DOCTANT THE MATERINE, INSEE STAPP, OR ENDROSENENT, AT ANOMING OF COL, DOCTANT THE MATERINE, INSEE STAPP, OR ENDROSENENT, AT INSEE STRESS MEED THAN 33 SKI. MILL CERTIFICATES PROM THE COLL RECOLLERS SHALL BE MADE AVAILABLE IF REQUESTED. MILL MOST THE COLL RECOLLERS SHALL BE MADE AVAILABLE IF REQUESTED. MILL CERTIFICATES FROM THE COLL RECOLLERS SHALL BE MADE AVAILABLE IF REQUESTED. MILL MOST THE STELL STIDS MAR SEE STADS IS REQUERD THAN 35 SKI. COLD FORM STEL. STIDS SHALL BE AND AND MEED THACK AED RYROL MILL STRESS AND MADE CONFERS MALL BE AND AND CONTRE THAN THE COLL RECOLL RESOLUTION OF ALLESS CORRESPOND OF THE ATTERNES AND DECEMPTING. COLD FORM STEL. STIDS MAR TRACKS SHALL BE ANTACHED BY MELDING OR (1) PRANSPAR ATACHEMERS AND MEET THE REQUERT ATTERNES AND AND CONTRE THAND AND CONTRE THAN AND AND CONTRE THAN AND AND				BE TEN SHALL APPLIC	SION TES BE TENS ATIONS,	57ED. F0 10N TES 50% 01	OR ALL OTH TED. WHEN E F ANCHORS	ER STRU POXY A BHALL E	JC N BE
THE COULD BOOK THELE USED FOR STUDS, TRACK, BLOCKING, SUSSETS, BRACE STRAPS, FTC. SHALL MEET THE REQUEREMENTS OF THE STEEL STUD MANUPACTURERS ASSOCATION (SIMM) DE SALALTON REPORT NO SOBAL DATED FERRILARY 2020. SEE DETAIL SHEETS FOR SUSSEAND THICKNESS REQUEREMENTS. COLD FORM STELL STUD TRAVING SHALL CONFORM TO THE FOLLOWING: 49 ML AND EAVERT ASSOCATION (SIMM) MANUPACTURERS ASSOCATION (SIMM) DE SATERIOR MEMBERS I: GALVAN ZED GOO NN. ALL STEEL STUD. TRAVING SHALL CONFORM TO THE FOLLOWING: 49 ML AND EAVERT ASSOCATION RESS SIG (SI MINIUM YELD) EXTENSIVE THE MATERIAL BASES SIG (SI MINIUM YELD) EXTENSIVE TO INCURE SIMMUNA YELD) EXTENSIVE TO INCURSE INMUNITY PLD.) EXTENSIVE TO INCURSE THE MATERIAL BASE METAL THICKNESS (INCOLTED) IN MILL CERTIFICATES FROM THE COLL FROMCURESS MANUEL COSC OR NULLAS. EVALUATION SERVICE REPORT THE MATERIAL BASE METAL THICKNESS (INCOLTED) IN MILL CERTIFICATES FROM THE COLL FROMCURES MANUEL SOGO OR NULLAS. EVALUATION SERVICE REPORT THE MATERIAL BASE METAL THICKNESS (INCOLTED) IN MILL CERTIFICATES FROM THE COLL FROM STELL STUDS AND CANTING THAN 35 KM. ALL SECTIONS TO REMAIN INFLIMENT AND SECTION SOLUTION AND SECTIONS ON THE ALL SECTIONS ON THE ALL SECTIONS ON THE ALL SECTION SOLUTION AND SECTIONS ON THE ALL SE			1	ANCHO	RS PASS	6. (PER I	IR-19.1 FOR I	25A PRO	Э.
EVALUATION REPORT NO. 3664P DATED FEBRUARY 2020. SEE DETAIL SHEETS FOR SIZES AND THICKNESS REALIZEMENTS. VERTIFY MINUM PENDING COLD FORM STEEL STUD FRAMING SHALL CONFORM TO THE FOLLOWING: SHIVE AND FEAVER-ASTM A883 56 150 kill MINIMUM YIELD? VERTIFY MINUM PENDING SHIVE AND FEAVER-ASTM A883 56 150 kill MINIMUM YIELD? VERTIFY MINUM PENDING SHIVE AND INFERVIEW AND ASS 56 150 kill MINIMUM YIELD? VERTIFY MINUM PENDING LID STELLSTOR MEMERSTER MARELED SEED CONFERSION. VERTIFY MINUM PENDING A MAXIMUM OF 49: OG, INDCATING THE YANJACTURERS NAME, LOGO CR NITALS. VERTIFY MINUM PENDING LID STENDERTH IP DIFFERENT THAN 38 kill. VERTIFY MINUM PENDING ALL SCOTOR TO TO PENAL INFORMET. THE ANTERNAL TRACK DES (INCOATED) NALL SCOTOR TO PENAL INFORMATION. VALUES ARE FOR SINGLE ANCACRES IN VITH CO HOLE SCEAND CONTING THE YANJACATORISED AND THE COLORIDAL TO PENAL INFORMATION. VALUES STRENGTH, ELONATION, AND COLD ROATING THE VANDANCE ON PENALTING INSTALL TO NEET STRENGTH. VALUES ARE FOR SINGLE ANCACRES IN VITH CO HOLE SCEAND CONTING THE VANDANCE ON PENALTING INSTALL TO NEET SIGNAL INSTALL TO NEET STRENGTH. DESEANT CONTINUE FULL HEGHT NULL STIDDS AND TRACKS SHALL BE ATTACHED IN ACCORDANCE WITH ASI SPECIFICATION REMAINS AND AND REST AND BENDING INSTALL STIDD SARE INSTALL STRENGT INSTALL TO NEET STRENGT IN STRENGT INSTALL STRENGT INSTALL STRENGT INSTALL STRENGT INSTALL STRENGT INSTALL TO NEET STRENGT INSTALL STRENGT I	C	OLD FORM STEEL USED FOR STUDS, TRACK, BLOCKING, GUSSETS, BRACE STRAPS, ETC.			RESSIVE	STRENG	ЭТН		
20.D PORM STELL STUD FRAMMS SHALL CONFORM TO THE FOLLONING: 54 ML AND LIGHTER-NETM A655 95 (05 kis) MINIUM YIELD) 21 ML AND LIGHTER-NETM A655 95 (05 kis) MINIUM YIELD) 21 ML AND LIGHTER-NETM A655 95 (05 kis) MINIUM YIELD) 21 ML AND LIGHTER-NETM A655 95 (05 kis) MINIUM YIELD) 21 ML AND LIGHTER-NETM A655 95 (05 kis) MINIUM YIELD 21 ML AND LIGHTER-NETM A655 95 (05 kis) MINIUM YIELD 21 ML AND LIGHTER HAND FRAME MARK METRICL BASE MATERAL THACKNESS (MLCATED) IN ANAMUM OF 49 CC., INDICATING THE MANUFACTURERS INAME, LOGG CR INTIALS, SWALLATCH STRENGTH FE OFFERENT TANKI S3 Kis. 21 ML CERTIFICATES TROM THE COLER SHALL BE MADE AVAILABLE IF REQUESTED, MILL CERTIFICATES TROMATIC CONTROLOGY SHALL CONFORMICE, INTELLED STRENGTH. 21 ML AND LIGHTER INFORMATION. AND COATING THE CHEMCAL COMPOSITION, YIELD STRENGTH. 21 ML CERTIFICATINE FULL MEMORY COATING THE FOLLOWING CR MITH ICC HOLE SIZE AND SPACING LIMITATIONS. 21 AND TRACK ATACHMENT TO SUDS AND REDIGING INSTALLATION SEE TYPICAL DEFENDING FULL HEIGHT ON BOTH SIDES. FOR BRIDGING INSTALLATION SEE TYPICAL DEFENDING FULL HEIGHT ON DEDIG REDIGING INSTALLATION SEE TYPICAL DEFENDING FULL SECTIFIED AND SALL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL BEARING AGAINST INSIDE TRACK HEB FRIOR TO SUT ALL ATOM RESISTING FULL	=	VALUATION REPORT NO. 3064P DATED FEBRUARY 2020. SEE DETAIL SHEETS FOR SIZES AND		VER	IFY MINI	MUM EX	ISTING CONC	RETE S	Tł
EXTRICAL MEMBERS : GALVANZED 660 MN. ALL STEEL STUDS, JUBIT 1 TRACK SHALL HAVE A LESIBLE LABEL, STAMP OR EMBOSSMENT, AT ANAMIMU OF 49° OC, NOCATING THE AVAILABLE ELABEL, STAMP OR EMBOSSMENT, AT ANAMIMU OF 49° OC, NOCATING THE AVAILABLE BASE METAL THICKNESS (INCOATED) N GUI III, AD THE YELD STRENGTH IP D PFERENT THAN 38 kil. MILL CERTIFICATES FROM THE COLL PRODUCER SHALL BE MADE AVAILABLE IF REQUESTED MILL CERTIFICATES FROM THE COLL PRODUCER SHALL BE MADE AVAILABLE IF REQUESTED MILL CERTIFICATES TO RUJDE 46 A MINIMUM THE CHEMICAL COMPOSITION, YIELD STRENGTH, MILL CERTIFICATES TO RUJDE 46 A MINIMUM THE CHEMICAL COMPOSITION, YIELD STRENGTH, MILL CERTIFICATE TO NULLES, AND EXCEPT WALL STUDS MAY BE PLACHED IN ACCORDANCE MITH ICA HOLE SIZE AND SPACING LIMITATIONS. ATERAL BRIDGING OF COLD FORM STELL STUDS IS RESURED WHEN SHALLATION SEE TYPICAL DETAL SHEET, STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRICK TO STUD AND TRACK ATTACHMENT. STUDS AND TRACKS SHALL BE ATTACHED BY AELDING OR RELAND TRACK SCRAPH (ONE EA, FLANGE). MELTING SHALL DE MEDISING IN A CORDANCE MITH ABI "SPECIFICATION ATERAL DRIDGING SCRAPH (ONE EA, FLANGE). MELTING SHALL BE NETWORK ICC ESR-2044 SELF-DRILLING SCRAPH (ONE EA, FLANGE). MELTING SHALL BE NETWORK ICC ESR-2044 SELF-DRILLING SCRAPH ONE BY CERTIFIC MELDING IN TRACK. SPLICES IN STUDS OR RECANN FASTENERS HAVE BEEN DESIGNED MACCORDANCE MITH ABI "SPECIFICATION ATEN AND LIGHTER SHEET TO SHEET - THE REQUIREMENTS OF "SIMPSON" OR RECANN FASTENERS HAVE BEEN DESIGNED BACCORDANCE MITH ABI "SPECIFICATION AND AND LIGHTER SHEET TO SHEET - THE REQUIREMENTS OF "SIMPSON" OR RECANN FASTENERS HAVE BEEN DESIGNED BACCORDANCE MITH ABI "SPECIFICATION AND AND LIGHTER SHEET TO SHEET - THE REQUIREMENTS OF "SIMPSON" OR RECANN FASTENERS HAVE BEEN REDENTING DE TRACK SPLICES IN STUDS OR RECANN FASTENERS HAVE BEEN REDENTED SHALL CONFORM TO SAE JTS. MIL AND LIGHTER SHEET TO SHEET - TOXX MIL AND LIGHTER SHEET TO SHEET - TOXX MIL AND LIGHTER SHEET OF SHEET DESIGNED THROUGH BY MELLINGS	5	54 MIL AND HEAVIER-ASTM A653 SS (50 ksi MINIMUM YIELD)		DOWE	L ANC	CHOR COD	DIAMETER	MIN. EMBE	
MAXIMUM OF 49' CC, NDICATING THE MANUFACTURERS INAME, LOGO CR INITIALS, SAVALIATION SERVICE REPORT NUMBER, THE MATERIAL BASE METAL THICKNESS (INICOATED) IN 201 In. AND THE YIELD STRENGTH IF DIFFERENT THAN 33 Sui. Image: Comparison of the Comparelevents of the Comparison of the Comparison of the		EXTERIOR MEMBERS : GALVANIZED G60 MIN.		#4	1/2	" DIA.	5/8"	3" 4" 5"	_
III.L CERTFICATES FROM THE COLIP RODUCER SHALL BE MADE AVAILABLE F REQUESTED. III.L CERTFICATE TO NOLLIDE AS A MINIMUM THE CHEMICAL COMPOSITION, YIELD STRENGTH, ENGLE STRENGTH, ELONGATION, AND COATING THICKNESS. UTH LC SECTIONS TO REMAIN INFINCHED EXCEPT NALL STUDS MAY BE PINCHED IN ACCORDANCE III.L CERTFICATES TO COLD FORM STELL STUDS IS REQUIRED ANEN SHEATHING, INSTALLED III.D DA AND TRACK STREAGINED ANEN SHEATHING, INSTALLED III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D DA AND TRACK SHALL BE ATTACHED BY NELDING OR III.D SCHWART SHEET SHALL BE ALL BEAL THE REQUIREMENTS OF ISMPSON' OR III.D SCHWART SHEET SHEET SHALL BE ALL CONFORM TO SAE JTS. III.D SCHWART SHEET TO SHEET - ECONX IIIII.D SHALL BE PERCOMED BY CERTIFIED WELDERS IN A FABRICATION SHOP ALL BE AND THE ANGENG SHALL BE FROM IIII.D CAR AND ADD THE ALSONG NICHTAR SHEET TO SHEET - ECONX IIII. AND HERY SHEET TO SHEET - ECONX IIIIII AND HERY SHEET TO SHEET - ECONX IIIIII AND HERY SHEET TO SHEET - ECONX IIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIIII		MAXIMUM OF 48" O.C., INDICATING THE MANUFACTURER'S NAME, LOGO OR INITIALS, VALUATION SERVICE REPORT NUMBER, THE MATERIAL BASE METAL THICKNESS (UNCOATED) IN		#6	3/4	" DIA.	7/8"	5" 6" 7"	
 LL SECTONS TO REMAN UNPINCHED EXCEPT WALL STUDS MAY BE PUNCHED IN ACCORDANCE ITH LC HOLE SIZE AND SPACING LIMITATIONS. ** VALUES ARE FOR SINGLE ANCHORS IN OTHER CASES, REDUCTION OF VALUES OTHER CASES, REDUCTION OF VALUES OTHER CASES, REDUCTION OF VALUES OTHER CASES, REDUCTION OF VALUES TENSION TEST VALUES CORRESPOND (STEENSTH). ** TENSION REST VALUES CORRESPOND (STEENSTH).<td>1</td><td>11LL CERTIFICATES FROM THE COIL PRODUCER SHALL BE MADE AVAILABLE IF REQUESTED. 11LL CERTIFICATE TO INCLUDE AS A MINIMUM THE CHEMICAL COMPOSITION, YIELD STRENGTH,</td><td>*</td><td>#8</td><td>1"</td><td>DIA.</td><td>1 1/8"</td><td>8"</td><td></td>	1	11LL CERTIFICATES FROM THE COIL PRODUCER SHALL BE MADE AVAILABLE IF REQUESTED. 11LL CERTIFICATE TO INCLUDE AS A MINIMUM THE CHEMICAL COMPOSITION, YIELD STRENGTH,	*	#8	1"	DIA.	1 1/8"	8"	
ATERAL BRIDGING OF COLD FORM STEEL STUDS IS REQUIRED WHEN SHEATHING, INSTALLED DOES NOT CONTINUE FULL HEIGHT ON BOTH SIDES. FOR BRIDGING INSTALLATION SEE TYPICAL COLD FORM STEEL STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRIOR TO STEEL STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRIOR TO TRACK ATTACHMENT. STUDS AND TRACKS SHALL BE ATTACHED BY WELDING OR 2) #3 SELF DRILLING SCREWS (ONE EA. FLANGE). REMANUFACTURED HANGERS, CLIPS, ETC. SHALL MEET THE REQUIREMENTS OF "SIMPSON" OR REMANUFACTURED HANGERS, CLIPS, ETC. SHALL MEET THE REQUIREMENTS OF "SIMPSON" OR REMANUFACTURED HANGERS, CLIPS, ETC. SHALL MEET THE REQUIREMENTS OF "SIMPSON" OR REPORT CONSTRACT STEEL NETWORK ICC ESR-2044. BELF-DRILLING FASTENERS HAVE BEEN DESIGNED IN ACCOORDANCE WITH AISI "SPECIFICATION REQUINES FOR SCREW CONNECTIONS". FASTENERS SHALL BE IN SAGLO STALL SCREWS TO SEGALVANIZED OR CORROGION RESISTANT. SCREWS SHALL BE IN SAGLO STALL SCREWS TO RELDING SHALL BE INFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL MELDING SHALL BE INFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL MELDING SHALL BE INFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL MELDING SHALL BE INFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL MELDING SHALL BE INFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL MELDING SHALL BE INFORMED TO SHEET - EDXX SITT WELDS ON SPLICES SHALL BE USED AT ALL JOINTS IN TRACK. SPLICES IN STUDS OR RRACES SHALL NOT BE FERTITED. HERT TO SHEET - EDXX SITT WELDS ON DET. 1/59.1 TESTED UNTL 20 CONSECUTIVE ANCH READING ON DET. 1/59.1 VERIFY MINIMUM EXISTING CONCRET FOR SCREW THAS STRUCTURES. STEEL DECK TABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS. STEEL DECK SHALL CONFORM TO ASTMA446. RROVIDE 16 6A MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N.	4	LL SECTIONS TO REMAIN UNPUNCHED EXCEPT WALL STUDS MAY BE PUNCHED IN ACCORDANCE	**	VALUES	ARE FC	OR SING	LE ANCHORS	MITH N	0
2ETAIL SHEET. COLD FORM STEEL STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRIOR TO STUD AND TRACK ATTACHMENT. STUDS AND TRACKS SHALL BE ATTACHED BY MELDING OR 2010 ALD TRACK ATTACHMENT. STUDS AND TRACKS SHALL BE ATTACHED BY MELDING OR 2010 ALDRT. EXPANSION BOLTS SHALL BE HILTI KM EXPANSION STALL ATTEN PROVIDE HILTI KM EXPANSION STALL ATTEND PROVIDE SIN STALL STATES STALLATION: INSTALL THE EXPANSION FROVIDE HILTI KM EXPANSION ANCHORS SHALL BE PROVIDE THIS IN STALL SCREWS SHALL BE PROV (1704A OF THE CACE FOR DSA PROLE) SEE GALVANIZED OR CORROPION RESISTANT. SCREWS SHALL DE MONTO STALL CONFORM TO SALL SCREWS TO STALLATION: INSTALL THE EXPANSION ANCHORS ARE USED OF THE ANCHORS SHALL BE PROV (1704A OF THE CACE FOR DSA PROLE) STALLATE OR SHELT TO SHEET T E SCREW STALL SCREWS SHALL DE NE ACCORDANCE WITH THE REQUIREMENTS OF THE LATEST AND DIS AML AND HEAVIER SHEET TO SHEET T E SCOXX SM IL AND HEAVIER SHEET TO SHEET T E SCOXX SM IL AND HEAVIER SHEET TO SHEET T E SCOXX SM IL AND HEAVIER SHEET TO SHEET T E SCOXX SM IL AND LIGHTER SHEET TO SHEET T E SCOXX SM IL AND LIGHTER SHELL STITCH PLATE OF SAME THICKNESS. SEEL DECK FABRICATE STEEL DECK IN ACCORDANCE WITH ANSI SPECIFICATIONS. STEEL DECK FABRICATE STEEL DECK IN ACCORDANCE WITH ANSI SPECIFICATIONS. STEEL DECK SHALL CONFORM TO ASTM A446. PROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC, U.O.N.	_	ATERAL BRIDGING OF COLD FORM STEEL STUDS IS REQUIRED WHEN SHEATHING, INSTALLED				/ALUES	CORRESPON	D WITH	1.
 EXAMPLE AND PRODUCT ALTERNATE PRODUCT		ETAIL SHEET. OLD FORM STEEL STUDS SHALL HAVE FULL BEARING AGAINST INSIDE TRACK WEB PRIOR TO	•	EXPANS	510N ANG	CHORS ((HIL⊤I)		
QUIVALENT. a. PROVIDE HILTI KWIK-BOLT 3 AND ERTICLIP SL/SLD BY THE STEEL NETWORK ICC ESR-2049. b. INSTALLATION: INSTALL THE EXPANSION OF CONSTRUCTIONS'. FASTENERS SHALL BE #3 SMS U.O.N. ALL SCREWS TO CONSTRUCTIONS'. FASTENERS SHALL CONFORM TO S.A.E. JTB. b. INSTALLATION: INSTALL THE EXPANSION OF THE CONTENTS OF ALL BE #3 SMS U.O.N. ALL SCREWS TO CONTENT IN CONFORMING SHALL BE PERFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL BLING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE LATEST AWS DI.S CODE. b. WHEN EXPANSION ANCHORS ARE USED OF THE CONTENT OF SAE. JTB. ELDING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE LATEST AWS DI.S CODE. c. SPECIAL INSPECTION SHALL BE TENSION ANCHORS HALL BE TENSION ANCHOR SHALL APPLICANT AND LIGHTER SHEET TO SHEET - EFOXX VIT WELDS ON SPLICES SHALL BE USED AT ALL JOINTS IN TRACK. SPLICES IN STUDS OR RACES SHALL NOT BE PERMITTED, WHERE STUDG ARE BURNED THROUGH BY WELDING ROVIDE SUITABLE STITCH PLATE OF SAME THICKNESS. c. SONCRETE AT TIME OF INSTALLATION COMPRESSIVE STRENGTH EE ADDITIONAL NOTES ON DET. 1/99.1 TEEL DECK MEN EXCORDANCE WITH ASI SPECIFICATIONS. VERIFY MINIMUM EXISTING CONCRETE AT ENDER DISTING ACCORDANCE WITH ASI SPECIFICATIONS. TEEL DECK NOVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. VERIFY MINIMUM EXISTING CONCRETE AT TIME OF INSTALL AFFILTACE VERIFY MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. VERIFY MINIMUM EXISTING CONCRETE AT TIME OF INSTALLATION COMPRESS	2,) #8 SELF DRILLING SCREMS (ONE EA. FLANGE).		EQUAL	PRODUC	T. ALTE	RNATE PRO	DUCTS N	11
LF-DRILLING FASTENERS HAVE BEEN DESIGNED IN ACCORDANCE WITH AISI "SPECIFICATION LF-DRILLING FASTENERS HAVE BEEN DESIGNED IN ACCORDANCE WITH AISI "SPECIFICATION CALVANIZED OR CORROSION RESISTANT. SCREWS SHALL BE #6 SMS U.O.N. ALL SCREWS TO CALVANIZED OR CORROSION RESISTANT. SCREWS SHALL CONFORM TO S.A.E. JT2. LDING SHALL BE PERFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL ELDING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE LATEST ANS D1.3 CODE. 43 MIL AND HEAVIER SHEET TO SHEET - E60XX 54 MIL AND HEAVIER SHEET TO SHEET - E60XX 54 MIL AND HEAVIER SHEET TO SHEET - E60XX 54 MIL AND HEAVIER SHEET TO SHEET - E0XX TTI WELDS ON SPLICES SHALL BE USED AT ALL JOINTS IN TRACK. SPLICES IN STUDS OR RACES SHALL NOT BE PERMITTED. WHERE STUDS ARE BURNED THROUGH BY WELDING COVIDE SUITABLE STITCH PLATE OF SAME THICKNESS. E ADDITIONAL NOTES ON DET. 1/39.1 ELL DECK BRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS. EEL DECK SHALL CONFORM TO ASTM A446. 20VIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. D. WHEN EXAMINED THE CALVER SHEET CONSTRUCTURE SHALL BE PROVE 1/2" $3 5/8"$ $4"$ $6"5/8"$ $4 1/2"$ $4 3/4"$ $6 3/4"$	G	QUIVALENT.							
E GALVANIZED OR CORROSION RESISTANT. SCREWS SHALL CONFORM TO S.A.E. JT8. ELDING SHALL BE PERFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL LUING SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF THE LATEST AWS D1.3 CODE. 43 MIL AND LIGHTER SHEET TO SHEET - E60XX 54 MIL AND HEAVIER SHEET TO SHEET - E70XX MIL AND HEAVIER SHEET TO SHEET - E70XX MIL AND HEAVIER SHEET TO SHEET - E70XX MIT WELDS ON SPLICES SHALL BE USED AT ALL JOINTS IN TRACK. SPLICES IN STUDS OR RACES SHALL NOT BE PERMITTED. WHERE STUDS ARE BURNED THROUGH BY WELDING ROVIDE SUITABLE STITCH PLATE OF SAME THICKNESS. E ADDITIONAL NOTES ON DET. 1/59.1 TEL DECK ABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS. TEL DECK SHALL CONFORM TO ASTM A446. ROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. BILLING ILDING SHALL BL INST INTERVIEW. ABRICATE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. STOKED IN THE ANDITION OF ALL DIATES ON DET. 1/2" 3 5/8" 4" 6" 5/8" 4 1/2" 4 3/4" 6 3/4"	E	ELF-DRILLING FASTENERS HAVE BEEN DESIGNED IN ACCORDANCE WITH AISI "SPECIFICATION		GIVEN I	N MANUF	ACTURE	ER'S RECOMI	IENDAT	10
43 MIL AND LIGHTER SHEET TO SHEET - E60XX 54 MIL AND HEAVIER SHEET TO SHEET - E70XX61 THE ANOHONS SHALL BUILDED CONSTRUCTURAL APPLICA ANY ANCHOR FAILS TESTING, TEST AL TESTED UNTIL 20 CONSECUTIVE ANCHOR ROVIDE SUITABLE STITCH PLATE OF SAME THICKNESS.EE ADDITIONAL NOTES ON DET. 1/59.1 $MIN.$ TEEL DECK ABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS.TEEL DECK ROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. $MIN.$ TO SHELL DECK UNTIL 20 CONSULT AND		E GALVANIZED OR CORROSION RESISTANT. SCREWS SHALL CONFORM TO S.A.E. J78. RELDING SHALL BE PERFORMED BY CERTIFIED WELDERS IN A FABRICATION SHOP. ALL		(1704A	OF THE	CBC FC	DR DSA PRO	JECTS)	
UTT MELDS ON SPLICES SHALL BE USED AT ALL JOINTS IN TRACK. SPLICES IN STUDS OR RACES SHALL NOT BE PERMITTED. WHERE STUDS ARE BURNED THROUGH BY WELDING ROVIDE SUITABLE STITCH PLATE OF SAME THICKNESS.TESTED UNTIL 20 CONSECUTIVE ANCHOR CONCRETE AT TIME OF INSTALLATION CONCRETE AT TIME OF INSTALLATION COMPRESSIVE STRENGTHTEEL DECK ABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS.TEEL DECK SHALL CONFORM TO ASTM A446.ROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N.	1	43 MIL AND LIGHTER SHEET TO SHEET - E60XX		OF THE ALL SUC USED F	ANCHOI CH EXPA OR NON	RS SHAI NSION A -STRUC	LL BE TENSIC ANCHOR SHA TURAL APPLI	DN TEST	EI El 5,
SEE ADDITIONAL NOTES ON DET. 1/59.1 STEEL DECK FABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS. STEEL DECK SHALL CONFORM TO ASTM A446. PROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. G. CONCRETE AT TIME OF INSTALLATION COMPRESSIVE STRENGTH G. CONCRETE AT TIME OF INSTALLATION COMPRESSIVE STRENGTH STEEL DECK FABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS. STEEL DECK SHALL CONFORM TO ASTM A446. PROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N.	3	BRACES SHALL NOT BE PERMITTED. WHERE STUDS ARE BURNED THROUGH BY WELDING	$\sqrt{1}$	ANY AN TESTED	ICHOR F VINTIL 2	AILS TES O CONS	STING, TEST SECUTIVE ANO	ALL ANG CHORS F	⇒,∕
FIGEL DECKFABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS.TEEL DECK SHALL CONFORM TO ASTM A446.STEEL DECK SHALL CONFORM TO ASTM A446.PROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N.FIGURATE STEEL DECK SHALL CONFORMS, CLOSURE PLATES, ETC. U.O.N.					RESSIVE	STRENG	ЭТН		
FABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS.DIA.MIN. EMBEDHOLE DEPTHMIN. EDGE DISTANCESTEEL DECK SHALL CONFORM TO ASTM A446.3/8"2 1/4"2 5/8"4"PROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N.5/8"4 1/2"4 3/4"6 3/4"	5	TEEL DECK			2500 P	SI (NORI	MAL WEIGHT		
PROVIDE 16 GA MINIMUM EDGE FORMS, CLOSURE PLATES, ETC. U.O.N. 1/2" 3 5/8" 4" 6" 5/8" 4 1/2" 4 3/4" 6 3/4"	=	ABRICATE STEEL DECK IN ACCORDANCE WITH AISI SPECIFICATIONS.			EMBED	HOLE DEPT	E MIN. EDI TH DISTAN		1 P.
				1/2"	3 5/8"	4"	6"		٩
* FOR SINGLE ANCHORS WITH NO EDGE	;	EE DETAILS 2/57.2 AND 3/57.2 FOR ADDITIONAL REQUIREMENTS	*	3/4"	5 3/8"	5 3/4	4" 9"		13 A
* FOR SINGLE ANCHORS WITH NO EDGE FOR OTHER CASES, REDUCTION OF V ** TENSION TEST VALUES ONLY AND COP				FOR O	THER CA	SES, RE	DUCTION OF	VALUES	3
*** TENSION TEST VALUES ONLY AND CON TENSION LOADS.							CILL AND C		C

NCHORS PASS. (PER IR-19.1 FOR DSA PROJECTS ONLY) ONCRETE AT TIME OF INSTALLATION SHALL HAVE ATTAINED ITS MINIMUM DESIGN OMPRESSIVE STRENGTH MIN. WITH F'C= 2,500 PSI OR 3,000 PSI CC VERIFY MINIMUM EXISTING CONCRETE ST THREADED REINF. HOLE MIN. ANCHOR DOWEL DIAMETER EMBED ROD #3 1/2" DIA. 1/2" 3" #4 1/2" DIA. 5/8" 4" #5 5/8" DIA. 3/4" 5" #6 3/4" DIA. 7/8" 6" #7 7/8" DIA. 1" 7"

- #8 1" DIA. 1 1/8" 8" 5" 24" 5,670# INIMUM EDGE DISTANCE LIMITATION ASSUMED FROM ONE EDGE ONLY.
- THER CASES, REDUCTION OF VALUES CALCULATED PER ACI 318 IS REQUIRED.
- TRENGTH). (PANSION ANCHORS (HILTI)
- RIOR TO INSTALLATION PER SPECIFICATIONS.
- IVEN IN MANUFACTURER'S RECOMMENDATIONS FOR THE SPECIFIC ANCHOR.
- 704A OF THE CBC FOR DSA PROJECTS)
- ONCRETE AT TIME OF INSTALLATION SHALL HAVE ATTAINED ITS MINIMUM DESIGN OMPRESSIVE STRENGTH

			NG CONCRETI _ WEIGHT COM	E STRENGTH NCRETE) *	N FIELD. MIN.
DIA.	MIN. EMBED	MIN. HOLE DEPTH	MIN. EDGE DISTANCE	MIN. SPACING	TENSION TEST VALUE **
3/8"	2 1/4"	2 5/8"	4"	6"	1,509#
1/2"	3 5/8"	4"	6"	9 3/4"	3,267#
5/8"	4 1/2"	4 3/4"	6 3/4"	12"	4656#
3/4"	5 3/8"	5 3/4"	q "	13 1/4"	5,850#

- OR SINGLE ANCHORS WITH NO EDGE DISTANCE OR SPACING REDUCTION. OR OTHER CASES, REDUCTION OF VALUES CALCULATED PER ACI 318 IS REQUIRED. ENSION TEST VALUES ONLY AND CORRESPOND WITH 1.5X CRACKED CONCRETE SEISMIC
- ENSION LOADS.

3. MINIMUM COMPRESSIVE RESISTANCE AT 1% DEFORMATION: 3 psi

2. MINIMUM COMPRESSIVE STRENGTH: 40 psi

POXY ADHESIVE SHALL BE HILTI HIT-RE 500-V3 ADHESIVE ANCHOR (ESR-3814) OR EQUAL RODUCT. ALTERNATIVE PRODUCTS MUST BE SUBMITTED TO E.O.R. FOR SUBSTITUTION PRIOR

STALLATION: INSTALL THE EPOXY ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS IVEN IN MANUFACTURER'S RECOMMENDATIONS FOR THE SPECIFIC ANCHOR.

PECIAL INSPECTION SHALL BE PROVIDED IN ACCORDANCE WITH SECTION 1704 OF THE CBC.

OTIFY ARCHITECT IMMEDIATELY IF ELEMENTS WITH EXISTING STRUCTURE PREVENT DRILLING IN

POXIED DOWELS DO NOT SUBSTITUTE FOR HOOKED BARS. CONTRACTOR TO NOTIFY

HEN EPOXY ANCHORS ARE USED FOR SILL PLATE BOLTING, 10% OF THE ANCHORS SHALL

E TENSION TESTED. FOR ALL OTHER STRUCTURAL APPLICATIONS, ALL SUCH EPOXY ANCHOR HALL BE TENSION TESTED. WHEN EPOXY ANCHORS ARE USED FOR NON-STRUCTURAL PPLICATIONS, 50% OF ANCHORS SHALL BE TENSION TESTED. IF ANY ANCHOR FAILS TESTING, EST ALL ANCHORS OF THE SAME TYPE NOT PREVIOUSLY TESTED UNTIL 20 CONSECUTIVE

ONCRETE (NORMAL WEIGHT CONCRETE) FRENGTH IN FIELD. **					
) .	MIN. EDGE DISTANCE *	MIN. SPACING	TENSION TEST VALUE ***		
	1 7/8"	q"	1,160#		
	2 1/2"	12"	2,050#		
	3 1/8"	15"	3,150#		
	3 3/4"	18"	4,375#		
4 3/8" 21" 4,665#					

ALVES ARE FOR SINGLE ANCHORS WITH NO EDGE DISTANCE OR SPACING REDUCTION. FOR

ENSION TEST VALUES CORRESPOND WITH 1.5X CRACKED CONCRETE SEISMIC TENSION LOADS

XPANSION BOLTS SHALL BE HILTI KWIK-BOLT TZ-CARBON STEEL ANCHOR (ESR-1917) OR RUAL PRODUCT. ALTERNATE PRODUCTS MUST BE SUBMITTED TO E.O.R. FOR SUBSTITUTION

PROVIDE HILTI KWIK-BOLT 3 ANCHOR (ICC ESR-1385) AT MASONRY APPLICATION

STALLATION: INSTALL THE EXPANSION ANCHORS IN ACCORDANCE WITH THE REQUIREMENTS

PECIAL INSPECTION SHALL BE PROVIDED IN ACCORDANCE WITH SECTION 1704 OF THE CBC.

HEN EXPANSION ANCHORS ARE USED FOR SILL PLATE BOLTING AWAY FROM THE EDGE, 10% F THE ANCHORS SHALL BE TENSION TESTED. FOR ALL OTHER STRUCTURAL APPLICATIONS, LL SUCH EXPANSION ANCHOR SHALL BE TENSION TESTED. WHEN EXPANSION ANCHORS ARE BED FOR NON-STRUCTURAL APPLICATIONS, 50% OF ANCHORS SHALL BE TENSION TESTED. IF NY ANCHOR FAILS TESTING, TEST ALL ANCHORS OF THE SAME TYPE NOT PREVIOUSLY ESTED UNTIL 20 CONSECUTIVE ANCHORS PASS. (PER IR-19.1 FOR DSA PROJECTS ONLY)

- A. IN NORMAL WEIGHT CONCRETE: 0.157"Φ X-U FASTENER, 1" MIN. EMBEDMENT 3" EDGE DISTANCE, MIN. 4" O.C. SPACING.
- B. IN LIGHT WEIGHT CONCRETE: 0.157"Φ X-U FASTENER, 1 1/2" MIN. EMBEDMENT 3" EDGE DISTANCE, MIN. 4" O.C. SPACING.
- C. IN STRUCTURAL STEEL: 0.157"\$ X-U FASTENER, 1/2" MIN. EDGE DISTANCE, 1" MIN. SPACING. THE ENTIRE POINTED PORTION OF L.V.F. MUST COMPLETELY PENETRATE THE STEEL.
- D. IN CMU: 0.157" \$ X-U FASTENER, 1" MIN. EMBEDMENT.
- <u>TESTING</u>
- REQUIRED STRUCTURAL TESTS ARE LISTED ON THE ATTACHED SCHEDULE OF STRUCTURAL TESTS. SEE THE STATEMENT OF SPECIAL INSPECTION FOR ADDITIONAL REQUIREMENTS. <u>STRUCTURAL OBSERVATION</u> (BY HOHBACH-LEWIN, INC.)

OBSERVATION BY HOHBACH-LEWIN OR THEIR DESIGNATED REPRESENTATIVE IS REQUIRED AT THE PROJECT MILESTONES GIVEN BELOW. IT IS THE CONTRACTOR'S RESPONSIBILITY TO NOTIFY HOHBACH-LEWIN AT LEAST 24 HOURS IN ADVANCE OF COMPLETING MILESTONES THAT REQUIRE OBSERVATION AND ALLOW SUITABLE TIME TO MAKE ANY REQUIRED CORRECTIONS TO THE WORK PRIOR TO ENGAGING IN THE NEXT PHASE OF THE PROJECT. STRUCTURAL OBSERVATIONS WILL BE PERFORMED PER CBC 1704.6.1.

- PRIOR TO PLACING CONCRETE: HOHBACH-LEWIN SHALL OBSERVE PLACEMENT OF REINFORCING. POST-TENSIONED STRANDS, EMBEDMENTS AND CAST-IN ANCHORAGES TO CONCRETE.
- PRIOR TO CONCEALMENT BY FINISHES: HOHBACH-LEWIN SHALL OBSERVE

1. STEEL MOMENT AND BRACED FRAME ELEMENTS AND CONNECTIONS. 2. WOOD SHEAR WALL NAILING AND FRAMING HARDWARE.

3. HOLDOWNS AND TIEDOWN SYSTEMS.

CONTRACTOR SUBMITTALS

THE FOLLOWING IS A LISTING OF REQUIRED ITEMS TO BE SUBMITTED TO STRUCTURAL ENGINEER OF RECORD (TO BE PROVIDED IF MARKED):

SUBMITTAL	CERTIFICATE	SHOP DRAWINGS (2)	CALCS W/ ENG. STAMP	DEFERRED SUBMITTAL (1)
CONCRETE REINF. STEEL	×	X		
CONCRETE MIX DESIGN		X X X		
SHOTCRETE MIX DESIGN		X		
SHOTCRETE PRE- CONCTRUCTION TEST PANEL		×		
UNBONDED P/T CONCRETE				
PLANS, ELEVATIONS, SECTIONS		×		
TENDON LAYOUT AND SUPPORT REINF.	×	×	×	×
STRESSING REPORTS	×			
ELONGATION REPORTS	X X			
CONCRETE MASONRY UNITS				
GROUT MIX DESIGN	×			
PRE-CONSTRUCTION PRISM TESTS	×			
MASONRY PRISM TESTS	×			
REINFORCING STEEL	X X X X	X		
STRUCTURAL STEEL	×	X X X		
GLUE LAMINATED BEAMS	×	X		
DRILLED DISPLACEMENT COLUMN (DDC)		×	×	×
STEEL STAIR SYSTEM		×		X
ELEVATORS		X X X		X X X
STOREFRONT SYSTEMS		X	X	X
COLD-FORMED STEEL EXTERIOR CURTAIN WALLS		×	×	×
STEEL AWNINGS/CANOPIES		×	X X	X
CONTINUOUS ROD TIEDOWN SYSTEM		X	X	X

- (1) DEFERRED SUBMITTALS SHALL FIRST BE SUBMITTED TO THE PROJECT ARCHITECT AND/OR ENGINEER FOR REVIEW AND COORDINATION, THEN SUBMITTED TO THE APPROPRIATE JURISDICTION FOR APPROVAL. THIS SUBMITTAL SHALL INCLUDE HOHBACH-LEWIN'S SHOP DRAWING STAMP INDICATING THE STRUCTURAL REVIEW HAS BEEN COMPLETED AND THAT THE PLANS AND CALCULATIONS FOR THE DEFERRED APPROVAL ITEMS ARE IN GENERAL COMPLIANCE WITH THE INFORMATION PROVIDED WITHIN THE CONTRACT DOCUMENTS.
- (2) ELECTRONIC SHOP DRAWINGS ARE TO BE SUBMITTED TO HOHBACH-LEWIN FOR REVIEW. AT HOHBACH-LEWIN'S REQUEST, THE CONTRACTOR IS RESPONSIBLE FOR PROVIDING HARD COPIES OF SHOP DRAWINGS FOR REVIEW.

NAILING SCHEDULE (CBC TABLE 2304.10.1)

	-		-			
	CONNECTION ¹	NAILING ²	NAILING ²			
1.	Blocking between ceiling joists,	(3) 8d	Ea. end, toenail			
'`	rafters or trusses to top plate	(3) 84				
	or other framing below (Roof)					
2.	Ceiling joist to top plate	(3) 8d	Toenail			
3.	Ceiling joist not attached to parallel rafter, laps over partitions	(3) 16d	Face nail			
4.	Collar tie to rafter	(3) 10d	Face nail			
5.	Rafter or roof truss to top plate	(3) 10d	Toenail			
6.	Roof rafters to ridge valley or hip rafters;	(2) 16d	End nail			
	or roof rafter to 2 inch ridge beam.	(3) 10d	Toenail			
7.		16d	24" o.c face nail			
	Stud to stud (not a braced wall panels)					
8.	Stud to stud and abutting studs at intersecting wall corners (at braced wall panels)	16d	16" o.c face nail			
٩.	Built-up header (2" to 2" header)	16d	16" o.c each edge, face nail			
10.	Continuous header to stud	(4) 8d	Toenail			
11.	Top plate to top plate	16d	16" o.c face nail			
12.	Top plate to top plate, at end joints	(8) 16d	Each side of end joint			
13.	Bottom plate to joist, rim joist, band joist or blocking (not at braced wall panels)	16d	12" o.c face nail			
14.	Bottom plate to joist, rim joist, band joist or blocking at braced wall panels	(2) 16d	16" o.c face nail			
15.	- .	(4) 8d	Toenail			
	Top plate to stud	(2) 16d	End nail			
17.	Top plates, laps at corners and intersections	(2) 16d	Face nail			
18.	Joist to sill, top plate, or girder	(3) 8d	Toenail			
19.		8d	6" o.c toenail			
20.	2" planks (plank & beam - floor & roof)	(2) 16d	Each bearing			
		200	32" O.C face nail			
21	Built-up girders and beams, 2" lumber layers		at top and bottom staggered on sides.			
Z 1.	Dunt-up yn der 5 and Deams, 2 iumber lager 5	(2) 20d	Ends and at each splice, face nail			
22.	Ledger strip supporting joists or rafters	(3) 16d	Each joist or rafter, face nail			
23.	Joists to band joist or rim joist	(3) 16d	End nail			
	Bridging or blocking to joist, rafter	(2) 8d	Each end toe nail			
	or truss (floor)	, _,				
NOT						
1.	 NAILING PER SCHEDULE ABOVE IS TO BE USED WHERE NAILING IS NOT SPECIFIED ON PLANS OR DETAILS. NAILING PER PLANS AND DETAILS SUPERCEDE NAILING SCHEDULE UNLESS APPROVED BY ENGINEER. 					
2.	2. NAIL SPECIFIED ARE COMMON: 8d= 2 1/2"x0.131" 10d= 3"x0.148" 16d= 3 1/2"x0.162"					
	FOR ALTERNATE NAILING AND INFORMATION NOT SHOWN, SEE COMPLETE TABLE CBC 2304.10.1					

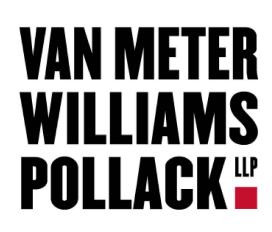
NAIL DESIGNATION AND SIZES				
NAIL	DIAMETER (in			
16d COMMON	0.162			
16d SINKER	0.148			
10d COMMON	0.148			
10d SHORT	0.148			
8d COMMON	0.131			

2

ABBREVIATIONS

	AND	MAX.	MAXIMUM
)	AT	MECH.	MECHANICAL
.В.	ANCHOR BOLT	MANUF.	MANUFACTURER
	ADDITIONAL	M.B.	MACHINE BOLTS
	ARCHITECTURAL	MIN.	MINIMUM
κΥ.C.	ALASKAN YELLOW CEDAR	MISC.	MISCELLANEOUS
		MTL.	METAL
	BUILDING	N 1	
	BLOCKING	N	NORTH
3M.	BEAM	(N)	
3.N.	BOUNDARY NAIL	NO.	NUMBER
	BOTTOM OF CONCRETE	N.S.	NEAR SIDE
30T.	BOTTOM	N.T.S.	NOT TO SCALE
		O.C.	ON CENTER
	CENTER LINE CANTILEVER	0.C. 0PG.	ON CENTER OPENING
ANT. BC	CALIFORNIA BUILDING	OPP.	OPPOSITE
	CODE	ОГР. О.Н.	OPPOSITE HAND
D.F.	CONTROLLED DENSITY FILL	0.n. 0550	OREGON STRUCTURAL
.G.S.	CENTER OF GRAVITY OF	0000	SPECIALTY CODE
.0.0.	POST-TENSIONING STRAND	0.W.S.J.	OPEN WEB STEEL JOIST
J.I.P.	CAST-IN-PLACE	0.M.M.J.	OPEN WED WOOD JOIST
У.J.	CONTROL JOINT		
LR.	CLEAR	Æ	PLATE
MU	CONCRETE MASONRY UNIT	PERP.	PERPENDICULAR
OL.	COLUMN	PLY	PLYWOOD
OMP.	COMPRESSION	P.T.	PRESERVATIVE TREATED
ONC.	CONCRETE	P/T	POST-TENSIONS
ONN.	CONNECTION	PSL	PARALLEL STRAND
ONT.	CONTINUOUS		LUMBER
TR.	CENTER		
		R.C.J.	ROUGHENED
BL	DOUBLE		CONSTRUCTION JOINT
D.D.C.	DRILLED DISPLACEMENT	REINF.	REINFORCEMENT
C		REQD.	REQUIRED
PET.	DETAIL		
2.F.	DOUGLAS FIR	5	SOUTH
NA.	DIAMETER	S.A.D.	SEE ARCHITECTURAL
0	DITTO		DRAWINGS
DMG.	DRAWINGS	5.C.	SLIP CRITICAL
	FLOT	S.C.D.	SEE CIVIL DRAWINGS
-)	EAST	SCHED.	SCHEDULE
E)	EXISTING	SDS	SELF-DRIVING SCREW
A.	EACH	SIM.	SIMILAR
.B.M.	EXTERIOR BUILDING MAINTENANCE	S.J.	SEISMIC JOINT
.F.	EACH FACE		SYSTEM
г 	EXPANSION JOINT	5.M.D.	SEE MECHANICAL
 E.	ELEVATION		DRAWINGS
. 	EDGE NAIL	SMS	SHEET METAL SCREW
Ν. Ε.Μ.	EACH WAY	5.0.G.	SLAB-ON-GRADE
XP.	EXPANSION	SPEC.	SPECIFICATION
EXT.	EXTERIOR	5Q.	SQUARE
		5.5. CTD	STAINLESS STEEL
		STD.	STANDARD
DN.	FOUNDATION	SSH	SHORT SLOTTED HOLE
IN.	FINISH	SYM.	SYMMETRICAL
.F.	FINISH FLOOR	T +D	TOP AND POTTON
.G.	FINISHED GRADE	T&B T&G	TOP AND BOTTOM TONGUE AND GROOVE
LR.	FLOOR	TD	TIEDOWN
.N.	FIELD NAIL	T.O.C.	TOP OF CONCRETE
.O.C.	FACE OF CONCRETE	T.O.F.	TOP OF FOOTING
.O.S.	FACE OF STUD	T.O.S.	TOP OF STEEL FRAMING
.R.T.	FIRE RETARDANT TREATED	T.O.P.	TOP OF PLATE/ TOP OF
.S.	FAR SIDE	1.0.1	PARAPET
TG.	FOOTING	TRANS.	
		TYP.	TYPICAL
A.	GAUGE		
B	GRADE BEAM	U.O.N.	UNLESS OTHERWISE NOTED
.С.	GENERAL CONTRACTOR	U.T.	ULTRASONIC TESTING
LB	GLUE LAMINATED (BEAM)		
		VERT.	VERTICAL
CA	HEADED CONC. ANCHOR		VERIFY IN FIELD
	(STUD)		
D	HOLDOWN	М	WEST
DR	HEADER	W/	MITH
GR.	HANGER	MF	WIDE FLANGE
ORIZ.	HORIZONTAL	M.H.S.	WELDED HEADED STUD
IT.	HEIGHT	M.J.	WALL JOINT
I.S.	HIGH STRENGTH	W/O	WITHOUT
	HIGH STRENGTH BOLTS	M.P.	WORK POINT
155 1661	HOLLOW STEEL SECTION		
ISSH	HORIZONTAL SHORT		
	SLOTTED HOLES		
NT.	INTERIOR		
NI.			
.H.	JOIST HANGER		
.8.	JUIUT FIRINGER		
I H	I ONG I FE LOPIT		
	LONG LEG HORIZ. LONG LEG VERT.		
	LONG LEG VERT. LONG SLOTTED HOLE		
.SH .SL	LONG SLOTTED HOLE LAMINATED STRAND		
.UL	LAMINATED STRAND LUMBER		
.ONG.	LONGITUDINAL		
unu.			
V.F.	LOW-VELOCITY FASTENER		
	LAMINATED VENEER		
	LUMBER		





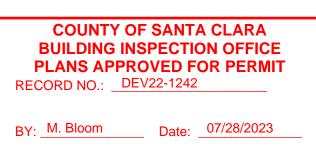
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- MEP ENGINEER EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- SUSTAINABILITY/ENERGY **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521
- JOINT TRENCH/DRY UTILITY MILLENIUM DESIGN PO BOX 737 ALAMO, CA, 94507



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HARD COPY OF THESE STAMPED PLANS MUST BE ON THE SITE FOR INSPECTIONS

ID	DATE	NAME
1	11/11/2022	PERMIT SET-CONV
Α	12/16/2022	BID SET
В	03/20/2023	BID ADDENDUM
2	03/20/2023	PLAN CHECK RESPONSE 2
3	05/12/2023	PLAN CHECK RESPONSE 3

Project:



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STRUCTURAL GENERAL NOTES

SPECIAL INSPECTIONS

SPECIAL INSPECTIONS AND TESTING WILL BE PERFORMED IN ACCORDANCE WITH THE APPROVED PLANS AND SPECIFICATIONS, THE ATTACHED "STATEMENT OF SPECIAL INSPECTION" AND CBC SECTIONS 1704, 1705.

EACH CONTRACTOR RESPONSIBLE FOR THE CONSTRUCTION OF A SEISMIC FORCE RESISTING SYSTEM, DESIGNATED SEISMIC SYSTEM OR A SEISMIC RESISTING COMPONENT LISTED IN THE STATEMENT OF SPECIAL INSPECTIONS SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE BUILDING OFFICIAL AND THE OWNER PRIOR TO THE COMMENCEMENT OF THE WORK ON THE SYSTEM OR COMPONENT. THE CONTRACTOR'S STATEMENT OF RESPONSIBILITY SHALL CONTAIN THE FOLLOWING:

- 1. ACKNOWLEDGEMENT OF AWARENESS OF THE SPECIAL REQUIREMENTS CONTAINED IN THE STATEMENT OF SPECIAL INSPECTIONS.
- 2. ACKNOWLEDGEMENT THAT CONTROL WILL BE EXERCISED TO OBTAIN CONFORMANCE WITH THE CONSTRUCTION DOCUMENTS APPROVED BY THE BUILDING OFFICIAL.
- 3. PROCEDURES FOR EXERCISING CONTROL WITHIN THE CONTRACTOR'S ORGANIZATION, THE METHOD AND FREQUENCY OF REPORTING AND THE DISTRIBUTION OF THE REPORTS.
- 4. IDENTIFICATION AND QUALIFICATION OF THE PERSON(S) EXERCISING SUCH CONTROL AND THEIR POSITION(S) IN THE ORGANIZATION.

STATEMENT OF SPECIAL INSPECTIONS

THIS STATEMENT OF SPECIAL INSPECTION IS SUBMITTED IN FULFILLMENT OF THE REQUIREMENTS OF CBC SECTIONS 1704.3. THE FOLLOWING ATTACHMENTS SUMMARIZE THE SPECIAL INSPECTIONS AND STRUCTURAL TESTS REQUIRED FOR THIS PROJECT.

THESE REQUIREMENTS HAVE ALSO BEEN MADE PART OF THE APPROVED PLANS. THE OWNER RECOGNIZES HIS OR HER OBLIGATION TO ENSURE THAT THE CONSTRUCTION COMPLIES

WITH THE APPROVED PERMIT DOCUMENTS AND TO IMPLEMENT THIS PROGRAM OF SPECIAL INSPECTIONS. IN PARTIAL FULFILLMENT OF THESE OBLIGATIONS, THE OWNER WILL RETAIN AND DIRECTLY PAY FOR THE SPECIAL INSPECTIONS AS REQUIRED IN CBC SECTION 1704.2 SPECIAL INSPECTORS WILL REFER TO THE APPROVED PLANS AND SPECIFICATIONS, THE ABOVE

REFERENCED SCHEDULES, AND THE RELEVANT CBC SECTIONS FOR DETAILED SPECIAL INSPECTION REQUIREMENTS. ANY ADDITIONAL TESTS AND INSPECTIONS REQUIRED BY THE APPROVED PLANS AND SPECIFICATIONS WILL ALSO BE PERFORMED.

INTERIM INSPECTION REPORTS SHALL BE SUBMITTED TO THE BUILDING DEPARTMENT AND TO HOHBACH-LEWIN, INC. IN ACCORDANCE WITH CBC SECTION 1704.2.4. A FINAL REPORT OF SPECIAL INSPECTIONS DOCUMENTING REQUIRED SPECIAL INSPECTIONS, TESTING, AND CORRECTION OF ANY DEFICIENCIES NOTED IN THE INSPECTIONS SHALL BE SUBMITTED PRIOR TO ISSUANCE OF A CERTIFICATE OF USE AND OCCUPANCY.

THIS PLAN HAS BEEN DEVELOPED WITH THE UNDERSTANDING THAT THE BUILDING OFFICIAL WILL:

- REVIEW AND APPROVE THE QUALIFICATIONS OF THE SPECIAL INSPECTORS PERFORMING THE INSPECTIONS
- MONITOR SPECIAL INSPECTION ACTIVITIES TO ASSURE COMPLIANCE WITH PROJECT REQUIREMENTS REVIEW SUBMITTED INSPECTION REPORTS PERFORM INSPECTION

WIND + SEISMIC REQUIREMENTS (CBC SECTION 1704.3.2 & 1704.3.3) THE STATEMENT OF SPECIAL INSPECTIONS SHALL IDENTIFY THE SEISMIC AND WIND FORCE LATERAL

RESISTING SYSTEMS THAT ARE SUBJECT TO SPECIAL INSPECTIONS PER CBC SECTIONS 1705.12 OR 1705.13 AND 1705.11

THE EXTENT OF THE SEISMIC LOAD RESISTING SYSTEM IS DEFINED IN MORE DETAIL IN THE CONSTRUCTION DOCUMENTS.

SCHEDULE OF SPECIAL INSPECTIONS

- INSPECTION FREQUENCY
- C CONTINUOUS INSPECTION P - PERIODIC INSPECTION
- X DENOTES INSPECTION THAT IS EITHER ONE-TIME OR AT A FREQUENCY DEFINED IN SOME OTHER MANNER N - INSPECTION IS NOT REQUIRED

* STEEL CONSTRUCTION PER CBC SECTION 1705.2.

ITEM			NOTES
1.	SPECIAL INSPECTION FOR STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE QUALITY ASSURANCE INSPECTION REQUIREMENTS OF AISC 360		SEE CBC SECTION 1705.2.1
2.	MATERIAL VERIFICATION OF HIGH- STRENGTH BOLTS, NUTS, AND WASHERS:		
	A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	P	SEE APPLICABLE ASTM MATERIAL SPECIFICATIONS AND AISC 360 SECTION A3.3
	B. MANUFACTURER'S CERTIFICATE OF COMPLIANCE	P	
З.	MATERIAL VERIFICATION OF STRUCT. STEEL		SEE AISC 360 SECTION A3 FOR ASTM DESIGNATIONS
	A. IDENTIFICATION MARKINGS TO CONFORM TO ASTM STANDARDS SPECIFIED IN THE APPROVED CONSTRUCTION DOCUMENTS	×	
	B. MANUFACTURER'S MILL TEST REPORTS	×	SEE ASTM A6 OR ASTM A 568
4.	INSPECTION OF HIGH-STRENGTH BOLTING		AISC 360 SECTION N5.6 AND TABLES N5.6-1, N5.6-2 & N5.6-3
5.	INSPECTION OF WELDING		AISC 360 SECTION N5.4, N5.5 AND TABLES N5.4-1, N5.4-2 & N5.4-3
	A. STRUCTURAL STEEL		
	1. COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	С	SEE AWS D1.1.
	2. MULTI-PASS FILLET WELDS	C	SEE AWS D1.1.
	3. SINGLE-PASS FILLET WELDS > 5/16"		SEE AWS D1.1.
	4. SINGLE-PASS FILLET WELDS \leq 5/16"	P	SEE AWS D1.1.
	5. FLOOR AND ROOF DECK WELDS	P	SEE AWS D1.1.
	6. STUDS USED FOR STRUCTURAL DIAPHRAGMS	P	SEE AMS D1.1.
	7. WELDED SHEET STEEL FOR STUDS AND JOISTS	P	
	8. STAIR AND RAILING SYSTEMS	P	
6.	INSPECTION OF STEEL FRAME JOINT DETAILS FOR COMPLIANCE W/ APPROVED CONSTRUCTION DOCUMENTS	P	SEE SLRS QUALITY ASSURANCE PLAN ON THE STRUCT. DRAWINGS FOR ADD'L REQUIREMENTS
٦.	COLD FORMED STEEL DECK		SPECIAL INSPECTIONS PER SDI REQUIREMENTS & CBC 1705.2.2

	• CONCRETE CONSTRUCTION PER CBC 1 TABLE 1705.3 CONCRETE
5	ITEM 1. INSPECTION OF REINFORCING STEEL,
	INCLUDING PRESTRESSING TENDONS A PLACEMENT.
	 INSPECTION OF REINFORCING STEEL M A. VERIFY WELDABILITY OF REINFORCE BARS OTHER THAN ASTM A706:
	 B. INSPECT SINGLE-PASS FILLET WELL MAXIMUM 5/16" C. INSPECT ALL OTHER WELDS.
	3. INSPECT ANCHORS TO BE CAST IN CO
	4. INSPECTION OF ANCHORS POST-INSTA HARDENED CONCRETE MEMBERS
	A. ADHESIVE ANCHORS INSTALLED IN HORIZONTALLY OR UPWARDLY INC ORIENTATIONS TO RESIST SUSTAINE TENSION LOADS. B. MECHANICAL ANCHORS AND ADHE
	5. VERIFY USE OF REQUIRED MIX DESIGN
	 6. PRIOR TO CONCRETE PLACEMENT, FABRICATE SPECIMENS FOR STRENGT TESTS, PERFORM SLUMP AND AIR COI TESTS, AND DETERMINE THE TEMPERA OF THE CONCRETE.
	7. INSPECTION OF CONCRETE AND SHOT PLACEMENT FOR PROPER APPLICATION TECHNIQUES, INCLUDING TEST PANELS.
Ē	8. INSPECTION FOR MAINTENANCE OF PR SPECIFIED CURING TEMPERATURE AND TECHNIQUES.
	9. INSPECTION OF PRESTRESSED CONCR A. APPLICATION OF PRESTRESSING F
	B. GROUTING OF BONDED PRESTRES TENDONS IN THE SEISMIC FORCE
	RESISTING SYSTEM. 10. INSPECTION OF PRECAST CONCRETE
	MEMBERS. 11. VERIFICATION OF IN-SITU CONCRETE STRENGTH, PRIOR TO STRESSING OF TENDONS IN POST-TENSIONED CONCR AND PRIOR TO REMOVAL OF SHORES FORMS FROM BEAMS AND STRUCTUR,
	12. INSPECT FORMWORK FOR SHAPE, LOC
	AND DIMENSIONS OF CONCRETE MEMI BEING FORMED.
	 POST-INSTALLED ANCHORS: A. VERIFY DIAMETER AND DEPTH OF TO CONFORM TO MANUFACTURER'S GUIDELINES AND VALUES SHOWN O STRUCTURAL DRAWINGS.
	B. VERIFY EPOXY OR EXPANSION AND OF TYPE INDICATED ON THE STRUC
	DRAWINGS. C. OBSERVE MIXING OF EPOXY AND INSTALLATION OF ANCHORS. VERIF TORQUE OF EXPANSION ANCHORS.
	MASONRY CONSTRUCTION PER CBC 5 ACI 530 TABLE 3.1.2 ITEM
	1. VERIFY COMPLIANCE WITH THE APPROV SUBMITTALS
	2. AS MASONRY CONSTRUCTION BEGINS, THAT THE FOLLOWING ARE IN COMPLIA A. PROPORTIONS OF SITE-PREPARED
	B. CONSTRUCTION OF MORTAR JOINTS
	C. GRADE AND SIZE OF PRESTRESSING TENDONS AND ANCHORAGES
	D. LOCATION OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES
	E. PRESTRESSING TECHNIQUE F. PROPERTIES OF THIN-BED MORTAR AAC MASONRY
	3. PRIOR TO GROUTING, VERIFY THAT THE FOLLOWING ARE IN COMPLIANCE:
	A. GROUT SPACE B. GRADE, TYPE, AND SIZE OF REINFORCEMENT AND ANCHOR BOL AND PRESTRESSING TENDONS AND
	ANCHORAGES C. PLACEMENT OF REINFORCEMENT, CONNECTORS, AND PRESTRESSING TENDONS AND ANCHORAGES
	D. PROPORTIONS OF SITE-PREPARED AND PRESTRESSING GROUT FOR BO TENDONS
	E. CONSTRUCTION OF MORTAR JOINTS 4. VERIFY DURING CONSTRUCTION:
	A SIZE AND LOCATION OF STRUCTURA ELEMENTS
	 B. TYPE, SIZE, AND LOCATION OF ANCH INCLUDING OTHER DETAILS OF ANCHORAGE OF MASONRY TO STRUCTURAL MEMBERS, FRAMES, C OTHER CONSTRUCTION C. WELDING OF REINFORCEMENT
	D. PREPARATION, CONSTRUCTION, AND PROTECTION OF MASONRY DURING WEATHER (TEMPERATURE BELOW 40
	(44°C) OR HOT WEATHER (TEMPERA ABOVE °90F (32.2°C) E. APPLICATION OF MEASUREMENT OF PRESTRESSING FORCE
	F. PLACEMENT OF GROUT AND PRESTRESSING GROUT FOR BONDE TENDONS IS IN COMPLIANCE
	G. PLACEMENT OF AAC MASONRY UNIT CONSTRUCTION OF THIN-BED MORT JOINTS
	5. OBSERVE PREPARATION OF GROUT

SPECIMENS, MORTAR SPECIMENS, ANI PRISMS * FOR FIRST 5000 SQ. FT. OF AAC MASONRY

1705.3

	FREQ.	NOTES
L,	P	SEE ACI 318: 20, 25.2, 25.3,
L, S AND		26.6.1-26.5.3
L WELDING		SEE AWS D.1.4; ACI 318: 26.6.4
DRCING	Ρ	
ELDS,	P	
	C	
CONCRETE	Ρ	ACI 318: 17.8.2
STALLED IN		
IN INCLINED AINED	С	ACI 318: 17.8.2.4
HESIVE	P	ACI 318: 17.8.2
GN	P	SEE ACI 318: CH.19, 26.4.3, 26.4.4
IGTH CONTENT RATURE	C	SEE ASTM C 172, C 31; ACI 318: 26.5, 26.12
OTCRETE ATION LS.	С	SEE ACI 318: 26.5
PROPER ND	P	SEE ACI 318: 26.5.3-26.5.5
ICRETE:		
5 FORCES.	C	SEE ACI 318: 26.10
RESSING E	С	SEE ACI 318: 26.10
Ē	P	SEE ACI 318: CH. 26.9
E OF CRETE RES AND URAL	P	SEE ACI 318: 26.11.2
LOCATION EMBERS	P	SEE ACI 318: 26.11.2 (b)
OF HOLE ER'S N ON THE	С	
ANCHOR IS RUCTURAL	×	SUBSTITUTIONS MUST BE SUBMITTED TO THE E.O.R. FOR ACCEPTANCE PRIOR TO USE.
D RIFY RS.	С	SEE DRAWINGS FOR LOCATIONS WHERE INSPECTION IS NOT REQUIRED.

SECTION 1705.4

	FREQ.	NOTES
OVED	Ρ	ACI 530.1 ART 1.5
5, VERIFY _IANCE:		
ED	Ρ	ACI 530.1 ART 2.1, 2.6A
NTS	Ρ	ACI 530.1 ART 3.3B
bing	P	ACI 530.1 ART 2.4B, 2.4H
NG	Ρ	ACI 530.1 ART 3.4, 3.6A
	P	ACI 530.1 ART 3.6B
AR FOR	*	ACI 530.1 ART 2.1C
ΉE		
	Ρ	ACI 530.1 ART 3.2D, 3.2F
BOLTS, ND	Ρ	ACI 530 SECTION 6.1 ACI 530.1 ART 2.4, 3.4
, NG	P	ACI 530 SECTION 6.1, 6.2.1, 6.2.6, 6.2.7 ACI 530.1 ART 3.2E, 3.4, 3.6A
ED GROUT BONDED	P	ACI 530.1 ART 2.6B, 2.4G.1.b
NTS	P	ACI 530.1 ART 3.3B
RAL	£	ACI 530.1 ART 3.3F
NCHORS, , OR	P	ACI 530 SECTION 1.2.1 (e), 6.1.4.3, 6.2.1
	С	ACI 530 SECTION 8.1.6.7.2, 9.3.3.4 (c), 11.3.3.4 (b)
AND NG COLD 1 40°F IRATURE	P	ACI 530.1 ART 1.8C, 1.8D
OF	C	ACI 530.1 ART 3.6B
DED	C	ACI 530.1 ART 3.5, 3.6C
NITS AND RTAR	د*	ACI 530.1 ART 3.3B.9, 3.3F.1.b
	Ρ	ACI 530.1 ART. 1.4B.2.a.3, 1.4B.2.b.3, 1.4B.2.c.3, 1.4B.3, 1.4B.4

SOILS PER CBC SECTION 1705.6 TABLE 1705.6 SOILS

ITEM			NOTES
1.	VERIFY MATERIALS BELOW FOOTINGS ARE ADEQUATE TO ACHIEVE THE DESIRED BEARING CAPACITY.	P	INSPECTIONS REQUIRED IN THIS TABLE SHALL BE PERFORMED BY THE PROJECT GEOTECHNICAL ENGINEER.
2.	VERIFY EXCAVATIONS ARE EXTENDED TO PROPER DEPTH AND HAVE REACHED PROPER MATERIALS.	P	
З.	PERFORM CLASSIFICATION AND TESTING OF CONTROLLED FILL MATERIALS.	P	
4.	VERIFY USE OF PROPER MATERIALS, DENSITIES AND LIFT THICKNESSES DURING PLACEMENT AND COMPACTION OF CONTROLLED FILL.	C	
5.	PRIOR TO PLACEMENT OF CONTROLLED FILL, OBSERVE SUBGRADE AND VERIFY THAT SITE HAS BEEN PREPARED PROPERLY.	P	

WOOD CONSTRUCTION PER CBC SECTION 1705.5

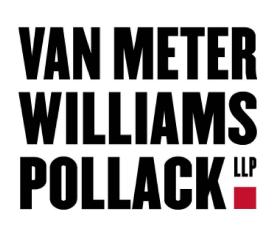
ITEM	FREQ.	NOTES
1. INSPECT SITE-BUILT ASSEMBLIES:		SEE CBC SECTION 1705.12.2
A. FIELD GLUING OF ELEMENTS OF THE SLRS.	C	
B. NAILING, BOLTING, ANCHORING, AND OTHER FASTENING OF COMPONENTS WITHIN THE SLRS, INCLUDING SHEAR WALLS, DIAPHRAGMS, DRAG STRUTS, BRACES, SHEAR PANELS, AND HOLD-DOWNS.	P	SPECIAL INSPECTION IS NOT REQUIRED FOR SHEAR WALLS, DIAPHRAGMS, AND SHEAR PANELS, INCLUDING ATTACHMENT TO OTHER COMPONENTS OF THE SLRS WHERE THE NOMINAL PANEL EDGE NAILING IS MORE THAN 4 INCHES ON CENTER.

SCHEDULE OF STRUCTURAL TESTS

STRUCTURAL TESTING FOR SEISMIC RESISTANCE CBC SECTION 1705.13

ITEN	Ч	FREQ.	NOTES
1.	STRUCTURAL STEEL:		
	A. TESTING CONTAINED IN THE QUALITY ASSURANCE PLAN AS REQUIRED BY AISC 341.	Y	SEE THE SLRS NOTES FOR TESTING REQUIREMENTS AND CBC 1705.13.1
2.	NON-STRCUTURAL COMPONENTS:		
	A. ARCHITECTURAL, MECHANICAL, AND ELECTRICAL COMPONENT, SUPPORTS AND ATTACHMENTS	Y	SEE CBC SECTON 1705.13.2 AND ASCE 7 13.2.1
З.	SPECIAL CERTIFICATION REQUIREMENTS FOR DESIGNATED SEISMIC SYSTEMS.	N	SEE CBC SECTION 1705.13.3 AND ASCE 7 13.2.2
4.	SEISMICALLY ISOLATED STRUCTURES	N	OBTAIN SYSTEM TESTS AS REQUIRED BY ASCE 7 SECTION 17.8 AND CBC 1705.13.4

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(4) #7X10B NOCATES (4) IT RENE BARS X 10-01 LONG BOTTOM REINT (4) #7X10T NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL TOF RENE. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTOM REINT. (4) #7X10B3B NOCATES (4) IT RENE BARS X 10-01 LONG AT 91 COL BOTOM REINT. (4) #7X10B3B DENOTES COMPRESENT LAF PER DET. 2/55.6 (5) TOFNE-01 AT 121 COL TYP. UON. BEARM FER FLAN AND FOST SUFFORT FER DET. 1/25.6 (5) #70F10F10F1 AT 121 COL TYP. UON. BEARM FER FLAN AND FOST SUFFORT FER DET. 1/25.6 (6) #70F10F10F10F1 AT 121 COL TYP. UON. BEARM FER FLAN AND FOST SUFFORT FER DET. 1/25.6 (7) #70F10F10F10F10F10F10F10F1 BEARM FER FLAN AND FOST SUFFORT FER DET. 4/25.6 (7) #70F10F10F10F10F10F10F10F10F10F1	ULE
(4) #7x10T NDCATES (4) #T REINF. BARS x 10-0" LONG TOP REINF. (4) #7x1083T NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (4) #7x1083B NDCATES (4) #T REINF. BARS x 10-0" LONG AT 9" 0.C. (5) TOPXIO-0" AT 12" 0.C. TYP. U.O.N. EEXEMPTE SPACE, BED ET 1250.7 (5) TOPXIO-0" AT 12" 0.C. TYP. U.O.N. EEXEMPTE SPACE, BED ET 1250.7 (6) #T TOPXIO-0" AT 12" 0.C. TYP. U.O.N. EEXEMPTE SPACE, BED ET 1250.7 (7) TOPXIO-0" AT 12" 0.C. TYP. U.O.N. EEXEMPTE SPACE, BED ET 1250.7 (7) TOPXIO-0" AT 12" 0.C. TYP. U.O.N. EEXEMPTE SPACE, BED ET 1250.7 (8) FOPXIO-0" AT 12" 0.C. TYP. U.O.N. EEXEMPTE SPACE, BED	
(4) #7x10@3T NDCATES (4) *** RENF. BARS × 10~0° LONG AT 3° O.C. PARAFET NOTE: (4) #7x10@33B NDCATES (4) *** RENF. BARS × 10~0° LONG AT 3° O.C. POTOTES CARS B TENSION LAP PER DET. 2/65.1, TYF. (4) #7x10@33B DENOTES CARS B TENSION LAP PER DET. 2/65.1, TYF. CONTEX OF STRAP (4) #7x10@33F DENOTES CARS B TENSION LAP PER DET. 2/65.1, TYF. CONTEX OF STRAP (4) #7x10@33F DENOTES CARS B TENSION LAP PER DET. 2/65.1, TYF. CONTEX OF STRAP (4) #7x10@33F DENOTES CARS B TENSION LAP PER DET. 2/65.1, TYF. CONTEX OF STRAP (5) TOPX/0~0° AT 12° O.C. TYF. JO.N. SEE 5C2.2B PARAFET DRACE. SEE DET. 12/85.7 (6) #100 CONTEX SCHOLE AND OFFSCT FROM ALL, SEE SCHOLE AND POST SUPPORT PER DET. 12/85.7 EDARM FER PLAN AND POST SUPPORT PER DET. 12/85.7 (7) #100 CONTEX SCHOLE AND OFFSCT FER DET. 12/55.6 EDEAR PER PLAN AND POST SUPPORT PER DET. 12/55.6 (7) #100 CONTEX SCHOLE AND OFFSCT FERSIONING TENDON, SEE DET. 40/55.4 AND 10/55.4 EDEAR PLAN AND POST SUPPORT PER DET. 4/00 FER PLAN LABOVE (10) #100 FER PER DET. 1/55.6 FOR TYP, TRIM REINF, AT OPENINGS, SEE DET. 12/55.6 EDEAR MALL ABOVE (10) #100 FER PER DET. 1/55.6 FOR TYP, TRIM REINF, AT OPENINGS, SEE DET. 12/55.6 SURE-BOARD STRAPPED SHEAR MALL ABOVE SEE DET. 4/0 SHEAR MALL ABOVE SEE DET. 4/0 SHEAR MALL ABOVE (11) EDAO EDA MACO AO F POST-TENSIONING TENDON, SEE DED EDAR MAL	10/58.1
(4) #7X10@3B NOCATES (4) HERENE, BARS X 10-01 LONG AT 91 0.C. BOTTOM RENE DENOTES CLASS 5 TENSION LAP PER DET. 2/55.1, TYP. U.N. DENOTES CLASS 5 TENSION LAP PER DET. 3/55.2, TYP. U.N. Image: Construction of the construction of	
DENOTES	VD 12/58.1.
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#5 TOPX6-0" AT 12" O.C. TYP. U.O.N. SEE SC2.2B #5 TOPX10-0" AT 12" O.C. TYP. U.O.N. #6 TOPX10-0" AT 12" O.C. TYP. TRUM RENT. #6 TOPX10-0" AT 12" OPX10-0" AT 12" OPX10-0" AT 12" OPX10-0" AT 12" OPX1	
#5 TOPXIO-O' AT 12' O.C. TYP. U.O.N. Image: Corner Reinf, FER DET. 1/55.6 Image: Corner Reinf, FER DET. 1/55.6 Image: Re-Entrant corner Reinf, FER DET. 2/55.6 Image: RE-Entrentrant corner Fostreneter. Imag	
CORNER REINF. PER DET. 1/55.6 U.O.N. ON PLAN Image: Non-State in the stress of the state in the stress of the stress o	
RE-ENTRANT CORNER REINF. PER DET. 2/55.6 RE-ENTRANT CORNER REINF. AT OPENINGS, SEE DETAILS 5/55.6, 6/55.6 AND 7/55.6. CHANGE IN BOTTOM OF SOFFIT SLOPE STRESSING END OF POST-TENSIONING TENDON. SEE DET. 4/55.4 AND 10/55.4 DEAD END ANCHOR OF POST-TENSIONING. SEE DET. 13/55.4 AND 10/55.4 STRESSING END OF POST-TENSIONING. SEE DET. 13/55.4 AND 10/55.4 SEE DET. 13/55.4 AND 10/55.4 SEE DET. 13/55.4 AND 14/55.4 SEE DET. 13/55.4 AND 16/55.4 SEE DET. 13/55.7 SEE DET. 13/55.7 SEE DET. 12/50.0 FOST	9/58.2,
FOR TYP. TRIM REINF. AT OPENINGS, SEE DETAILS 5/55.6, SURE-BOARD SHEAR WALL ABOVE SEE DET. 4/4 Image: Stressing end of sofFit slope SURE-BOARD SHEAR WALL ABOVE SEE DET. 4/4 Image: Stressing end of Post-tensioning tendon, Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of Post-tensioning. Stressing end of Post-tensioning. Image: Stressing end of tendon Above slab or Beam sofFit. Image: Stressing end of tendons Stressing end of tendons Image: Stressing end of tendons Stressing end of tendons Image: Stressing end of tendons Stressing end of tendons Image: Stressing end of tendons Stressing end of tendons Image: Stressing end of tendons Stressing end of tendons Image: Stressing end of tendons Stressing end	
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SEE DET. 9/55.4 AND 10/55.4 Market Dead End Anchor of Post-tensioning. SEE DET. 13/55.4 AND 14/55.4 SEE DET. 13/55.4 AND 14/55.4 <	
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ABOVE SLAB OR BEAM SOFFIT. X= MIN. CALCULATED SHEAR WALL LENGTH X= MIN. CALCULATED SHEAR WALL LENGTH	
STRUCTURAL STEEL COLUMN SEE SHEET ST.1. * "X" DENOTES LEVEL OF COLUMN TERMINATION * "R" = ROOF, "P" = PARAPET	
パー (R" = ROOF, "P" = PARAPET	<u>_1</u>
Y → Y+ "*" DENOTES COLUMN REQUIRES FIRE PROTECTION, S.A.D. I □ FOR REQUIREMENTS. FIRE PROTECTION (INTUMESCENT PAINT	
L GR REQUIREMENTS. FIRE PROTECTION (INTUMESCENT PAINT RECOMMENDED), S.A.D. FOR REQUIREMENT. DENOTE NON-SEISMIC MOMENT RESISTING CONNECTIONS.	
PER DETAIL 8/57.1	
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- LANDSCAPE ARCHITECT PLURAL STUDIO 2742 17TH STREET SAN FRANCISCO, CA 94110
- STRUCTURAL ENGINEER HOHBACH-LEWIN INC 250 SHERIDAN AVE STE 100 PALO ALTO, CA 94306
- MEP ENGINEER EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- SUSTAINABILITY/ENERGY **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521
- JOINT TRENCH/DRY UTILITY MILLENIUM DESIGN PO BOX 737 ALAMO, CA, 94507



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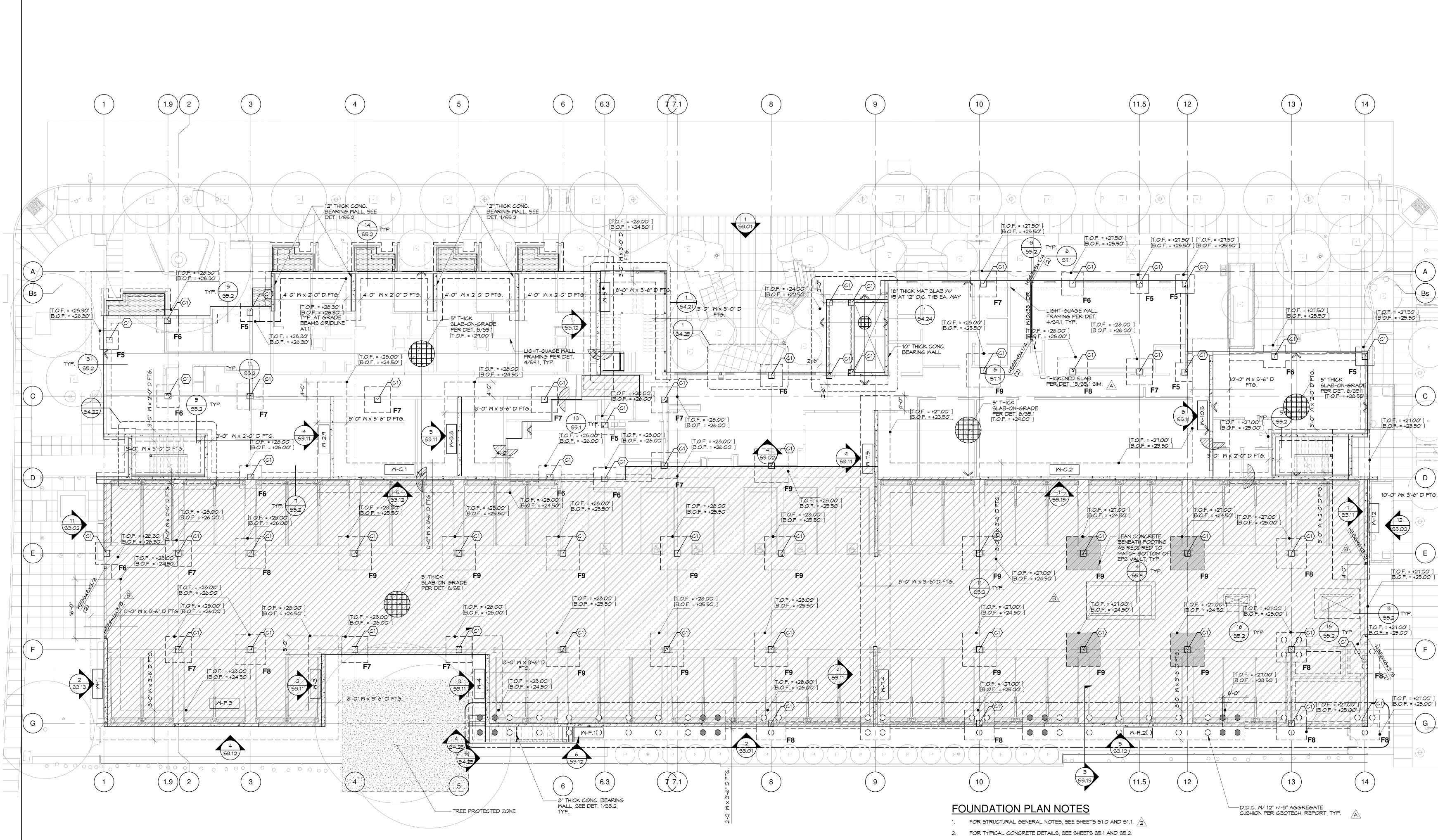
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ID	DATE	NAME
1	11/11/2022	PERMIT SET-CONV
Α	12/16/2022	BID SET
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2	03/20/2023	PLAN CHECK RESPONSE 2
3	05/12/2023	PLAN CHECK RESPONSE 3

Project:



231 GRANT AVENUI PALO ALTO, CA 9430	
Client:	
HOUSING	
abode	
MERCY HOUSING/ ABODE COMMUNITIE	ES
STRUCTURAL	
GENERAL NOT	
JOB #: 1925	1451
SCALE: As indicated	# Z
S1.2	HBACH-LEWIN # 14515

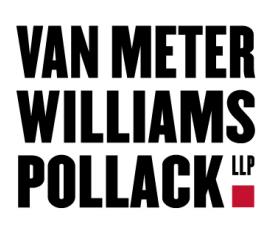


Ρ 5/12/2023 1:53:47 LEVEL 1 - FOUNDATION PLAN 3/32" = 1'-*0*"

- FOR BUILDING LAYOUT AND DIMENSIONS, SEE ARCHITECTURAL DRAWINGS, TYP. U.O.N.
- 4. FOR FINISH FLOOR ELEVATIONS, DEPRESSIONS, DRAINS, ETC., SEE ARCHITECTURAL DRAWINGS.
- 5. FOR PAD ELEVATIONS, SEE CIVIL DRAWINGS.
- FOR MECHANICAL, ELECTRICAL, AND PLUMBING OPENINGS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
- TOP OF FOOTINGS SHALL BE AT ELEVATION [-1'-0"] WITH RESPECT TO DATUM ELEVATION OR [-0'-6"] BELOW LOWEST ADJACENT SOIL PAD GRADE ELEVATIONS, WHICHEVER IS LOWER. TYP. U.O.N.
- 8. EXCAVATIONS SHALL BE MADE AS NEAR AS POSSIBLE TO THE LINES
- REQUIRED BY THE FOUNDATION. NO MATERIAL IS TO BE OVER EXCAVATED UNNECESSARILY. 9. VERIFY LOCATION OF UNDERGROUND UTILITIES PRIOR TO EXCAVATION. IN THE EVENT THAT SUCH UTILITIES ARE ENCOUNTERED DURING
- EXCAVATION, NOTIFY ARCHITECT IMMEDIATELY. 10. FOR DRAINAGE DETAILS, SUMPS, PITS, WATERPROOFING, MOISTURE
- BARRIERS, TRENCHES, CURBS, EXTERIOR WALKS, UTILITIES, EQUIPMENT DETAILS, STEPS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL. 11. SETBACK CONDITIONS SHALL BE VERIFIED IN THE FIELD PRIOR TO TRENCHING OR FORMING FOUNDATIONS. THE FOUNDATION
- SUBCONTRACTOR SHALL COORDINATE WITH THE GENERAL CONTRACTOR, SURVEYOR, AND THE ARCHITECT. 12. THE GEOTECHNICAL ENGINEER SHALL BE RETAINED TO PROVIDE OBSERVATION AND TESTING SERVICES DURING THE GRADING AND FOUNDATION PHASE OF CONSTRUCTION. THEIR INSPECTION AND
- DEPARTMENT. 13. RAMMED AGGREGATE PIERS, GEOPIERS, OR OTHER SOIL IMPROVEMENT SYSTEMS ARE DESIGN-BUILD BY THE GENERAL CONTRACTOR. SEE THE DESIGN-BUILD DRAWINGS PREPARED BY

OTHERS FOR PIER INFO. AND ORIENTATION.

TESTING REPORTS SHALL BE SUBMITTED TO THE BUILDING



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- STRUCTURAL ENGINEER HOHBACH-LEWIN 260 SHERIDAN AVE STE 150 PALO ALTO, CA 94306
- MEP ENGINEER **EMERALD CITY ENGINEERS** 21705 HIGHWAY 99 LYNWOOD, WA 98036
- ENERGY CONSULTANT **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521



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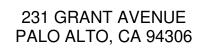
COUNTY OF SANTA CLARA BUILDING INSPECTION OFFICE PLANS APPROVED FOR PERMIT RECORD NO.: DEV22-1242

BY: <u>M. Bloom</u> Date: <u>07/28/2023</u> HARD COPY OF THESE STAMPED PLANS MUST BE ON THE SITE FOR INSPECTIONS

ID	DATE	NAME
1	11/11/22	PERMIT SET-CONV
А	12/16/22	BID SET
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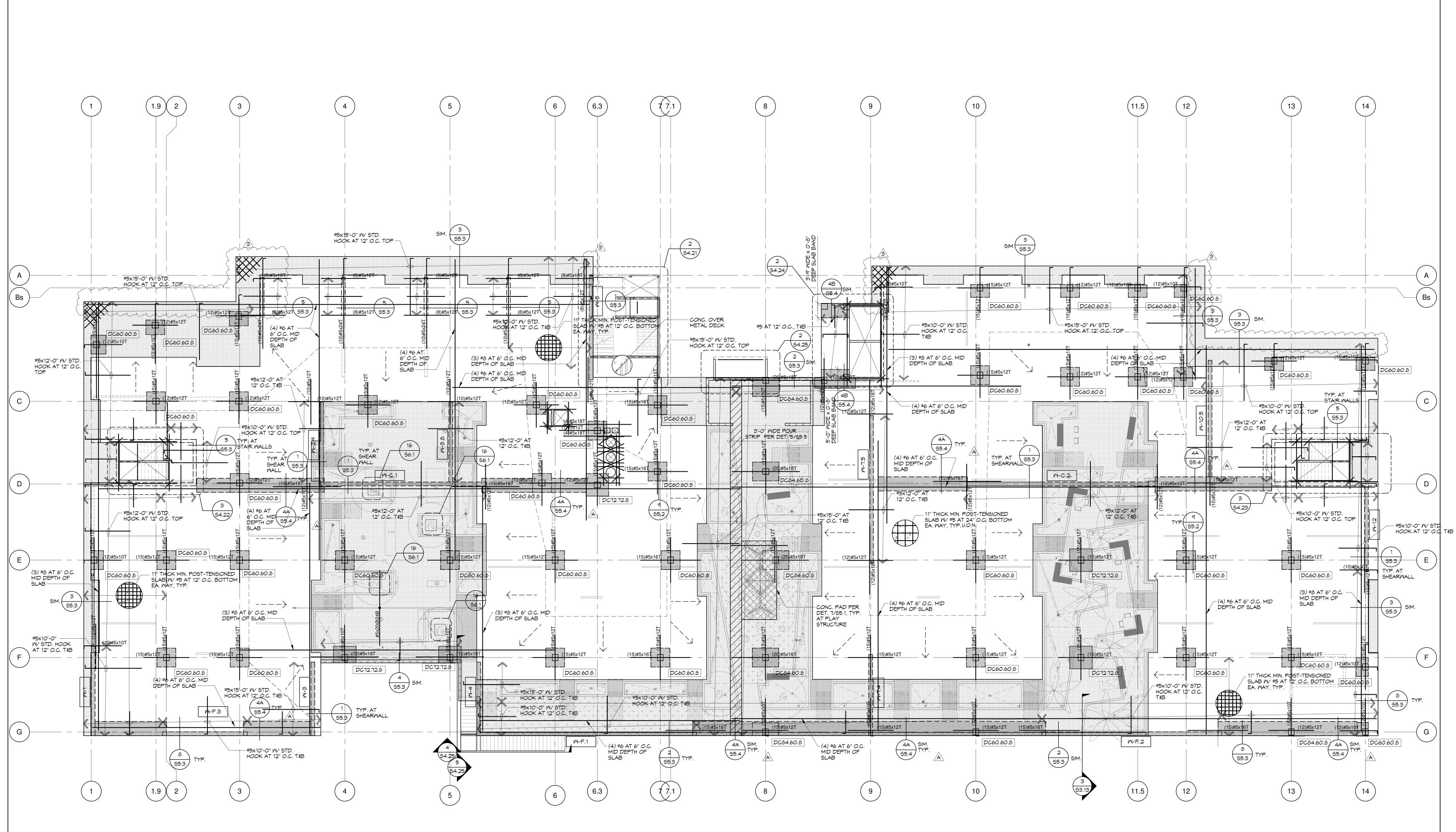
Project:

EDUCATOR HOUSING 231 GRANT AVENUE





JOB #: 1925 SCALE: As indicated S2.1



5/12/2023 2:33:20 PM

LEVEL 2 PODIUM - REINFORCEMENT PLAN 3/32" = 1'-0"

REINFORCEMENT PLAN NOTES:

- 1. FOR STRUCTURAL GENERAL NOTES, SEE SHEETS S1.0 AND S1.1.
- FOR TYPICAL CONCRETE DETAILS, SEE SHEETS 55.2 AND 55.3.
 FOR BUILDING LAYOUT AND DIMENSIONS, FINISH FLOOR ELEVATIONS, SLAB SLOPES, DEPRESSIONS, DRAINS, FINISHES, ETC., SEE ARCHITECTURAL DRAWINGS, TYP. U.O.N.
- 4. FOR MECHANICAL, ELECTRICAL, AND SHAFT OPENINGS, ETC., SEE
- DRAWINGS OTHER THAN STRUCTURAL.5. FOR TYP. SLAB REINF. LAYOUT AND SPLICE LOCATIONS, SEE DET.
- 4/55.5.6. FOR TIEDOWN LOCATIONS, SEE PODIUM FRAMING PLAN AND FOR
- ADDITIONAL REINF. REQUIRED AT TIEDOWNS, SEE SHEET S5.8, TYP.7. FOR TYP. TRIM REINF. AT OPENINGS, SEE DETAILS 5/S5.6, 6/S5.6 AND
- 7/55.6.
 CONDUITS EMBEDDED IN SLABS SHALL MEET THE REQUIREMENTS OF SHEET S1.0 CONCRETE NOTE K. FOR CONDUIT LAYOUT, SEE DETAIL 11/55.6.
- 9. SEE P/T TENDON PLANS FOR POST-TENSION TENDON LAYOUT.10. IF CONSTRUCTION JOINTS OR INTERMEDIATE STRESSING JOINTS ARE
- DEEMED NECESSARY, SEE DETAILS 6/S5.5 AND 7/S5.5.
 11. DO NOT EXCEED A MAX OF 24" OF SOIL ON PODIUM (EXCEPT AT LARGE TREES). FOR AREAS WHERE ADDITIONAL SOIL DEPTH IS REQUIRED, PROVIDE FOAM FILL AS REQD., SEE DETAILS 15/S6.1 AND 16/S6.1.
- 12. SEE ARCHITECTURAL AND LANDSCAPE DRAWINGS FOR LANDSCAPE FEATURES, DIMENSIONS, AND REINFORCEMENT NOT SHOWN ON PLAN.

LOCATE P/T TENDONS VIA RADIOGRAPHIC METHODS PRIOR TO ANY POST-INSTALLED ANCHOR INSTALLATION. DO NOT CUT, NICK, OR OTHERWISE MAR P/T TENDONS AND SLAB REINFORCEMENT DURING THE POST-INSTALLED ANCHOR INSTALLATION.

PODIUM - DESIGN	LOADINGS	
	DL	LL
3-STORY WOOD FRAMED BLG. RESIDENTIAL UNITS CORRIDORS STAIRS	160 PSF 160 PSF 160 PSF	84 PSF* 180 PSF 300 PSF
COURTYARD: PAVERS PLANTER (AVG. 24" SOIL) SMALL TREE BOX TREE	85 PSF 345 PSF 1500 LB. 3000 LB.	100 PSF 40 PSF

<u>/1</u>



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- LANDSCAPE ARCHITECT
 PLURAL STUDIO
 2742 17TH STREET
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- STRUCTURAL ENGINEER
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- MEP ENGINEER
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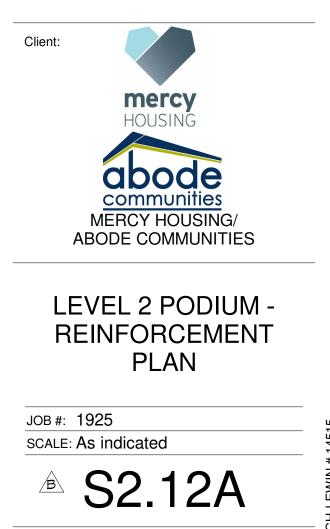
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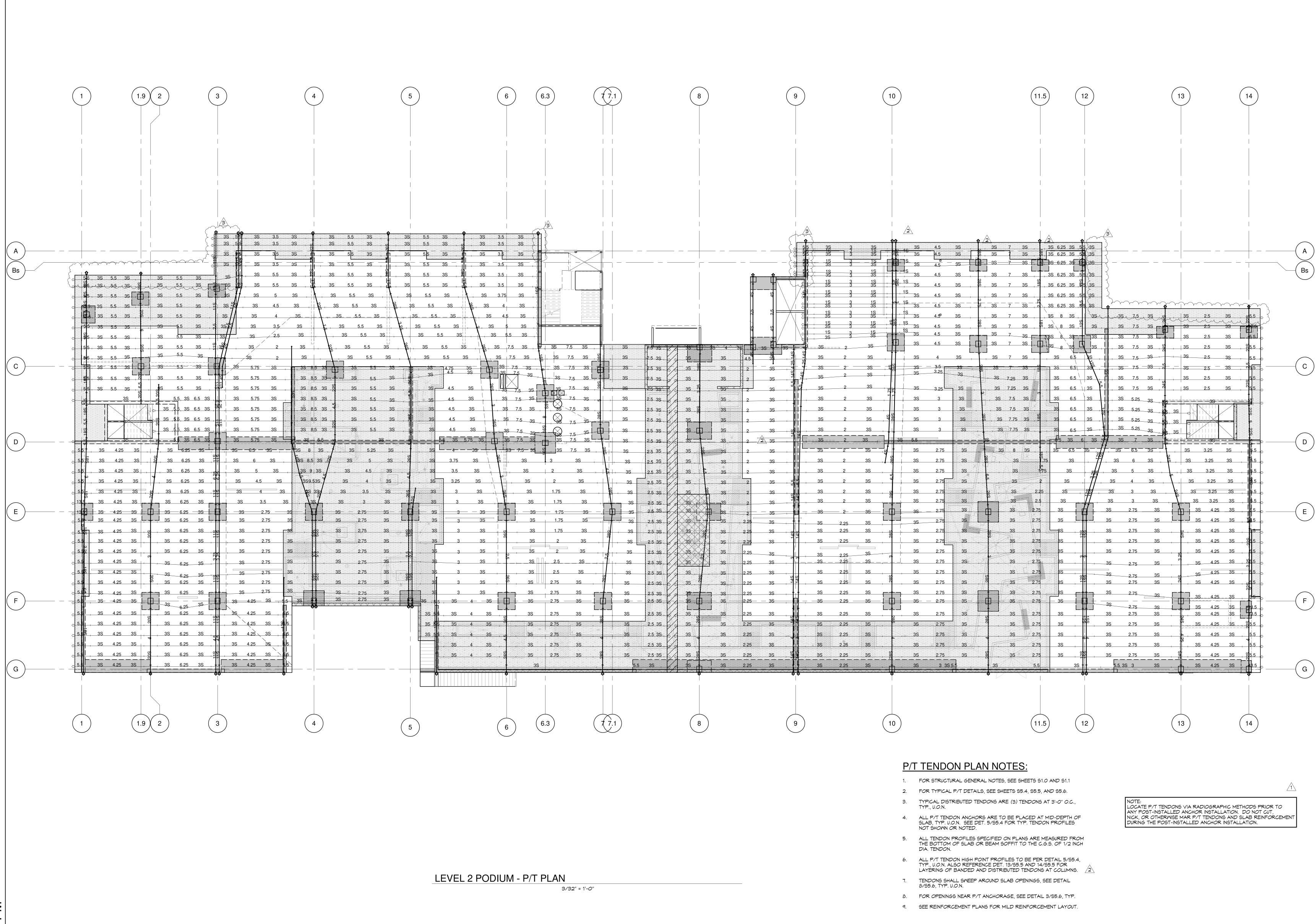
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Project:

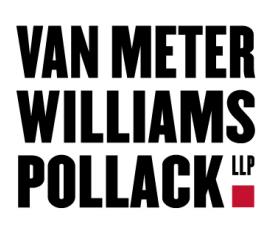
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COUNTY OF SANTA CLARA BUILDING INSPECTION OFFICE PLANS APPROVED FOR PERMIT RECORD NO.: DEV22-1242 BY: <u>M. Bloom</u> Date: <u>07/28/2023</u>

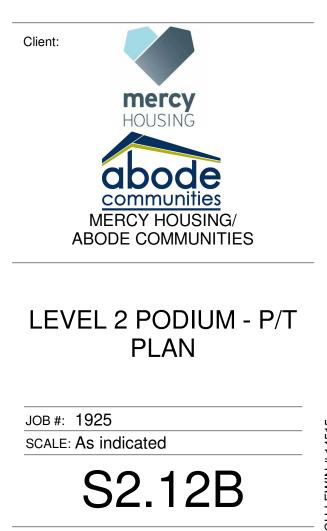
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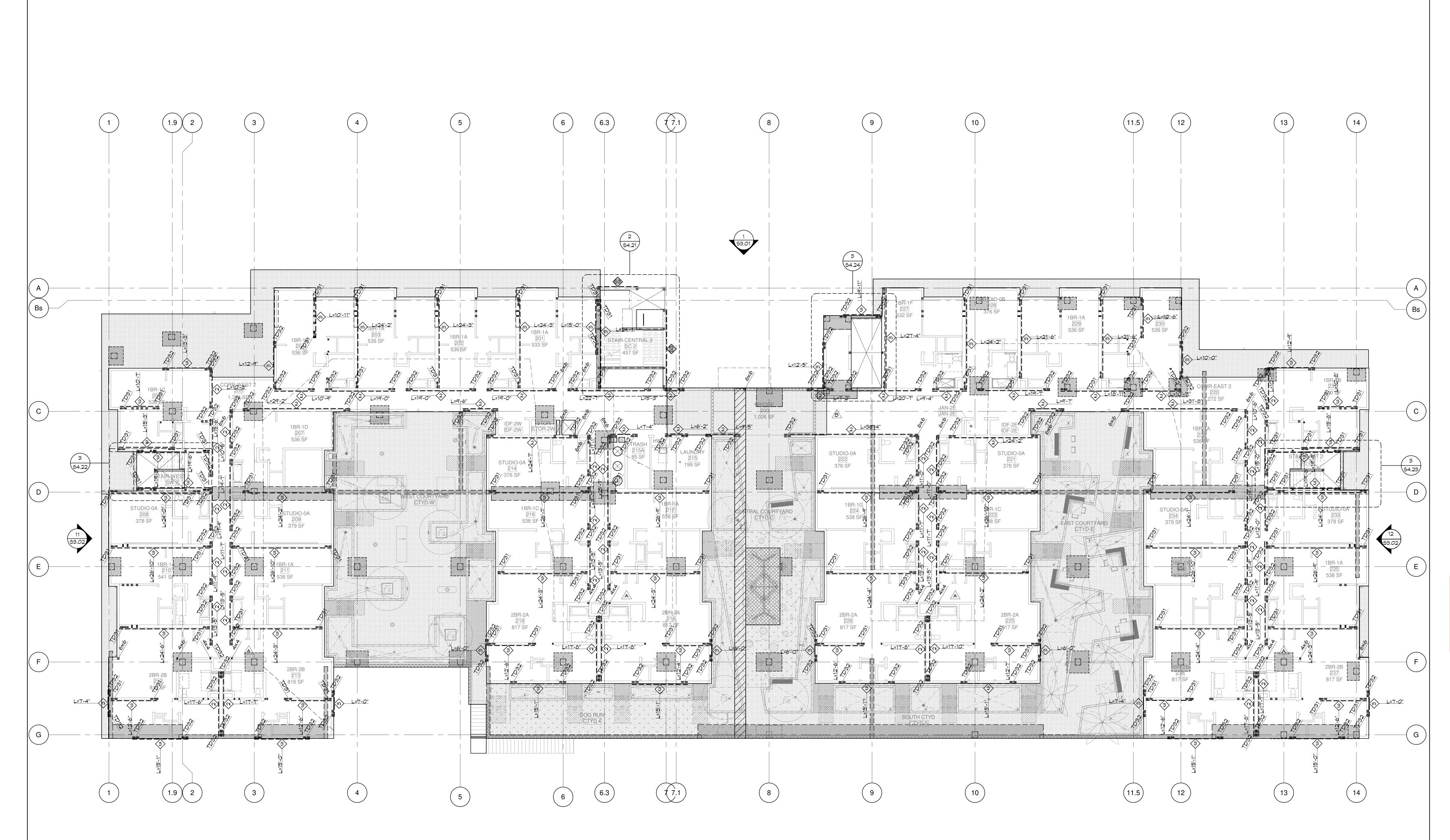
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LEVEL 2 - FLOOR FRAMING PLAN

PM 5/12/2023 1:53:57 3/32" = 1'-*0*"

FRAMING PLAN NOTES

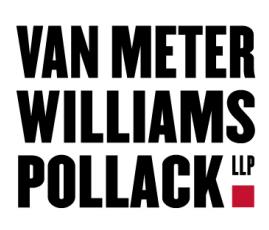
- FOR STRUCTURAL GENERAL NOTES, SEE SHEETS S1.0 AND S1.1. FOR TYPICAL CONCRETE DETAILS, SEE SHEETS 55.1 AND 55.2.
- FOR BUILDING LAYOUT AND DIMENSIONS, FINISH FLOOR
- ELEVATIONS, SLAB SLOPES, DEPRESSIONS, DRAINS, FINISHES, ETC., SEE ARCHITECTURAL DRAWINGS, TYP. U.O.N.
- 4. FOR MECHANICAL, ELECTRICAL, AND PLUMBING OPENINGS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
- UNTREATED SAWN LUMBER BEAMS SPECIFIED MAY BE SUBSTITUTED 5. WITH PREFABRICATED BEAMS (I.E. LSL, LVL, ETC.) AS A CONTRACTOR OPTION. THIS SUBSTITUTION SHALL BE SUBMITTED TO AND APPROVED BY THE ENGINEER OF RECORD. P.T. OR F.R.T. SHALL NOT BE SUBSTITUTED.
- 6. FOR JOIST TO BEAM CONNECTION DETAILS, SEE DETAIL 15/58.2. 7. ALL WOOD BEAMS SHOWN ON PLANS SHALL BE SUPPORTED ON
- 8. FOR TYP. STUD WALL SIZE AND SPACING, SEE DETAIL 1/58.2, TYP.
- 9. FOR ROOF AND FLOOR PLYWOOD NAILING SEE SHEET S1.0, WOOD NOTES AND DETAILS 6/58.1 AND 7/58.1.
- 10. FOR ALLOWABLE HOLES AND NOTCHES AT FRAMING MEMBERS, SEE DETAIL 5/5802. FOR TOP PLATE PENETRATIONS, SEE DETAILS 3/58.2 AND 12/58.4.

WOOD POSTS PER SCHEDULE, SEE DETAIL 9/58.2, TYP. U.O.N. U.O.N. ON THE UNIT PLANS. FOR TALL WALLS, SEE DETAIL 4/58.2.

- FOR SHEAR WALL FRAMING AND HARDWARE REQUIREMENTS, SEE SHEET S8.1.
- 12. FOR VERTICAL TIEDOWN STRAP AT FLOOR, SEE DETAIL 10/58.1 13. FOR INTERIOR NON-SHEAR BEARING WALLS, SEE DETAIL 5/58.4.
- 14. FOR INTERIOR NON-SHEAR NON-BEARING WALLS, SEE DETAIL 6/58.4.
- ALL LUMBER AND SHEATHING EXPOSED TO WEATHER SHALL BE PRESSURE TREATED.
- PROVIDE PRESERVATIVE TREATED FRAMING AND PLYWOOD AT 16.
- EXTERIOR BALCONIES, DECKS, AND WALKWAYS. 17. FOR ALL HSS COLUMNS INSIDE WOOD SHEAR WALLS, PROVIDE 3x NAILER PER DETAIL 10/57.3.
- ~~~<u>/3</u> 18. FOR TYPICAL STEEL DETAILS, SEE SHEET ST.1.
- 19. FOR TIEDOWN FRAMING AND HARDWARE REQUIREMENTS, SEE SHEETS S8.10 AND S8.11.
- 20. FOR TYPICAL UNIT FRAMING PLANS, SEE SHEET S4.11

NOTE: LOCATE P/T TENDONS VIA RADIOGRAPHIC METHODS PRIOR TO ANY POST-INSTALLED ANCHOR INSTALLATION. DO NOT CUT, NICK, OR OTHERWISE MAR P/T TENDONS AND SLAB REINFORCEMENT DURING THE POST-INSTALLED ANCHOR INSTALLATION.

<u>∕1</u>∖



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COUNTY OF SANTA CLARA BUILDING INSPECTION OFFICE PLANS APPROVED FOR PERMIT RECORD NO.: DEV22-1242 BY: <u>M. Bloom</u> Date: <u>07/28/2023</u>

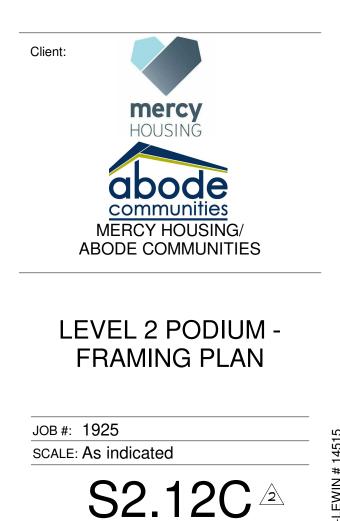
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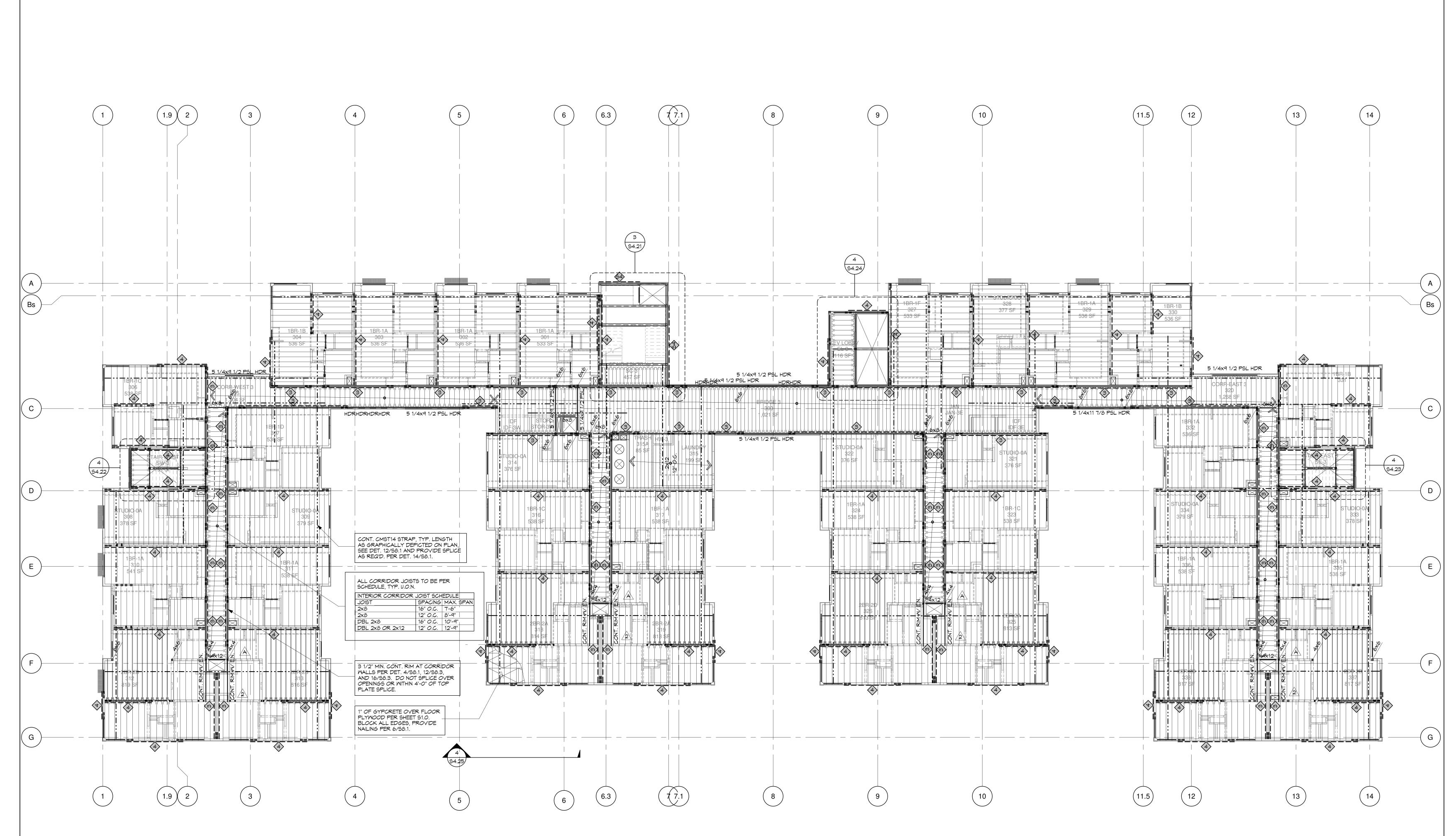
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В	03/20/23	BID ADDENDUM
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3	05/12/23	PLAN CHECK RESPONSE 3
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Project:

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LEVEL 3 - FLOOR FRAMING PLAN

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3/32" = 1'-0"

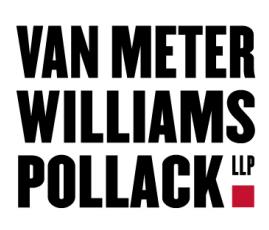
FRAMING PLAN NOTES

- FOR BUILDING LAYOUT AND DIMENSIONS, FINISH FLOOR ELEVATIONS, SLAB SLOPES, DEPRESSIONS, DRAINS, FINISHES, ETC., SEE ARCHITECTURAL DRAWINGS, TYP. U.O.N. 4.
- FOR MECHANICAL, ELECTRICAL, AND PLUMBING OPENINGS, ETC., SEE DRAWINGS OTHER THAN STRUCTURAL.
- 5. UNTREATED SAWN LUMBER BEAMS SPECIFIED MAY BE SUBSTITUTED WITH PREFABRICATED BEAMS (I.E. LSL, LVL, ETC.) AS A CONTRACTOR OPTION. THIS SUBSTITUTION SHALL BE SUBMITTED TO AND APPROVED BY THE ENGINEER OF RECORD. P.T. OR F.R.T. SHALL NOT BE SUBSTITUTED.
- 6. FOR JOIST TO BEAM CONNECTION DETAILS, SEE DETAIL 15/58.2. ALL WOOD BEAMS SHOWN ON PLANS SHALL BE SUPPORTED ON WOOD POSTS PER SCHEDULE, SEE DETAIL 9/S8.2, TYP. U.O.N. 7. 8. FOR TYP. STUD WALL SIZE AND SPACING, SEE DETAIL 1/S8.2, TYP. U.O.N. ON THE UNIT PLANS. FOR TALL WALLS, SEE DETAIL 4/58.2. 9. FOR ROOF AND FLOOR PLYWOOD NAILING SEE SHEET S1.0, WOOD

- NOTES AND DETAILS 6/S8.1 AND 7/S8.1. 10. FOR ALLOWABLE HOLES AND NOTCHES AT FRAMING MEMBERS, SEE DETAIL 5/S802. FOR TOP PLATE PENETRATIONS, SEE DETAILS 3/58.2 AND 12/58.4.

FOR STRUCTURAL GENERAL NOTES, SEE SHEETS S1.0 AND S1.1. FOR TYPICAL CONCRETE DETAILS, SEE SHEETS 55.1 AND 55.2.

- 11. FOR SHEAR WALL FRAMING AND HARDWARE REQUIREMENTS, SEE SHEET S8.1.
- 12. FOR VERTICAL TIEDOWN STRAP AT FLOOR, SEE DETAIL 10/58.1.
- 13. FOR INTERIOR NON-SHEAR BEARING WALLS, SEE DETAIL 5/58.4. 14. FOR INTERIOR NON-SHEAR NON-BEARING WALLS, SEE DETAIL
- 6/58.4. 15. ALL LUMBER AND SHEATHING EXPOSED TO WEATHER SHALL BE
- PRESSURE TREATED. 16. PROVIDE PRESERVATIVE TREATED FRAMING AND PLYWOOD AT
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- ~~~<u>/</u>3 18. FOR TYPICAL STEEL DETAILS, SEE SHEET S7.1.
- 19. FOR TIEDOWN FRAMING AND HARDWARE REQUIREMENTS, SEE SHEETS S8.10 AND S8.11.
- 20. FOR TYPICAL UNIT FRAMING PLANS, SEE SHEET S4.11



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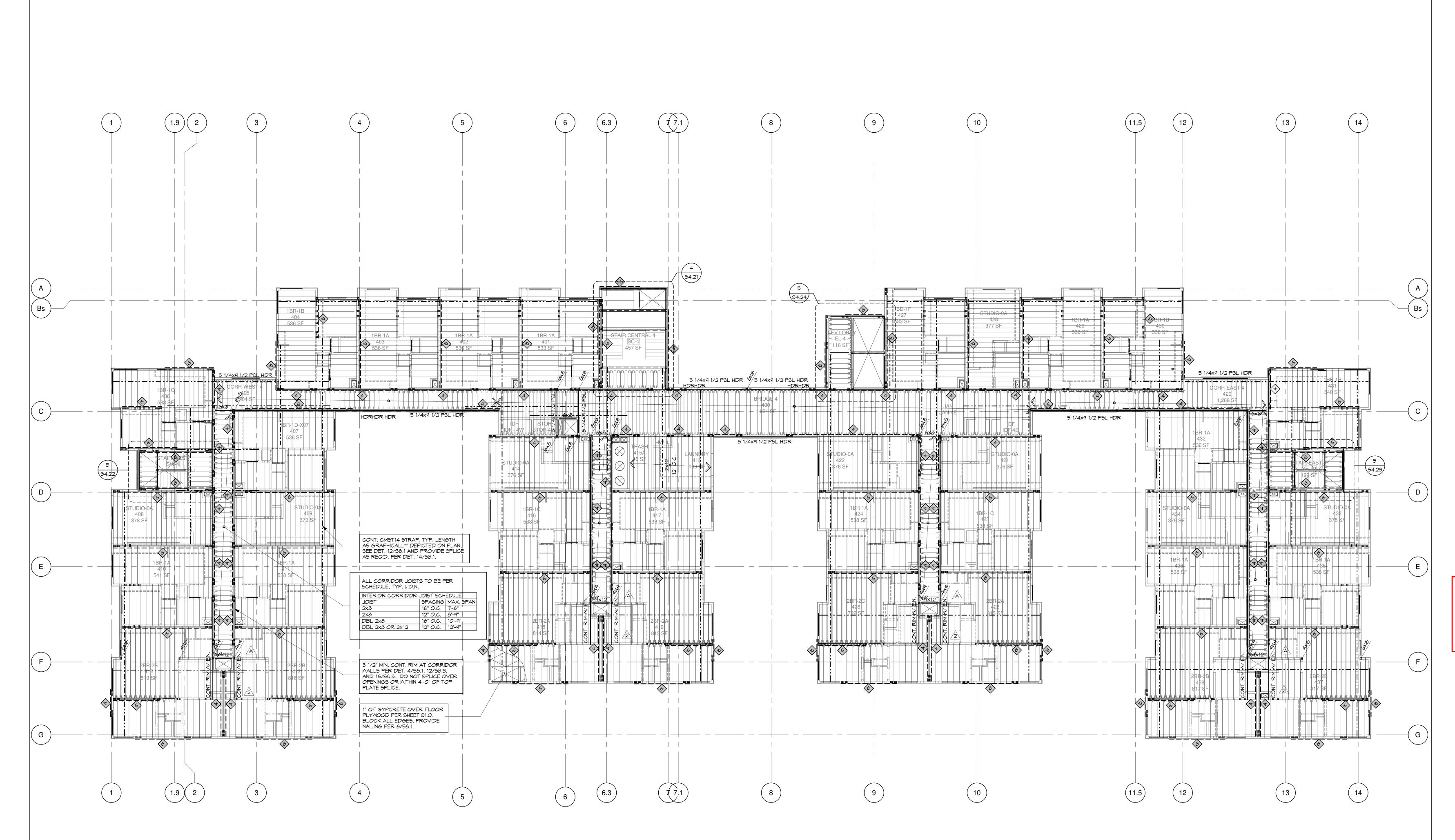
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<u>ک</u> S2.13 PLAN CHECK RESPONSE 2 DATE: 03/20/23



LEVEL 4 - FLOOR FRAMING PLAN

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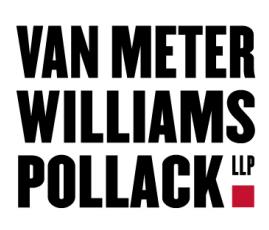
FRAMING PLAN NOTES

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- 9

DETAILS 3/58.2 AND 12/58.4.

FOR ROOF AND FLOOR PLYWOOD NAILING SEE SHEET S1.0, WOOD NOTES AND DETAILS 6/S8.1 AND 7/S8.1. 10. FOR ALLOWABLE HOLES AND NOTCHES AT FRAMING MEMBERS, SEE DETAIL 5/5802. FOR TOP PLATE PENETRATIONS, SEE

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- 19. FOR TIEDOWN FRAMING AND HARDWARE REQUIREMENTS, SEE SHEETS S8.10 AND S8.11.
- 20. FOR TYPICAL UNIT FRAMING PLANS, SEE SHEET 54.11



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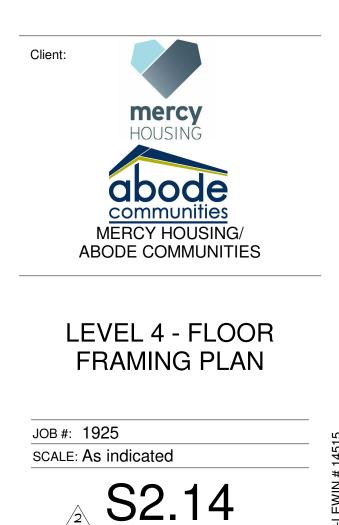
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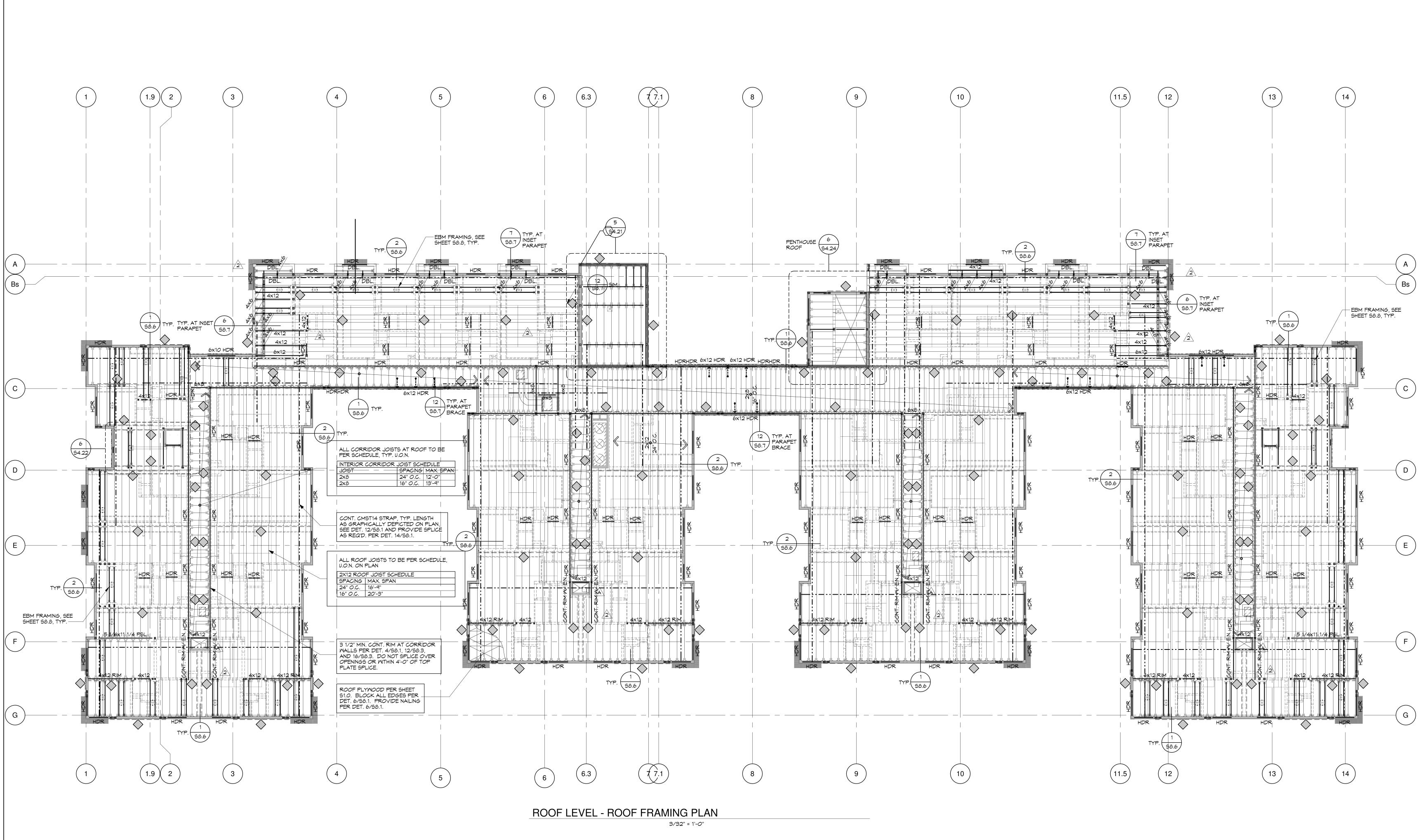
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- FRAMING PLAN NOTES FOR STRUCTURAL GENERAL NOTES, SEE SHEETS S1.0 AND S1.1.
- 2. FOR TYPICAL CONCRETE DETAILS, SEE SHEETS 55.1 AND 55.2. FOR BUILDING LAYOUT AND DIMENSIONS, FINISH FLOOR ELEVATIONS, SLAB SLOPES, DEPRESSIONS, DRAINS, FINISHES, ETC., SEE ARCHITECTURAL DRAWINGS, TYP. U.O.N.
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- 8. FOR TYP. STUD WALL SIZE AND SPACING, SEE DETAIL 1/58.2, TYP. U.O.N. ON THE UNIT PLANS. FOR TALL WALLS, SEE DETAIL 4/58.2.
- 9. FOR ROOF AND FLOOR PLYWOOD NAILING SEE SHEET S1.0, WOOD NOTES AND DETAILS 6/S8.1 AND 7/S8.1.
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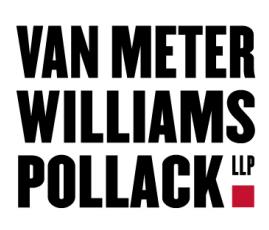
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- 12. FOR VERTICAL TIEDOWN STRAP AT FLOOR, SEE DETAIL 10/58.1.

11.

- 13. FOR INTERIOR NON-SHEAR BEARING WALLS, SEE DETAIL 5/58.4.
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- 18. FOR TYPICAL STEEL DETAILS, SEE SHEET ST.1.
- SHEETS S8.10 AND S8.11.

ROOF FRAMING PLAN NOTES

- FOR MECHANICAL UNIT ROOF ANCHORAGE AND SUPPORT, SEE DETAILS 2/S808, 3/S808, AND 4/S808. SEE ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ADDITIONAL INFO.
- FOR EBM SUPPORT FRAMING DETAILS, SEE SHEET S808.
- FOR PREFABRICATED TRUSS REQUIREMENTS, SEE TRUSS NOTES ON SHEET S100.
- 4. ALL 2X STRUCTURAL FASCIA TO BE 10'-0" MIN.
- 19. FOR TIEDOWN FRAMING AND HARDWARE REQUIREMENTS, SEE
- 20. FOR TYPICAL UNIT FRAMING PLANS, SEE SHEET S4.11



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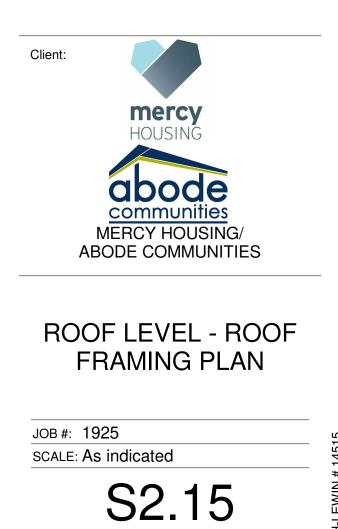
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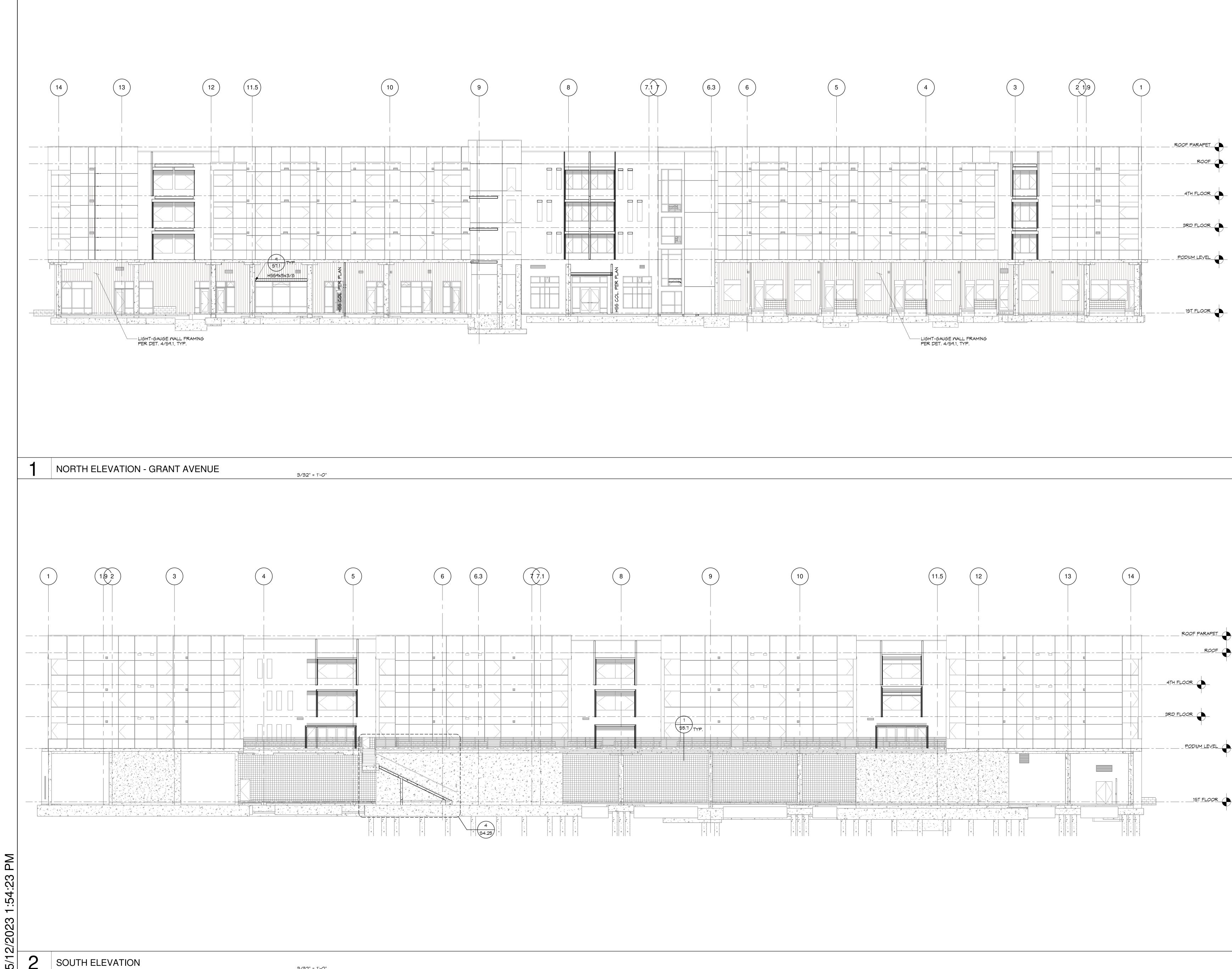
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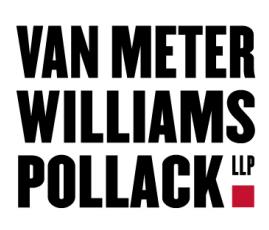
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SOUTH ELEVATION

3/32" = 1'-*0*"



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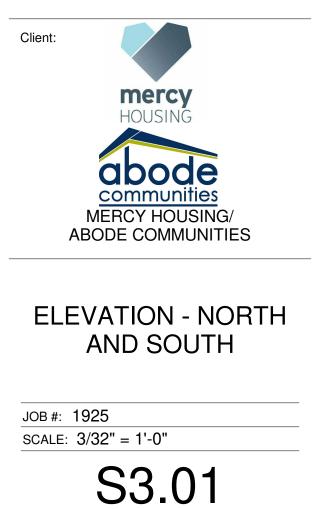
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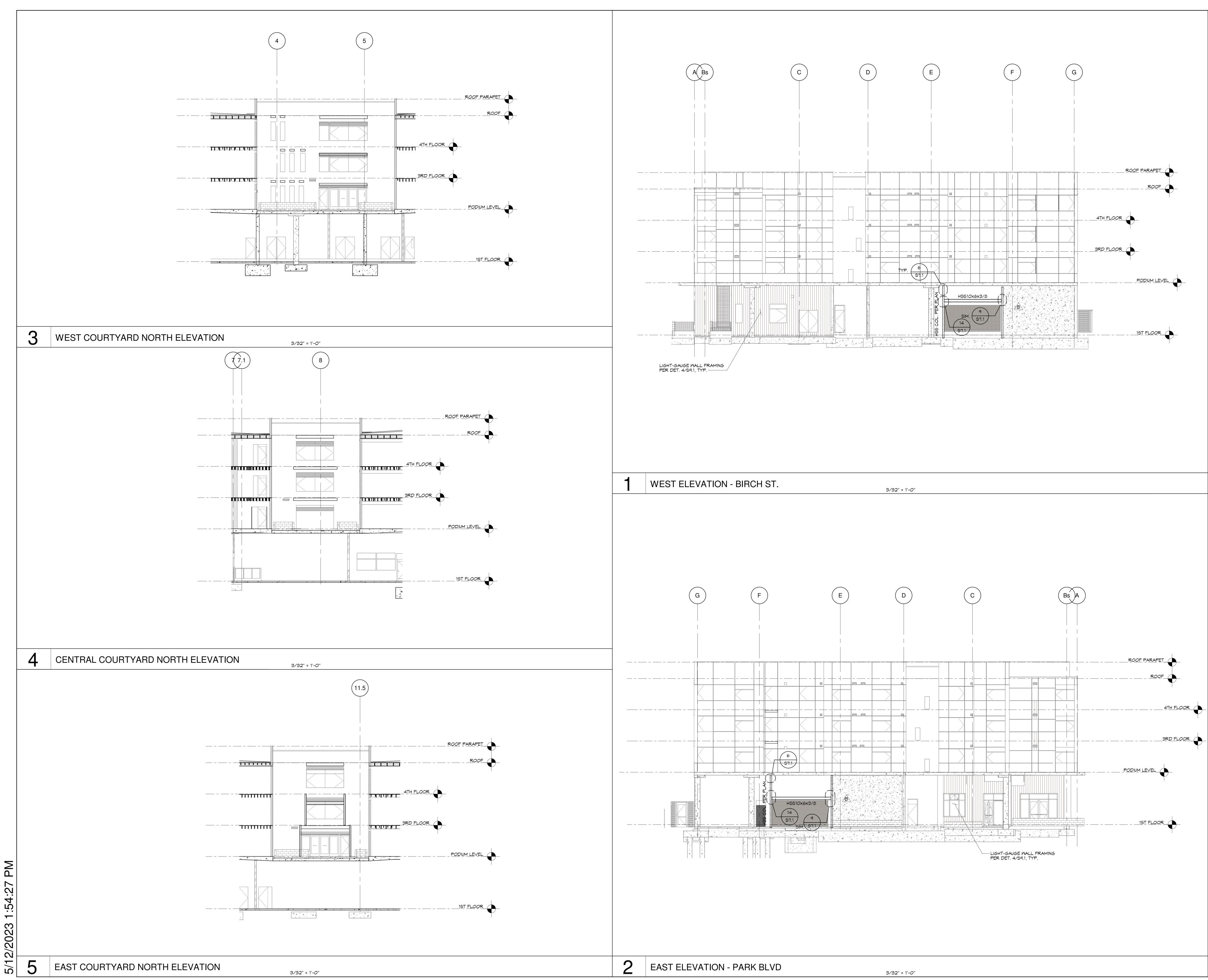
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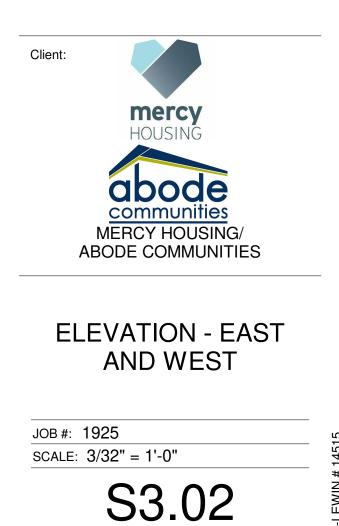
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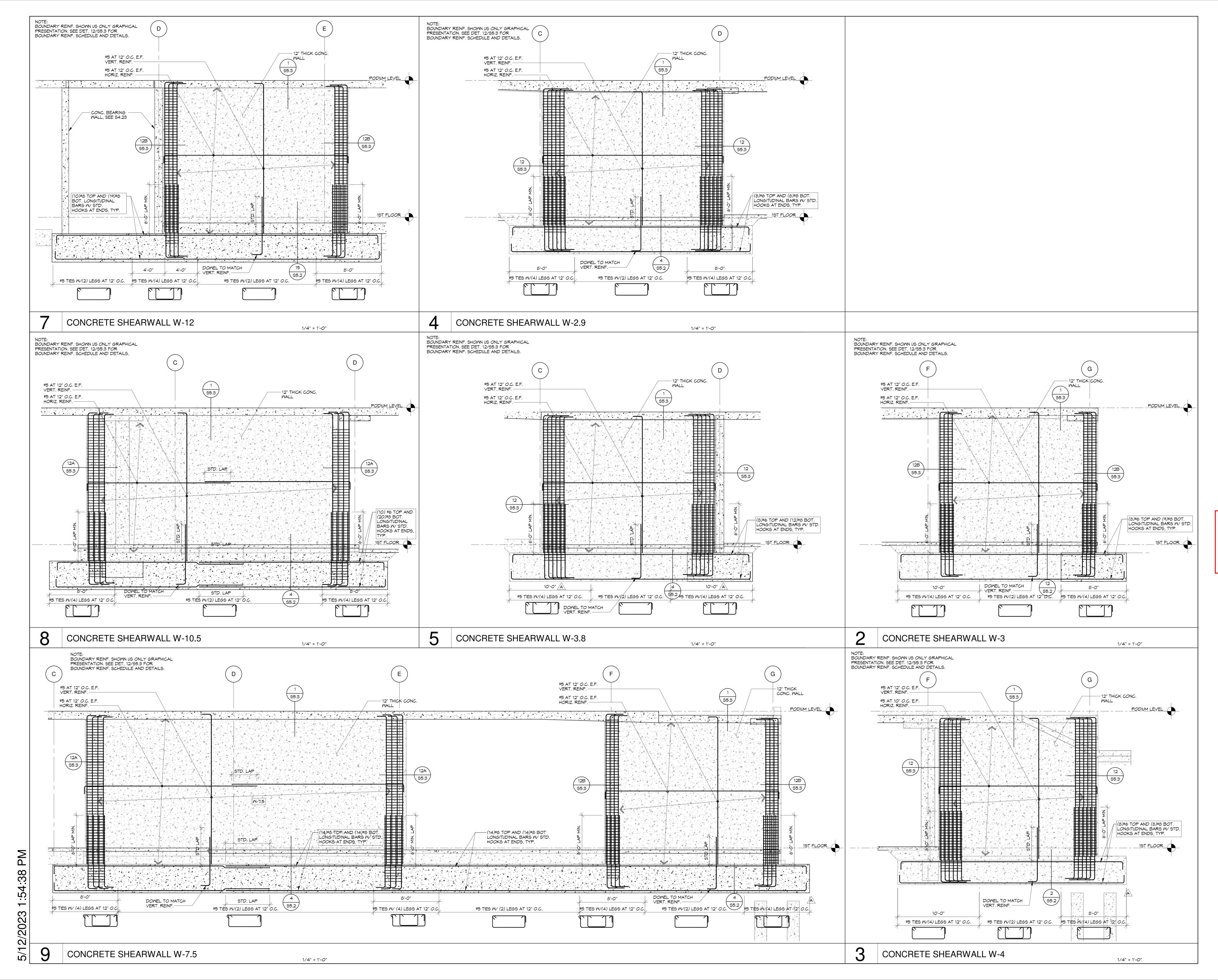
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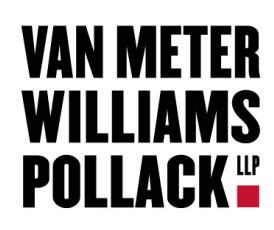
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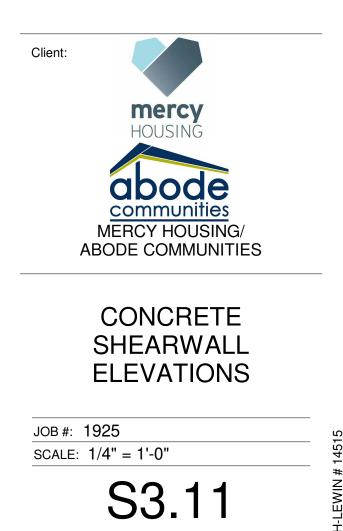
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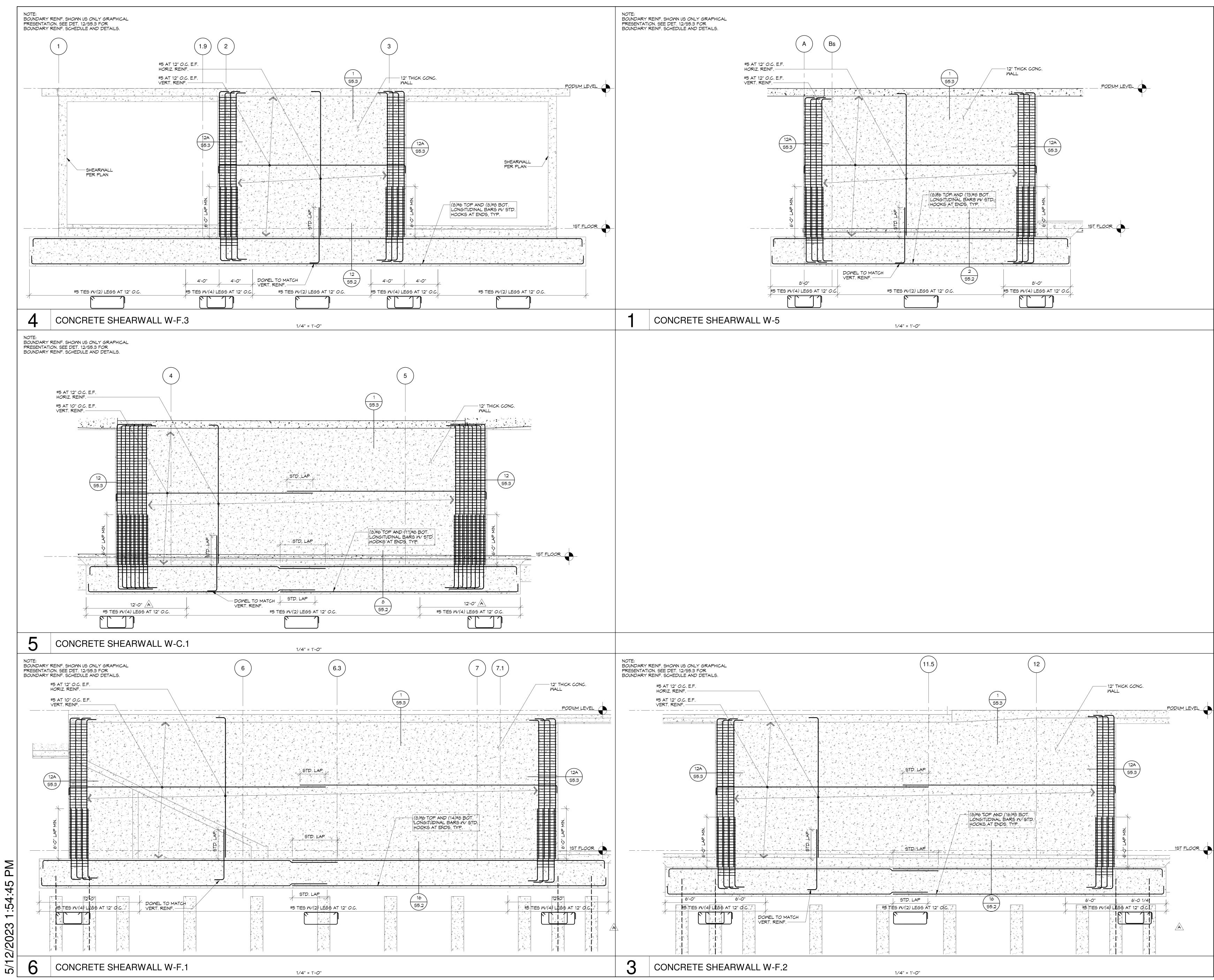
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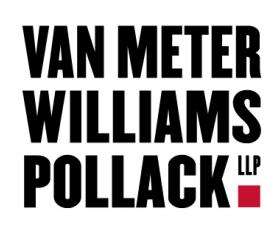
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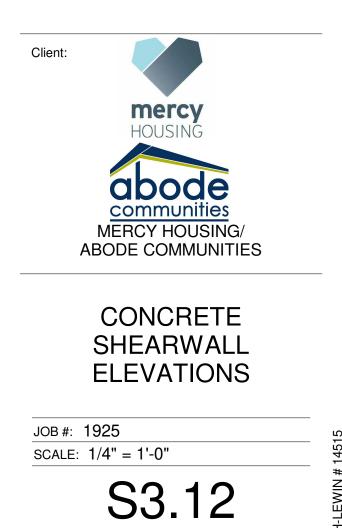
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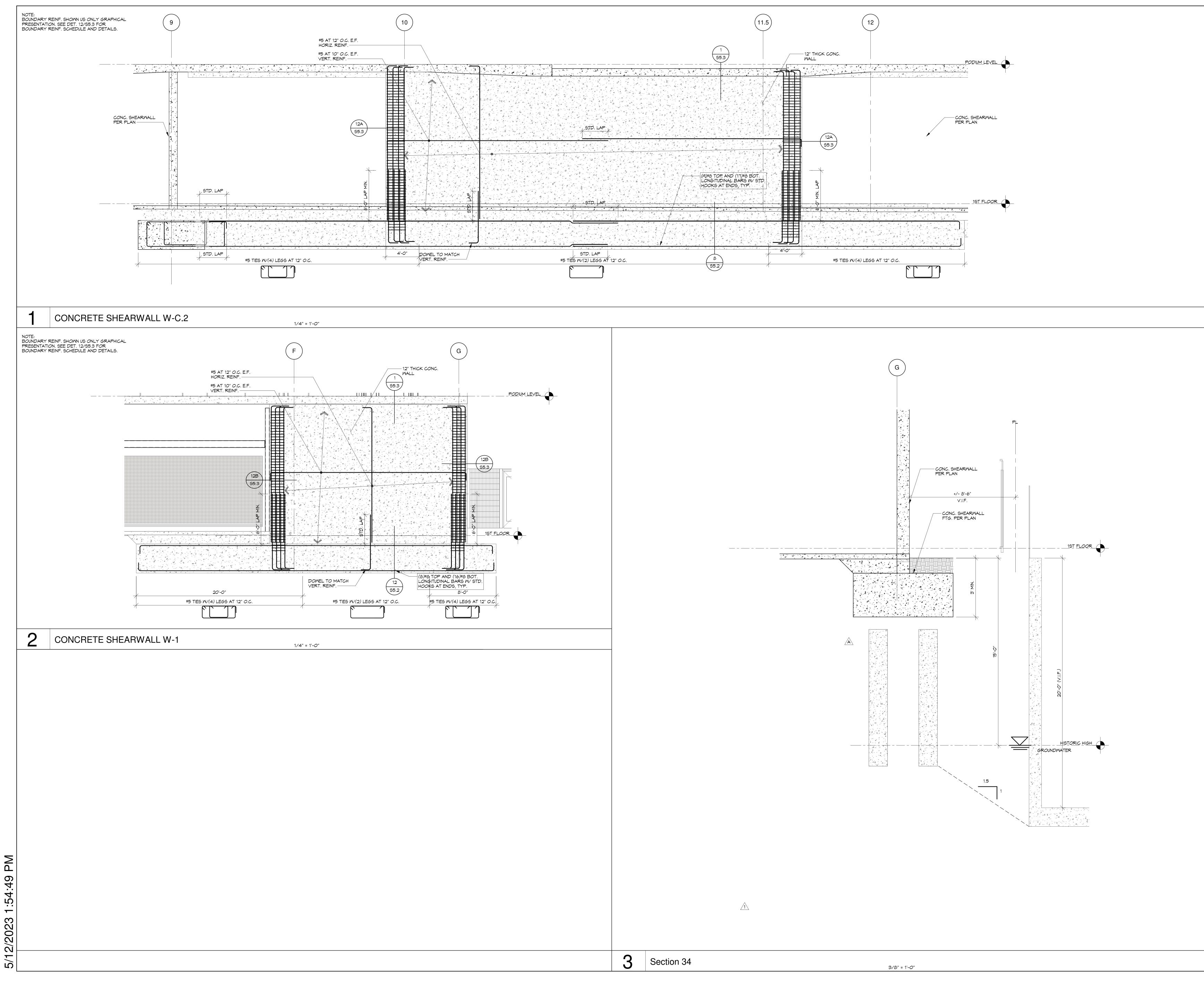
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В	03/20/23	BID ADDENDUM
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3	05/12/23	PLAN CHECK RESPONSE 3

Project:

EDUCATOR HOUSING 231 GRANT AVENUE

231 GRANT AVENUE PALO ALTO, CA 94306







- **CIVIL ENGINEER BKF - SAN JOSE** 1730 N. FIRST ST. STE 600 SAN JOSE, CA 95112
- **JOINT TRENCH / DRY UTILITIES** MILLENIUM DESIGN PO BOX 737 ALAMO, CA 94507
- LANDSCAPE ARCHITECT PLURAL STUDIO 2742 17TH STREET SAN FRANCISCO, CA 94110
- STRUCTURAL ENGINEER HOHBACH-LEWIN 260 SHERIDAN AVE STE 150 PALO ALTO, CA 94306
- MEP ENGINEER EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- ENERGY CONSULTANT **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521



HOHBACH-LEWIN, INC. **HOHBACH-LEWIN, INC.** STRUCTURAL & CIVIL ENGINEERS 260 Sheridan Avenue, Palo Alto, CA 94306 650) 617-5930



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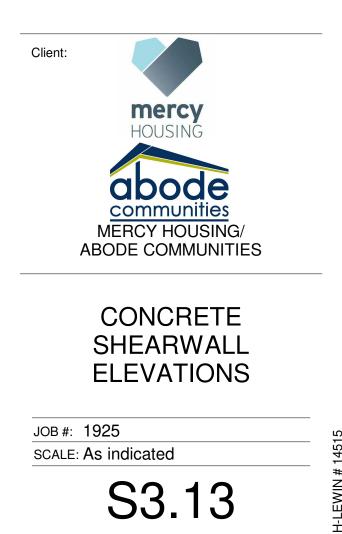
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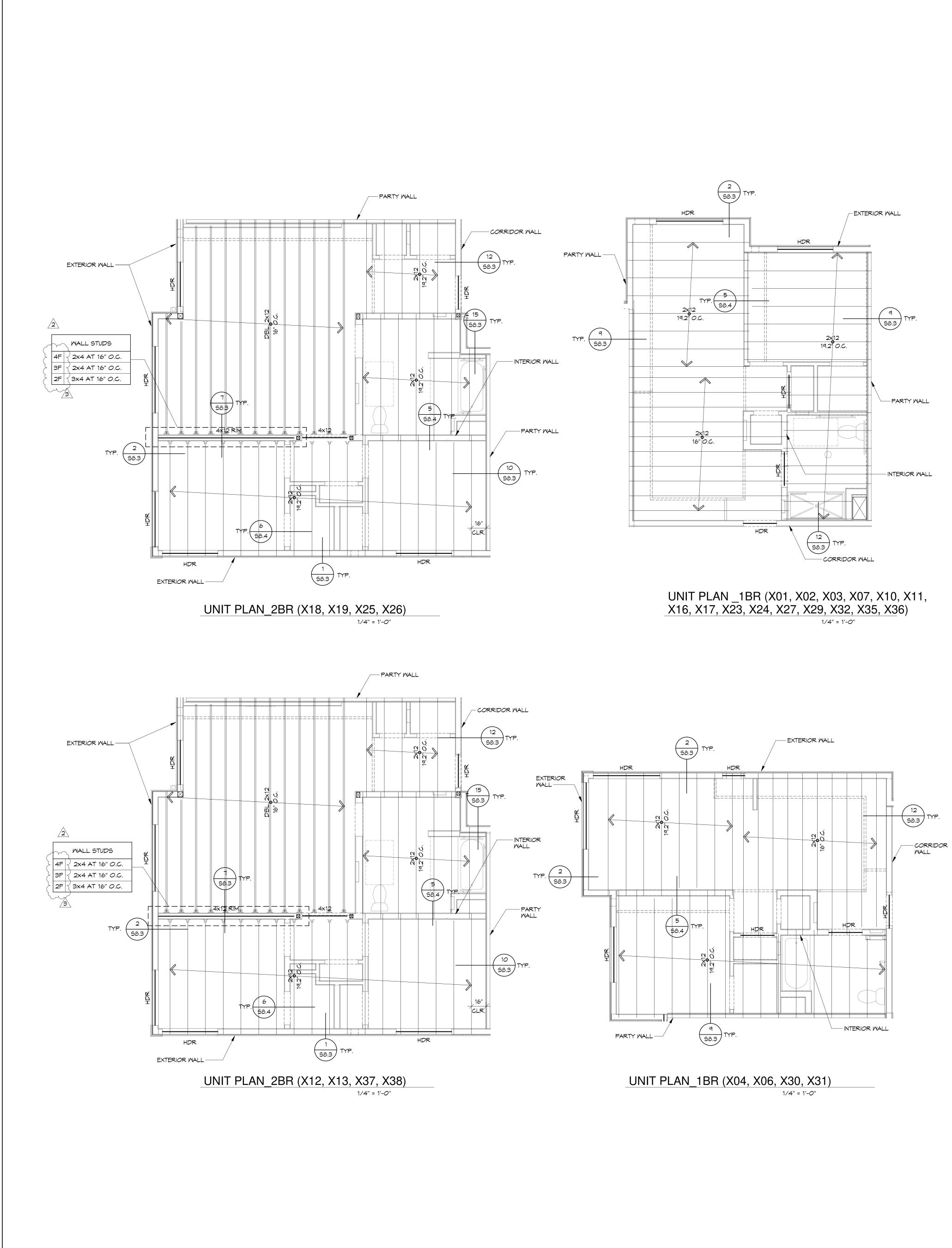
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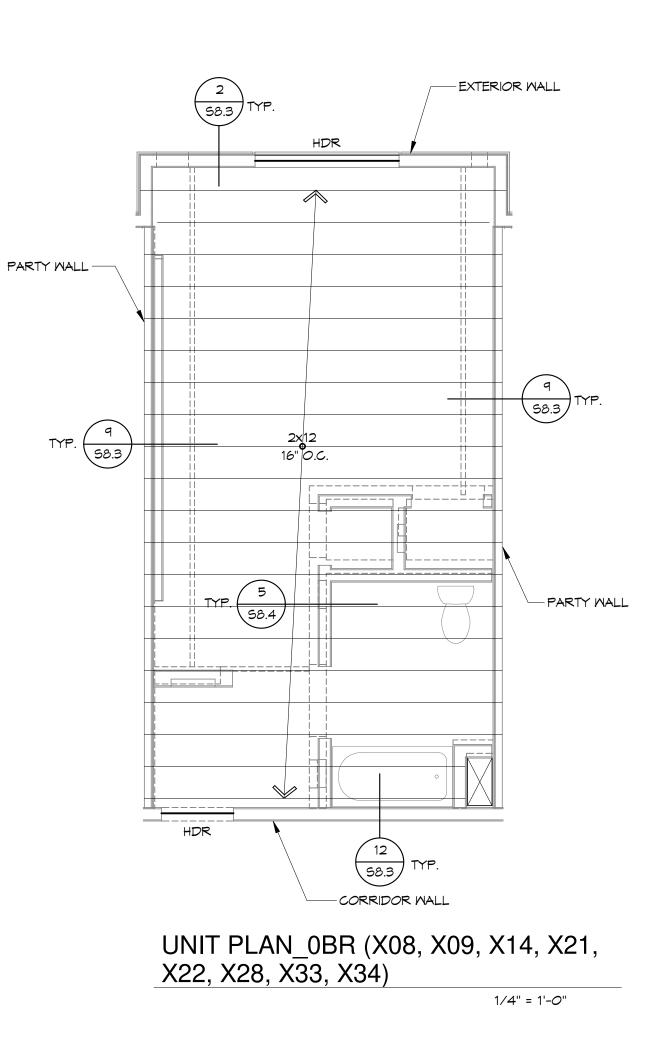
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UNIT FRAMING PLAN NOTES

- FOR UNIT LAYOUT AND DIMENSIONS, SEE ARCHITECTURAL DRAWINGS, TYP. U.O.N.
- 2. FOR SHEAR WALLS AND TIEDOWN LOCATIONS, SEE PLANS.
- FOR ADDITIONAL FRAMING NOT SHOWN OR NOTED, SEE З. PLANS.
- 4. FOR UNIT LAYOUTS NOT SHOWN, SEE PLANS. TYPICALLY, THESE UNITS DO NOT STACK AT ALL LEVELS.
- 5. ALL FLOOR JOISTS TO BE PER SCHEDULE.

2x12 (DF. #2) FLOOR	JOIST SCHEDULE
SPACING	MAX. SPAN
19.2" O.C.	14'-0"
16" O.C.	15'-3"
12" O.C.	17'-6"
DBL. AT 16" O.C.	21'- <i>O</i> "

- DBL. AT 12" O.C. 23'-0
- FOR TYPICAL WALL FRAMING INFORMATION, SEE DET.
 1/58.2 U.O.N. ON PLANS OR TYPICAL UNIT PLANS.
- 7. FOR BEAM TO POST CONNECTIONS, SEE DET. 9/58.2 U.O.N. ON PLANS OR TYPICAL UNIT PLANS.
- 8. FOR BUILT-UP POSTS, SEE DET. 9/58.4.
- CEILING AND SOFFIT FRAMING NOT SHOWN FOR CLARITY, 9. SEE DETAIL 4/58.4 FOR SCHEDULE.
- 10. IT IS ACCEPTABLE TO PROVIDE DOUBLE JOISTS AT 18" O.C. IN LIEU OF JOIST AT 12" O.C. WHERE WIDER JOIST SPACING IS REQUIRED FOR MECH. DUCTS, SEE MECH. DRAWINGS FOR LOCATIONS.
- 11. EBM SUPPORT POSTS NOT SHOWN FOR CLARITY, SEE PLANS.



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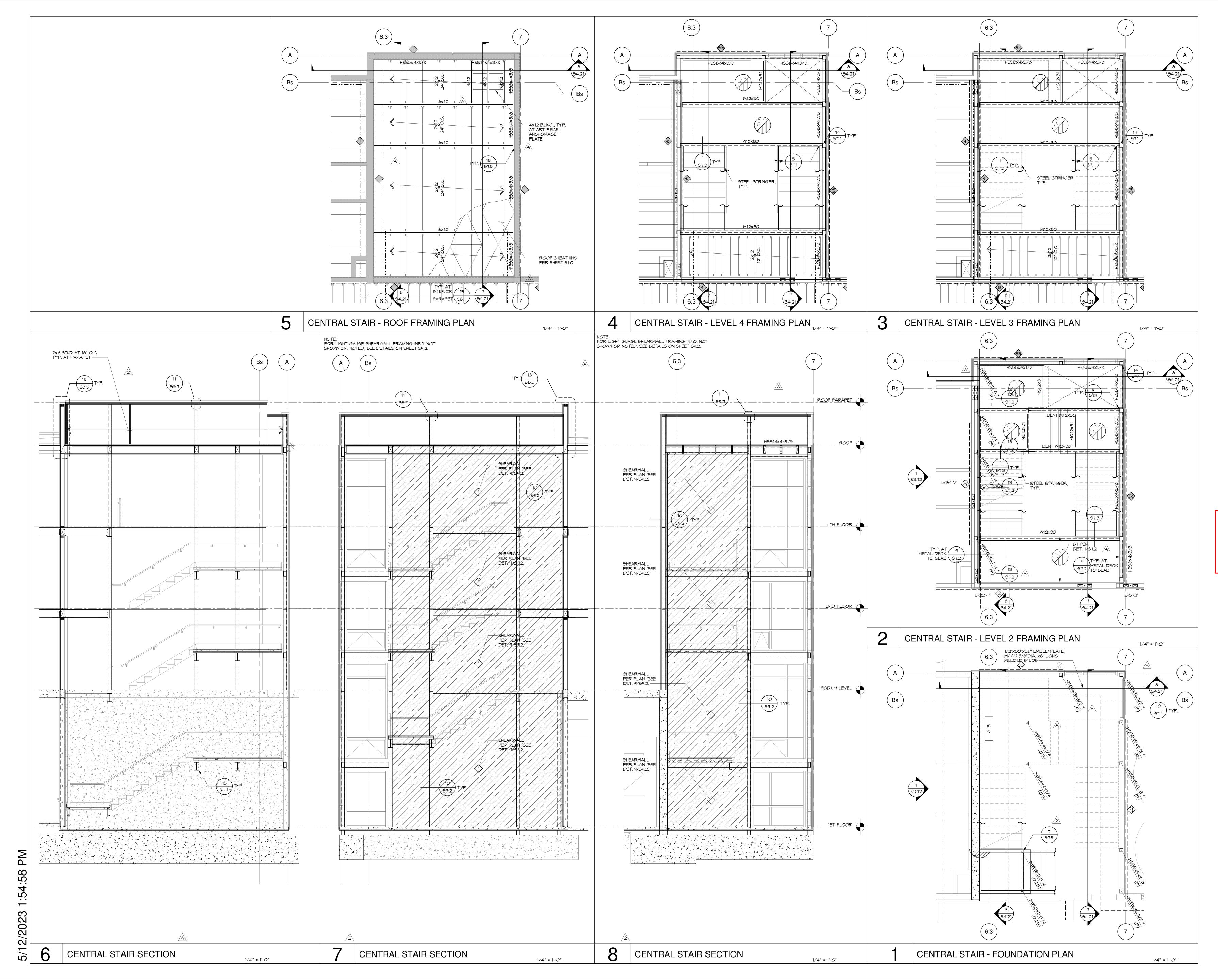
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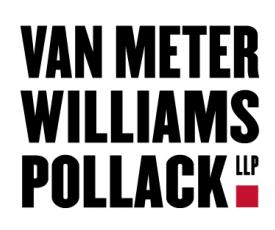
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UNIT FRAMING PLANS

JOB #: 1925 SCALE: As indicated S4.11 PLAN CHECK RESPONSE 2 DATE: 03/20/23





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COUNTY OF SANTA CLARA
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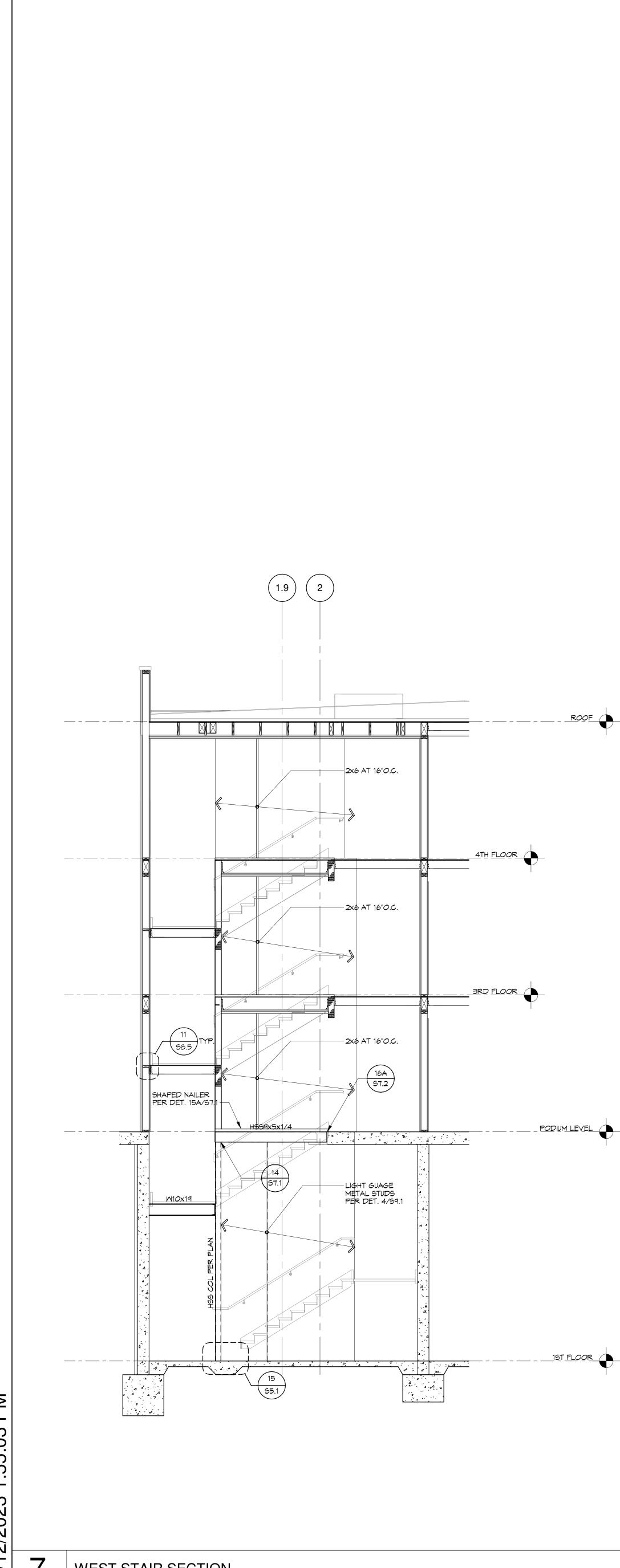
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EDUCATOR HOUSING 231 GRANT AVENUE

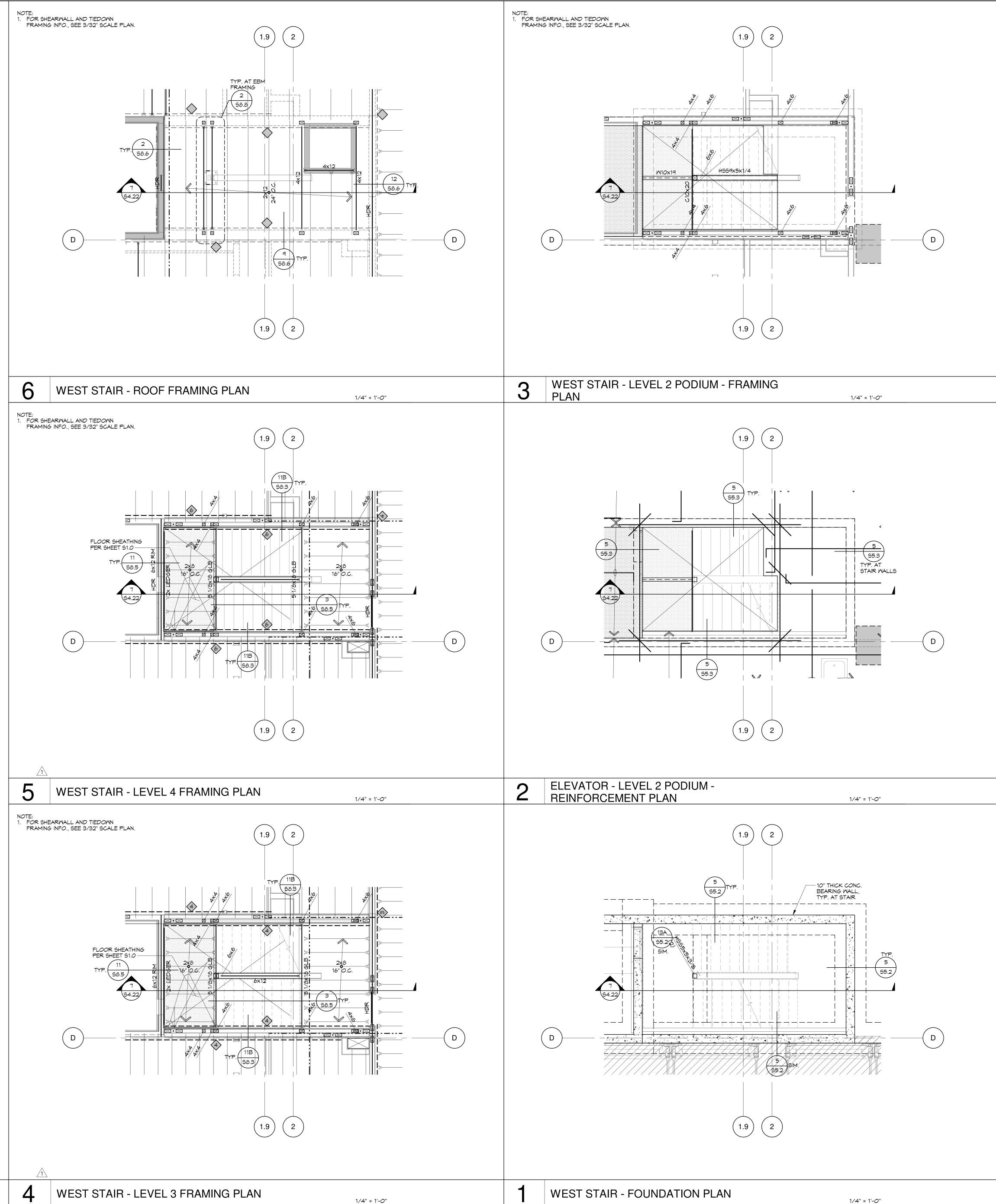
231 GRANT AVENUE PALO ALTO, CA 94306





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1/4" = 1'-0"





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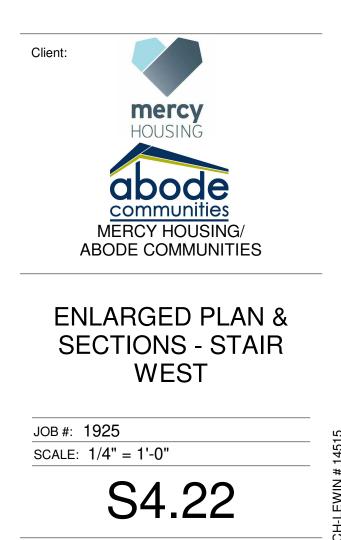
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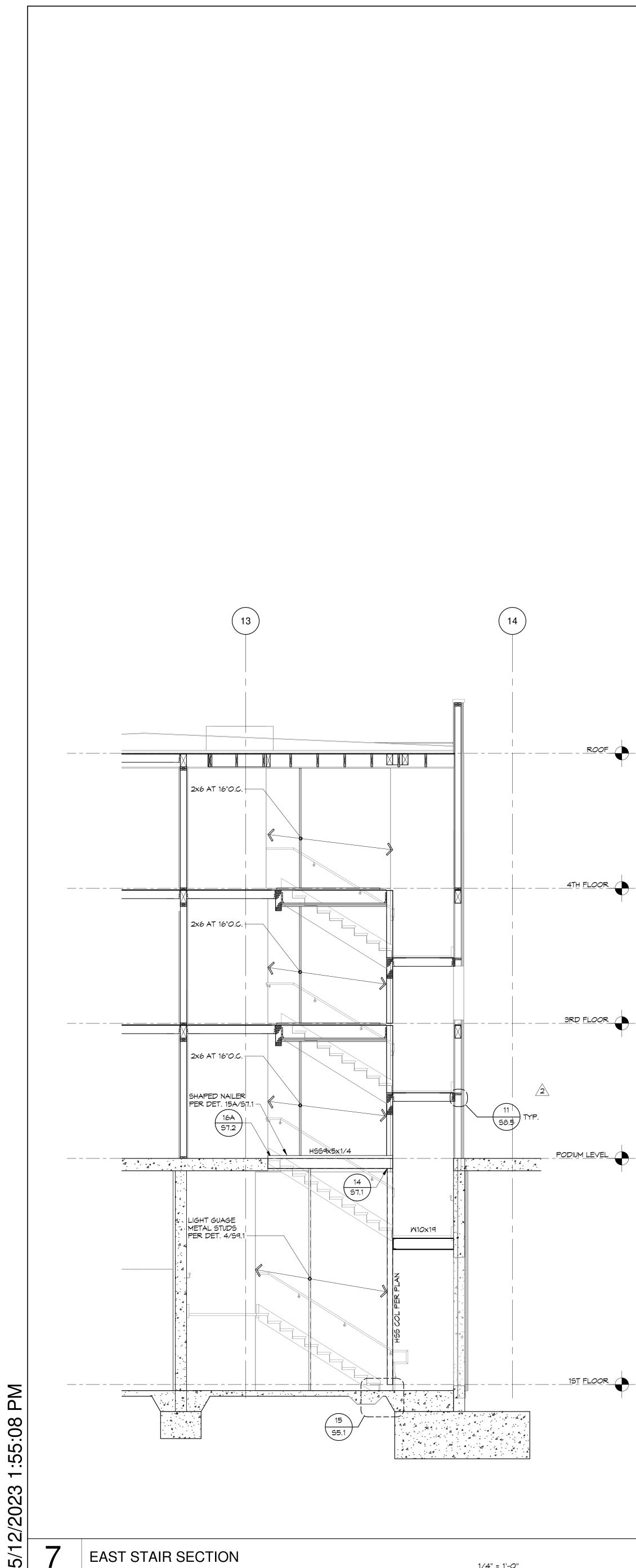
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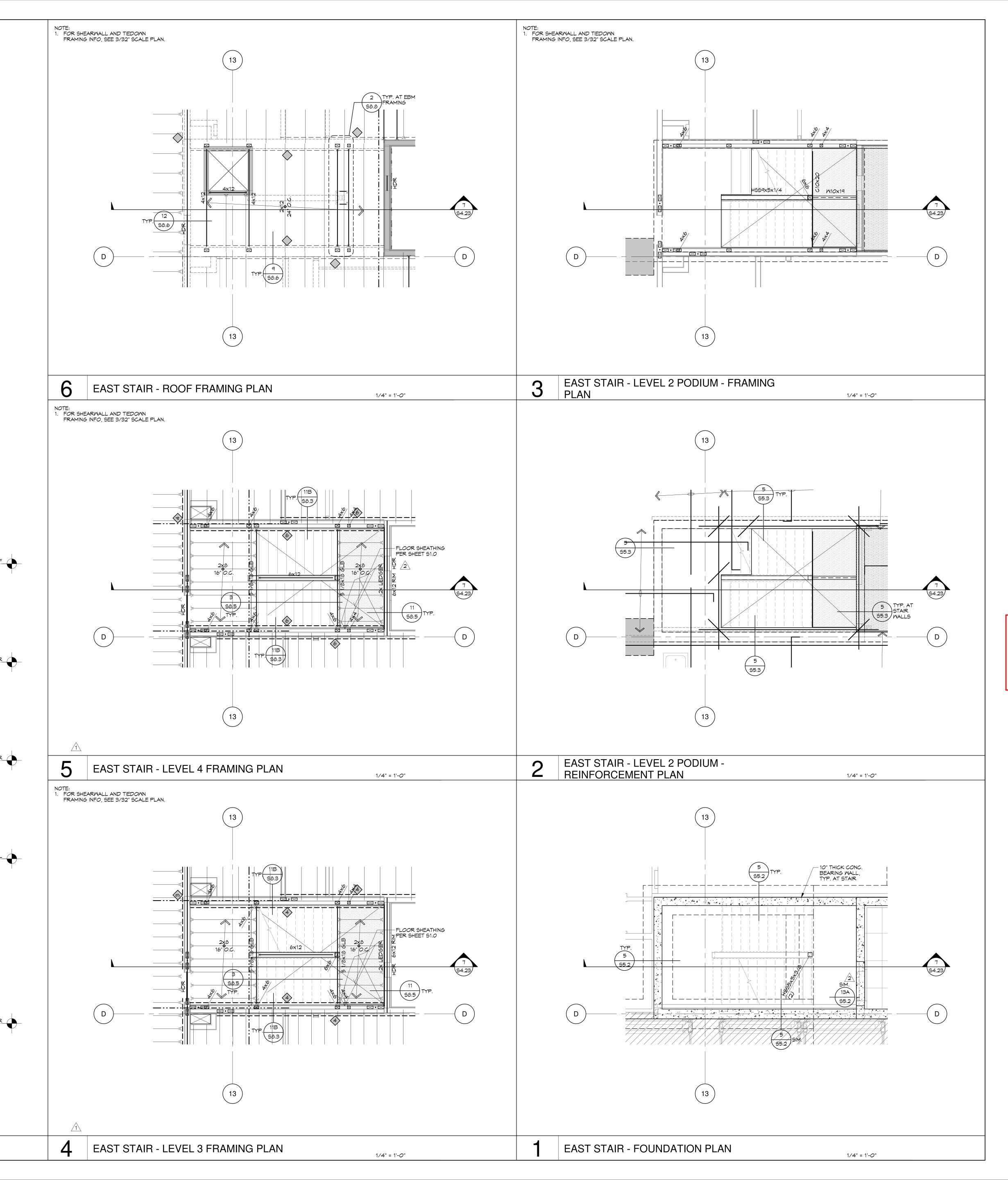
231 GRANT AVENUE PALO ALTO, CA 94306





EAST STAIR SECTION

1/4" = 1'-0"





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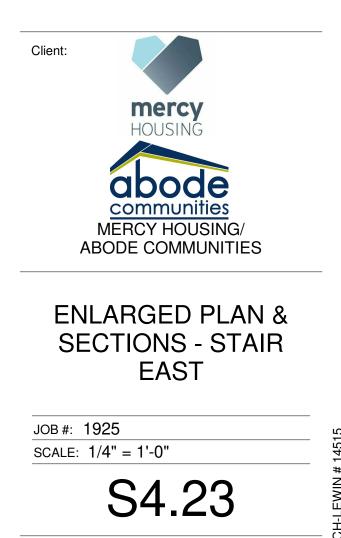
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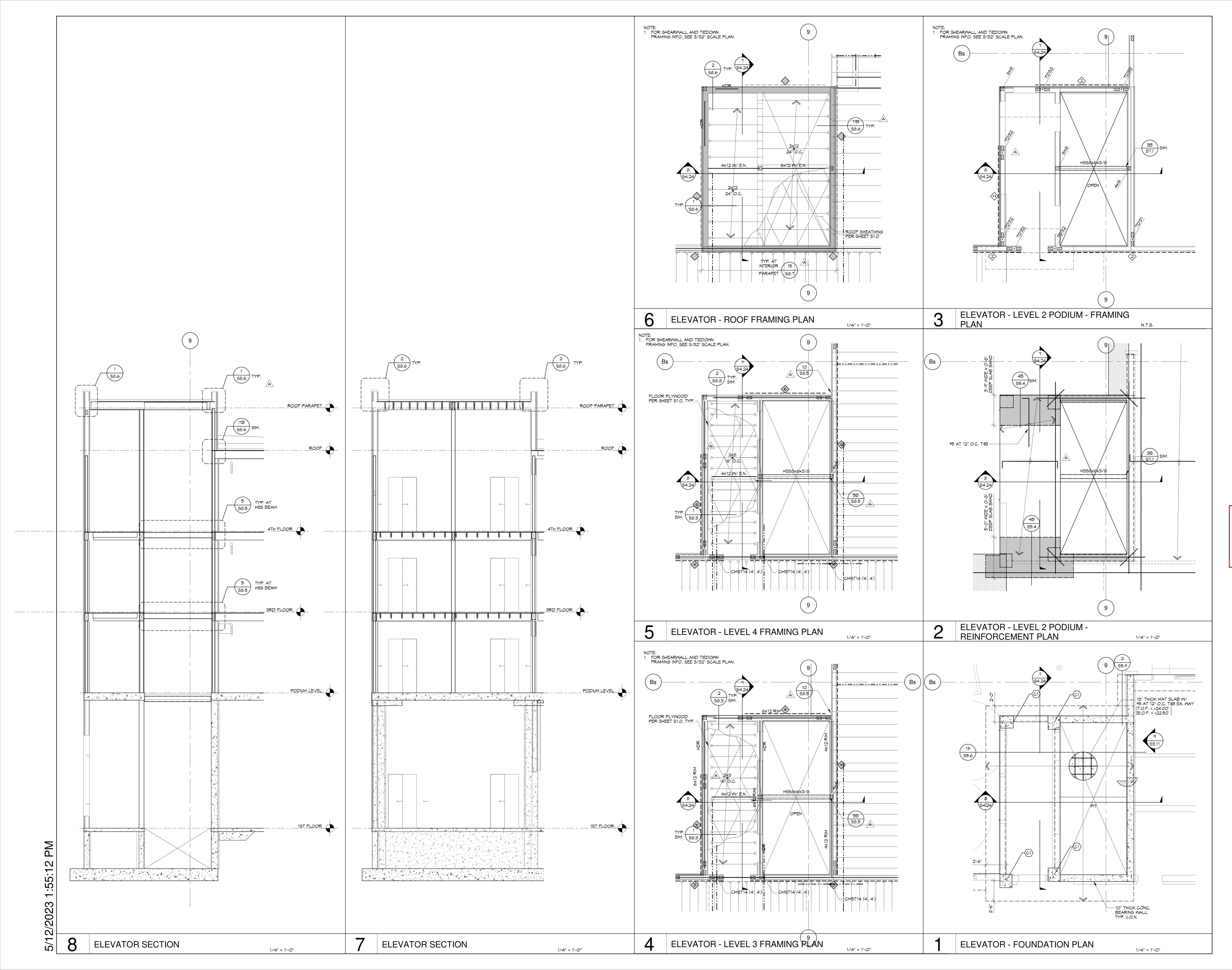
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231 GRANT AVENUE PALO ALTO, CA 94306







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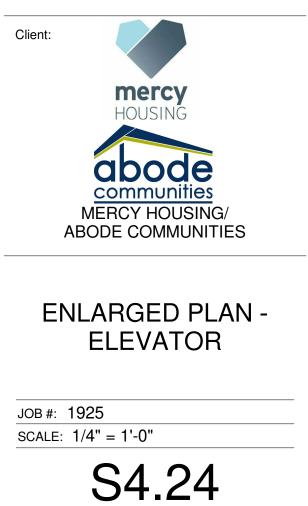
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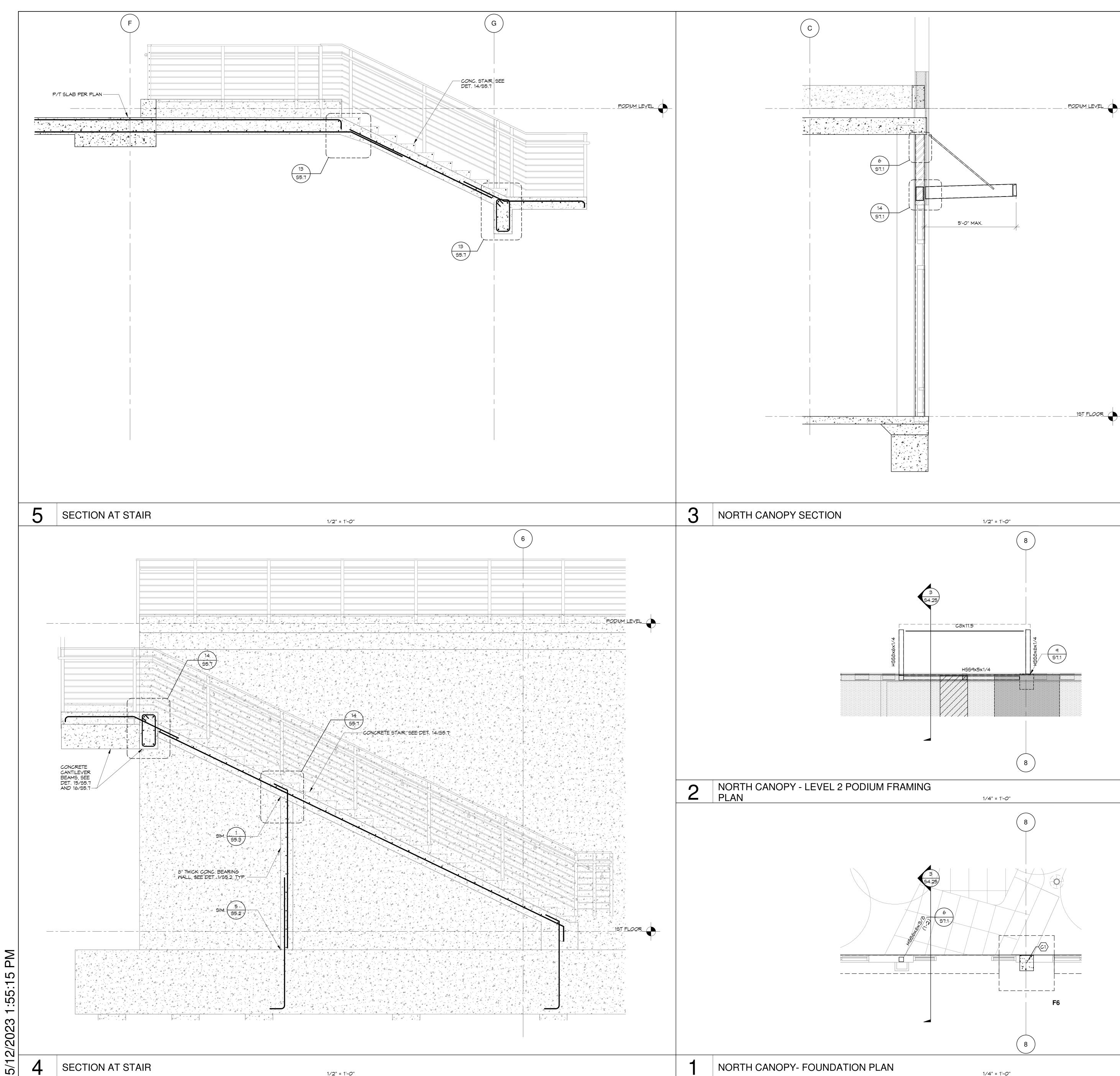
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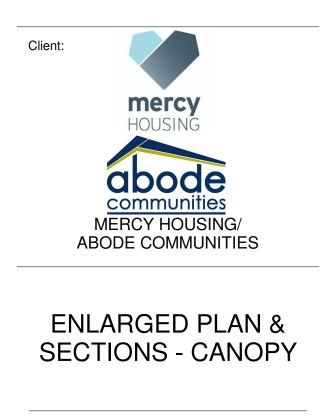
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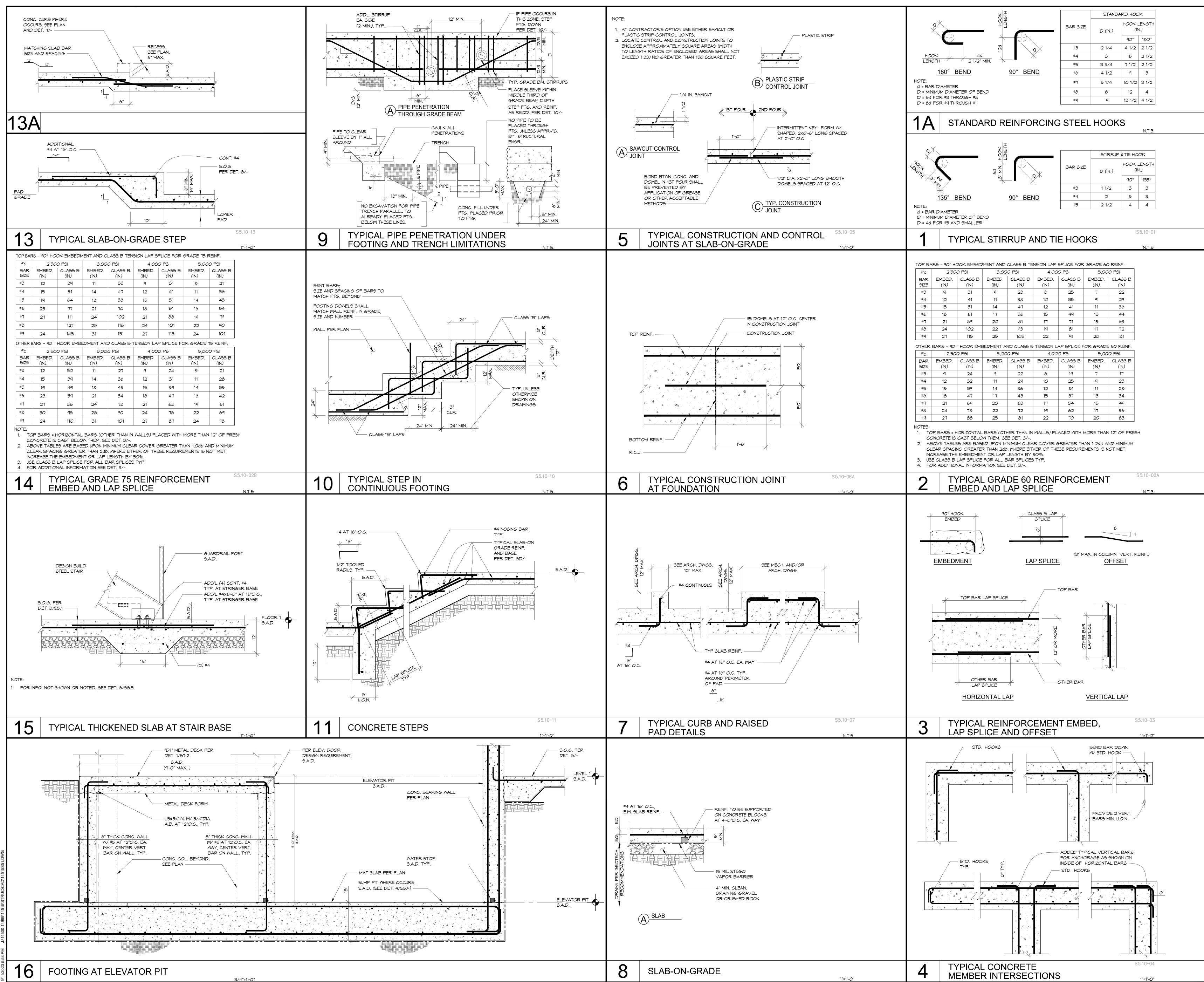
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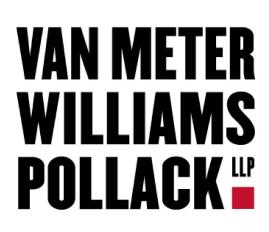
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JOB #: 1925 SCALE: As indicated S4.25





- **BKF-SAN JOSE** 1730 N. FIRST ST. STE 600 SAN JOSE, CA 95112
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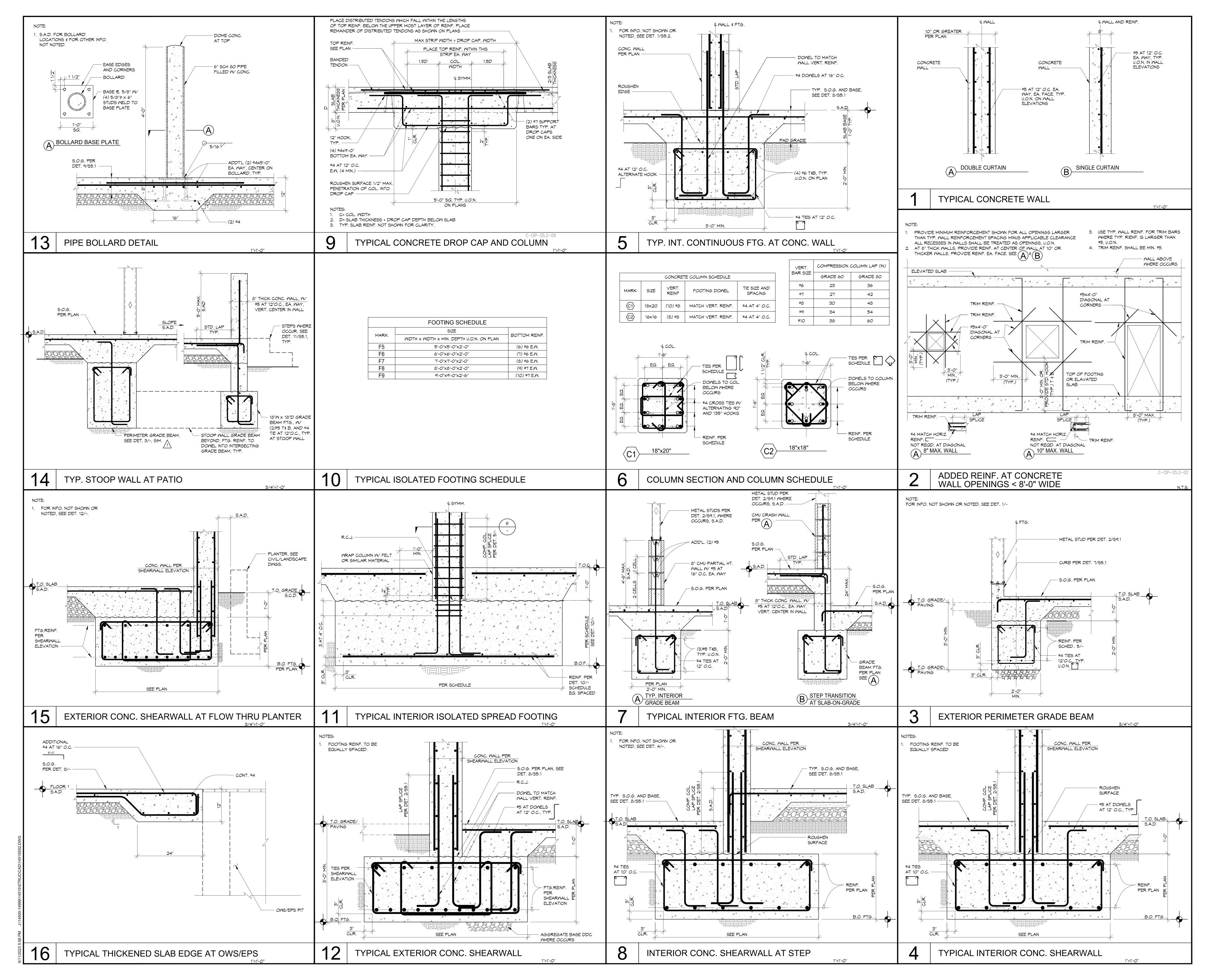
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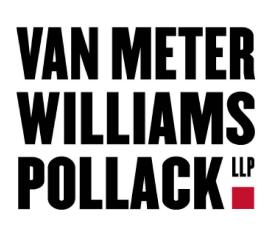
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Project:



231 GRANT AVENU PALO ALTO, CA 943	
Client: mercy HOUSING	
MERCY HOUSING ABODE COMMUNITI	
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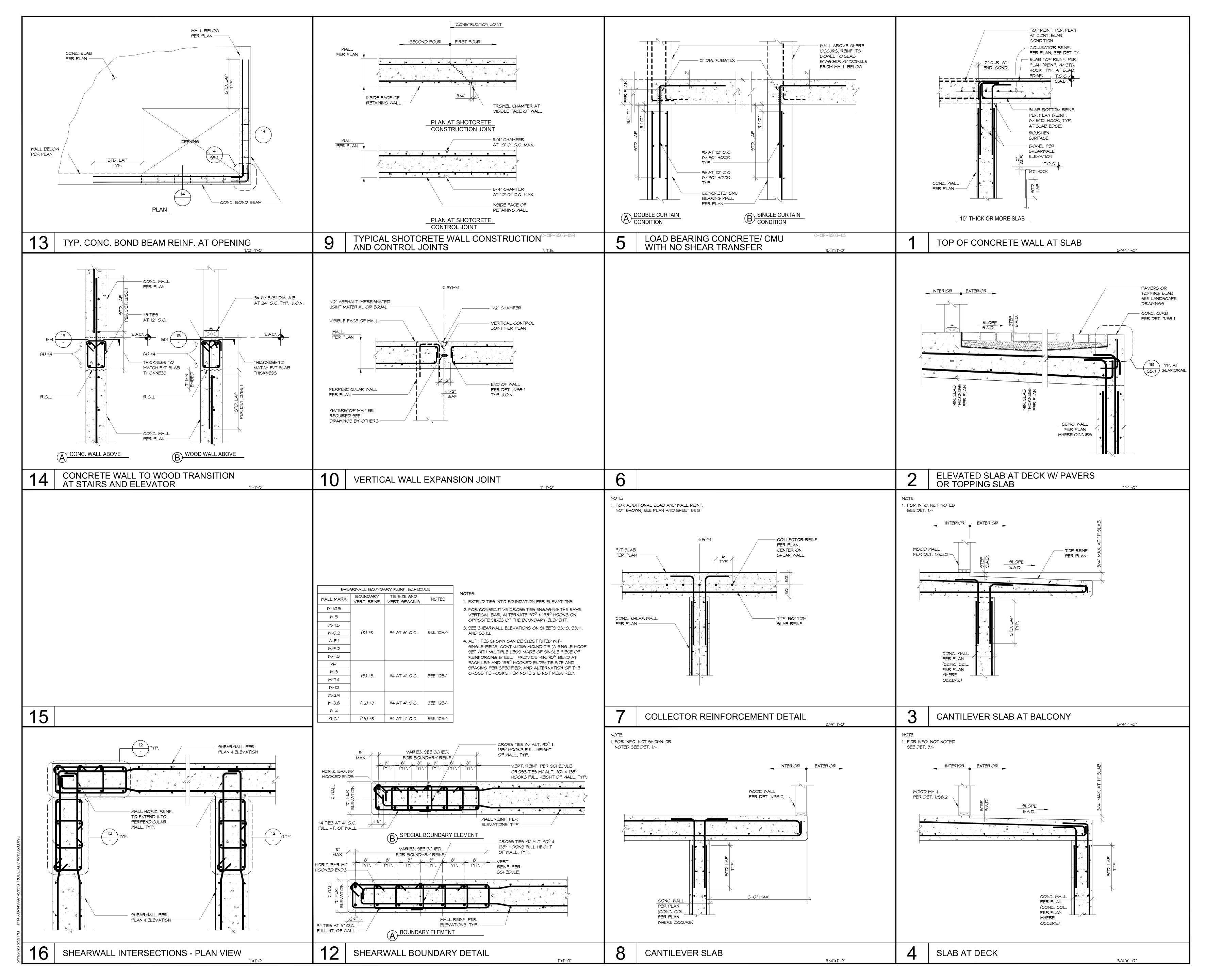
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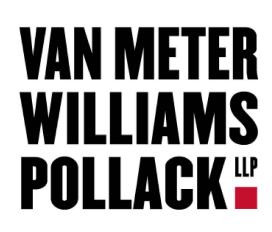
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231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	_
HOUSING	
MERCY HOUSING/ ABODE COMMUNITIES	_
TYPICAL CONCRETE DETAILS	-
ЈОВ #: 1925	ا 14515
SCALE: As indicated	# = NI
S5.2	HOHBACH-LEWIN # 14515
PLAN CHECK RESPONSE 2 DATE: 03/20/2023	유





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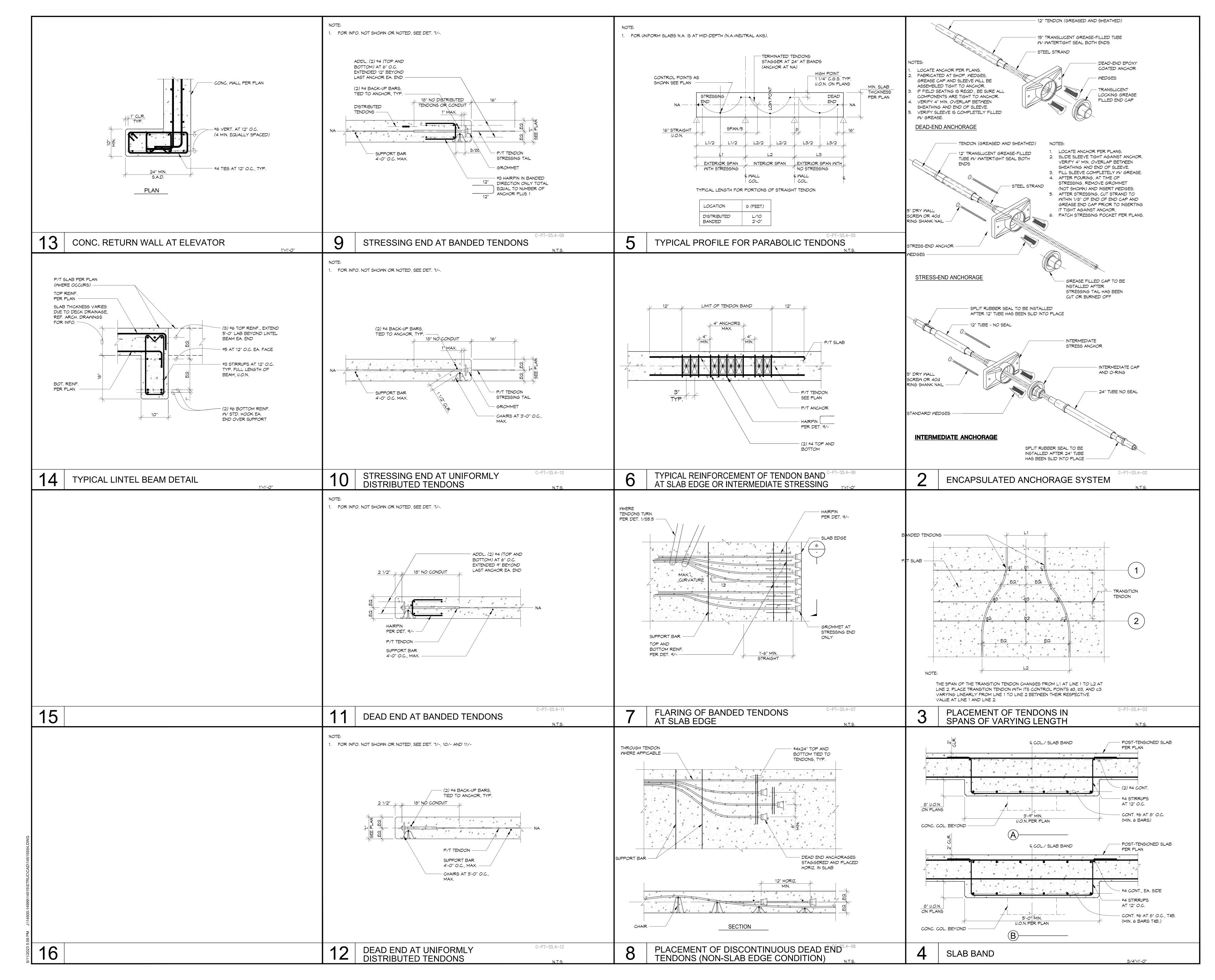
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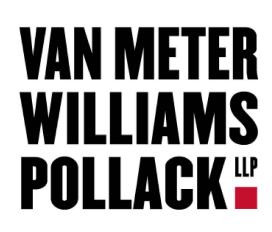
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	231 GRANT AVENUE PALO ALTO, CA 94306	
-	Client:	
	mercy HOUSING	
	abode	
-	MERCY HOUSING/ ABODE COMMUNITIES	_
	TYPICAL CONCRETE DETAILS	
	JOB #: 1925 SCALE: As indicated	/IN # 14515
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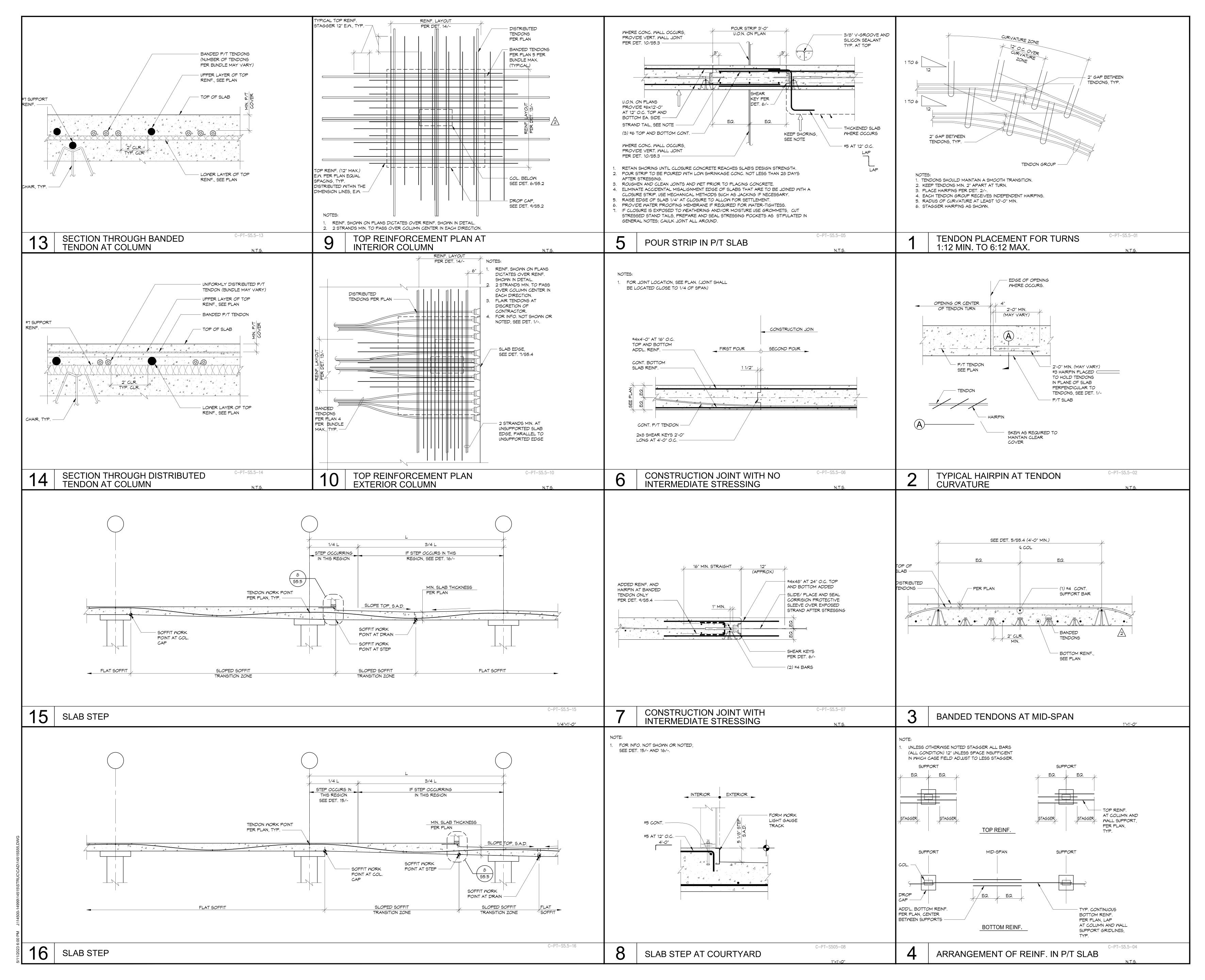
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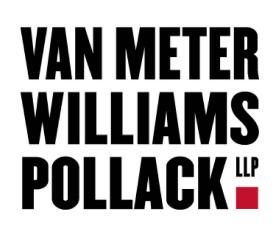
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Client:		_
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- **BKF-SAN JOSE** 1730 N. FIRST ST. STE 600 SAN JOSE, CA 95112
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- STRUCTURAL ENGINEER **HOHBACH-LEWIN INC** 250 SHERIDAN AVE STE 100 PALO ALTO, CA 94306
- MEP ENGINEER EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- SUSTAINABILITY/ENERGY **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521
- JOINT TRENCH/DRY UTILITY MILLENIUM DESIGN PO BOX 737 ALAMO, CA, 94507



→_I HOHBACH-LEWIN, INC. STRUCTURAL & CIVIL ENGINEERS 260 Sheridan Avenue, Suite 150 Palo Alto, CA 94306 650) 617-5930





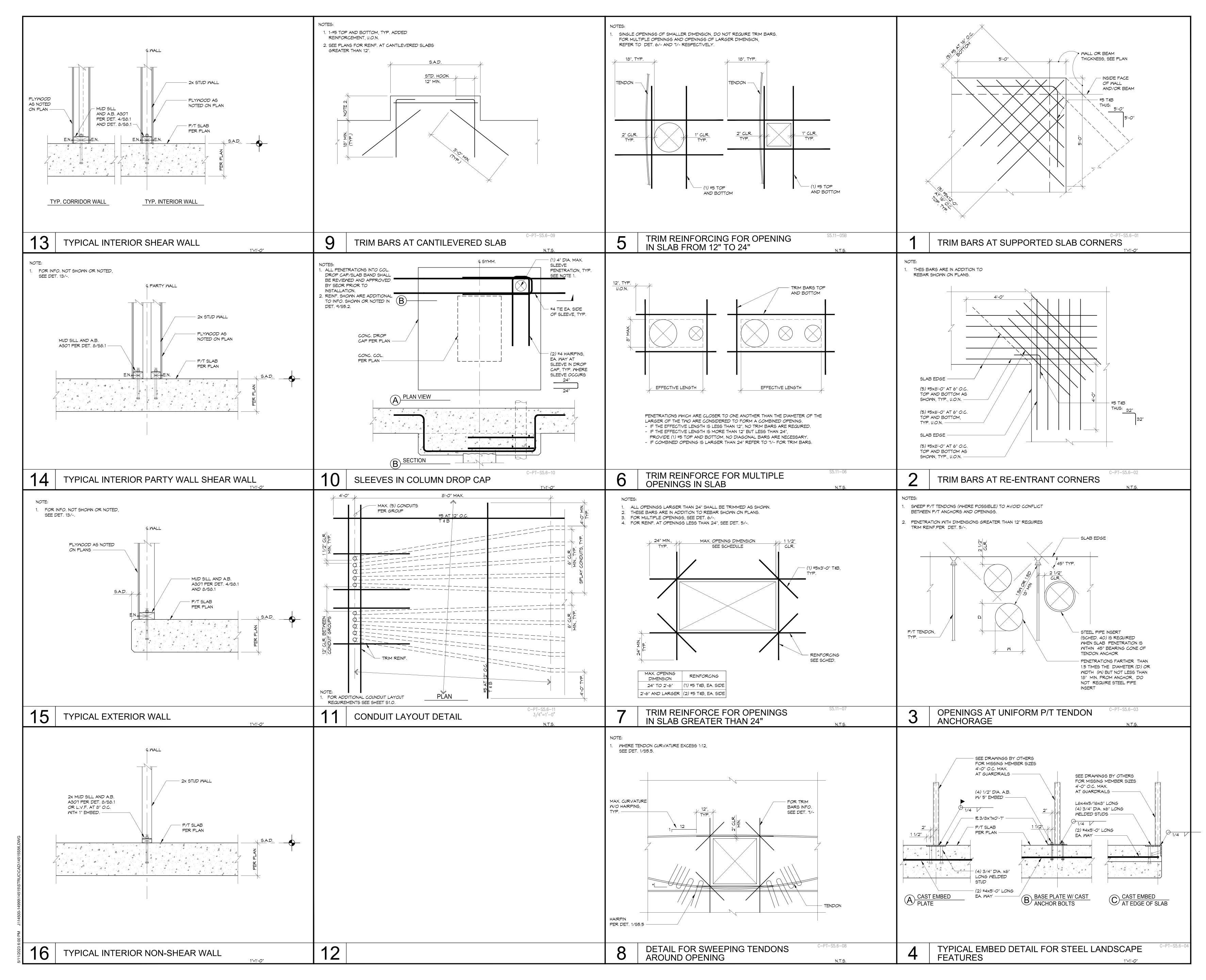
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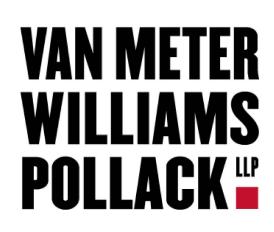
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В	03/20/2023	BID ADDENDUM
2	03/20/2023	PLAN CHECK RESPONSE 2
3	05/12/2023	PLAN CHECK RESPONSE 3

Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client: mercy	_
HOUSING COMMUNITIES MERCY HOUSING/	
ABODE COMMUNITIES	_
TYPICAL CONCRETE DETAILS	
JOB #: 1925	14515
SCALE: As indicated	- #
S5.5	OHBACH-LEWIN # 14515
PLAN CHECK RESPONSE 2 DATE: 03/20/2023	



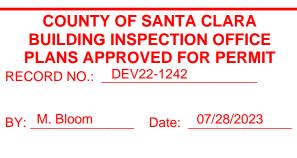


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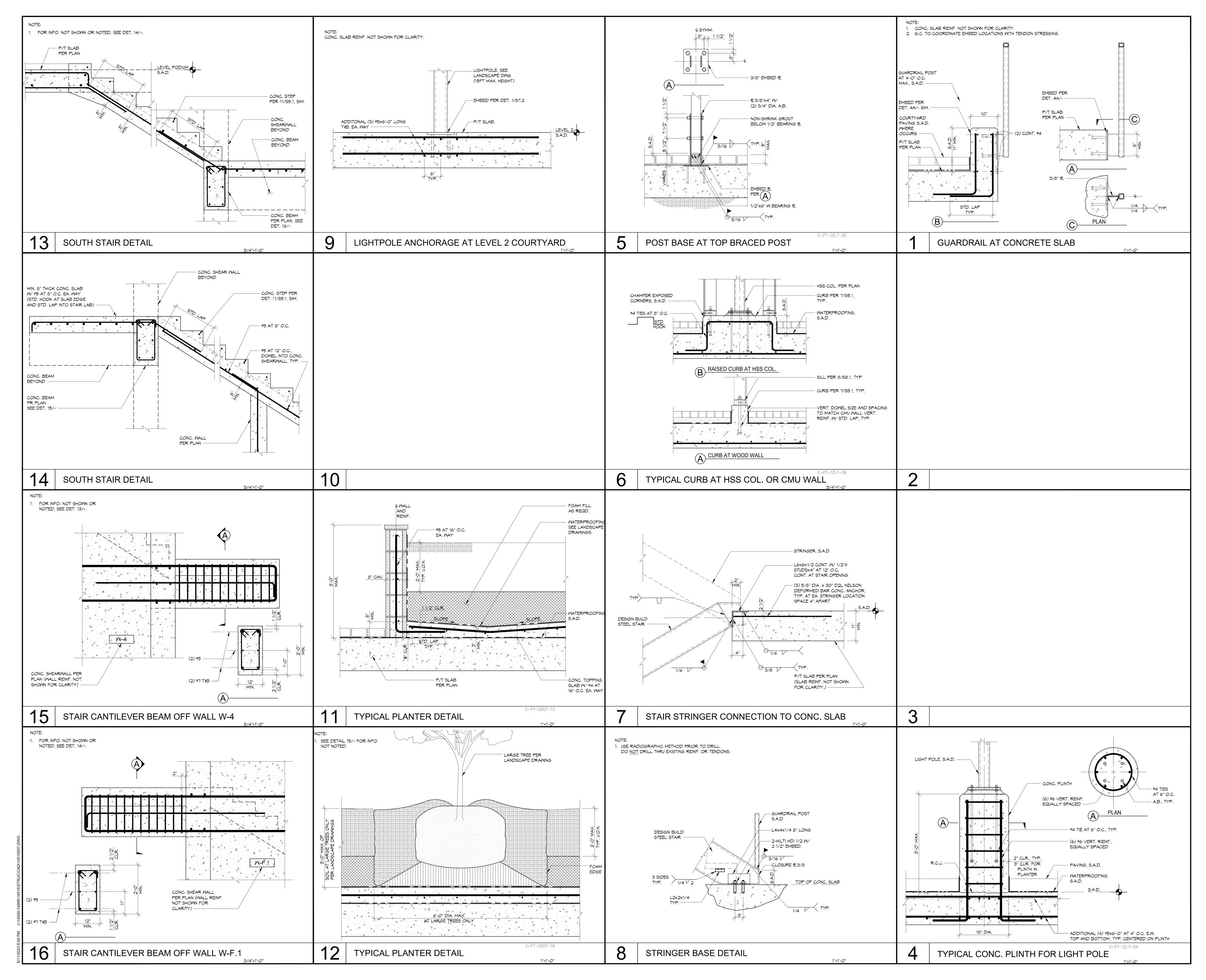
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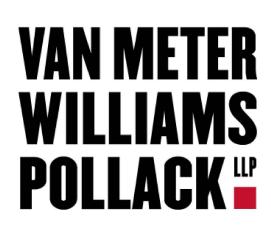
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Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	_
HOUSING	
MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL CONCRET DETAILS	Ξ
 Job #: 1925	ا 14515
SCALE: As indicated	- # N #
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PLAN CHECK RESPONSE 2 DATE: 03/20/202	





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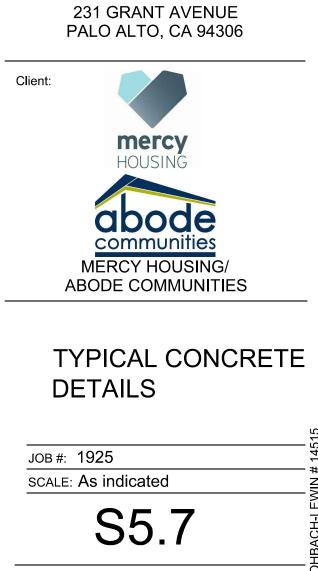


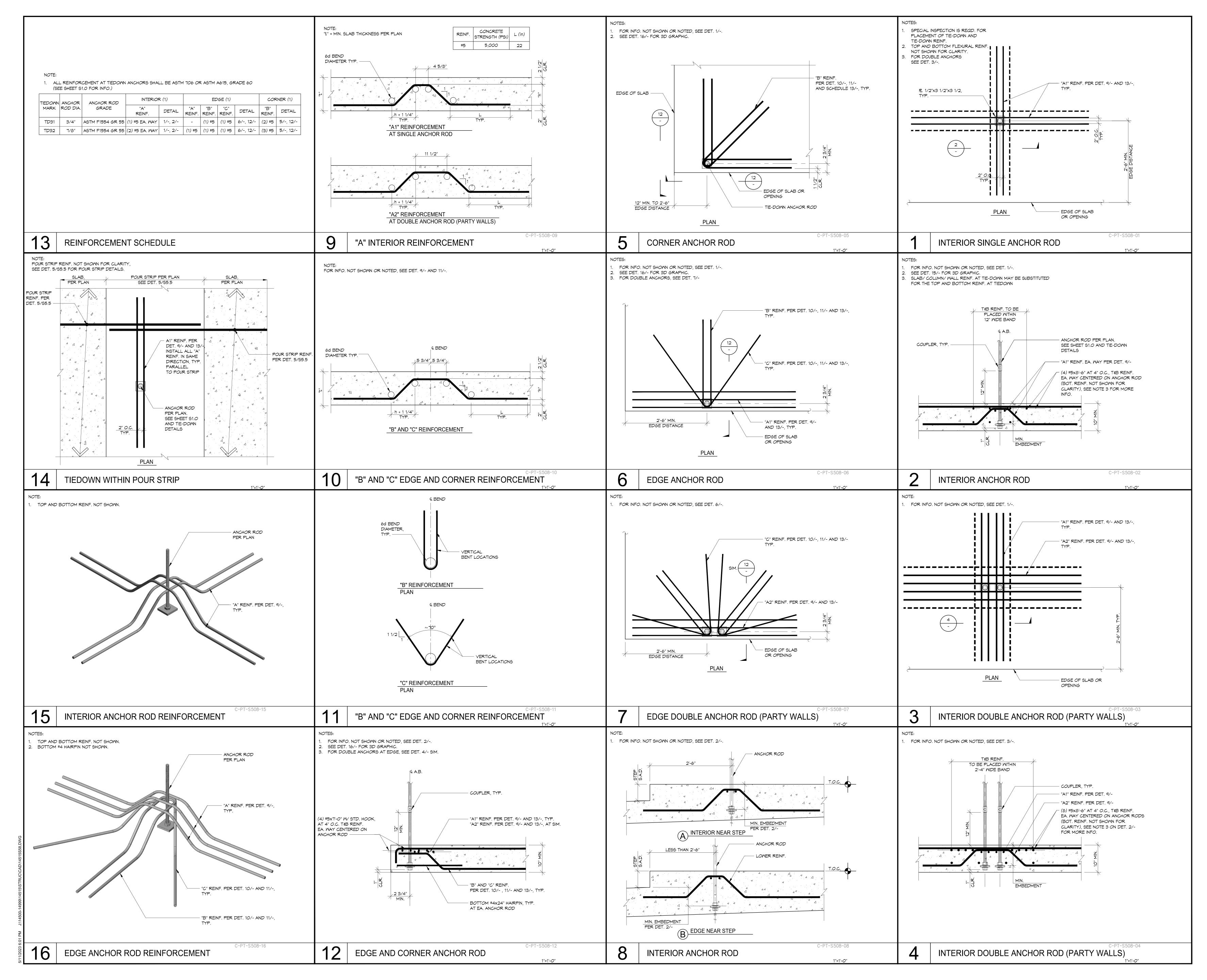
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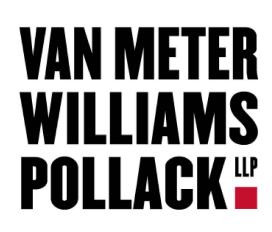
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Project:







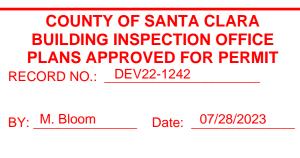


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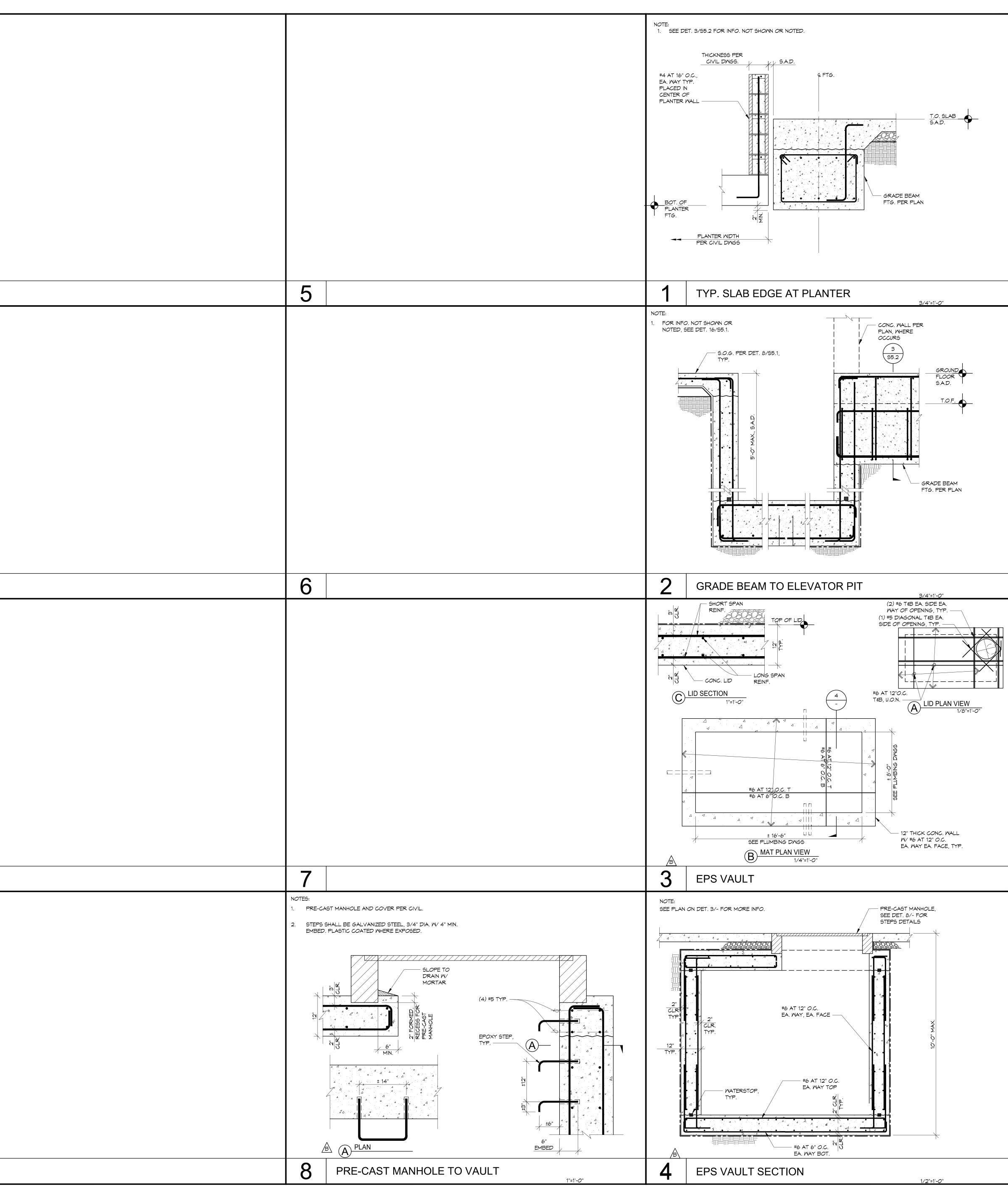
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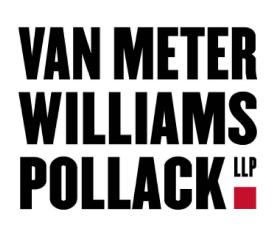
Project:



	231 GRANT AVENUE ALO ALTO, CA 94306	
Client:	mercy	-
	HOUSING	
AI	Communities MERCY HOUSING/ BODE COMMUNITIES	
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JOB #:		+ 14515
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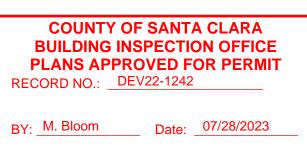


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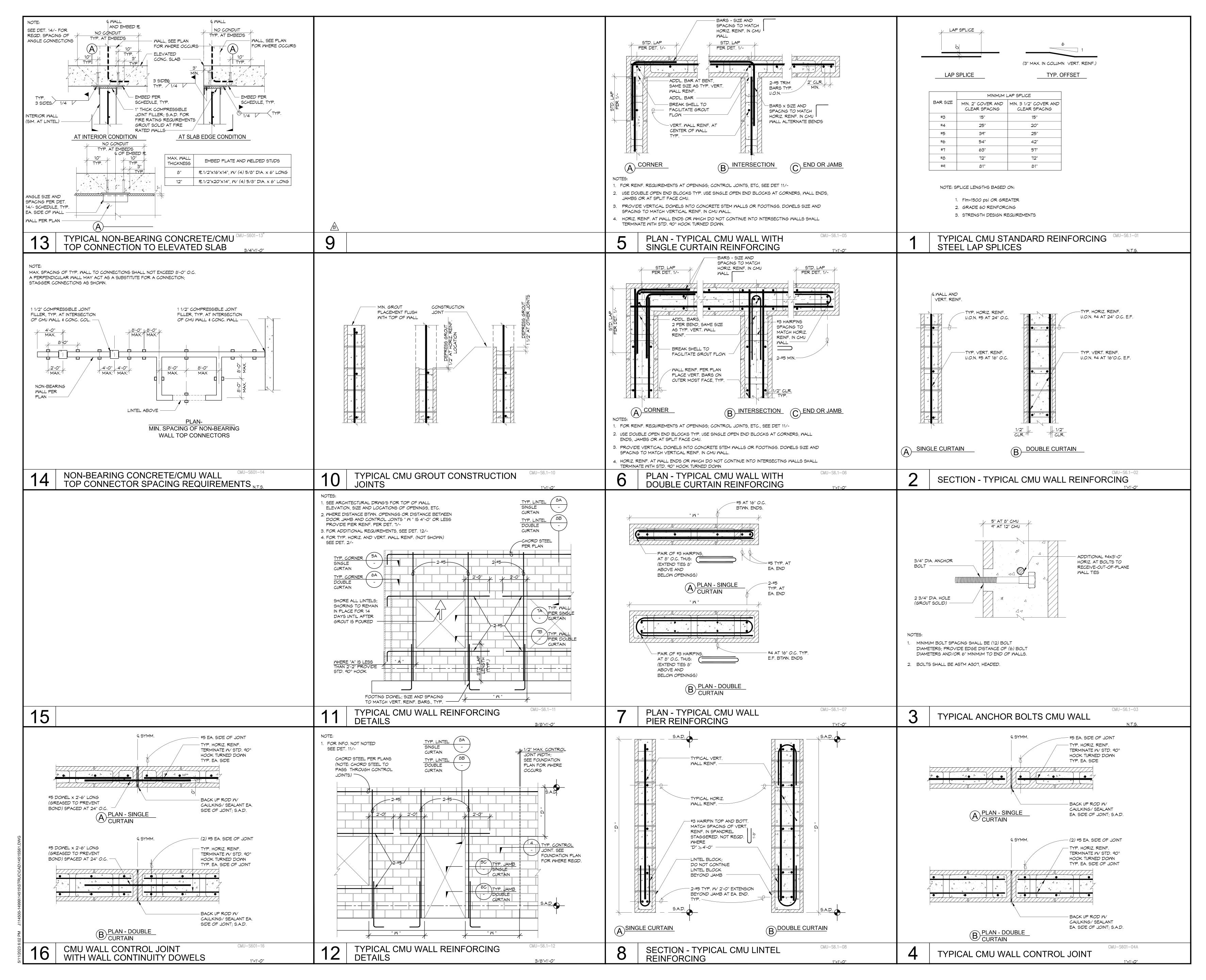
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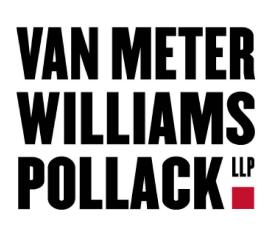
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Project:



	31 GRANT AVENUE LO ALTO, CA 94306	
Client:		
	HOUSING	
	Dercy Housing/	
	PICAL CONCRET TAILS	E
		515
JOB #: 1	925	+ 4
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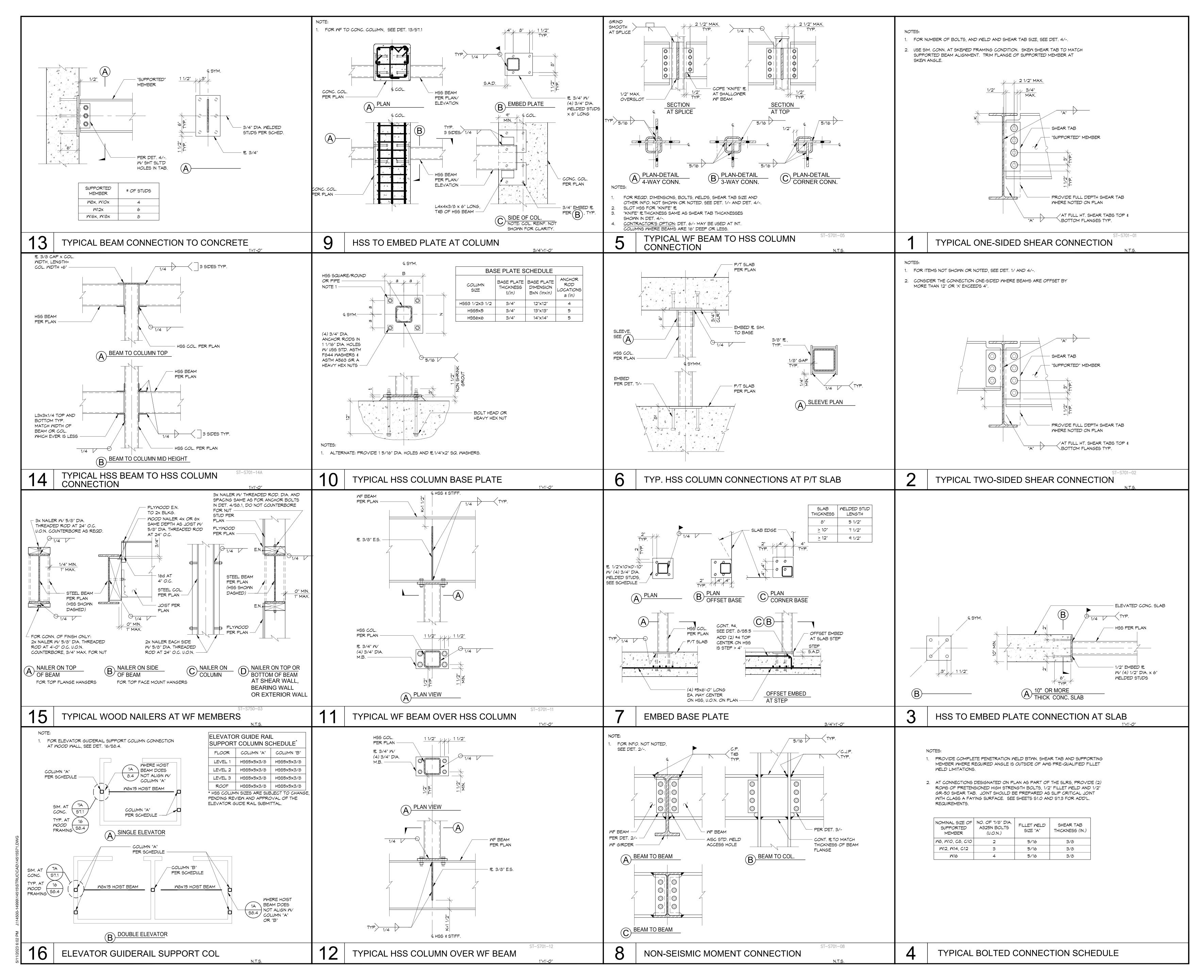
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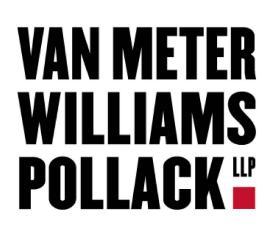
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Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	-
mercy HOUSING	
abode	
MERCY HOUSING/ ABODE COMMUNITIES	_
TYPICAL CMU DETAILS	
JOB #: 1925 SCALE: As indicated	/IN # 14515
S6.1	OHBACH-LEWIN # 14515
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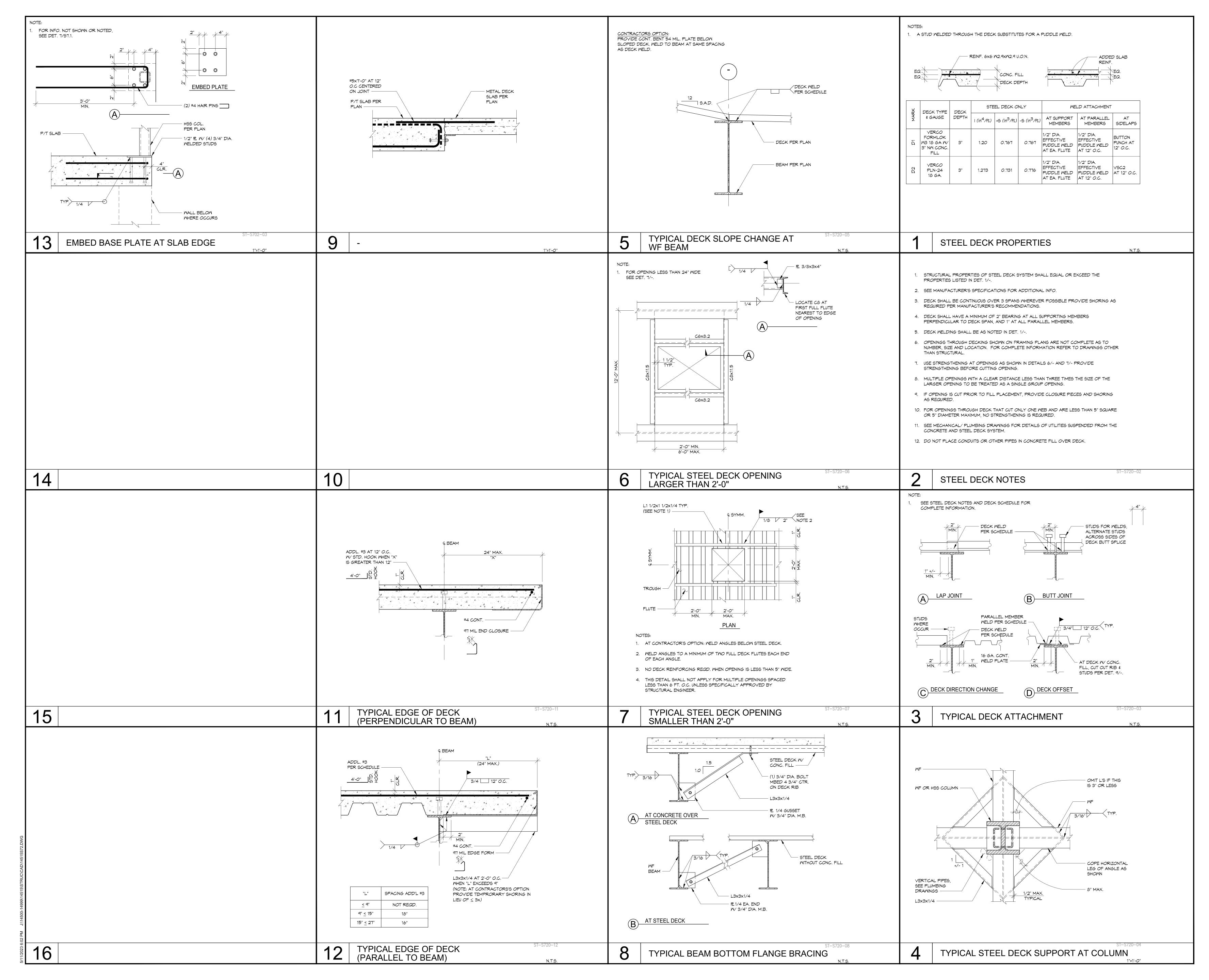
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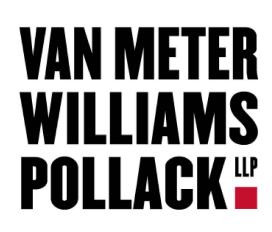
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Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	-
HOUSING	
MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL STEEL DETAILS	Q
JOB #: 1925 SCALE: As indicated	NIN # 1451
S7.1	OHBACH-LEWIN # 14515





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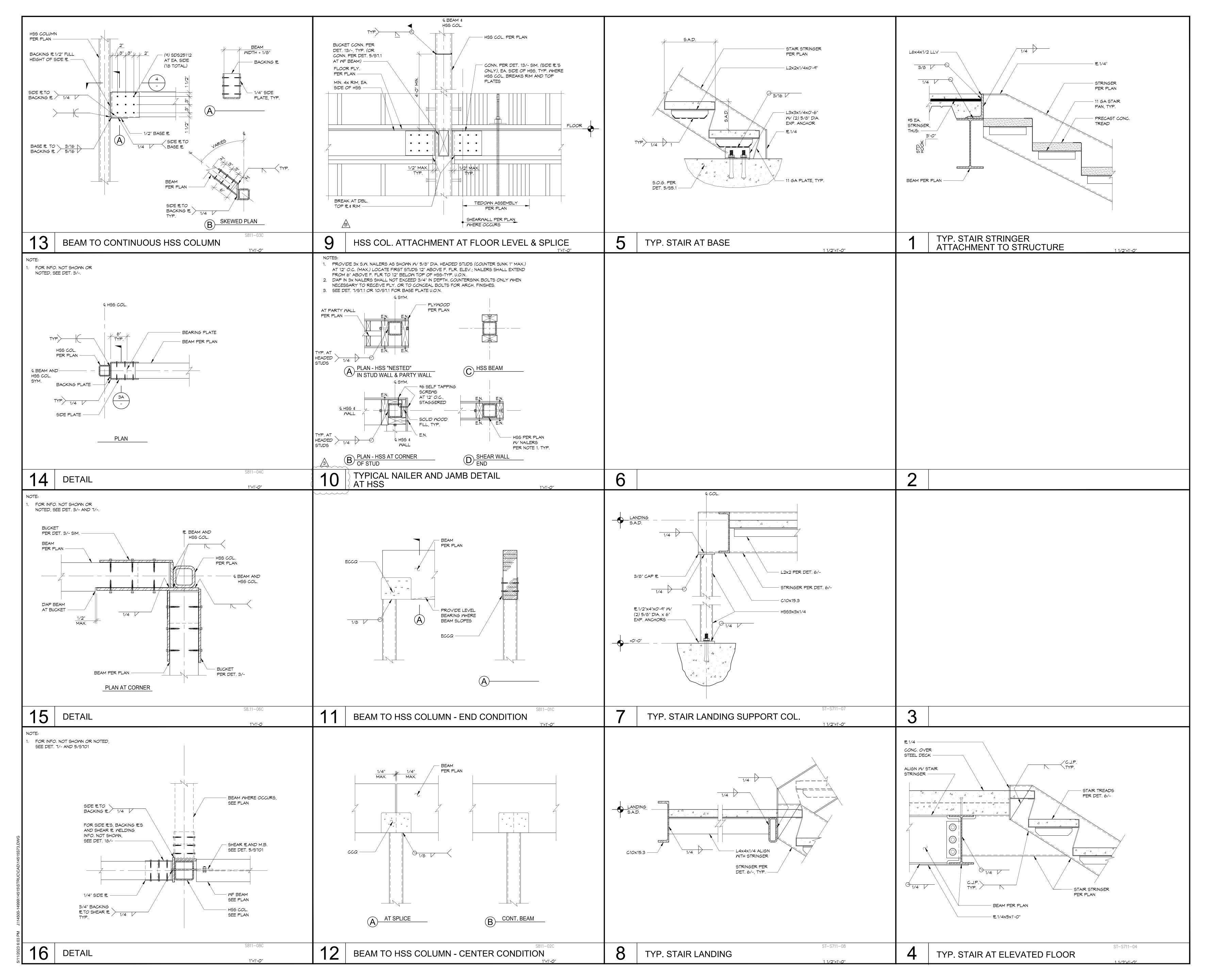
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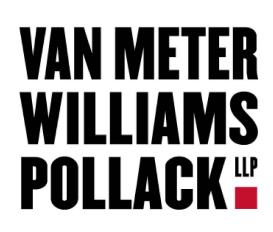
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Project:



231 GRANT AVENU PALO ALTO, CA 943	
Client:	
HOUSING	
abode	
MERCY HOUSING ABODE COMMUNIT	
TYPICAL STEE DETAILS	
JOB #: 1925 SCALE: As indicated	//////////////////////////////////////
S7.2	OHBACH-LEWIN # 14515





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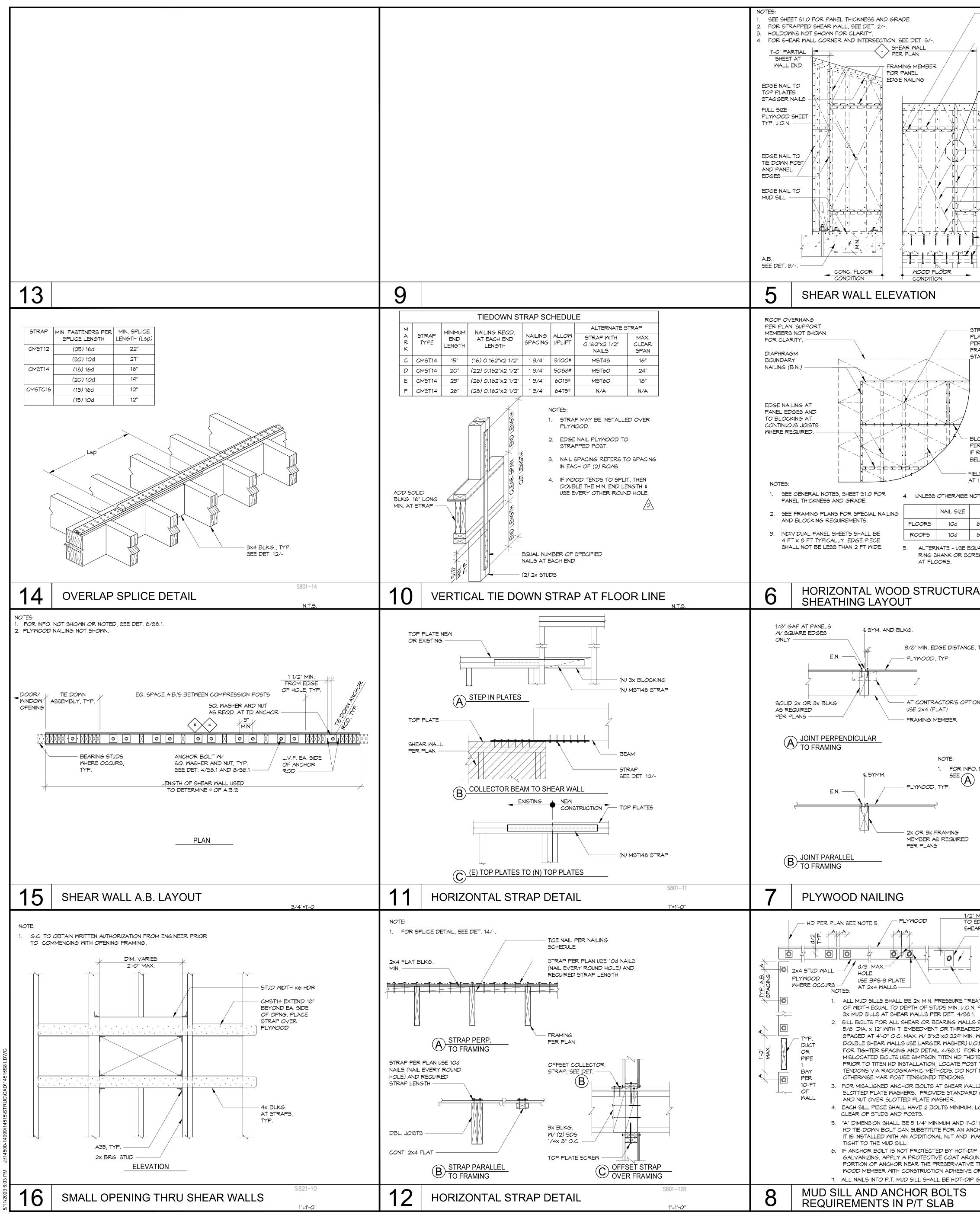
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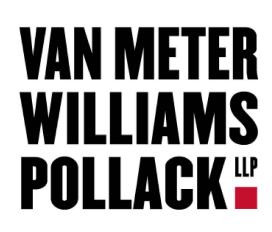
Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	
mercy HOUSING	
abode	
Communities MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL STEEL	
DETAILS	
JOB #: 1925 SCALE: As indicated	N # 14515
S7.3	DHBACH-LEWIN # 14515
	OHB.



		_
— STAGGER HORIZ. PANEL JOINTS, TYP. OR USE FULL HEIGHT SHEETS	1. USE 0.148"X3" (10d) COMMON WIRE NAILS U.O.N.	
WHERE POSSIBLE — BLKG. AT PANEL	2. ** NUMBER SHOWN IN SYMBOL REPRESENTS PLYWOOD PANEL EDGE NAILING IN INCHES.	
EDGES SAME SIZE AS PANEL EDGE STUD., SEE DET. 7A/	THE DENOTES DOUBLE SHEAR WALL WITH PLYWOOD ON BOTH SIDES. NUMBER SHOWN IN SYMBOL REPRESENTS PLYWOOD PANEL EDGE	
PANEL JOINT1/2" MIN.	NAILING IN INCHES.	
	 SEE DET. 4/- FOR SHEAR WALL SCHEDULE. SEE DET. 5/- FOR SHEAR WALL ELEVATION AND FRAMING INFORMATION. 	
	5. SEE DET. 2/- FOR STRAPPED SHEAR WALL ELEVATION AND FRAMING INFORMATION.	
	6. ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PLYWOOD PANELS SHALL BE 3X OR GREATER, SEE DET. 4/- FOR NAILING PATTERN.	
FIELD NAILING	ALTERNATIVELY, USE DOUBLE 2X STUDS NAILED TOGETHER WITH 16d NAILS SPACED TO MATCH SHEAR WALL E.N. SPACING, SEE NOTE 2.	
AT 12" O.C. EDGE NAIL AT PANEL EDGES	7. FOLLOW MANUFACTURER'S INSTALLATION REQUIREMENTS.	
SEE NOTE 6 DET. 1/ TYP. STUDS	 A35, LTP4, OR RBC MAY BE USED FOR SHEAR CLIPS AS APPLICABLE PER DET. 4/- SCHEDULE. INSTALL LTP4 IN HORIZONTAL ORIENTATION, AND WITH 0.131"x2 1/2" (8d COMMON) NAILS. 0.148"x2 1/2" (10d) NAILS 	
PLYWOOD FACE GRAIN PARALLEL TO STUD	SHALL BE USED WITH RBC SHEAR CLIPS. 9. SOLE PLATE NAILING AND SCREWS AT FLOOR SHALL BE SPACED PER	
EDGE NAIL TO SOLE	SCHEDULE FOR SHEAR WALL BELOW AS SHOWN ON DET. 4A/	
	 FOR ANCHOR BOLT AND MUD SILL DETAIL, SEE DET. 8/ 2X MUD SILL MAY BE USED AT 6" NAILED SHEAR WALLS WITH 5/8" DIA. 	
SEE SCHEDULE DET. 4/	ANCHOR BOLTS AT 3'-4" O.C. MINIMUM.	
	S801-01	
N.T.S.	1 SHEAR WALL SCHEDULE NOTES	
RUCTURAL PANEL; .ACE FACE GRAIN RPENDICULAR TO	NOTES: 1. HOLDOWNS LOCATIONS NOT COMPLETE FOR CLARITY.	
AF LINDICULAR TO RAMING MEMBERS; TAGGER JOINTS.	2. FOR INFORMATION NOT NOTED, SEE DET. 5/	
	SHEAR WALL LENGTH AS DESIGNATED PER PLAN	
	PROVIDE PLYMOOD WITH SAME NAILING AS CMST14 STRAP EXTENDING	
	ADJACENT WALLS ABOVE AND BELOW OPENING UNDER CONTRACT OF SHEAR WALL U.O.N. NAIL EVERY OTHER HOLE	
.OCKING AT PANEL EDGES IR DET. 7/- THIS SHEET REQUIRED (SEE SCHEDULE	$\frac{1}{ \nabla } = \frac{1}{ \nabla } = $	
LOW AND FRAMING PLANS).	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
LD NAILING 12" O.C.	$\begin{array}{c ccccccccccccccccccccccccccccccccccc$	
TED, NAILING SHALL BE		
E.N. B.N. (5) 6" O.C. 6" O.C. YES		
6" O.C. 6" O.C. YES		
JAL STRENGTH CAPACITY EW TYPE NAILS (3x0.135)		
	U HOLDOWN PER PLAN	
AL PANEL	2 STRAPPED SHEAR WALL ELEVATION	
N.T.S.	N.T.S. SHEAR WALL	
	BEARING POST AT OPENING ONE SIDE OR BOTH OR BOTH OR BOTH	
TYP.	BEARING POST AT OPENING H H H H H H H H H H H H H	
TYP.	BEARING POST AT OPENING H H H H H H H H H H H H H	
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ΤΥΡ. N	BEARING POST AT OPENING UNE SIDE OR BOTH UNE SIDE OR BOTH UNE SIDE OR BOTH UNE SIDE OR UNE SIDE ONE SIDE	
	BEARING POST AT OPENING INC SIDE OR BOTH INC OF SYM. INC OF SYM.	
	BEARING POST AT OPENING AT OPENING NE SIDE OR BOTH SHEAR WALL EA, SIDE OR BOTH A END OR OPENING BOTH I6d SPACE SDS SCREM SPACING SAME AS SULE PLATE PER SCHEDULE 4/- SAME AS E.N. I6d SPACE SAME AS E.N. I6	
N:	BEARING POST AT OPENING BOTH	
	BEARING POST AT OPENING AT OPENING BOTH	
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N:	BEARING POST AT OPENING AT OPENING BOTH COTE SIDE OR BOTH COTE SIDE ONE SIDE OR BOTH COTE SIDE COTE STAGE SAME AS EN. IGd SPACE SAME AS EN. STAGERED SIDE OR BOTH SIDE OR BOTH	
N:	BEARING POST AT OPENING AT OPENING BOTH BOTH COPENING SHEAR WALL SHEAR WALL SHEAR WALL COPENING	
N:	BEARING POST AT OPENING BOTH	
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ν. . NOT SHOWN, 	BEARING POST AT OPENING BOTH	
N: NOT SHOWN, S801-07 N.T.S. MAX. IDGE OF PLYWOOD AT NR WALL, TYP. - USE BPS-6 PLATE	BEARING POST AT OPENING BOTH	
N: NOT SHOWN, S801–07 N.T.S. MAX. DGE OF PLYWOOD AT AR WALL, TYP. USE BPS-6 PLATE AT 2X6 WALLS 2 X6 STUD WALL	BLARNS POST AT OFENNS BOTH	
N: NOT SHOWN, S801–07 N.T.S. MAX. IDGE OF PLYWOOD AT NR WALL, TYP. USE BPS-6 PLATE AT 2X6 WALLS DYG 6 FILM WALLS DYG 6 FILM WALLS	BEAR NS POST AT OPENNS BOTH BOTH BOTH BOTH BOTH BOTH BOTH BOTH	
N: NOT SHOWN, S801–07 N.T.S. MAX. DGE OF PLYWOOD AT N.T.S. MAX. DGE DF PLYWOOD AT N.T.S. MAX. COS STUD WALL OR OR OR	BEARNS FOST ONE BUE OR ONE BUE OR SCHEDULE AT ENTRON FOST ONE BUE OR ONE BUE SCHEDULE BUTH Ido SPACE Ido SPACE Ido SPACE SHEAR NALL SHEAR NALL ONE SAME AS BUTH SAME AS EN SHEAR NALL ONE SOLE PLATE SAME AS EN SAME AS EN SHEAR NALL ONE SHEAR NALL ONE SAME AS EN SAME AS EN SHEAR NALL ONE SHEAR NALL ONE SAME AS EN SAME AS EN NISTAL PLY SHEAR NALL ONE SAME AS EN SAME AS EN NISTAL PLY SHEAR NALL ONE SHEAR NALL ONE SAME AS EN NISTAL PLY SHEAR NALL ONE SHEAR NALL ONE SAME AS ENDER NISTAL PLY SHEAR NALL ONE SHEAR NALL ONE SAME AS ENDER NISTAL PLY SHEAR NALL ONE SHEAR NALL ONE SAME AS ENDER NISTAL PLY SHEAR NALL ONE SHEAR NALL ONE SHEAR NALL ONE SHEAR NALL ONE	
N: NOT SHOWN, S801-07 N.T.S. MAX. DGE OF PLYWOOD AT NR WALL, TYP. - USE BPS-6 PLATE AT 2x6 STUD WALL OR STAGGER BOLTS - 2x6 STUD WALL OR STAGGER BOLTS - SLOTTED PLATE WASHER,	BEAR PROT TO CENNO BOTH BOT	
N: NOT SHOWN, S801-07 N.T.S. MAX. DGE OF PLYNOOD AT AR WALL, TYP. USE BPS-6 PLATE AT 2X6 WALLS STAGGER BOLTS SLOTTED PLATE WASHER, AS REQD. AT SHEAR WALLS ATED D.F. PROVIDE SHALL BE D ROD W/ NUT	BEAK IN PLYWOOD SHEAT HING SIDE AT EXTERIOR WALL NOTE: 1. ALL PLY BEAK TRANSFER AND/ OR THE DOWN LOCATION SHEAR TRANSFER AND/ OR THE DOWN LOCATION SIDE SHEAR TRANSFER AND/ OR SIDE SHEA	
N: NOT SHOWN, NOT SHOWN, S801-07 NT.5. MAX. DGE OF PLYMOOD AT NR WALL, TYP. USE BPS-6 PLATE AT 2x6 WALLS STAGGER BOLTS STAGGER BOLTS STAGGER BOLTS STAGGER BOLTS SLOTTED PLATE WASHER, AS REQD. AT SHEAR WALLS ATED D.F. PROVIDE SHALL BE	BEAR MALL ONE SOLE PLATERNON BOTH	
N: NOT SHOWN, S801-07 N.T.S. MAX. DGE OF PLYWOOD AT AR WALL, TYP. USE BPS-6 PLATE AT 2x6 WALLS STAGGER BOLTS STAGGER STAG STAGGER STAG STAGGER STAG STAGGER STAG STAG STAGGER STAG STA	EE-ANE FOST ONE SOE OR ONE SOE SCHEDULE AT OPENNO END OR OPENING Iso BPACE END OR OPENING Iso STAGGERED SHEAR MULL SOE SOE BOTH Iso STAGGERED STAGGERED SHEAR MULL SOE SOE BOTH Iso SPACE STAGGERED SHEAR MULL SOE SOE BOTH Iso SPACE STUD WALL SHEAR ALL ONE SOE OR BOTH SOE OR BOTH SOE OR BOTH SHEAR ALL ONE SOE OR BOTH SOE OR BOTH SOE OR BOTH NOTES Iso SPACE SOE OR BOTH SOE OR BOTH NOTES PARTY WALL END D BREAK IN PLYWOOD SHEATHING NOTES Iso SPACE AND/ OR TE EDOWN NP. SOE OR BOTH 1. ALL SHEATHING VALUES CHOW IS I. POR TEDOWN NP. SOE SHEATHING I. POR TEDOWN NP. SOE SHEATHING 1. ALL SHEATHING VALUES CHOW IS I. POR TEDOWN NP. SOE SHEATHING I. POR TEDOWN NP. SOE SHEATHING SHEAR TRANSFER AND/ OR SOE OF ROW IS I. POR TEDOWN NP. SOE SHEATHING SHEAR TRANSFER AT LONG SOE OF ROW IS I. STAGGERED SHEAR TRANSFER AT LONG SOE OF ROW IS I. POR TEDOWN NP. SEE SHEAT SHEAR CLEP OF TRANSFERATION OR SHEAR TRANSFERAT TRANSFERE AT LONG	
N: NOT SHOWN, S801–07 N.T.S. MAX EDGE OF PLYNOOD AT AR WALL, TYP. USE BPS-6 PLATE AT 2x6 WALLS STAGGER BOLTS SLOTTED PLATE WASHER, AS REQD. AT SHEAR WALLS STAGGER BOLTS SLOTTED PLATE WASHER, AS REQD. AT SHEAR WALLS ATED D.F. PROVIDE SHALL BE D ROD W/ NUT WASHERS (AT IN. (SEE PLANS MISSING OR IS512HMG. TENSIONED INICK, CUT, OR LS PROVIDE	BEAR PROT ONE SOL OR ONE SOL OR BOTLOW ONE SOL OR BOTLOW SCHEDULE AL OPENNING BEAR PALL END OR OPENING END OR OPENING END OR OPENING END OR OPENING BEER PALL ONE SOL OF SYM END OR OPENING END OR OPENING END OR OPENING END OR OPENING Idd SPACE END OR OPENING Idd SPACE END OR OPENING Idd SPACE END OR OPENING SOLE OR DOT END OR OPENING END OR OPENING Idd SPACE END OR OPENING SOLE OR DOT END OR OPENING END OR OPENING Idd SPACE END OR OPENING END OR OPENING END OR OPENING END OR OPENING Idd SPACE PARTY WALL END END OR OPENING END OR OPENING END OPENING Idd SPACE PARTY WALL END END OPENING END OPENING END OPENING Idd SPACE PARTY WALL END END OPENING END OPENING END OPENING Idd SPACE PARTY WALL END END OPENING END OPENING END OPENING	
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N: NOT SHOWN, S801–07 N.T.S. MAX. DGE OF PLYNOOD AT R WALL, TYP. UGE BPS-6 PLATE AT 2X6 WALLS - 2X6 STUD WALLS TAGGER BOLTS - SLOTTED PLATE WASHER, AS READ. AT SHEAR WALLS ATED D.F. PROVIDE SHALL BE D ROD W/ NUT WASHERS (AT 2N. (SEE PLANS MISSING OR ISB12HMG, TENSIONED NICK, CUT, OR LS PROVIDE CUT WASHER LOCATE BOLTS 'MAXIMUM A	PERF RAL ONE SIDE OR SCHEDU S SOTO OR SCHEDU S SCHEDU S SOTO OR OPENING SCHEDU S </th <th></th>	
N: NOT SHOWN, S801–07 N.T.S. MAX. DGE OF PLYWOOD AT K NALL, TYP. USE BPS-6 PLATE AT 2x6 WALLS STAGGER BOLTS STAGGER BOLTS STAGGER BOLTS STAGGER BOLTS STAGGER BOLTS SHALL BE D ROD W/ NUT WASHERS (AT N. (SE PLANS MISSING OR ISB12HMG. TENSIONED NICK, CUT, OR LS PROVIDE SHALL BE D ROD W/ NUT WASHERS (AT N. (SE PLANS MISSING OR ISB12HMG. TENSIONED NICK, CUT, OR LS PROVIDE SCUT WASHER LOCATE BOLTS	PERMENT ONE STORE ONE STORE SCHEDULE PERMENT ONE STORE ONE STORE STAGGERED PERMENT ONE STORE STAGGERED PERMENT SCHEDULE STAGGERED SCHEDULE SCHEDULE SCHEDULE PERMENT SCHEDULE SCHEDULE SCHEDULE SCHEDULE SCHED	
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N: NOT SHOWN, S801–07 N.T.S. MAX. DGE OF PLYWOOD AT AR WALL, TYP. USE BPS-6 PLATE AT 2x6 WALLS - 2x6 STUD WALL OR STAGGER BOLTS - SLOTTED PLATE WASHER, AS READ. AT SHEAR WALLS ATED D.F. PROVIDE SHALL BE D ROD W. NUT WASHERS (AT SHALL BE D ROD W. NUT SHALL BE D ROD W. NUT SHALL BE D	PREAR MALL ONE SIZE OR ONE SIZE OR SOURCE SOURCE <td< th=""><th></th></td<>	
N: NOT SHOWN, S801–07 N.T.S. MAX. DAX	PARTY WALL END PARTY WA	

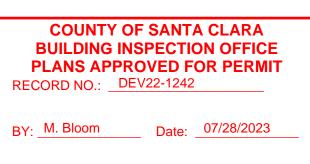


- **BKF-SAN JOSE** 1730 N. FIRST ST. STE 600 SAN JOSE, CA 95112
- LANDSCAPE ARCHITECT PLURAL STUDIO 2742 17TH STREET SAN FRANCISCO, CA 94110
- STRUCTURAL ENGINEER HOHBACH-LEWIN INC 250 SHERIDAN AVE STE 100 PALO ALTO, CA 94306
- MEP ENGINEER EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- SUSTAINABILITY/ENERGY **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521
- JOINT TRENCH/DRY UTILITY MILLENIUM DESIGN PO BOX 737 ALAMO, CA, 94507



→ HOHBACH-LEWIN, INC. STRUCTURAL & CIVIL ENGINEERS 260 Sheridan Avenue, Suite 150 Palo Alto, CA 94306 (650) 617-5930





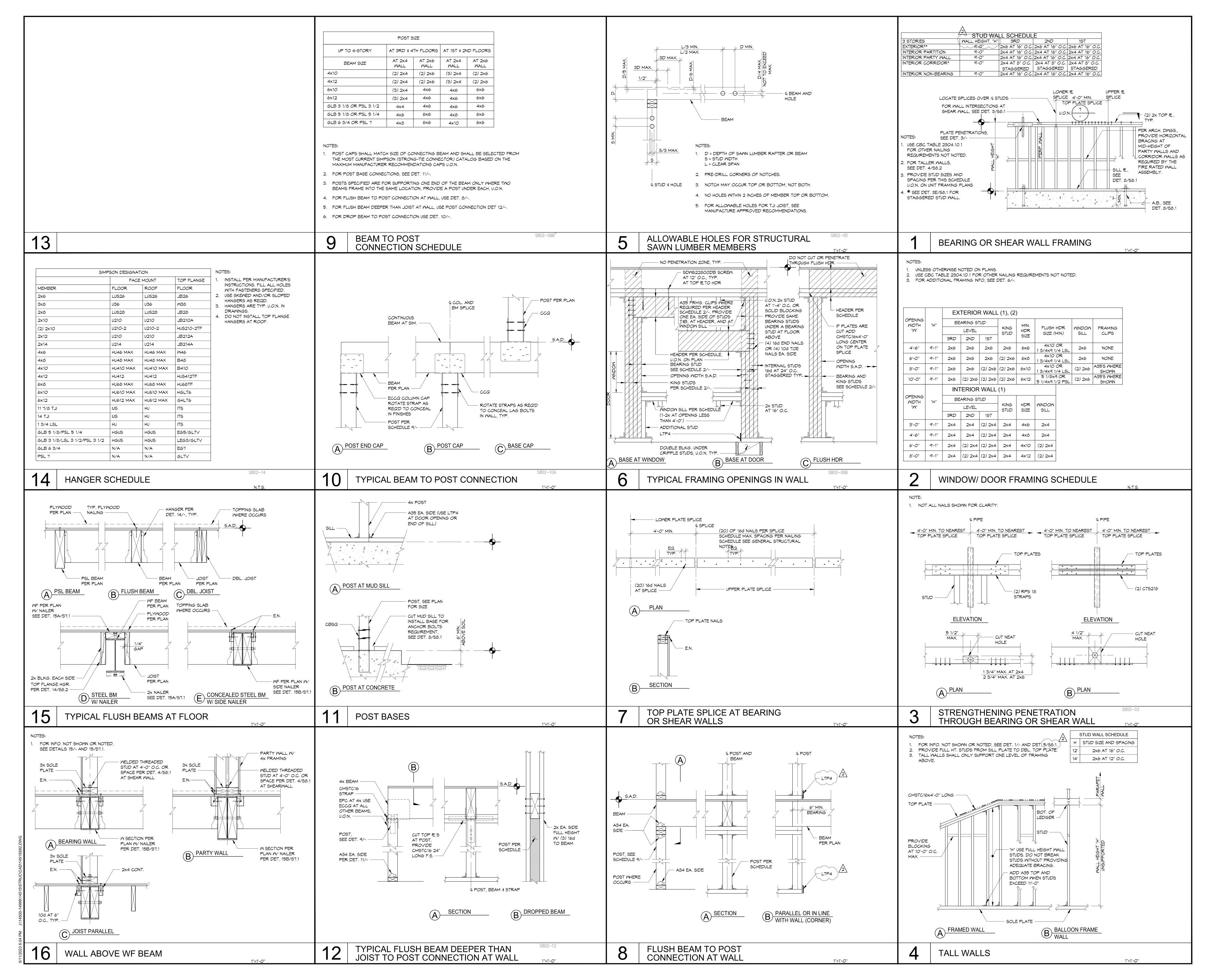
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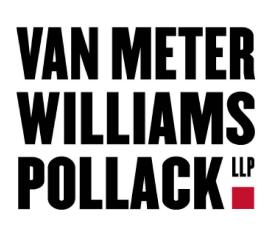
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В	03/20/2023	BID ADDENDUM
2	03/20/2023	PLAN CHECK RESPONSE 2
3	05/12/2023	PLAN CHECK RESPONSE 3

Project:



	31 GRANT AVENUE LO ALTO, CA 94306	
Client:		
	mercy HOUSING	
	Communities IERCY HOUSING/ ODE COMMUNITIES	
	PICAL WOOD AMING DETAILS	
JOB #: 19 SCALE: As	925 s indicated	/IN # 1451
	S8.1	DHBACH-LEWIN # 14515



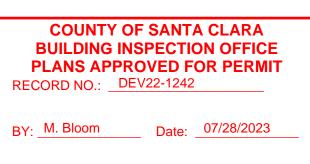


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STRUCTURAL & CIVIL ENGINEERS 260 Sheridan Avenue, Suite 150 Palo Alto, CA 94306 650) 617-5930



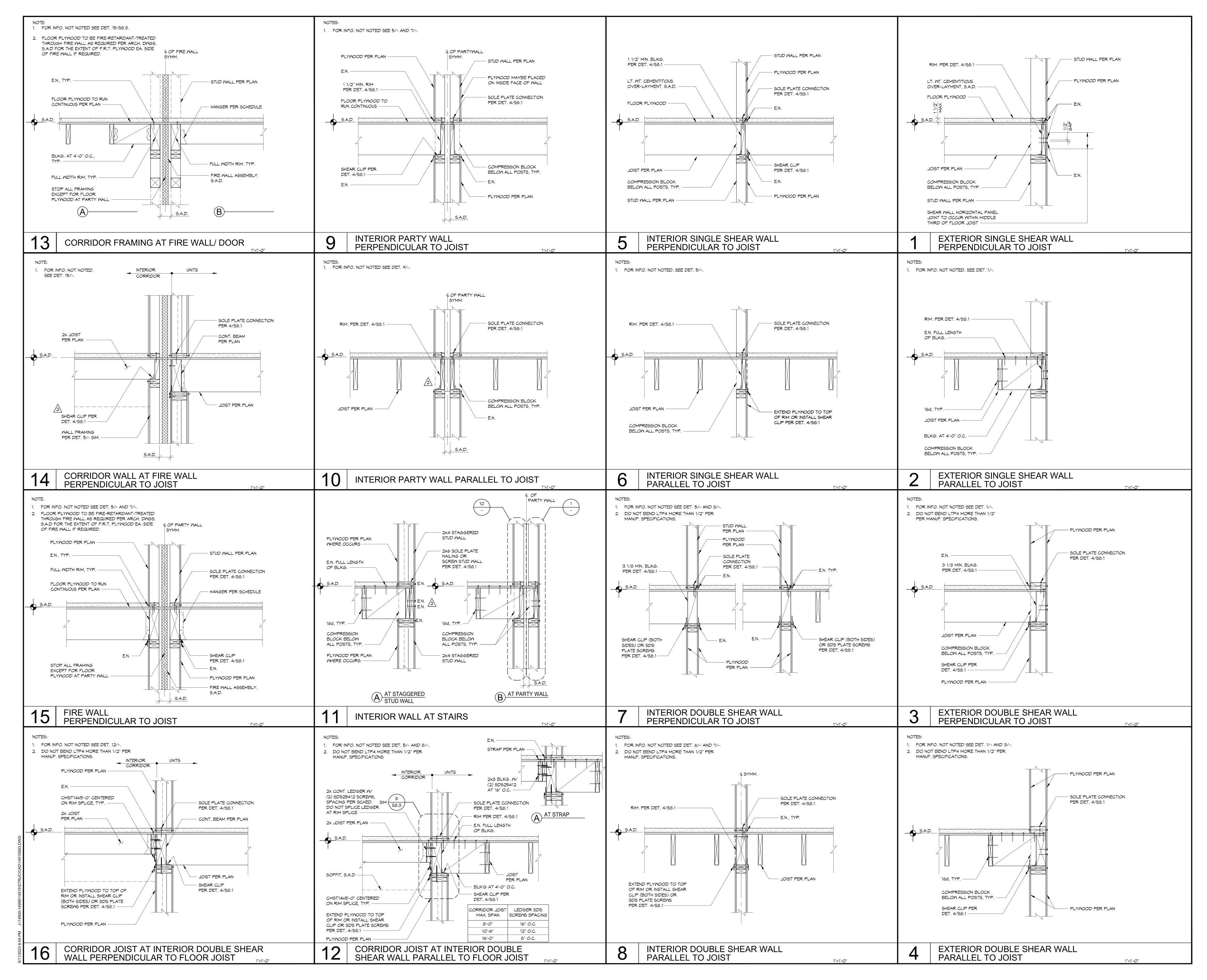


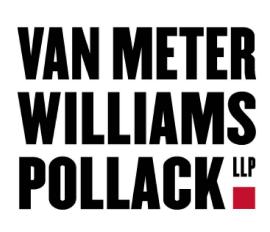
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231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	
mercy HOUSING	
abode	
MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL WOOD FRAMING DETAILS	5
JOB #: 1925 SCALE: As indicated	 VIN # 1451
S8.2	 DHBACH-LEWIN # 14515





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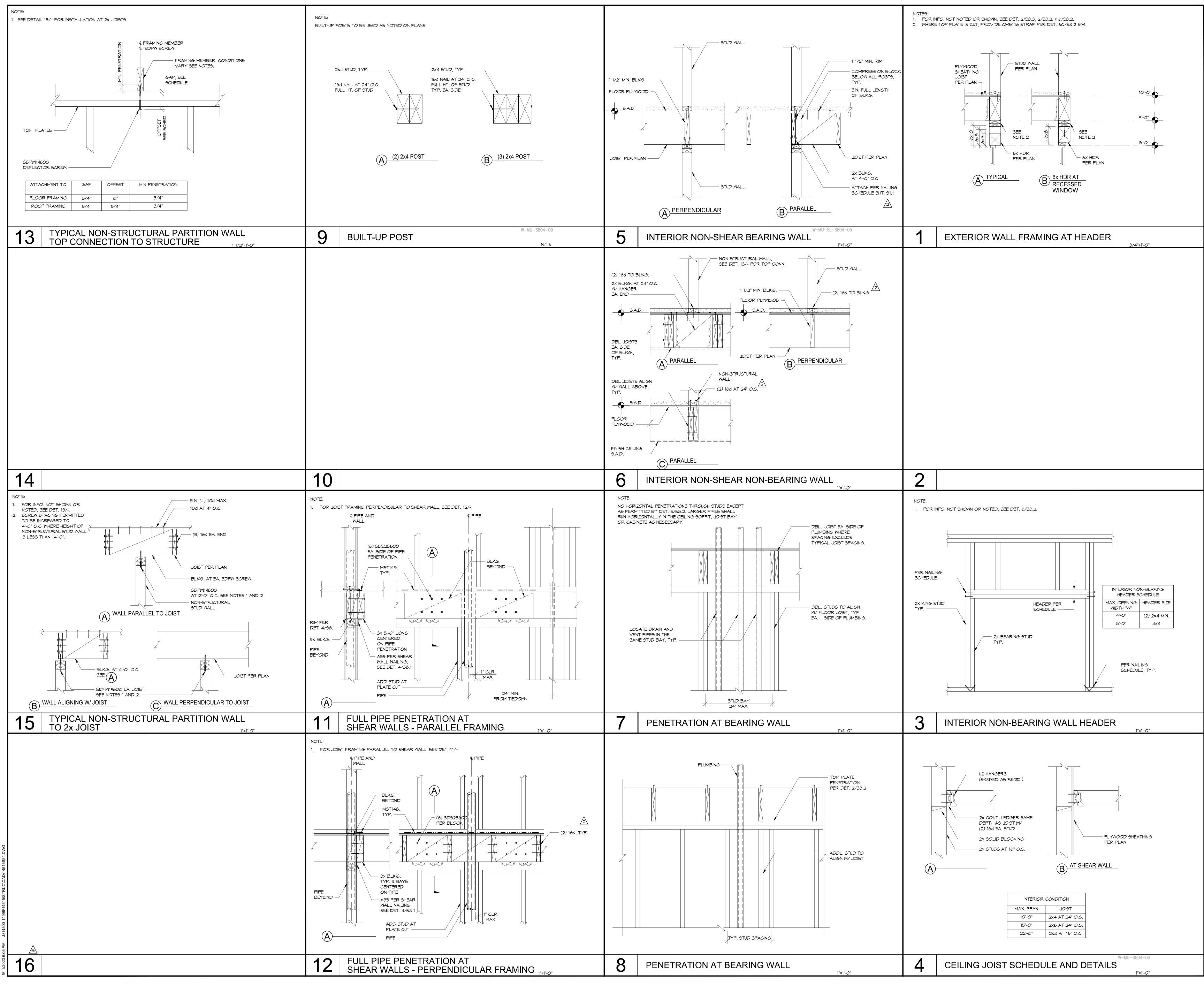
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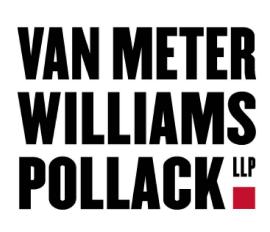
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Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	
HOUSING	
abode	
MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL WOOD FRAMING DETAILS	
	 4515
JOB #: 1925 SCALE: As indicated	
S8.3	HBACH-LEWIN # 14515





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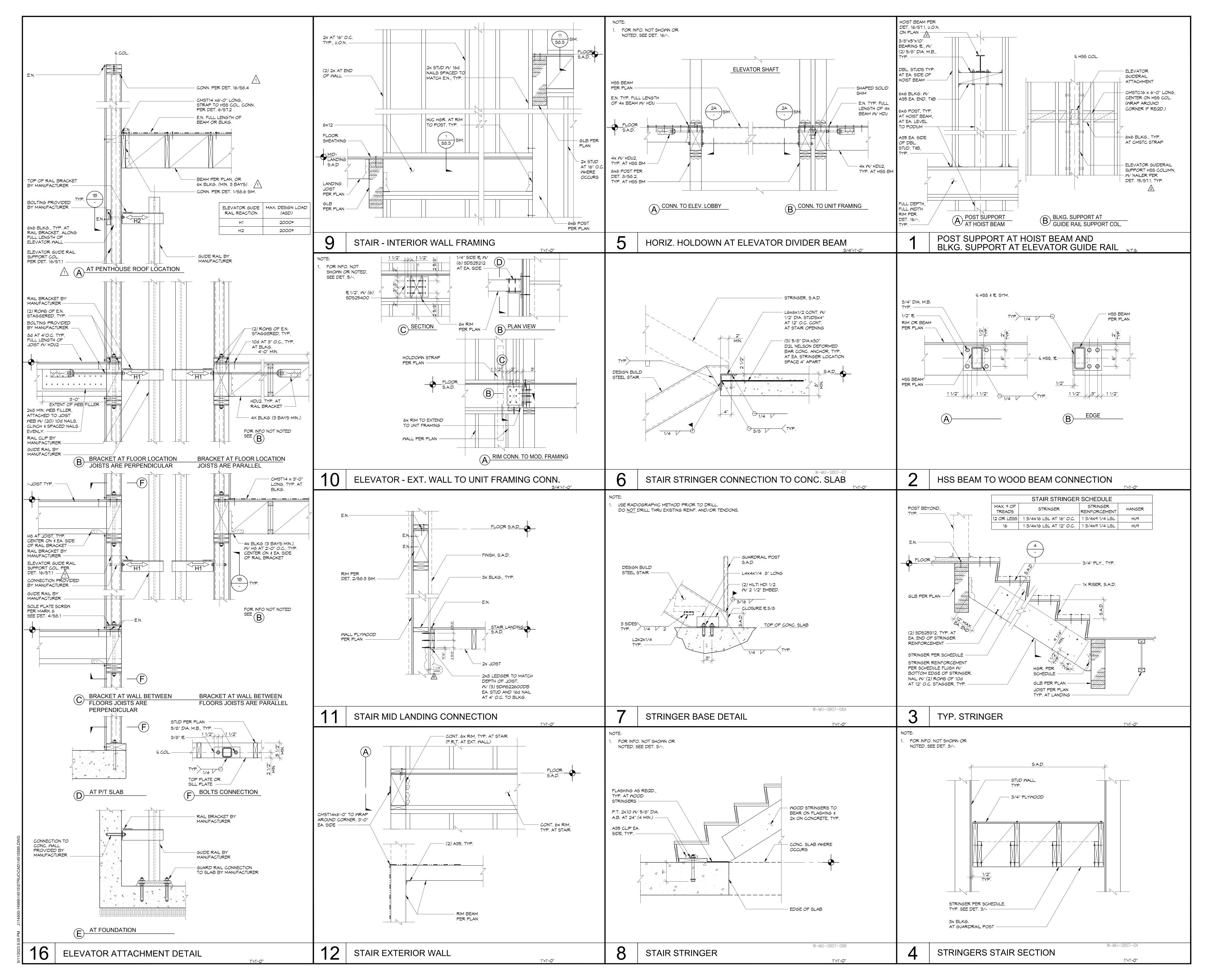
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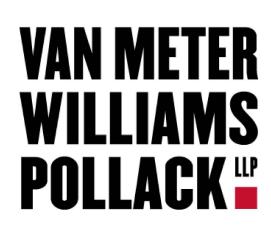
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Project:







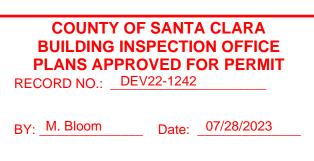


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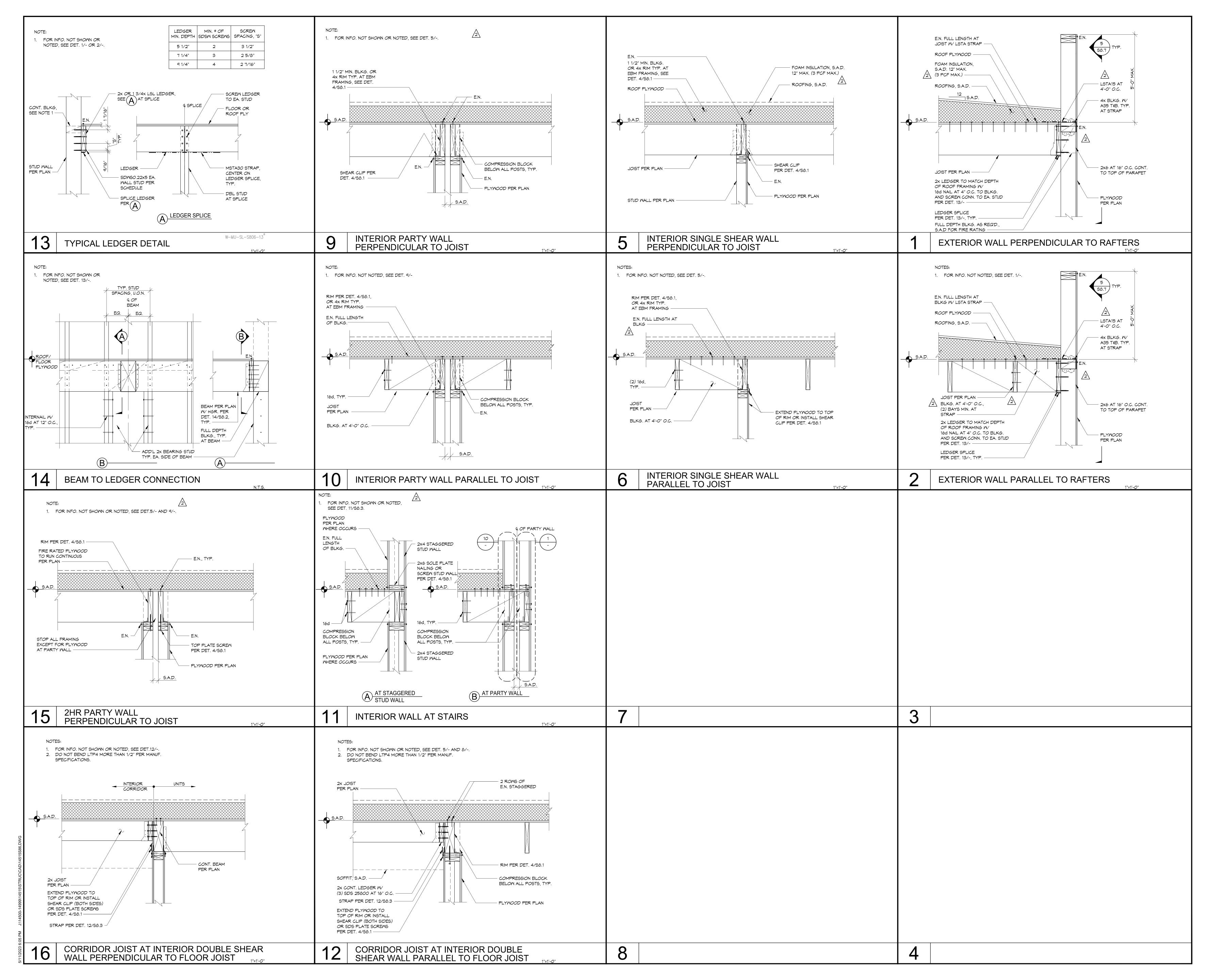
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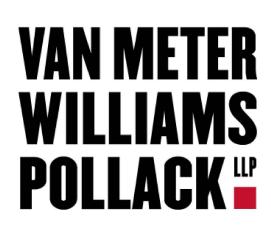
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Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	_
HOUSING	
abode communities	
MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL WOOD	
FRAMING DETAILS	15
JOB #: 1925	 # 145
SCALE: As indicated	- MIN
S8.5	I I I I HBACH-LEWIN # 14515



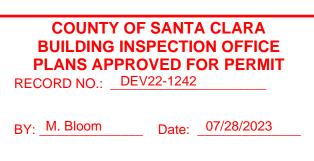


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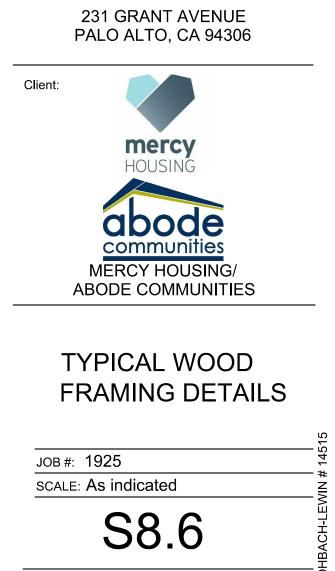


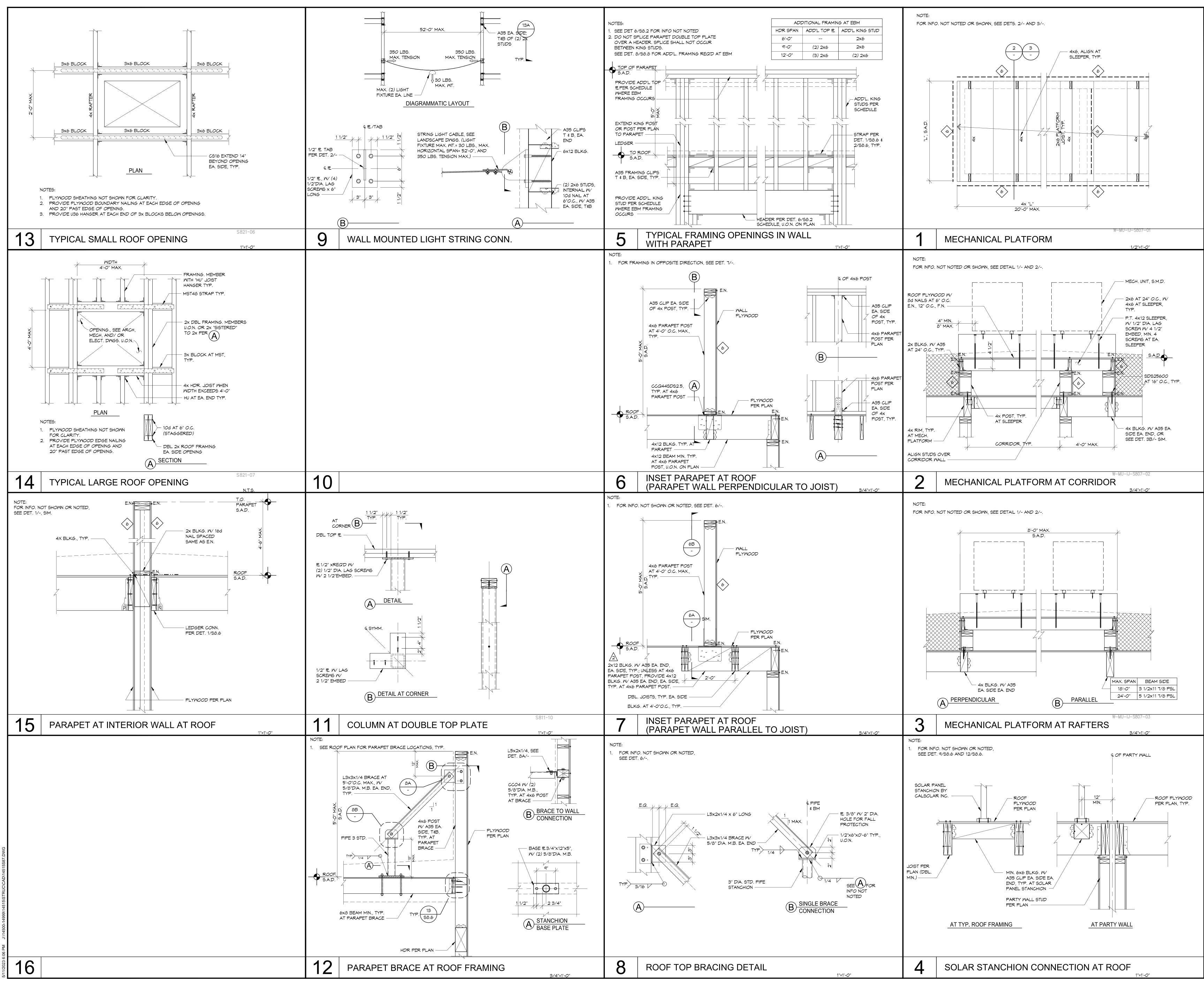
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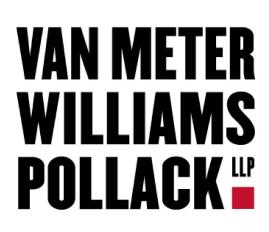
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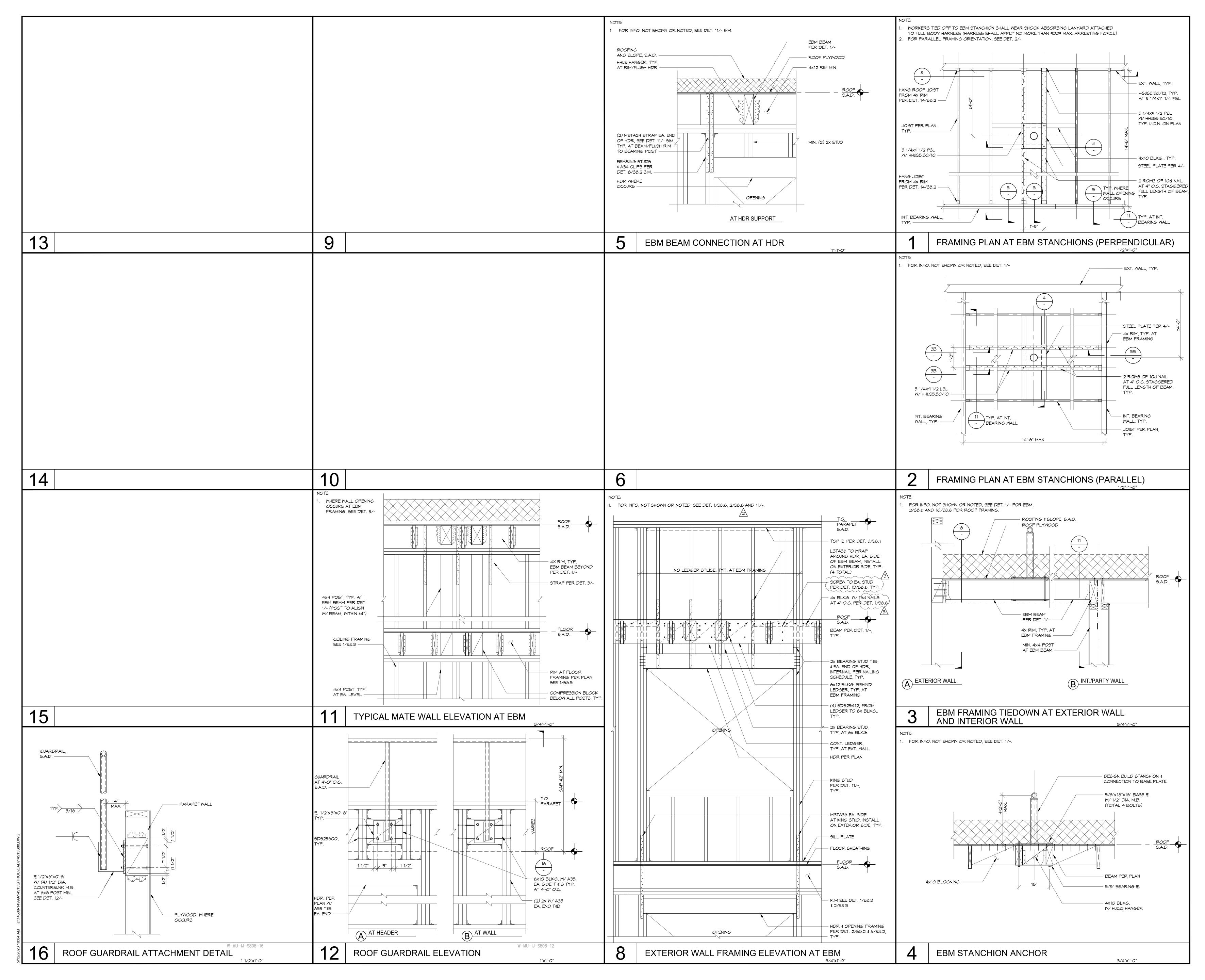
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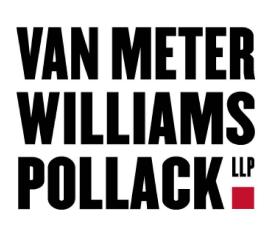
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Project:



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Client:	
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MERCY	nunities HOUSING/
ABODE C	COMMUNITIES
	AL WOOD NG DETAILS
JOB #: 1925 SCALE: As indic	ated z
	ated



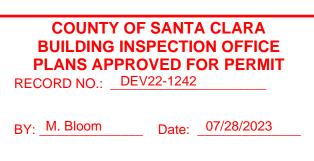


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Project:



	1 GRANT AVENUE LO ALTO, CA 94306	
Client:		
	mercy HOUSING	
	abode	
	COMMUNITIES	
	PICAL WOOD	
	AMING DETAILS	•
JOB #: 19 SCALE: As	925 s indicated	DHBACH-LEWIN # 14515
	S8.8	BACH-LEW
		—— F

	GENERAL N	OTES
1.	THE <u>SLACKJACK[™] DEVICE</u> COMPONENTS INCLUDE: A. SJA INNER AND OUTER SLEEVE CYLINDERS B. PRE-COMPRESSED COMPRESSION SPRING	18. <u>DRILLED OR BORED HOLES:</u> DR SHALL NOT EXCEED 25% OF TH IS MAXIMUM ALLOWED IN 3 1/2
_	 C. NUT: ALL NUTS TO CONFORM TO ASTM A563 GRADE "A" FOR 60 ksi AND GRADE "C" FOR 120 ksi TENSILE STRENGTH THREADED RODS. D. SWIVEL WASHER 	THE BORED HOLE SHALL BE A 19. <u>CUTTING AND NOTCHING:</u> WOOL CUT OR NOTCHED TO A DEPTH
2.	ALL THREADED RODS SHALL CONFORM TO THE FOLLOWING SHOP DRAWING MARKS: 1. $Rx = ASTM A36 \text{ OR } A307$, $fu = 60 \text{ ksi.}$ 2. $RxM = ASTM F1554 \text{ GRADE } 55$, $fu = 75 \text{ ksi.}$	IBC 2018. 20. STUD/POST/TRIMMER NOTCHING
	 3. RxHS = ASTM A449 (fu = 120 ksi UP 1". fu = 105 ksi 1-1/8" THRU 2-1/2") or ASTM A193B7 (fu = 125 ksi). NOTES: a. x = ROD DIAMETER IN EIGHTS OF AN INCH. (ie. R7M = 7/8" ASTM F1554 GRADE 55) 	A. THE MEMBER SHALL BE NO PLATE TO MAINTAIN THE FU B. IF THE REQUIREMENTS OF I COMPRESSION MEMBERS SH
3.	b. "M" ROD ARE ZINC PLATED FINISH. STANDARD AND HIGH STRENGTH (HS) RODS ARE PLAIN (UNCOATED) CARBON STEEL,	AREA. ADDITIONAL COMPRE PER THE REQUIREMENTS OF
	ALL BEARING PLATES ARE FABRICATED FROM ASTM A36 STEEL.	21. MINIMUM REQUIREMENTS FOR B A. BLOCKING SHALL SPAN THE
	ALL STEEL BEARING PLATES SHALL HAVE FULL BEARING CONTACT AREA ON WOOD MEMBERS. COUPLING NUT CONNECTION: CONTRACTOR SHALL BE RESPONSIBLE TO VERIFY	FLOOR LEVEL BELOW THE J B. BLOCKING SHALL EQUAL TH TOTAL HOLDOWN WOOD COM
	THAT AT CONNECTIONS WITH THE SAME SIZE ROD THAT EACH ROD IS THREADED HALF WAY INTO COUPLER FROM EACH SIDE. HIGH STRENGTH (HS) COUPLERS SHALL BE USED WITH HIGH STRENGTH (HS) RODS UNLESS NOTED OTHERWISE. STANDARD STRENGTH COUPLERS ARE ZINC PLATED. HIGH STRENGTH (HS) ARE NOT PLATED. WHERE REDUCING COUPLERS ARE USED, COUPLER SHALL BE FULLY SEATED ON THE LARGER DIAMETER ROD BEFORE ENGAGING THE SMALLER DIAMETER ROD. SIGHT HOLES ARE PROVIDED ON STRAIGHT COUPLERS ONLY, SIGHT HOLES ARE NOT REQUIRED ON REDUCING COUPLERS.	ABOVE THE JOIST SPACE. C. THE SPACE BETWEEN BLOCK D. COMPRESSION CAPACITIES O THAN THE RELATED JOISTS. NOTE "E" FOR EXCEPTION.
	COLORS REFERENCED ON THIS DRAWING ARE PROVIDED FOR CONVENIENCE TO THE INSTALLER AND FOR INSPECTION PURPOSES.	E. VERTICAL GRAIN BLOCKING SHRINKAGE OF THE JOISTS. INFORMATION.
	SILICONE OR CAULKING SHALL NOT BE PLACED INTO THE SLACKJACK DEVICE OR ANY DRILLED HOLE FOR RODS AT ANY TIME. CONTRACTOR SHALL USE FIRE RATED ROCKWOOL OR NON-HARDENING FIRESTOP. (PROVIDED BY OTHERS). ANCHOR BOLT ASSEMBLIES AND EMBEDMENTS DEPTHS ARE PROVIDED BY OTHERS.	F. FLOOR JOISTS SHALL NOT E OF BLOCKING. BLOCKING IS G. WEB STIFFENERS SHALL BE "I" JOIST MATERIAL USED A
	ANCHOR BOLTS ARE SHOWN FOR CONVENIENCE TO THE INSTALLER. HOLDOWN RUN ELEVATION VIEWS ARE DIAGRAMS AND MAY NOT DEPICT THE CORRECT NUMBER OF COLLECTOR STUDS REQUIRED. REFER TO COLLECTOR STUD	T COLOT MATERIAL COLO P
11	PLAN DIAGRAMS FOR REQUIRED AMOUNT OF ADDITIONAL STUDS DEPENDING ON SHEAR WALL FRAMING TYPE. ADDITIONAL COMPRESSION STUDS OR POSTS MAY BE REQUIRED BY THE STRUCTURAL PLANS FOR DEAD AND LIVE LOADS. THIS DRAWING WILL BE REVIEWED BY THE ENGINEER OF RECORD. UPON APPROVAL,	JOB SPECIFIC NOTES:
	U.N.O. THIS DRAWING REPLACES THE HOLDOWN SYSTEM PER THE STRUCTURAL PLANS. ENGINEER OF RECORD SHALL VERIFY COMPLETE LOAD TRANSFER TO FOUNDATION	
13.	LEVEL INCLUDING ALL CODE REQUIRED LOAD FACTORS. WOOD FLOOR LEVELS SHOWN ARE RELATIVE TO THE CONCRETE FOUNDATION AND	
14.	MAY NOT REFLECT THOSE SHOWN IN THE PLANS. CONTRACTOR MAY SUBSTITUTE POSTS WITH 2x MEMBERS OR VICE VERSA ONLY IF TOTAL BEARING PLATE AREA ON BOTTOM PLATE IS EQUAL.	
15.	REFER TO THE COLLECTOR STUD SCHEDULE FOR WOOD STUD AND FLOOR PLATE MATERIALS USED IN LOAD CALCULATION ASSUMPTIONS. THE MATERIALS SHALL	
16.	 MATCH STRUCTURAL DRAWING REQUIREMENTS UNLESS NOTED OTHERWISE. FULL HEIGHT COLLECTOR STUD REQUIREMENTS (NOT AT BRIDGE LEVEL – SEE 17A): A. FULL HEIGHT BUNDLED 2x, 3x COLLECTOR/COMPRESSION STUDS, THE OUTERMOST STUD FROM EACH SIDE OF THE THREADED ROD SHALL RECEIVE EDGE NAILING PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS. EDGE NAILING MAY BE STAGGERED/DISPERSED ON (2) MEMBERS MAX. ANY REMAINING INTERIOR STUDS SHALL RECEIVE FIELD NAILING PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS. 	ANS.
	B. FULL HEIGHT 4x COLLECTOR/COMPRESSION AT OUTERMOST POST FROM EACH SIDE OF THE THREADED ROD SHALL RECEIVE ONE (1) ROW OF EDGE NAILING PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS. ANY REMAINING 2x, 3x, 4x INTERIOR MEMBERS SHALL RECEIVE FIELD NAILING PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS.	AE
	C. FULL HEIGHT 6x AND LARGER COLLECTOR/COMPRESSION AT OUTERMOST POST FROM EACH SIDE OF THE THREADED ROD SHALL RECEIVE TWO (2) ROWS OF EDGE NAILING PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS. ANY REMAINING INTERIOR POSTS CAN BE FIELD NAILED PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS.	AB ANCHOR BOLT PER R SCHEDULE BLKG BLOCKING C COUPLER FND FOUNDATION G HOT DIPPED GALVANI
	 D. WINDOW AND DOOR TRIMMERS MAY BE USED AS HOLDOWN COLLECTOR STUDS IF THE FOLLOWING CONDITIONS ARE MET: A CONTINUOUS VERTICAL LOAD PATH SHALL EXIST FROM WALL TOP PLTS. TO WALL BOTTOM PLTS. ALL WOOD MEMBERS IN THE CONTINUOUS VERTICAL LOAD PATH SHALL BE NAILED ACCORDING TO THE EARTHBOUND SYSTEM REQUIREMENTS. THE SHEAR PANEL MUST EXTEND TO INCLUDE THE TRIMMERS USED. 	GG COUPLER GALV. TAPP GS COUPLER GALV. TAPP CB CORROSION BARRIER HS HIGH STRENGTH SJA 4XY SLACKJACK™ COMPO (X) = SLACKJACK DE 1=1 INCH (RED), 2=2
	 E. COMPRESSION STUDS/POSTS SHALL BE INSTALLED SYMMETRICALLY ABOUT ROD. WHERE QUANTITY OF STUDS IS GREATER THAN THE SPACE BETWEEN ROD OR BEARING PLATE AND THE END OF SHEAR WALL THE ADDITIONAL REQUIRED STUDS SHALL BE ADDED TO THE OPPOSITE SIDE OF ROD (ASYMMETRICALLY). F. STITCH NAILING OF BUNDLED OR BUILT UP STUD PACKS SHALL CONFORM TO 	(Y) = ROD SIZE $JST JOIST MIN. MINIMUM O.C. ON CENTER P PLATE$
	IBC TABLE 2304.10.1. COMPRESSION BRIDGE REQUIREMENTS:	PG PERPENDICULAR TO C R ROD U.N.O. UNLESS NOTED OTHE
17.	A. ALL FULL HEIGHT 2x AND 3x MEMBERS ON EITHER END OF THE BRIDGE SHALL RECEIVE	SHOP D
17.	EDGE NAILING PER THE SHEAR WALL SCHEDULE OF THE STRUCTURAL PLANS. B. ALL FULL HEIGHT 4x COLLECTOR/COMPRESSION POSTS ON EITHER SIDE OF THE BRIDGE SHALL RECEIVE ONE (1) ROW OF EDGE NAILING PER THE SHEAR WALL SCHEDULE OF	
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LED HOLES THROUGH VERTICAL COMPRESSION MEMBERS STUD OR POST WIDTH, FOR EXAMPLE: 7/8" DIA. HOLE WIDE STUD OR POST UNLESS APPROVED BY EOR. LEAST 5/8" AWAY FROM EDGE OF STUD.

VERTICAL COMPRESSION MEMBERS ARE PERMITTED TO BE NOT EXCEEDING 25% OF IT'S WIDTH PER 2308.5.9 PER

AT CONFLICTING BEARING PLATES INTERFERENCE. CHED TO THE EXACT THICKNESS AND AREA OF THE STEEL BEARING AREA OF THE NOTCHED COMPRESSION MEMBER. EM "A" ABOVE ARE NOT MET, THEN ADDITIONAL L BE ADDED TO ACHIEVE THE REQUIRED TOTAL BEARING SION MEMBERS SHALL RECEIVE SHEAR PANEL NAILING THE EARTHBOUND SHOP DRAWINGS. I PLATE MAY BE USED IN LIEU OF NOTCHING.

OCKING IN THE JOIST SPACE AT HOLDOWN LOCATIONS ARE: ULL WIDTH OF THE SHEAR WALL TOP PLATE OF THE ST SPACE.

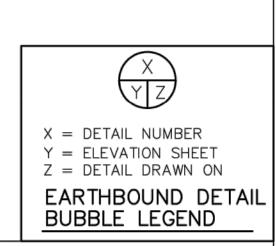
OUTERMOST DIMENSIONS OF THE "FOOTPRINT" OF THE PRESSION MEMBERS OF THE FLOOR LEVELS BELOW AND

ING WHERE THE ROD IS LOCATED SHALL NOT EXCEED 3". BLOCKING MATERIALS SHALL BE EQUAL TO OR GREATER THE VERTICAL DIMENSIONS SHALL BE EQUAL. SEE

SHALL BE TRIMMED TO ACCOUNT FOR POSSIBLE CONTACT JOIST MANUFACTURER FOR SHRINKAGE

REMOVED TO ALLOW FOR THE INSTALLATION IN ADDITION TO THE EXISTING JOISTS.

ADDED TO ENGINEERED LUMBER "I" JOISTS AND S BLOCKING. SEE NOTE "B" ABOVE.



BREVIATIONS

COMPONENT

ED ROD OR GALV. TAPPED COMPONENT D ON BOTH ENDS D ON SMALLER END INT VICE SERIES TRAVEL INCHES (GREEN)

AIN /ISF

RAWING DISCLAIMER

THE DETAILS OF THE EARTHBOUND SEISMIC HOLDOWN ONFORMANCE WITH THE STRUCTURAL DESIGN PROVIDED CORP.") BY THE PROJECT OWNER OR ITS REPRESENTATIVE. EPARATION OR REVIEW OF SAID STRUCTURAL DESIGN AND E . THE STAMP OR SEAL OF AN EB CORPORATION EMPLOYEE S PERTAIN ONLY TO THE TRANSFER OF THE FORCES ORD ON THE STRUCTURAL DRAWINGS AND NOT TO THE UCTURAL DESIGN.

IED, AS TO THE ADEQUACY OF THE STRUCTURAL DESIGN IS

<u>or stud diagram key</u>

COLLECTOR STUDS REQUIRED '4x WALLS: APPROX. ROD LOCATION (TYP) 4x8 4x6 6x WALLS:

ANCHOR BOLT LAYOUT DIMENSION (TYP) MM ★ MA ★(5)2x's TOTAL STUD OR POST (TYP) STUD/POST (QTY) & DESCRIPTION (TYP)

TES 1/2 HEIGHT TRIMMER (UNLESS NOTED OTHERWISE) BRIDGE.

HOWN ARE THE MINIMUM REQUIREMENT TO EQUAL FORCES CALLED OUT BY THE STRUCTURAL DWGS. SSION STUDS OR POSTS MAY BE REQUIRED BY ER OF RECORD. APPLIES TO ALL HOLDOWN RUNS.

	COMPONENT	SELECTION	SCHEDULES
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	EA	RTHBOUND THREADED	O ROD CAPACITIES (IBC	2018/CBC 2019)
ROD SIZE	ROD SIZE (INCHES)	ALLOWABLE TENSION LOAD IBC 2018/CBC 2019	ROD REMARKS	SLACKJACK SIZE
R4	1/2" DIA.	4,470 LBS	ASTM A 307 (UNC)	MJ100, MJ200
R5	5/8" DIA.	7,120 LBS	ASTM A 307 (UNC)	SJA 215, 225, 415, 425
R6	3/4" DIA.	10,540 LBS	ASTM A 307 (UNC)	SJA 216, 226, 416, 426, HJA 716, 726
R7	7/8" DIA.	14,540 LBS	ASTM A 307 (UNC)	SJA 217, 227, 417, 427, HJA 717, 727
R8	1" DIA.	19,080 LBS	ASTM A 307 (UNC)	SJA 218, 228, 418, 428, HJA 718, 728
R9	1 1/8" DIA.	24,040 LBS	ASTM A 307 (UNC)	SJA 219, 229, 419, 429, HJA 719, 729
R10	1 1/4" DIA.	30,530 LBS	ASTM A 307 (UNC)	SJA 4110, 4210, HJA 7110, 7210, HJS 4110, 4
R12	1 1/2" DIA.	44,270 LBS	ASTM A 307 (UNC)	SJST 4112, 4212, HJA 7112, 7212, HJS 4112, 4
R14	1 3/4" DIA.	59,830 LBS	ASTM A 307 (UNC)	
R6M	3/4" DIA.	13,170 LBS	ASTM F1554 GR55 (UNC)	SJA 216, 226, 416, 426, HJA 716, 726
R7M	7/8" DIA.	18,180 LBS	ASTM F1554 GR55 (UNC)	SJA 217, 227, 417, 427, HJA 717, 727
R8M	1" DIA.	23,850 LBS	ASTM F1554 GR55 (UNC)	SJA 218, 228, 418, 428, HJA 718, 728
R9M	1 1/8" DIA.	30,050 LBS	ASTM F1554 GR55 (UNC)	SJA 219, 229, 419, 429, HJA 719, 729
R10M	1 1/4" DIA.	38,160 LBS	ASTM F1554 GR55 (UNC)	SJA 4110, 4210, HJA 7110, 7210, HJS 4110, 4
R12M	1 1/2" DIA.	55,330 LBS	ASTM F1554 GR55 (UNC)	SJST 4112, 4212, HJA 7112, 7212, HJS 4112, 4
R14M	1 3/4" DIA.	74,790 LBS	ASTM F1554 GR55 (UNC)	
R6HS	3/4" DIA.	21,950 LBS	ASTM A193 B7 (UNC)	SJA 216, 226, 416, 426, HJA 716, 726
R7HS	7/8" DIA.	30,300 LBS	ASTM A193 B7 (UNC)	SJA 217, 227, 417, 427, HJA 717, 727
R8HS	1" DIA.	39,750 LBS	ASTM A193 B7 (UNC)	SJA 218, 228, 418, 428, HJA 718, 728
R9HS	1 1/8" DIA.	50,090 LBS	ASTM A193 B7 (UNC)	SJA 219, 229, 419, 429, HJA 719, 729
R10HS	1 1/4" DIA.	63,600 LBS	ASTM A193 B7 (UNC)	SJA 4110, 4210, HJA 7110, 7210, HJS 4110, 4
R12HS	1 1/2" DIA.	92,220 LBS	ASTM A193 B7 (UNC)	SJST 4112, 4212, HJA 7112, 7212, HJS 4112, 4
R14HS	1 3/4" DIA.	124,650 LBS	ASTM A193 B7 (UNC)	

					<u> </u>			
	EARTHBO	UND BEAF	RING PLATE	CAPACITIES	(DF)	SJA ROD	SIZE	со
PLATE	DIFFERENTIAL	COLOR	PL	ATE DIMENSIONS (INCHI	ES)	PART	ROD	
SIZE	LOAD	CODE	WIDTH	LENGTH	THICK.	NO.	DIAMETER	
P6	6,650 LBS	GREEN	3"	3.5"	1/4"	MJ100 or MJ200	1/2"	
P8	8,470 LBS	BLACK	3-1/4"	4.25"	1/4"	SJA 2x5 or 4x5	5/8"	
P10	10,320 LBS	BLUE	3-1/4"	5"	3/8"	SJA 2x6 or 4x6, HJA 7x6	3/4"	R6
P12	12,230 LBS	GRAY	3-1/4"	6"	5/8"	SJA 2x7 or 4x7, HJA 7x7	7/8"	R7
P14	13,660 LBS	RED	3-1/4"	7"	3/4"	SJA 2x8 or 4x8, HJA 7x8	1"	R8
P18	18,840 LBS	YELLOW	3-1/2"	9"	1"	SJA 2x9 or 4x9, HJA 7x9	1 1/8"	RS
P20	21,020 LBS	BROWN	3-1/2"	10"	1"	SJA 4x10, HJA 7x10, HJS 4x10	1 1/4"	R10
P22	23,210 LBS	WHITE	3-1/2"	11"	1 1/4"	6JST 4x12, HJA 7x12, HJS 4x12	1 1/2"	R12
P24	24,310 LBS	GOLD	3-1/2"	11.5"	1 1/4"	"x" = SLACKJA	CK TRAVEL	IN "II
P26	26,490 LBS	ORANGE	3-1/2"	12.5"	1 1/2"	1		

TENSION LOAD WAS CALCULATED FROM THE FOLLOWING EQUATION: A. ASTM A 307 Threaded Rod Capacities are F'u = 60,000 psi. UNC thread pitch M Rods are based on ASTM F1554 GR55, F'u = 75 ksi.

HS Rods are based on ASTM A193 B7, Fu = 125 ksi B. The IBC 2018 column is calculated in accordance to Section 1613 and ASCE 7-16.

2. PLATE STEEL SHALL BE ASTM A36: F'u = 60,000 PSI.

S. SUBSTITUTIONS OF DESIGNATED BEARING PLATE SHALL NOT BE PERMITTED, OBTAIN WRITTEN APPROVAL FROM THE ENGINEER.

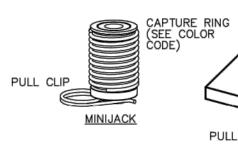
4. ALL BEARING PLATE CAPACITIES ARE BASED ON 1-1/16" HOLE, UNLESS NOTED OTHERWISE.

SLACKJACK SELECTION NOTES: THE SIZES SHOWN ABOVE ARE FOR BOTH ONE AND TWO INCH TRAVEL ("SJA 4xy") (x = TRAVEL HEIGHT IN INCHES, y = ROD SIZE). TWO INCH TRAVEL SLACKJACKS

ARE REQUIRED ON FOURTH WOOD FRAME LEVELS AND HIGHER. THIS TABLE IS FOR REFERENCE OF FULL PRODUCT LINE, SOME ROD AND PLATE SIZES MAY NOT BE IN USE.

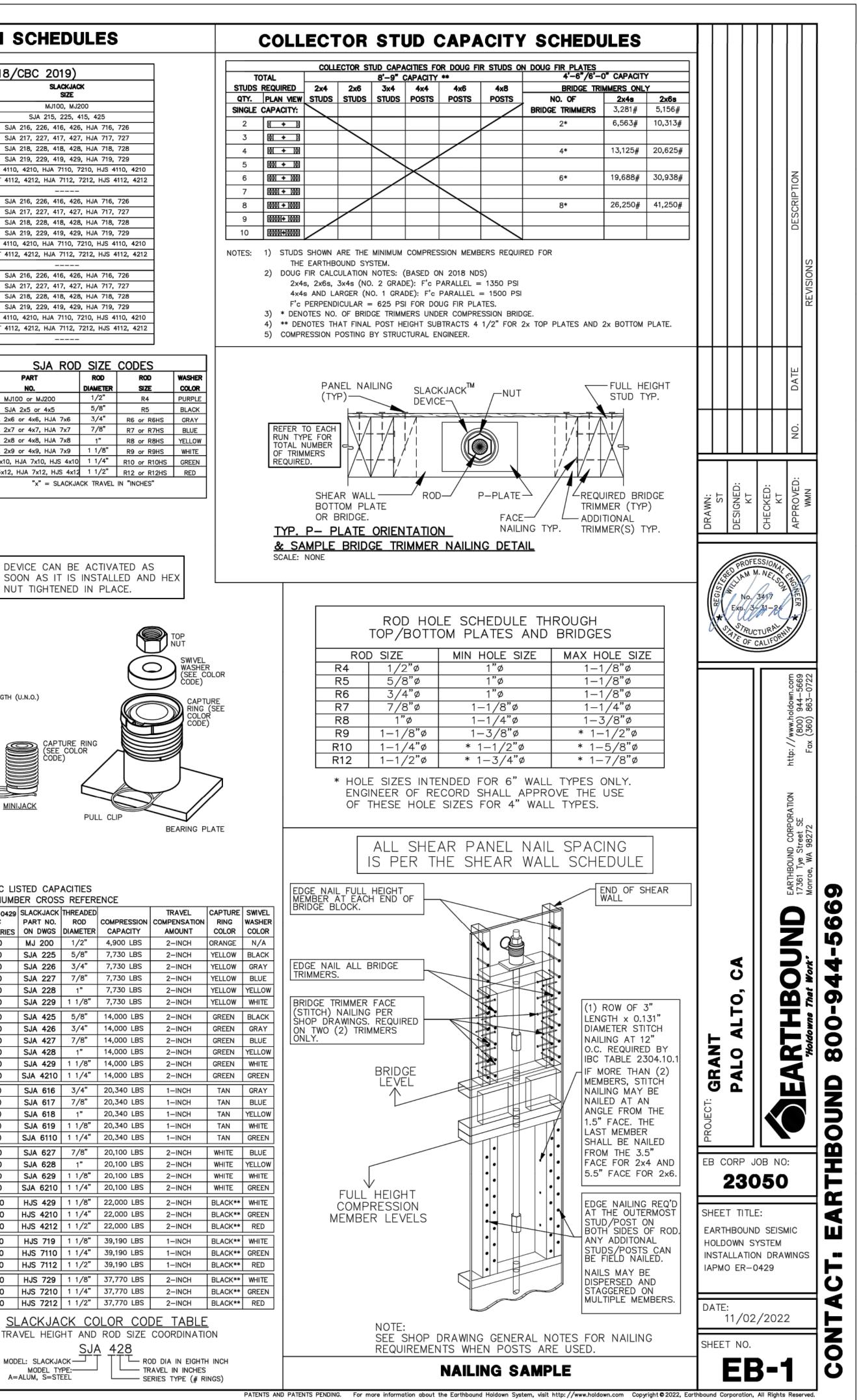
THE ENGINEER OF RECORD SHALL REVIEW AND APPROVE CAPACITIES. ALL A36/A307 THREADED ROD WILL BE 10' IN LENGTH AND ALL A193 B7 THREADED ROD WILL BE 12' IN LENGTH (U.N.O.)

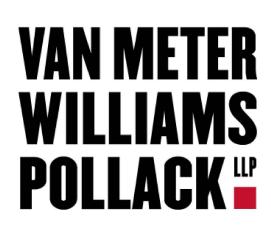
Г	TYPICAL T	
	<u>SLACKJACK[™] COLOR</u>	KEY
	P.	ART NO.
	SJA 418 RED YELLOW	
*	*CAPTURE RING	- WASHER COLOR
**	SECONDARY COLOR APPLIES TO HJS OUTER CYLINDER ONLY.	MODEL/SERIE



PULL CLIP

IAPMO ER-0429 /LABC	PART NO.	ROD	COMPRESSION	TRAVEL COMPENSATION	CAPTURE RING	WASHER		IAPMO ER-0429 /LABC	PART NO.	ROD	COMPRESSI
MODEL/SERIES	ON DWGS	DIAMETER	CAPACITY	AMOUNT	COLOR	COLOR		MODEL/SERIES	ON DWGS	DIAMETER	CAPACITY
M 100	MJ 100	1/2"	5,000 LBS	1-INCH	RED	N/A		M 200	MJ 200	1/2"	4,900 LB
A 210	SJA 215	5/8"	7,360 LBS	1-INCH	BLUE	BLACK		A 220	SJA 225	5/8"	7,730 LB
A 210	SJA 216	3/4"	7,360 LBS	1-INCH	BLUE	GRAY		A 220	SJA 226	3/4"	7,730 LB
A 210	SJA 217	7/8"	7,360 LBS	1-INCH	BLUE	BLUE		A 220	SJA 227	7/8"	7,730 LB
A 210	SJA 218	1"	7,360 LBS	1-INCH	BLUE	YELLOW		A 220	SJA 228	1"	7,730 LB
A 210	SJA 219	1 1/8"	7,360 LBS	1-INCH	BLUE	WHITE	L	A 220	SJA 229	1 1/8"	7,730 LB
A 410	SJA 415	5/8"	14,000 LBS	1-INCH	RED	BLACK		A 420	SJA 425	5/8"	14,000 LE
A 410	SJA 416	3/4"	14,000 LBS	1-INCH	RED	GRAY		A 420	SJA 426	3/4"	14,000 LE
A 410	SJA 417	7/8"	14,000 LBS	1-INCH	RED	BLUE		A 420	SJA 427	7/8"	14,000 LE
A 410	SJA 418	1"	14,000 LBS	1-INCH	RED	YELLOW		A 420	SJA 428	1"	14,000 LE
A 410	SJA 419	1 1/8"	14,000 LBS	1-INCH	RED	WHITE		A 420	SJA 429	1 1/8"	14,000 LE
A 410	SJA 4110	1 1/4"	14,000 LBS	1-INCH	RED	GREEN		A 420	SJA 4210	1 1/4"	14,000 LI
T 410	SJT 415	5/8"	9,000 LBS	1-INCH	ORANGE	BLACK	Γ	A 610	SJA 616	3/4"	20,340 L
T 410	SJT 416	3/4"	9,000 LBS	1-INCH	ORANGE	GRAY		A 610	SJA 617	7/8"	20,340 LI
T 410	SJT 417	7/8"	9,000 LBS	1-INCH	ORANGE	BLUE		A 610	SJA 618	1"	20,340 L
T 410	SJT 418	1"	9,000 LBS	1-INCH	ORANGE	YELLOW		A 610	SJA 619	1 1/8"	20,340 L
T 410	SJT 419	1 1/8"	9,000 LBS	1-INCH	ORANGE	WHITE		A 610	SJA 6110	1 1/4"	20,340 L
T 410	SJT 4110	1 1/4"	9,000 LBS	1-INCH	ORANGE	GREEN	Ī	A 620	SJA 627	7/8"	20,100 LI
HA 710	HJA 717	7/8"	15,650 LBS	1-INCH	PURPLE	BLUE		A 620	SJA 628	1"	20,100 LI
HA 710	HJA 718	1"	15,650 LBS	1-INCH	PURPLE	YELLOW		A 620	SJA 629	1 1/8"	20,100 LI
HA 710	HJA 719	1 1/8"	15,650 LBS	1-INCH	PURPLE	WHITE		A 620	SJA 6210	1 1/4"	20,100 LE
HA 710	HJA 7110	1 1/4"	15,650 LBS	1-INCH	PURPLE	GREEN	Ē	HS 420	HJS 429	1 1/8"	22,000 L
HA 710	HJA 7112	1 1/2"	15,650 LBS	1-INCH	PURPLE	RED		HS 420	HJS 4210	1 1/4"	22,000 L
HA 720	HJA 727	7/8"	15,590 LBS	2-INCH	GRAY	BLUE		HS 420	HJS 4212	1 1/2"	22,000 L
HA 720	HJA 728	1"	15,590 LBS	2-INCH	GRAY	YELLOW	Ē	HS 710	HJS 719	1 1/8"	39,190 LI
HA 720	HJA 729	1 1/8"	15,590 LBS	2-INCH	GRAY	WHITE		HS 710	HJS 7110	1 1/4"	39,190 L
HA 720	HJA 7210	1 1/4"	15,590 LBS	2-INCH	GRAY	GREEN	F	HS 710	HJS 7112	,	39,190 LI
HA 720	HJA 7212	1 1/2"	15,590 LBS	2-INCH	GRAY	RED	Ē	HS 720	HJS 729	1 1/8"	37,770 L
HS 410	HJS 419	1 1/8"	22,000 LBS	1-INCH	BLACK**	WHITE	F	HS 720	HJS 7210	1 1/4"	37,770 LI
HS 410	HJS 4110	1 1/4"	22,000 LBS	1-INCH	BLACK**	GREEN		HS 720	HJS 7212	1 1/2"	37,770 L
HS 410	HJS 4112	1 1/2"	22,000 LBS	1-INCH	BLACK**	RED		21	ACKJA		
	*SJST4210	1 1/4"	22,000 LBS	1 1/2-INCH	BLACK	GREEN			VEL HEIG		
	*SJST4110	1 1/4"	22,000 LBS	1-INCH	RED	GREEN			,		428
	*SJST4112	1 1/2"	22,000 LBS	1-INCH	RED	RED		MOD			₽ 7
* OUTSIDE CYL	INDER COL	[*SJST4112] 1 1/2" 22,000 LBS 1-INCH RED RED * SLACKJACK TAKE UP DEVICE WITH ΔR = 0.003 IN. NOT SHOWN IN ER-0429. MODEL: SLACKJACK MODEL: TYPE: * OUTSIDE CYLINDER COLOR AS FOLLOW: HJS41=BLUE, HJS42=YELLOW, HJS71=GREEN, HJS72=ORANGE MODEL: SLACKJACK MODEL: SLACKJACK									





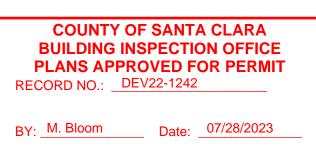
ARCHITECTURE | URBAN DESIGN
SAN FRANCISCO | DENVER | MINNEAPOLIS 333 Bryant Street, Suite 300, San Francisco, CA 94107 T 415.974.5352

- **BKF-SAN JOSE** 1730 N. FIRST ST. STE 600 SAN JOSE, CA 95112
- LANDSCAPE ARCHITECT PLURAL STUDIO 2742 17TH STREET SAN FRANCISCO, CA 94110
- STRUCTURAL ENGINEER HOHBACH-LEWIN INC 250 SHERIDAN AVE STE 100 PALO ALTO, CA 94306
- MEP ENGINEER EMERALD CITY ENGINEERS 21705 HIGHWAY 99 LYNWOOD, WA 98036
- SUSTAINABILITY/ENERGY **REDWOOD ENERGY** 1887 Q STREET ARCATA, CA 95521
- JOINT TRENCH/DRY UTILITY MILLENIUM DESIGN PO BOX 737 ALAMO, CA, 94507



→ HOHBACH-LEWIN, INC. STRUCTURAL & CIVIL ENGINEERS 260 Sheridan Avenue, Suite 150 Palo Alto, CA 94306 (650) 617-5930



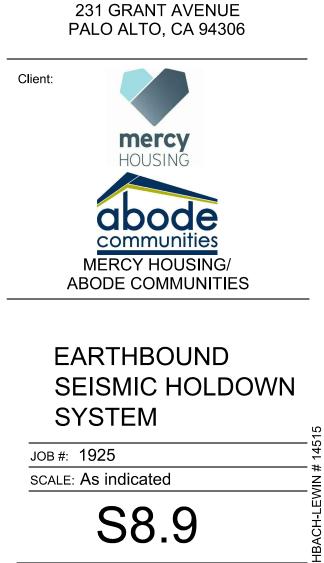


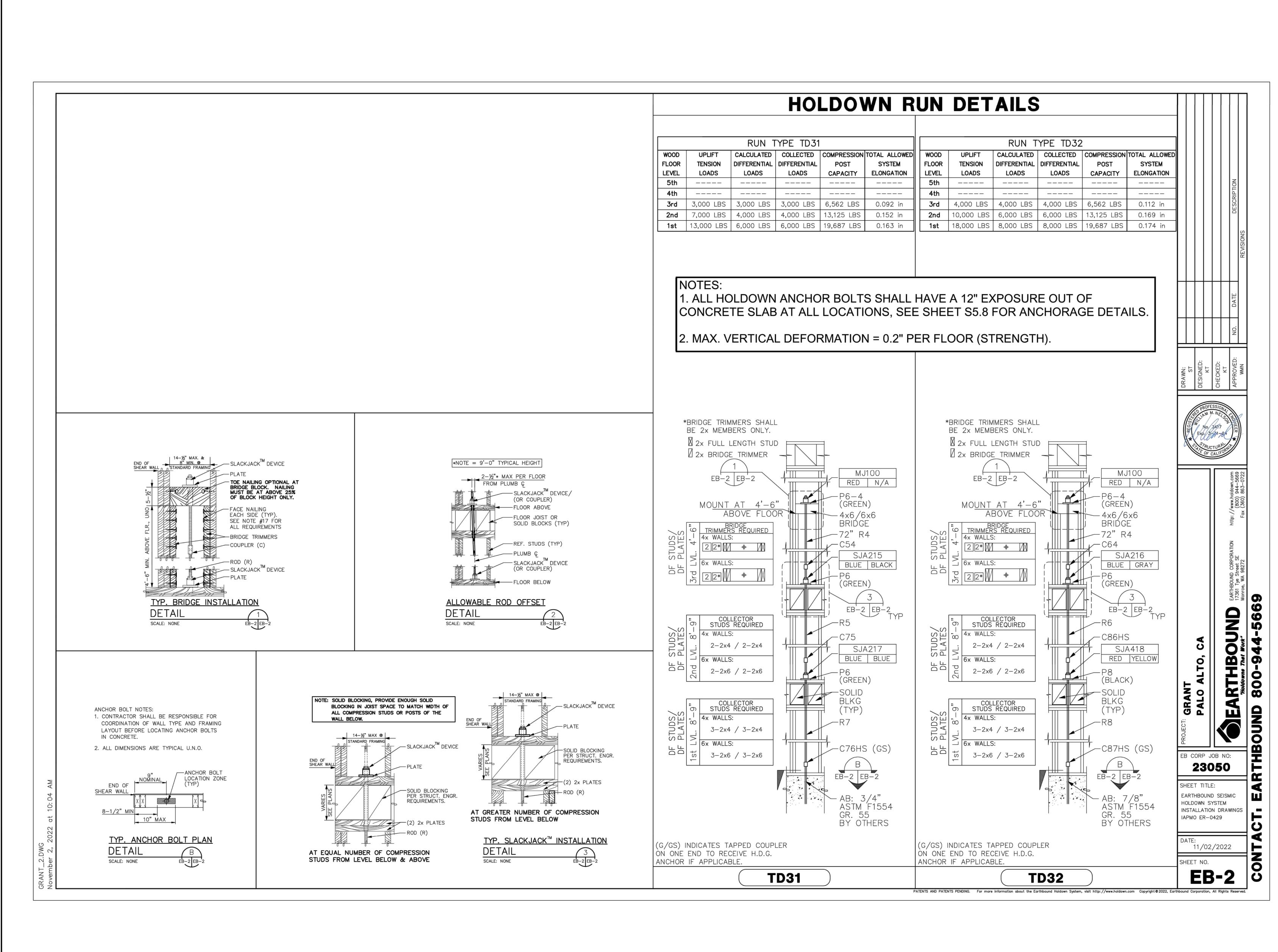
HARD COPY OF THESE STAMPED PLANS MUST BE ON THE SITE FOR INSPECTIONS

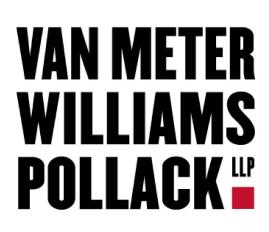
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Α	12/16/2022	BID SET
В	03/20/2023	BID ADDENDUM
2	03/20/2023	PLAN CHECK RESPONSE 2
3	05/12/2023	PLAN CHECK RESPONSE 3

Project:







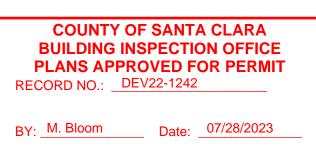


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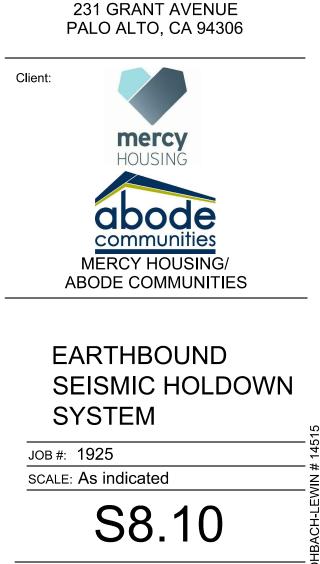


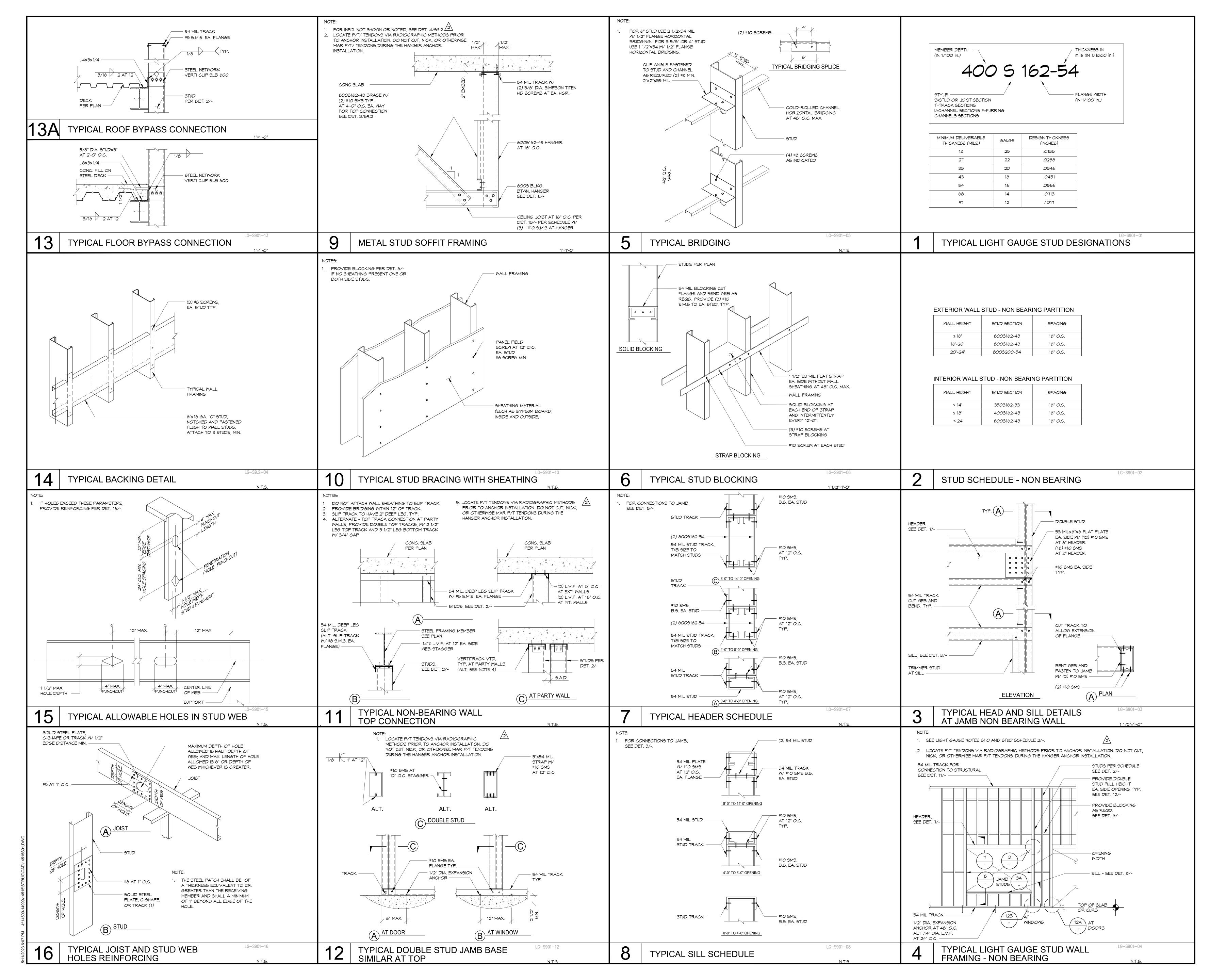
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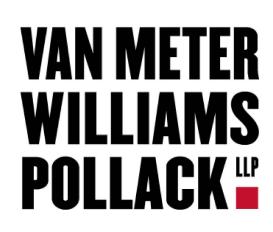
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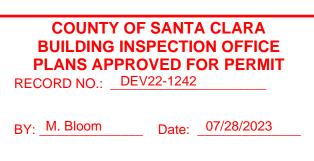


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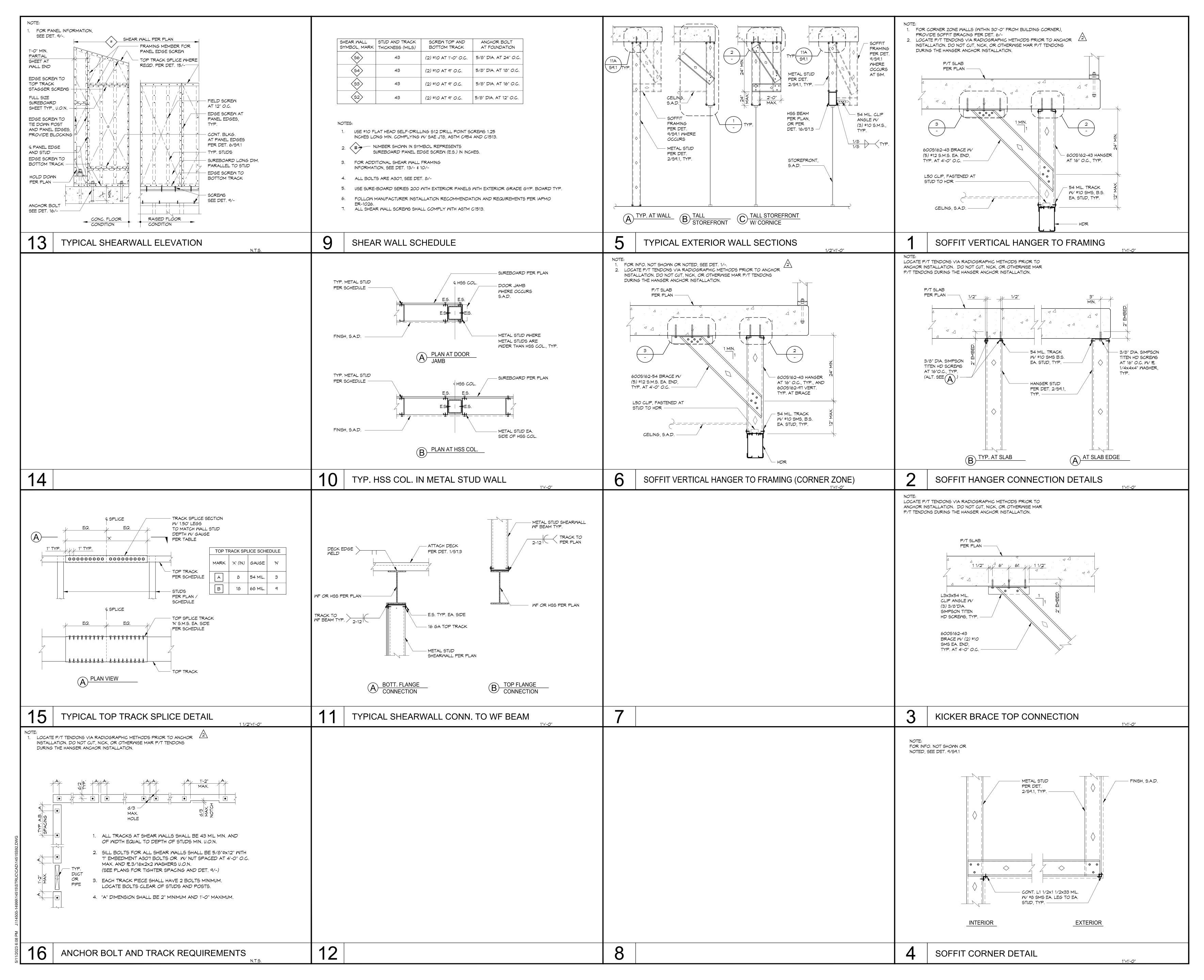
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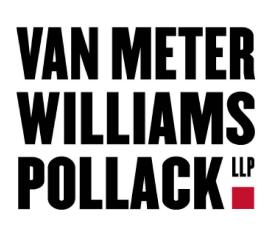
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Project:



231 GRANT AVENUE PALO ALTO, CA 94306	
Client:	
mercy HOUSING	
abode	
MERCY HOUSING/ ABODE COMMUNITIES	
TYPICAL LIGHT GAUGE DETAILS	
JOB #: 1925 SCALE: As indicated	# Z
S9.1	HBACH-LEWIN # 14515





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Project:



	1 GRANT AVENUE LO ALTO, CA 94306	
Client:		
	Mercy Housing	
	abode	
	IERCY HOUSING/ ODE COMMUNITIES	
	PICAL LIGHT	
GA	UGE DETAILS	ני
JOB #: 19)25	145
SCALE: AS	s indicated	# NI
	S9.2	DHBACH-LEWIN # 14515