

22558 COUNTRY VIEW DRIVE,
SAN JOSE, CA 95120

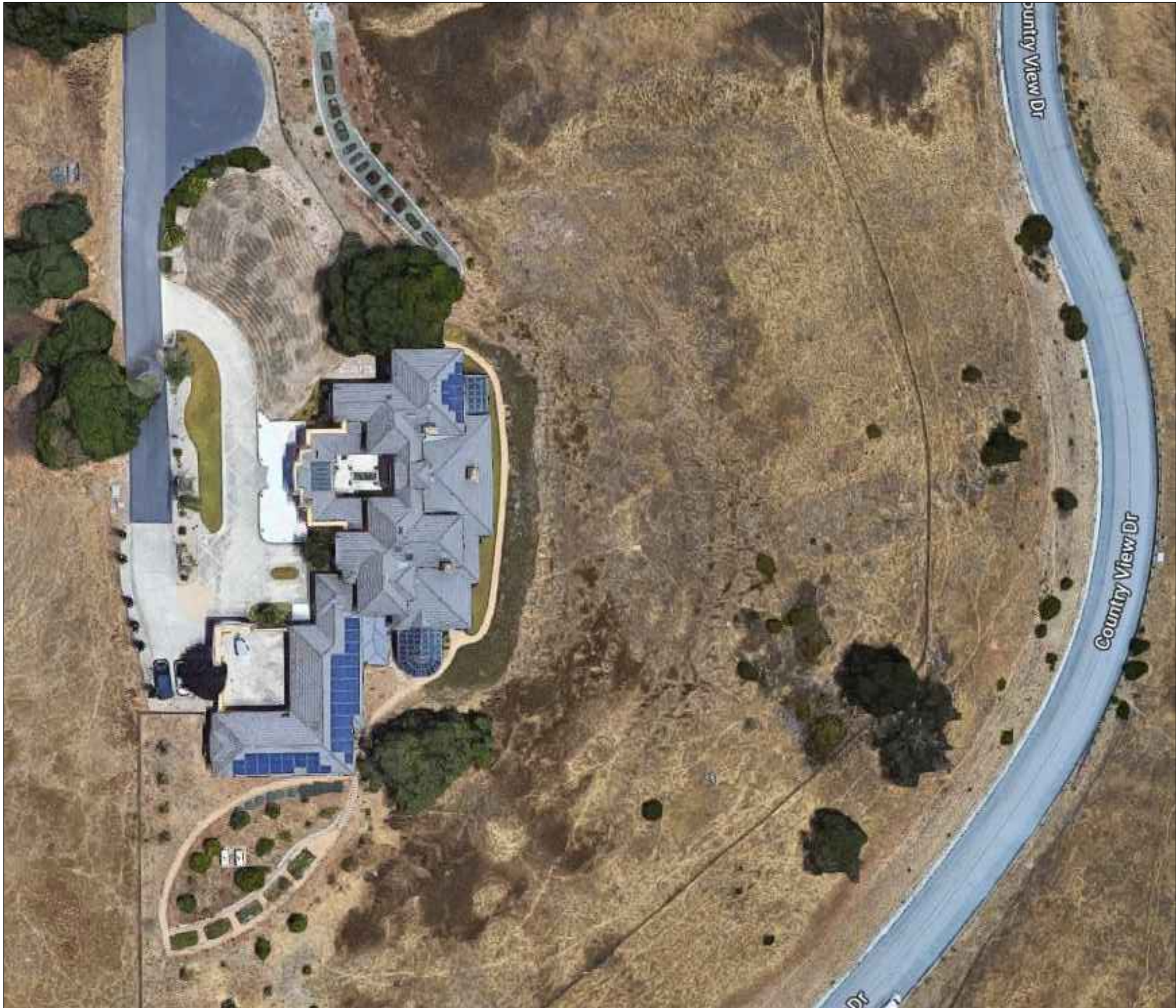
SCOPE OF WORK:

- 1. LEGALIZATION OF THE EXISTING DECK

SPECIAL INSPECTION REQUIRED:

- 1. HOLD-DOWN AND STRAPS
- 2. HIGH-STRENGTH SHEAR WALLS.
- 3. CAST IN PLACE ANCHORS
- 4. DOWEL EPOXY TO EXISTING CONCRETE

SHEET INDEX	
SHEET NO.	SHEET TITLE
COVER	COVER
S0.1	GENERAL NOTES
S0.2	ABBREVIATIONS AND NAILING SCHEDULE
S0.3	TYPICAL DETAILS
S0.4	TYPICAL DETAILS
S0.5	TYPICAL DETAILS
S0.6	TYPICAL DETAILS
S1.0	FOUNDATION PLAN
S1.1	DECK FRAMING PLAN
S2.0	STRUCTURAL DETAILS

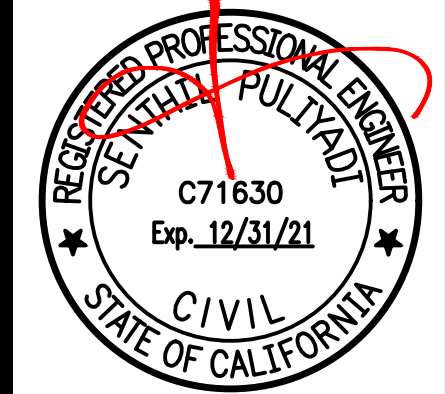


SITE LOCATION



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REV:
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COVER

22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120

DATE: 12.17.2019
SCALE: AS SHOWN
DESIGN BY: AZ
DRAWN BY: AZ
REVIEWED BY: SP
191929 CO1

COVER

PAGE 01 of 10

GENERAL

1.

NOTES AND DETAILS ON THE STRUCTURAL DRAWINGS TAKE PRECEDENCE OVER THESE STANDARD STRUCTURAL NOTES. DETAILS NOTED AS "TYPICAL" SHALL BE USED WHENEVER APPLICABLE. REFER TO SPECIFICATIONS FOR INFORMATION NOT COVERD BY THESE NOTES OR DRAWINGS.
2.

THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS, ELEVATIONS AND SITE CONDITIONS BEFORE STARTING WORK, AND THE ENGINEER/ ARCHITECT SHALL BE IMMEDIATELY NOTIFIED, IN WRITING OF ANY DISCREPANCIES. IN NO CASE SHALL DIMENSIONS BE SCALED FROM PLANS, SECTIONS, OR DETAILS ON THE STRUCTURAL DRAWINGS.
3.

ALL OMISSIONS AND CONFLICTS BETWEEN THE VARIOUS ELEMENTS OF THE STRUCTURAL DRAWINGS AND /OR SPECIFICATIONS SHALL BE BROUGHT TO THE ATTENTION OF, AND RESOLVED WITH, THE ENGINEER BEFORE PROCEEDING WITH ANY WORK SO INVOLVED.
4.

WHERE A CONSTRUCTION DETAIL IS NOT SPECIFICALLY SHOWN OR NOTED, THE DETAIL SHALL BE THE SAME AS FOR OTHER SIMILAR WORK.
5.

THE CONTRACTOR SHALL DETERMINE THE LOCATION OF UTILITY SERVICES IN THE AREA TO BE EXCAVATED, BEFORE BEGINNING EXCAVATION.
6.

NO PIPES, DUCTS, SLEEVES, CHASES, ETC. SHALL BE PLACED IN OR THRU SLABS, BEAMS, OR WALLS, NOR SHALL ANY STRUCTURAL MEMBER BE CUT FOR PIPES, DUCTS, ETC. EXCEPT AS INDICATED ON THE STRUCTURAL DRAWINGS. THE CONTRACTOR SHALL OBTAIN PRIOR APPROVAL FOR INSTALLATION OF ANY PIPES, DUCTS, ETC.
7.

ALL MATERIAL AND WORKMANSHIP SHALL CONFORM TO THE REQUIREMENTS OF THE CBC 2016.
8.

THE CONTRACTOR SHALL ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THIS PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS.
9.

RETAIN A CALIFORNIA REGISTERED CIVIL ENGINEER TO DESIGN ALL TEMPORARY BRACING, SHORING, AND SUPPORT REQUIRED DURING CONSTRUCTION.
10.

INCLUDE ENGINEERING FEES, ENGINEERING DESIGN TIME AND BUILDING DEPARTMENT APPROVAL TIME IN THE COST OF PROPOSED MATERIAL ALTERNATES. CONTACT ENGINEER FOR FEE AMOUNT. SUBMIT MATERIAL ALTERNATE FOR REVIEW BEFORE CONSTRUCTION.

DESIGN CRITERIA

1.

VERTICAL LOADS :

A. DEAD LOADS:

DECK 20 PSF

B. LIVE LOADS:(REDUCIBLE UNLESS NOTED OTHERWISE)

DECK..... 60 PSF

2.

LATERAL LOADS:

A. EARTHQUAKE DESIGN DATA:

RESPONCE MODIFICATION FACTOR (R)= 6.5
(LIGHT-FRAME WOOD WALL w/STRUCTURAL PANEL)

V=0.208W (ASD) (INCLUDES rho=1.3)

SEISMIC IMPORTANCE FACTOR.....= I
OCCUPANCY CATEGORY.....= II
MAPPED REFERENCE RESPONSE ACCEL.....

Ss = 1.566
S1 = 0.6

SITE CLASS= D
SPECTRAL RESPONSE COEFF.....

Sps = 1.044
Sp1 = 0.6

SEISMIC DESIGN CATEGORY.....= D
SEISMIC RESPONSE COEFF=0.161
ANALYSIS PROCEDURE :
EQUIVALENT LATERAL FORCE PROCEDURE

B. WIND DESIGN DATA:

BASIC WIND SPEED.....= 110 MPH
SITE EXPOSURE.....= B
I.....= 1.00
Kzt.....= 1.00
Kd.....= 1.30
DESIGN WIND PRESSURE (p).....= 16 PSF

FOUNDATION

- SOIL INFORMATION:

1.

SOIL DESIGN PRESSURES:

PER CBC 2016 TABLE 1806.2
ALLOWABLE BEARING PRESSURES:

1,500 PSF
(½ INCREASE FOR WIND OR SEISMIC)

LATERAL BEARING PRESSURE:

100 PCF

FRICTIONAL FACTOR:

0.25
2.

BOTTOM OF THE FOOTING SHALL BE AT LEAST 18 INCHES BELOW LOWEST ADJACENT FINISHED GRADE INTO NATURAL GRADE OR ENGINEERED FILL.
3.

REFER TO AND CHECK WITH ARCHITECTURAL DRAWINGS FOR VARIOUS FLOOR SLOPES, DROPPED SLABS, DEPRESSIONS CURBS, STEPS, WALKS, DRAINS, DEPRESSED FLOOR, ETC. & DIMENSIONS NOT SHOWN.
4.

NO CONCRETE SHALL BE POURED IN ANY FOUNDATION UNTIL EXCAVATION HAS BEEN INSPECTED, EXCAVATION SHALL BE KEPT FREE OF LOOSE MATERIAL AND STANDING WATER.
5.

NO SLEEING OF ANY GRADE BEAM WILL BE PERMITTED UNLESS SHOWN ON STRUCTURAL DRAWINGS APPROVED BY THE ENGINEER.
6.

THE ENGINEER HAS NO CONTROL OR RESPONSIBILITY FOR THE DESIGN OF TEMPORARY SHORING, SCAFFOLDING, FORMING, UNDERPINNING, ETC., NOT DETAILED ON THESE PLANS.
7.

ALL EXCAVATIONS SHALL BE PROPERLY BACKFILLED. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS BEFORE CONCRETE (OR GROUT) HAS ATTAINED FULL DESIGN STRENGTH. CONTRACTOR SHALL BRACE OR PROTECT ALL BUILDING AND PIT WALLS BELOW GRADE FROM MATERIAL LOADS UNTIL ATTACHING FLOORS ARE COMPLETELY IN PLACE AND HAVE ATTAINED FULL STRENGTH. CONTRACTOR SHALL PROVIDE FOR DESIGN, PERMITS, AND INSTALLATION OF SUCH BRACING.
8.

FOUNDATION CONTRACTOR SHALL VERIFY AND PROVIDE CROSS VENTILATION THROUGH ALL PORTIONS OF CRAWL SPACE FOR PROPER AIR FLOW.
9.

CONTRACTOR SHALL COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. NOTIFY STRUCTURAL ENGINEER, PRIOR TO FABRICATION OR ERECTION, WHEN DISCREPANCIES ARE FOUND.

REINFORCED CONCRETE

1.

CEMENT SHALL CONFORM ASTM C150, TYPE II.
2.

AGGREGATES FOR NORMAL WEIGHT CONCRETE SHALL CONFORM TO ASTM C33.
3.

CONCRETE SHALL HAVE MINIMUM COMPRESSIVE STRENGTH OF 2,500 PSI AT 28 DAYS: (4" MAX SLUMP FOR FLATWORK)
4.

ADMIXTURES MAY BE USED WITH PRIOR APPROVAL OF THE ENGINEER. ADMIXTURES SHALL COMPLY WITH ASTM C494 & C1017 AND BE OF A TYPE THAT INCREASES THE WORKABILITY OF THE CONCRETE, BUT SHALL NOT BE CONSIDERED TO REDUCE THE SPECIFIED MINIMUM CEMENT CONTENT.
5.

NO CONDUIT PLACED IN CONCRETE SLAB SHALL HAVE AN OUTSIDE DIAMETER GREATER THAN ½ THE THICKNESS OF THE SLAB. NO CONDUIT SHALL BE EMBED. IN A SLAB THAT IS LESS THAN 3½" THICK. EXCEPT FOR LOCAL OFFSETS, MINIMUM CLEAR DISTANCE BETWEEN CONDUITS SHALL BE THREE DIAMETERS ON CENTER. (EXCEPT IF THE CONDUIT IS PASSING THROUGH).
6.

PROJECTING CORNERS OF SLAB , BEAMS, WALLS, COLUMNS, ETC. SHALL BE FORMED WITH ¾" CHAMFERS.
7.

REFER TO DRAWINGS OF OTHER DISCIPLINES FOR MOLDS, GROOVES, CLIPS, ORNAMENTS OR GROUNDS REQUIRED TO BE CAST INTO CONCRETE.
8.

ALL SLABS ON GRADE SHALL HAVE CONTROL JOINTS INSTALLED TO PROVIDE APPROXIMATELY 15 FOOT SQUARES UNLESS DETAILED OTHERWISE ON THE PLANS. WHERE CONCRETE POURS ARE STOPPED, THE JOINTS SHALL BE FORMED.

REINFORCED STEEL

1.

NO BRICK OR POROUS MATERIAL SHALL BE USED TO SUPPORT FOOTING STEEL OFF THE GROUND. PRECAST CONCRETE DOBIES ARE APPROVED.
2.

BAR REINFORCEMENT SHALL BE ASTM A615, GRADE 60.
3.

BAR REINFORCEMENT THAT IS TO BE WELDED SHALL BE ASTM A706, GRADE 60. WELDING OF REINFORCING BARS SHALL CONFORM TO AWS D1.4-92 AND CBC STANDARD 19-1. E70XX ELECTRODES SHALL BE USED. SPECIAL INSPECTION IS REQUIRED FOR ALL FIELD WELDING.
4.

SPLICES IN REINFORCING STEEL SHALL LAP AS FOLLOWS, UNLESS NOTED OTHERWISE: #3 THROUGH #6 = 48 DIA., #7 THROUGH #9 = 68 DIA., #10 THROUGH #11 = 86 DIA. HORIZONTAL SPLICES SHALL BE STAGGERED. NON-CONTACT SPLICES SHALL NOT BE SPACED TRANSVERSELY FARTHER APART ½ THE REQUIRED LAP SPLICE LENGTH, OR 6 INCHES.

5.

THE CLEAR DISTANCE BETWEEN PARALLEL BARS SHALL BE FOUR BAR DIAMETERS BUT NO LESS THAN 1½" U.N.O. IN WALLS AND SLABS OTHER THAN CONCRETE JOIST CONSTRUCTION, REINFORCEMENT SHALL BE SPACED NOT FARTHER APART THAN THREE TIMES THE WALL OR SLAB THICKNESS, NOR 18".
6.

REINFORCING STEEL SHALL HAVE A PROTECTED CONCRETE COVERING AS FOLLOWS, U.N.O.

WALL STEEL BELOW GRADE:

ON DIRT SIDE WHEN POURED AGAINST DIRT = 3"
ON DIRT SIDE WHEN FORMED = 2"

WALL STEEL ABOVE GRADE:

IN ALL OTHER CASES = 1-½"

OTHER ITEMS :

FOOTING PADS = 3"
SLABS (ON EARTH) = 2"
SLABS (HARD ROCK CONCRETE) = 1"
JOISTS (SIDES, TOPS & SOFFITS) = 1"
COLUMNS (TO MAIN STEEL) = 2"
BEAMS/GIRDERS (SIDES, TOPS & SOFFITS) = 2"
7.

WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185 AND SHALL BE LAPPED 12" MINIMUM.
8.

ALL WALLS SHALL BE DOWELED TO SUPPORTING FOOTINGS, BEAMS, PADS, ETC., WITH BARS THE SAME SIZE AND SPACING AS VERTICAL BARS IN THE WALL UNLESS OTHERWISE DETAILED. ANCHORAGE OF DOWELS SHALL BE EQUIVALENT OF A BAR SPLICE.
9.

DOWEL REINFORCED SLABS TO WALLS AND OTHER EDGE MEMBERS PER TYPICAL DETAILS.
10.

ALL REINFORCING STEEL IS TO BE PLACED IN RELATIVE POSITION SHOWN ON DRAWINGS. NO SPLICES IN ANY REINFORCING WILL BE PERMITTED EXCEPT THOSE SHOWN ON THE STRUCTURAL DRAWINGS.
11.

REINFORCING DETAILING, BENDING, AND PLACING SHALL BE IN ACCORDANCE WITH THE CONCRETE REINFORCING STEEL INSTITUTES "MANUAL OF STANDARD PRACTICE", LATEST EDITION.
12.

ALL REINFORCING STEEL, ANCHOR BOLTS, DOWELS, AND INSERTS SHALL BE WELL SECURED IN POSITION WITH WIRE POSITIONERS BEFORE PLACING CONCRETE OR GROUT. VERTICAL BARS IN MASONRY WALLS SHALL BE TIED IN POSITION AT THE TOP AND BOTTOM AND INTERVALS NOT EXCEEDING 200 x BAR DIAMETERS.

STRUCTURAL WOOD

1.

ALL FRAMING LUMBER SHALL BE DOUGLAS FIR-LARCH GRADE MARKED BY A RECOGNIZED GRADING AGENCY (WCLIB & WWPA)

JOISTS & PLANKS: NO.1
BEAMS & STRINGERS: NO.1
POST & TIMBERS: NO.1
GLUED LAMINATED TIMBERS: COMBINATION 24F-V8
PARALLAM, PSL: 2.0E (2900Fb)

VERTICAL STUDS:

2X4 STUDS, 8'-0" LONG: NO.1
2X4 STUDS, 8'-1"TO 14"-0" LONG: NO.1
2X6 STUDS: NO.1
ALL OTHER LUMBER: NO. 1 STRUCTURAL" LIGHT FRAMING
2.

ALL SILL PLATES RESTING ON CONCRETE OR MASONRY, WHICH IS IN CONTACT WITH EARTH OR RESTING ON FOUNDATIONS SHALL BE PRESSURE TREATED DOUGLAS FIR (P.T.D.F.). ALL FASTENERS SUCH AS NAILS, BOLTS, SCREWS, ANCHOR BOLTS, ETC. ATTACHING P.T.D.F. OR FIRE-RETARDANT TREATED WOOD SHALL BE HOT-DIPPED ZINC COATED GALVANIZED OR STAINLESS STEEL (ASTM A153).
3.

NEW MUDSILLS SHALL BE MINIMUM 3x PRESSURE TREATED w/ 8"ø ANCHOR BOLTS w/ 8" MIN. EMBED. IN CONCRETEV@ 4'-0" MAX. SPACING w/ 3"x3"x.229" THK PLATE WASHERS. PROVIDE MINIMUM TWO BLTS PER MUDSILL EACH ONE NOT MORE THAN 12" OR LESS THAN 1 BOLT DIAMETERS FROM END.
4.

ANCHOR BOLT SPACING @ SHEAR WALL PER SHAR WALL SCHEDULE.
5.

WHERE STUD PARTITIONS JOIN CONCRETE OR MASONRY WALLS THE END STUD SHALL BE ANCHORED THERE TO WITH ½" BOLTS NEAR THE TOP & BOTTOM AND AT EACH ROW OF FIRE BLOCKING. SUCH BOLTS SHALL BE EMBEDDED IN THE WALL NOT LESS THAN ⅓ OF THE WALL THICKNESS OR 8" MAX.
6.

CUTTING, NOTCHING, OR BORING OF STUDS SHALL BE PERMITTED ONLY AS DETAILED OR APPROVED BY ENGINEER AND/OR PER 2015 NDS.
7.

ALL NAILING SHALL CONFORM TO CBC TABLE 2304.10.1, AND SHALL BE COMMON NAILS, UNLESS NOTED OTHERWISE ON PLANS AND DETAILS.
8.

ALL BOLT HEADS AND NUTS BEARING ON WOOD SHALL HAVE STANDARD CUT WASHERS. HOLES FOR BOLTS SHALL BE BORED ⅙" LARGER THAN THE NOMINAL BOLT DIAMETER. BOLTS IN WOOD SHALL NOT BE LESS THAN 7 DIAMETERS FROM THE END AND 4 DIAMETERS FROM THE EDGE OF THE MEMBER.
9.

TOP PLATES OF ALL WOOD STUD WALLS TO BE (2) 2x MINIMUM

- (SAME WIDTH AS STUDS), LAP 48" MIN. WITH NOT LESS THAN (6) 16d NAILS AT EACH LAP AND NOT MORE THAN 12" BETWEEN NAILS.
10.

PLYWOOD SHALL BE APA STRUCTURAL-I RATED SHEATHING WITH EXTERIOR GLUE.
11.

PROVIDE DOUBLED JOISTS UNDER ALL PARALLEL PARTITIONS. SEE DETAIL 2/S0.5.
12.

ALL LAG SCREWS TO BE PREDRILLED, DRILL DIAMETER TO BE 60 PERCENT OF SHANK DIAMETER.
13.

RE-TIGHTEN ALL ANCHOR BOLTS JUST BEFORE CLOSING IN.
14.

ALL FRAMING ANCHORS, POST CAPS, BASES, HANGERS, STRAPS, ETC. SHALL BE AS MANUFACTURED BY "SIMPSON COMPANY" OR ENGINEER APPROVED EQUALS.
15.

PROVIDE BLOCKING OR BRIDGING PER 2015 NDS SECTION 4.4.1.
16.

MOISTURE CONTENT OF WOOD AT TIME OF PLACING SHALL NOT EXCEED 19 PERCENT.
17.

MACHINE BOLTS AND ANCHOR BOLTS SHALL BE GRADE-A CONFORMING TO ASTM A307. NUTS FOR MACHINE BOLTS AND ANCHOR BOLTS SHALL CONFORM TO ASTM A563, HEX GRADE-A. THREADED RODS SHALL CONFORM TO ASTM A36. ROUND WASHERS SHALL CONFORM TO ASTM F436 AND SQUARE PLATE WASHERS SHALL CONFORM TO ASTM A36.

SPECIAL INSPECTION

1.

SPECIAL INSPECTION SHALL MEET THE REQUIREMENTS OF CBC SECTION 1704.
2.

SPECIAL INSPECTOR SHALL:

A. BE UNDER THE SUPERVISION OF CALIFORNIA REGISTERED CIVIL ENGINEER.

B. OBSERVE THE WORK ASSIGNED FOR CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS.

C. FURNISH INSPECTION REPORTS TO THE ENGINEER AND BUILDING DEPARTMENT. DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION; THEN IF NOT CORRECTED, TO THE ENGINEER AND BUILDING DEPARTMENT.

D. SUBMIT TO THE ENGINEER AND BUILDING DEPARTMENT A FINAL REPORT, SIGNED BY A CALIFORNIA REGISTERED CIVIL ENGINEER, STATING THAT THE WORK WAS IN CONFORMANCE WITH THE APPROVED DRAWINGS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE CBC.
3.

INSPECTION NOTES:

A. CONSTRUCTION INSPECTIONS LISTED ARE IN ADDITION TO THE CALLED INSPECTIONS REQUIRED BY CBC SECTION 109, APPENDIX CHAPTER 1. SPECIAL INSPECTION IS NOT A SUBSTITUTE FOR INSPECTION BY A BUILDING OFFICIAL. SPECIALLY INSPECTED WORK WHICH IS INSTALLED OR COVERED WITHOUT APPROVAL OF THE BUILDING OFFICIAL IS SUBJECT TO REMOVAL OR EXPOSURE.

B. CONTINUOUS INSPECTION IS ALWAYS REQUIRED DURING PERFORMANCE OF THE WORK UNLESS SPECIFICALLY NOTED.

C. SPECIAL INSPECTORS MUST BE CERTIFIED BY THE BUILDING DEPARTMENT TO PERFORM THE TYPES OF INSPECTIONS SPECIFIED.

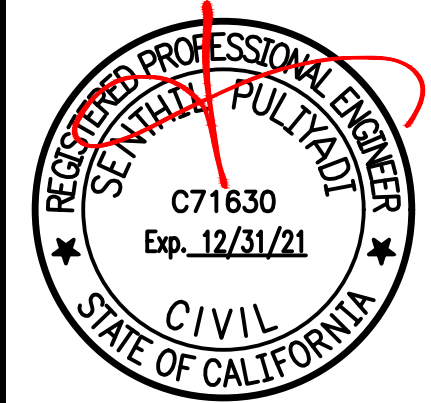
D. IT IS THE RESPONSIBILITY OF THE OWNER TO INFORM THE SPECIAL INSPECTOR OR INSPECTION AGENCY AT LEAST ONE WORKING DAY BEFORE PERFORMING ANY WORK THAT REQUIRES SPECIAL INSPECTION. ALL WORK PERFORMED WITHOUT REQUIRED SPECIAL INSPECTION IS SUBJECT TO REMOVAL.



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GENERAL NOTES

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S0.1

ABBREVIATIONS

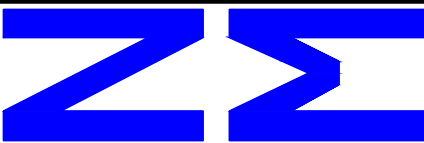
AB ANCHOR BOLT
ABV ABOVE
ADDL ADDITIONAL
ALT ALTERNATE
APA AMERICAN PLYWOOD ASSOCIATION
ARCH ARCHITECTURAL
AWPA AMERICAN WOOD PRESERVERS ASSOCIATION
B BOTTOM
BEL BELOW
BLKG BLOCKING
BM BEAM
BN BOUNDARY NAIL
BRG BEARING
BTWN BETWEEN
BOT BOTTOM
CBC CALIFORNIA BUILDING CODE
CIP CAST-IN-PLACE
CJ CONSTRUCTION JOINT
CL CENTER LINE
CLR CLEAR
COL COLUMN
COLL COLLECTOR
CONC CONCRETE
CONN CONNECTION
CONST CONSTRUCTION
CONT CONTINUOUS
DBL DOUBLE
DEMO DEMOLISH
DF DOUGLAS FIR/LARCH
DP DEEP
DIA DIAMETER
DIM DIMENSION
DIR. DIRECTION
DO DITTO (REPEAT)
DWG DRAWING
DWL DOWEL
(E) EXISTING
EA EACH
EF EACH FACE
EL ELEVATION
ELEC ELECTRICAL
ELEV ELEVATOR
EMBED EMBEDMENT
EN EDGE NAIL
EQ EQUAL OR EQUIVALENT
ES EACH SIDE
E.W. EACH WAY
EXT EXTERIOR
FDN FOUNDATION
FF FINISHED FLOOR
FJ FLOOR JOIST
FOC FACE OF CONCRETE
FOM FACE OF MASONRY
FOS FACE OF STUD
FRM FRAMING
FRT FIRE-RETARDANT TREATED
FS FAR SIDE
FTG FOOTING
GA GAUGE
GALV GALVANIZED
GLB GLUED LAMINATED BEAM
GR GRADE
GYP GYPSUM
H HORIZONTAL
HDR HEADER
H.D.G. HOT DIP GALVANIZED
HDG HOT DIP GALVANIZED
HGR HANGER
HGT HEIGHT
HORIZ HORIZONTAL

IBC INTERNATIONAL BUILDING CODE
ID INSIDE DIAMETER
IF INSIDE FACE
I JST I-JOIST
INT INTERIOR
JST JOIST
JT JOINT
LDGR LEDGER
LTWT LIGHT WEIGHT
MATL MATERIAL
MAX MAXIMUM
MB MACHINE BOLT
MECH MECHANICAL
MFR MANUFACTURER
MIN MINIMUM
MISC MISCELLANEOUS
MTL METAL
NIC NOT INCONTRACT
NO NUMBER
NS NEAR SIDE
NTS NOT TO SCALE
O.C. ON CENTER
OD OUTSIDE DIAMETER
OF OUTSIDE FACE
OH OPPOSITE HAND
OPNG OPENING
OPP OPPOSITE
ORIG ORIGINAL
OSB ORIENTED STRAND BOARD
PL PLATE
PLCS PLACES
PLYWD PLYWOOD
PSL PARALLEL STRAND LUMBER
PT PRESERVATIVE TREATED
PW PLATE WASHER
R RADIUS
REF REFERENCE
REINF REINFORCEMENT
REQD REQUIRED
RF ROOF
RFT RAFTERS
S.A.D. SEE ARCHITECTURAL DWGS
SCHED SCHEDULE
SECT SECTION
SIM SIMILAR
S.O.G. SLAB ON GRADE
SN SHEAR NAIL
SPEC SPECIFICATION
SQ SQUARE
STD STANDARD
STGR STAGGER
STIFF STIFFENERS
STR STRUCTURAL
STR-I STRUCTURAL I TOP
T&B TOP & BOTTOM
THK THICKNESS/THICK
THR THREADED
TN TOENAIL
TOP TOP OF PARAPET
TOW TOP OF WALL
TYP TYPICAL
UON UNLESS OTHERWISE NOTED
VERT VERTICAL
VSH VERTICAL SLOTTED HOLES
WD WOOD
WP WATERPROOF OR WORK POINT
WT WEIGHT
WWF WELDED WIRE FABRIC
w/ WITH
w/o WITHOUT
YD YARD

TABLE 2304.10.1 FASTENING SCHEDULE

TABLE NO. 2304.10.1 : NAILING SCHEDULE		
DESCRIPTION OF BUILDING ELEMENTS	NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION
ROOF		
1. BLOCKING BETWEEN CEILING JOISTS, RAFTERS OR TRUSSES TO TOP PLATE OR OTHER FRAMING BELOW	5-8d COMMON (2½" X 0.131"); OR 5-10d BOX (3" X 0.128"); OR 5-3" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	EACH END, TOENAIL
BLOCKING BETWEEN RAFTERS OR TRUSS NOT AT THE WALL TOP PLATE, TO RAFTER OR TRUSS	2-8d COMMON (2½" X 0.131") 2-3" X 0.131" NAILS 5-3" 14 GAGE STAPLES	EACH END, TOENAIL
FLAT BLOCKING TO TRUSS AND WEB FILLER	2-16d COMMON (3½" X 0.162") 5-3" X 0.131" NAILS 5-3" 14 GAGE STAPLES	END NAIL
2. CEILING JOISTS TO TOP PLATE	16d COMMON (3½" X 0.162") @ 6" C. 5" X 0.131" NAILS @ 6" O.C. 5" X 14 GAGE STAPLES @ 6" O.C	FACE NAIL
3. CEILING JOIST NOT ATTACHED TO PARALLEL RAFTER, LAPS OVER PARTITIONS (NO THRUST)(SEE SECTION 2308.7.3.1, TABLE 2308.7.3.1)	5-8d COMMON (2½" X 0.131"); OR 5-10d BOX (3" X 0.128"); OR 5-3" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	EACH JOIST, TOENAIL
4. CEILING JOIST ATTACHED TO PARALLEL RAFTER (HEEL JOINT) (SEE SECTION 2308.7.3.1, TABLE 2308.7.3.1)	5-16d COMMON (3½" X 0.162"); OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	FACE NAIL
5. COLLAR TIE TO RAFTER	PER TABLE 2308.7.3.1	FACE NAIL
6. RAFTER OR ROOF TRUSS TO TOP PLATE (SEE SECTION 2308.7.5, TABLE 2308.7.5)	5-10d COMMON (3" X 0.148"); OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	FACE NAIL
7. ROOF RAFTERS TO RIDGE VALLEY OR HIP RAFTERS; OR ROOF RAFTER TO 2-INCH RIDGE BEAM	5-10 COMMON (3" X 0.148"); OR 5-16d BOX (3½" X 0.135"); OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131 NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	TOENAIL
8. STUD TO STUD (NOT AT BRACED WALL PANELS)	2-16d COMMON (3½" X 0.162"); OR 5-10d BOX (3" X 0.128"); OR 5-3" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	END NAIL
9. STUD TO STUD AND ABUTTING STUDS AT INTERSECTING WALL CORNERS (AT BRACED WALL PANELS)	3-10d COMMON (3½" X 0.148");OR 5-16d BOX (3½" X 0.135");OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	TOENAIL
10. BUILT-UP HEADER (2"TO 2" HEADER)	WALL	
11. CONTINUOUS HEADER TO STUD	16d COMMON (3½" X 0.162");OR 10d BOX (3" X 0.128"); OR 5" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	24" O.C. FACE NAIL
12. TOP PLATE TO TOP PLATE	16d COMMON (3½" X 0.162");OR 10d BOX (3" X 0.128"); OR 5" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	16" O.C. FACE NAIL
13. TOP PLATE TO TOP PLATE, AT END JOINTS	16d COMMON (3½" X 0.162");OR 10d BOX (3" X 0.128"); OR 5" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	12" O.C. FACE NAIL
14. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCK-ING (NOT AT BRACED WALL PANELS)	16d COMMON (3½" X 0.162");OR 16d BOX (3½" X 0.135"); OR 5" X 0.131" NAILS; OR 5" 14 GAGE STAPLES, ⅞" CROWN	16" O.C. FACE NAIL
15. BOTTOM PLATE TO JOIST, RIM JOIST, BAND JOIST OR BLOCKING AT BRACED WALL PANELS	2-16d COMMON (3½" X 0.162"); OR 5-16d BOX (3½" X 0.135"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	12" O.C. FACE NAIL
16. STUD TO TOP OR BOTTOM PLATE	4-8d COMMON (2½" X 0.131"); OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN; OR	TOENAIL
17. TOP OR BOTTOM PLATE TO STUD	2-16d COMMON (3½" X 0.162"); OR 5-10d BOX (3" X 0.128"); OR 5-3" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	END NAIL
18. TOP PLATES, LAPS AT CORNERS AND INTERSECTIONS	2-16d COMMON (3½" X 0.162"); OR 5-10d BOX (3" X 0.128"); OR 5-3" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	END NAIL
19. 1*BRACE TO EACH STUD AND PLATE	2-8d COMMON (2½" X 0.131"); OR 2-10d BOX (3" X 0.128"); OR 2-3" X 0.131" NAILS; OR 2-3" 14 GAGE STAPLES, ⅞" CROWN	FACE NAIL
20. 1" X 6" SHEATHING TO EACH BEARING	2-8d COMMON (2½" X 0.131"); OR 5-10d BOX (3" X 0.128"); OR	FACE NAIL
21. 1" × 8" AND WIDER SHEATHING TO EACH BEARING	5-8d COMMON (2½" X 0.131"); OR 5-10d BOX (3" X 0.128"); OR	FACE NAIL
FLOOR		
22. JOIST TO SILL, TOP PLATE, OR GIRDER	5-8d COMMON (2½" X 0.131"); OR FLOOR 5-10d BOX (3" X 0.128"); OR 5-3" X 0.131" NAILS; OR 5-3" 14 GAGE STAPLES, ⅞" CROWN	TOENAIL
23. RIM JOIST, BAND JOIST, OR BLOCKING TO TOP PLATE, SILL OR OTHER FRAMING BELOW	8d COMMON (2½" X 0.131"); OR 10d BOX (3" X 0.128"); OR 5" X 0.131" NAILS; OR 5" 14 GAGE STAPLES, ⅞" CROWN	6" O.C., TOENAIL

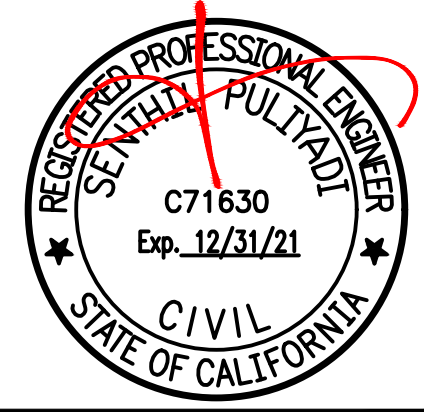
TABLE NO. 2304.10.1 CONTINUED : NAILING SCHEDULE				
DESCRIPTION OF BUILDING ELEMENTS		NUMBER AND TYPE OF FASTENER	SPACING AND LOCATION	
24. 1"x6" SUBFLOOR OR LESS TO EACH JOIST		2-8d COMMON (2½" X 0.131"); OR 2-10d BOX (3" X 0.128");	FACE NAIL	
25. 2" SUBFLOOR TO JOIST OR GIRDER		2-16d COMMON (3½" X 0.162")	FACE NAIL	
26. 2" PLANKS (PLANK & BEAM – FLOOR & ROOF)		2-16D COMMON (3½" X 0.162")	EACH BEARING, FACE NAIL	
27. BUILT-UP GIRDERS AND BEAMS, 2" LUMBER LAYERS		20d COMMON (4" X 0.192")	32" O.C., FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES	
		10d BOX (3"x 0.128"); OR 3" X 0.131" NAILS; OR 3" 14 GAGE STAPLES, ⅞" CROWN AND 2-20d COMMON (4" X 0.192"); OR 3-10d BOX (3" X 0.128"); OR 3-3" X 0.131" NAILS; OR 3-3" 14 GAGE STAPLES, ⅞" CROWN	24" O.C., FACE NAIL AT TOP AND BOTTOM STAGGERED ON OPPOSITE SIDES	
28. LEDGER STRIP SUPPORTING JOISTS OR RAFTERS		5-16d COMMON (3½" X 0.162"); OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	ENDS AND AT EACH SPLICE, FACE NAIL	
29. JOIST TO BAND JOIST OR RIM JOIST		5-16d COMMON (3½" X 0.162"); OR 4-10d BOX (3" X 0.128"); OR 4-3" X 0.131" NAILS; OR 4-3" 14 GAGE STAPLES, ⅞" CROWN	END NAIL	
30. BRIDGING OR BLOCKING TO JOIST, RAFTER OR TRUSS		2-8d COMMON (2½" X 0.131"); OR 2-10d BOX (3" X 0.128"); OR 2-3" X 0.131" NAILS OR 2-3" 14 GAGE STAPLES, ⅞" CROWN	EACH END, TOENAIL	
WOOD STRUCTURAL PANELS (WSP), SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING				
			EDGES (INCHES)	INTERMEDIATE SUPPORTS (INCHES)
31. ⅜" – ½"		8d COMMON OR DEFORMED (2" X0.113") (SUBFLOOR AND WALL)	6	12
		8d BOX OR DEFORMED (2½"X0.113") (ROOF)	6	12
		2½"X0.113" NAIL (SUBFLOOR AND WALL)	6	12
		⅝" 16 GAGE STAPLE, ⅞" CROWN (SUBFLOOR AND WALL)	4	8
		2⅝"X0.113" NAIL (ROOF)	4	8
		⅝" 16 GAGE STAPLE, ⅞" CROWN (ROOF)	3	6
32. 1⅝" – ¾"		8d COMMON (2½" X 0.131"); OR 8d DEFORMED (2" X 0.113")	6	12
		2⅝" X 0.113" NAIL; OR 2" 16 GAGE STAPLE, ⅞" CROWN	4	8
33. ⅞" – 1¼"		10d COMMON (3" X 0.148");OR 8d DEFORMED (2½"X 0.131")	6	12
OTHER EXTERIOR WALL SHEATHING				
34. ½" FIBERBOARD SHEATHING		1½" GALVANIZED ROOFING NAIL (⅞" HEAD DIAMETER); OR 1½" 16 GAGE STAPLE WITH ⅞" OR 1" CROWN	3	6
35. 2⅝" FIBERBOARD SHEATHING		1½" GALVANIZED ROOFING NAIL (⅞" DIAMETER HEAD);OR 1½" 16 GAGE STAPLE WITH ⅞" OR 1"CROWN	3	6
WOOD STRUCTURAL PANELS, COMBINATION SUBFLOOR UNDERLAYMENT TO FRAMING				
36. ¾" AND LESS		8d COMMON (2½" X 0.131"); OR 8d DEFORMED (2" X 0.113")	6	12
37. ⅞" – 1"		8d COMMON (2½" X 0.131"); OR 8d DEFORMED (2½" X 0.131")	6	12
38. 1⅞" – 1¼"		10d COMMON (3" X 0.148");OR 8d DEFORMED (2½"X 0.131")	6	12
PANEL SIDING TO FRAMING				
39. ½" OR LESS		8d CORROSION-RESISTANT SIDING (1½" X 0.106"); OR 8d CORROSION-RESISTANT CASING (2" X 0.099")	6	12
40. ⅝"		8d CORROSION-RESISTANT SIDING (2½" X 0.128"); OR 8d CORROSION-RESISTANT CASING (2½" X 0.113")	6	12
WOOD STRUCTURAL PANELS (WSP), SUBFLOOR, ROOF AND INTERIOR WALL SHEATHING TO FRAMING AND PARTICLEBOARD WALL SHEATHING TO FRAMING				
			EDGES (INCHES)	INTERMEDIATE SUPPORTS (INCHES)
INTERIOR PANELING				
41. ¼"		4d CASING (1½" X0.080"); OR 4d FINISH (1½" X 0.072")	6	12
42. ⅜"		8d CASING (2" X 0.099"); OR 8d FINISH (PANEL SUPPORTS AT 24 INCHES)	6	12
FOR SI: 1 INCH = 25.4MM				
b. NAILS SPACED AT 6 INCHES AT INTERMEDIATE SUPPORTS WHERE SPANS ARE 48 INCHES OR MORE. FOR NAILING OF WOOD STRUCTURAL PANEL AND PARTICLEBOARD DIAPHRAGMS AND SHEAR WALLS, REFER TO SECTION 2305. NAILS FOR WALL SHEATHING ARE PERMITTED TO BE COMMON, BOX OR CASING.				
c. SPACING SHALL BE 6 INCHES ON CENTER ON THE EDGES AND 12 INCHES ON CENTER AT INTERMEDIATE SUPPORTS FOR NONSTRUCTURAL APPLICATIONS. PANEL SUPPORTS AT 16 INCHES (20 INCHES IF STRENGTH AXIS IN THE LONG DIRECTION OF THE PANEL, UNLESS OTHERWISE MARKED).				
d. WHERE A RAFTER IS FASTENED TO AN ADJACENT PARALLEL CEILING JOIST IN ACCORDANCE WITH THIS SCHEDULE AND THE CEILING JOIST IS FASTENED TO THE TOP PLATE IN ACCORDANCE WITH THIS SCHEDULE, THE NUMBER OF TOENAILS IN THE RAFTER SHALL BE PERMITTED TO BE REDUCED BY ONE NAIL.				



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REV:

REV:



ABBREVIATIONS AND NAILING SCHEDULE

22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120

DATE: 12.17.2019

SCALE: AS SHOWN

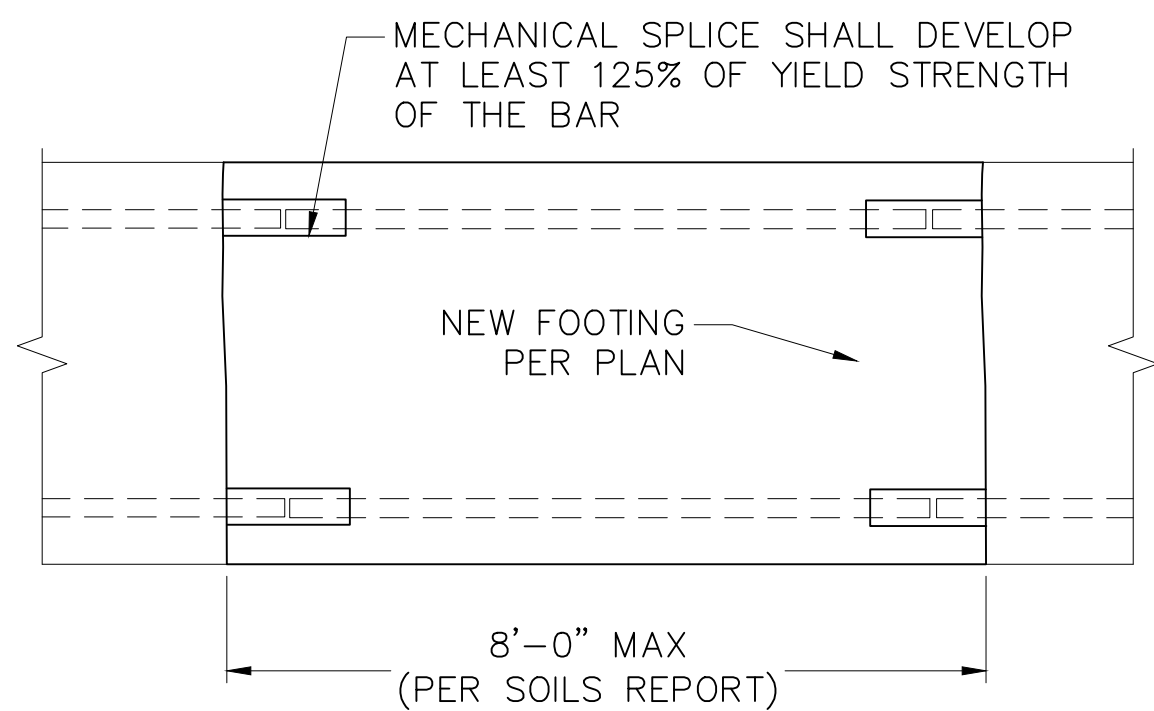
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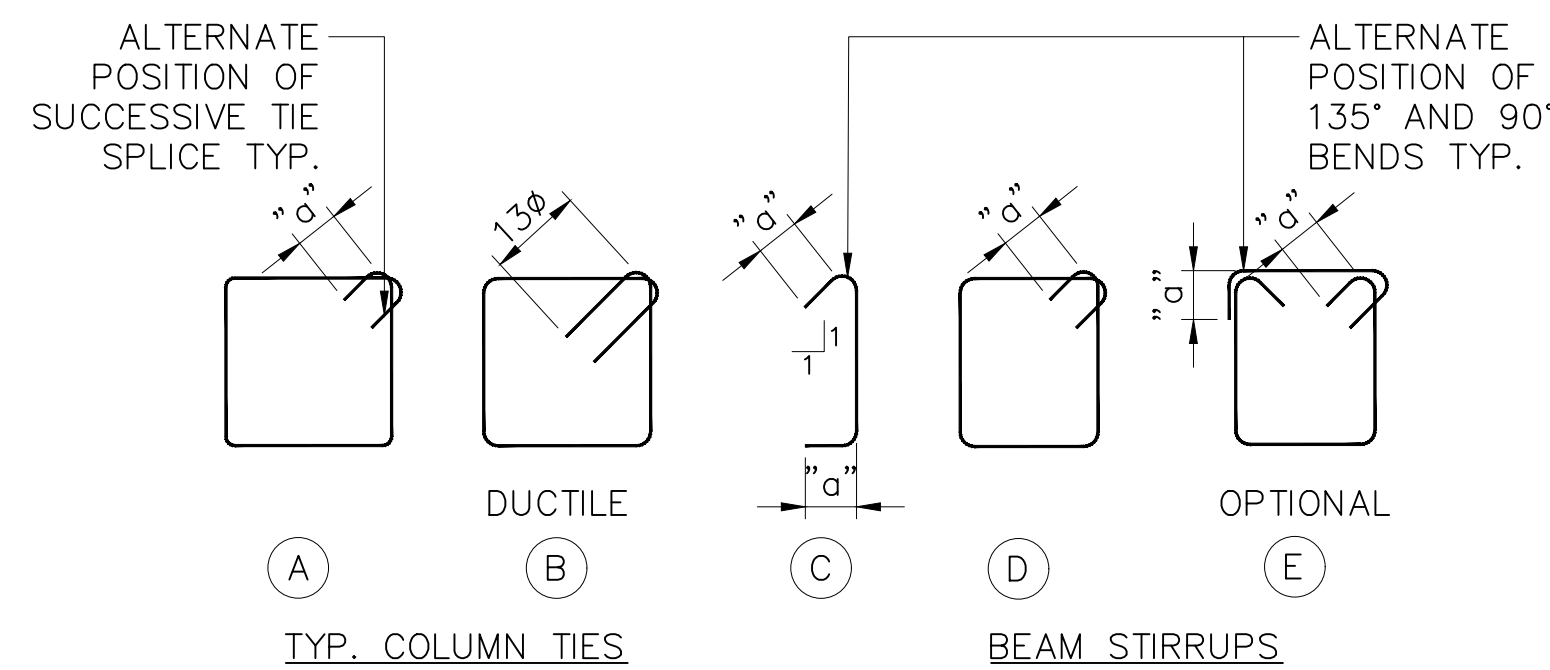
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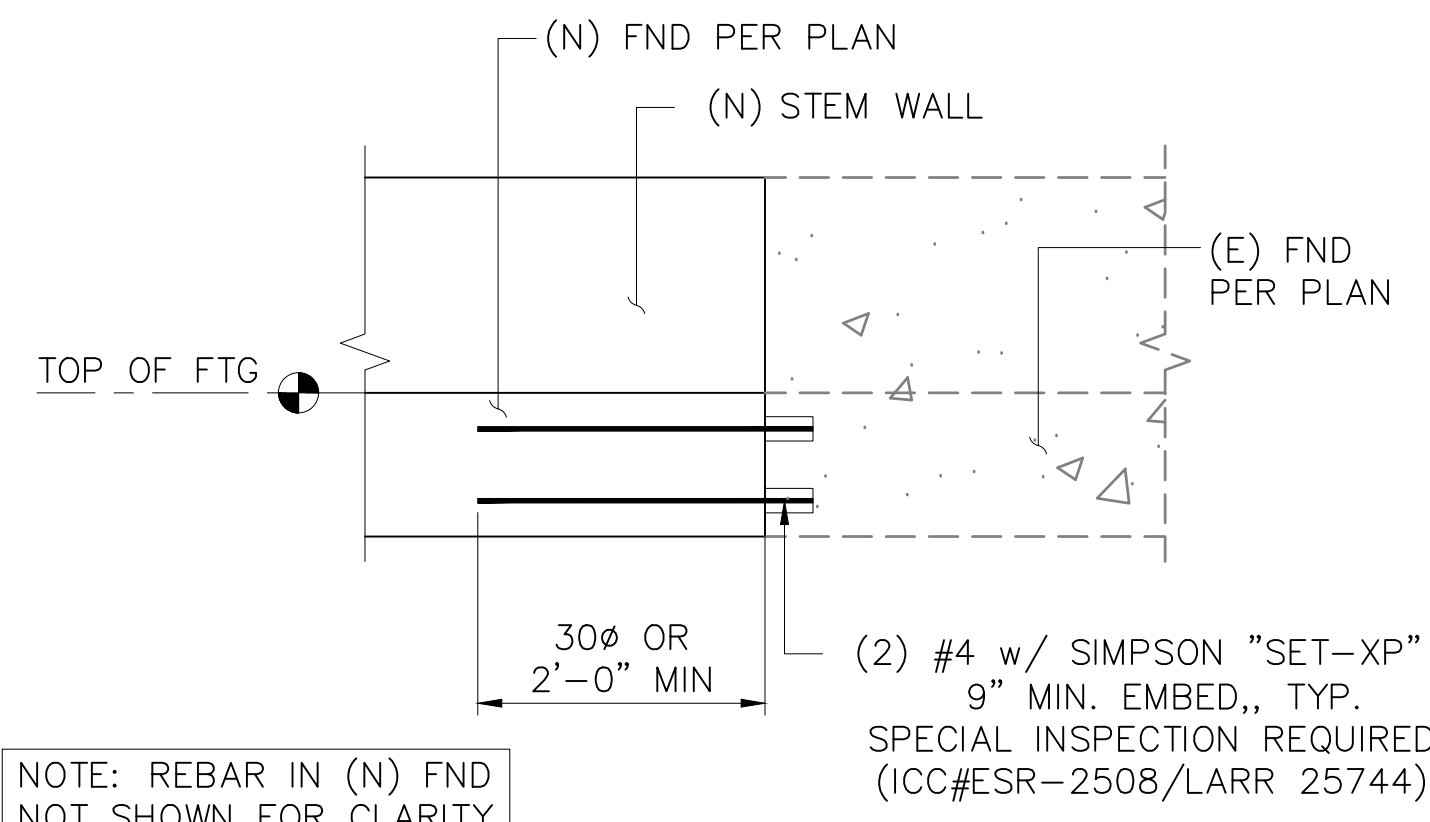


NOTE: KEEP END OF COUPLER PLUGGED TO PREVENT DUST/DIRT UNTIL NEXT PORTION REBAR IS INSTALLED.

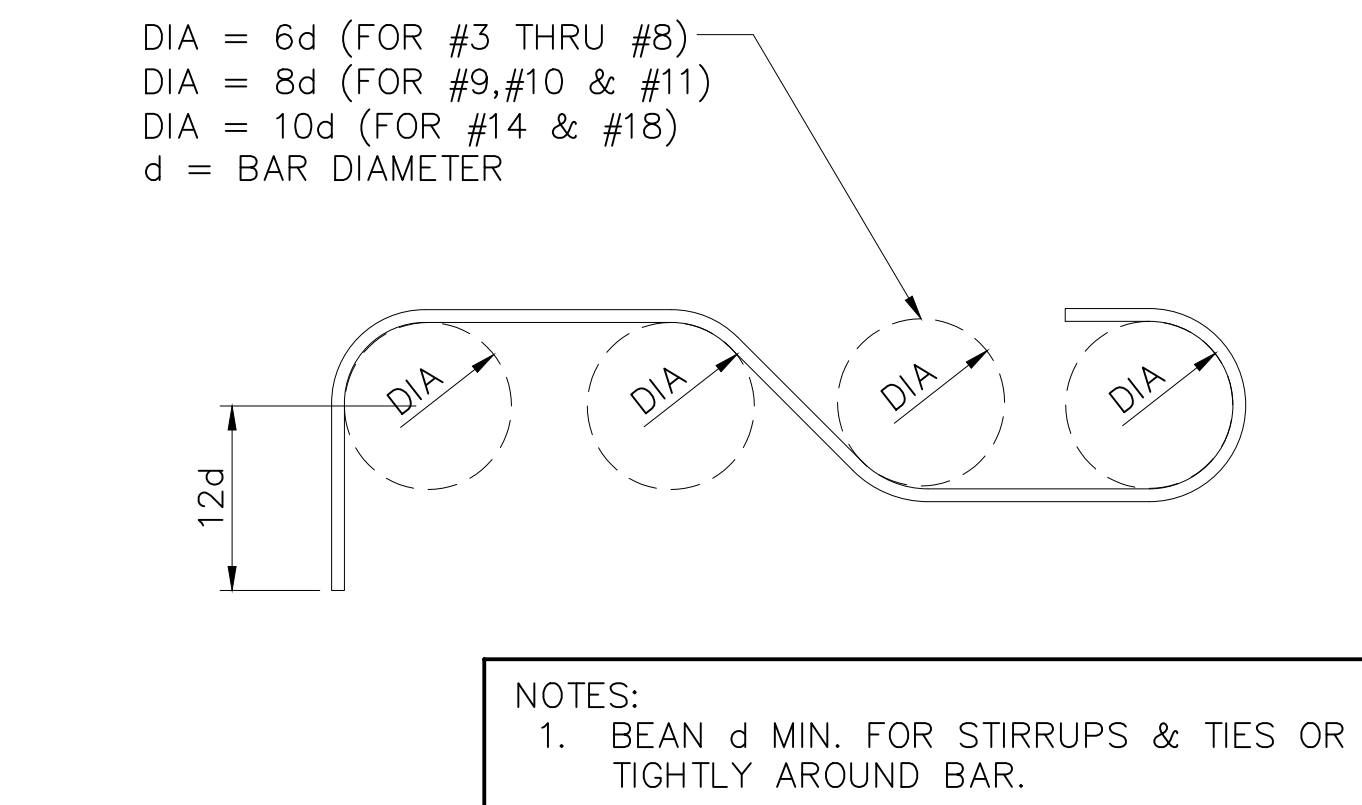


DIMENSIONS "a" FOR #3 TIES = 5"
FOR #4 TIES = 5"
FOR #5 TIES = 6"

NOTE: FOR SIZE AND SPACING OF ALL TIES AND STIRRUPS REFER TO SCHEDULE AND/OR DETAILS TYPICAL.

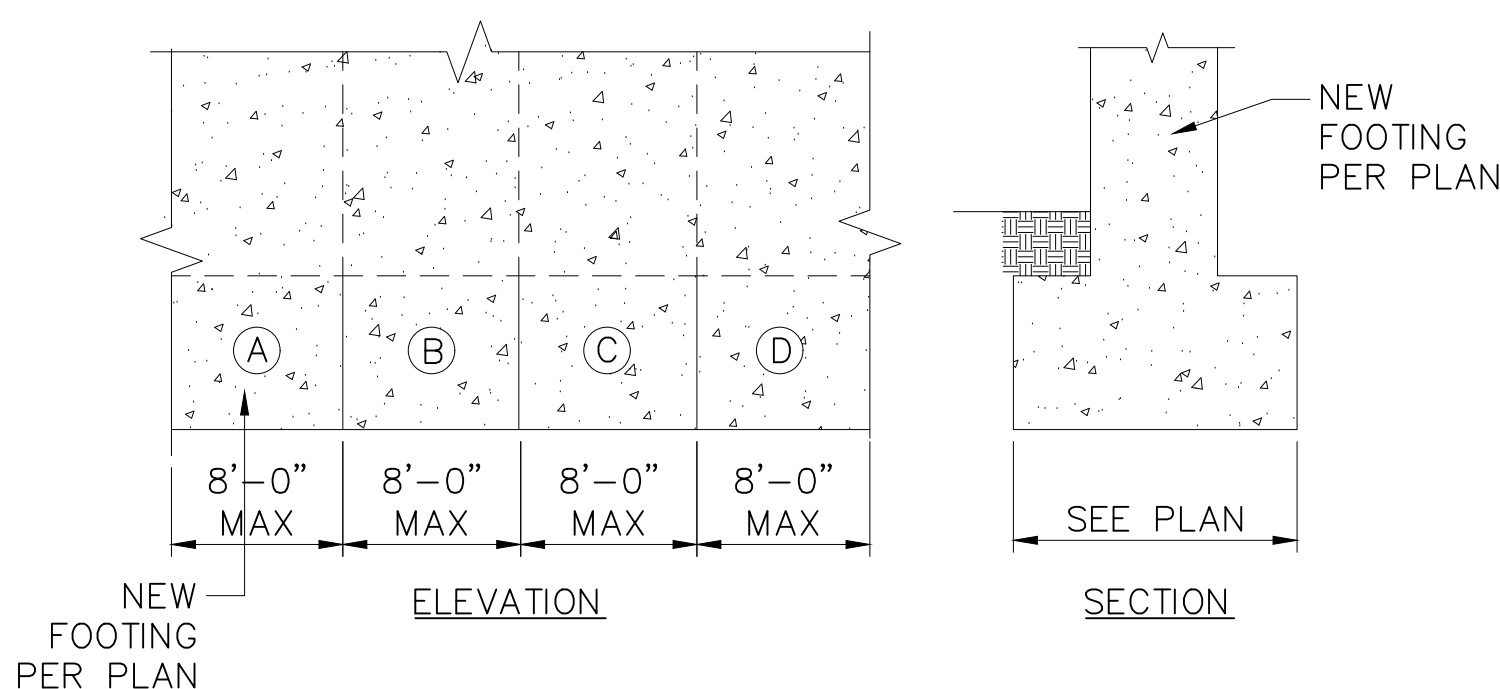


NOTE: REBAR IN (N) FND NOT SHOWN FOR CLARITY



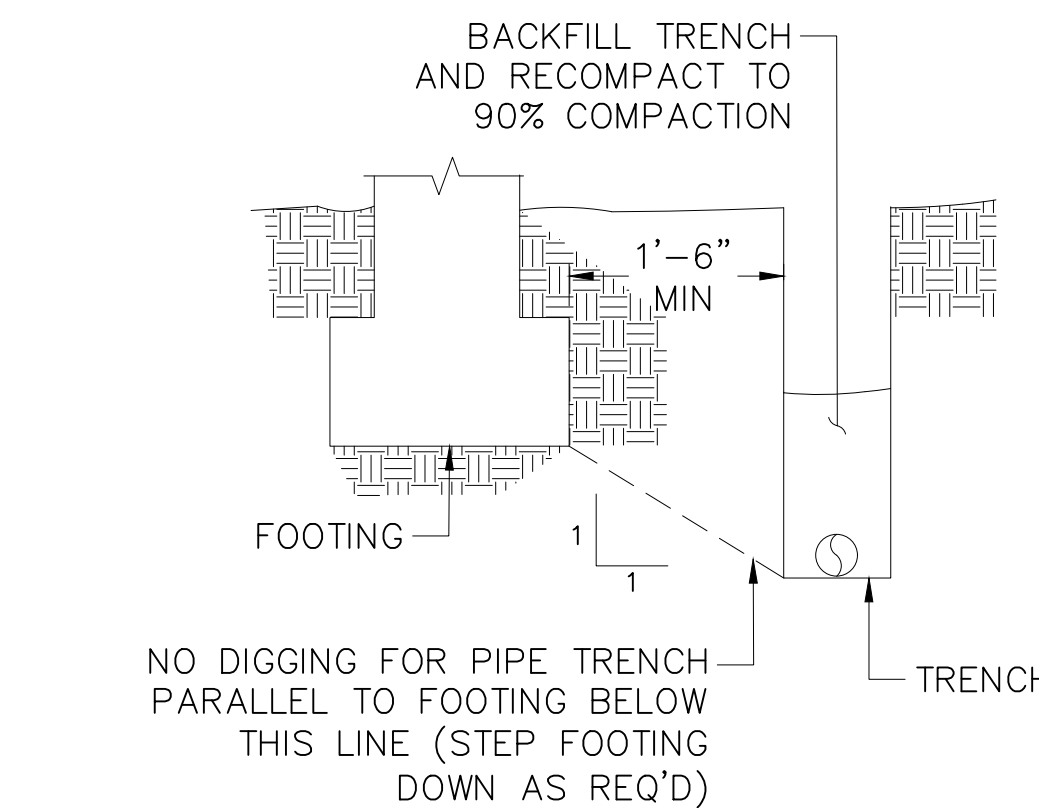
NOTES:
1. BEAN d MIN. FOR STIRRUPS & TIES OR TIGHTLY AROUND BAR.

7 STANDARD REINF. HOOKS AND BENDS Scale: NTS

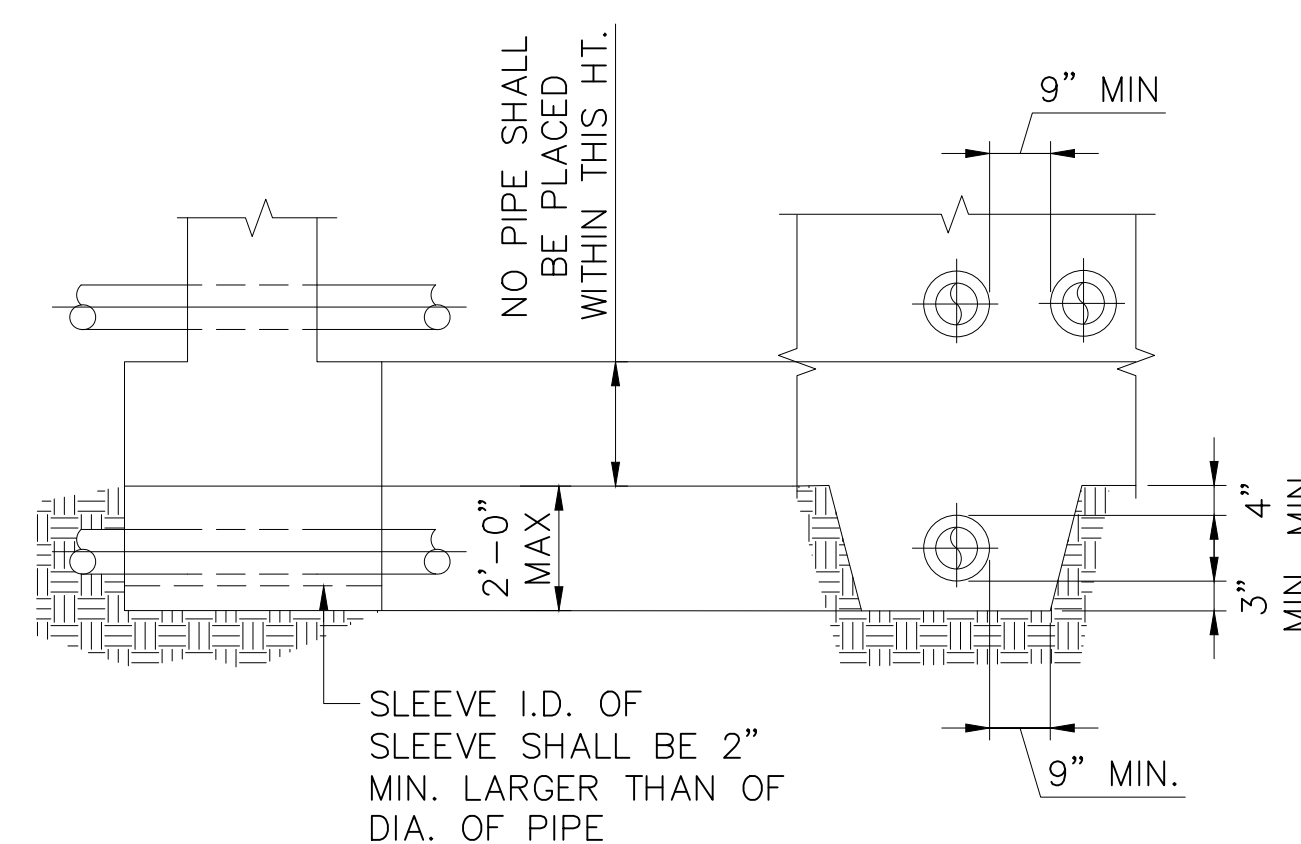


NOTES:
1. SLOT-CUTTING OF THE NEW ADJACENT BUILDING WALLS SHALL BE CONTINUOUS ALONG THE WALLS AND BEAR ON UNDISTURBED EARTH.
2. ALTERNATE UNITS AS FOLLOWS: INSTALL ALL 'A' UNITS. BEFORE PROCEEDING WITH 'B' UNITS. ADJACENT UNIT SHALL NOT BE PLACED WITHIN 4 DAYS OF EACH OTHER.
3. ALL SLOTS-CUTTING IS PER SOILS ENGINEER RECOMMENDATIONS PER APPROVED SOILS REPORT.

8 ABC SLOT-CUTTING DETAIL Scale: NTS

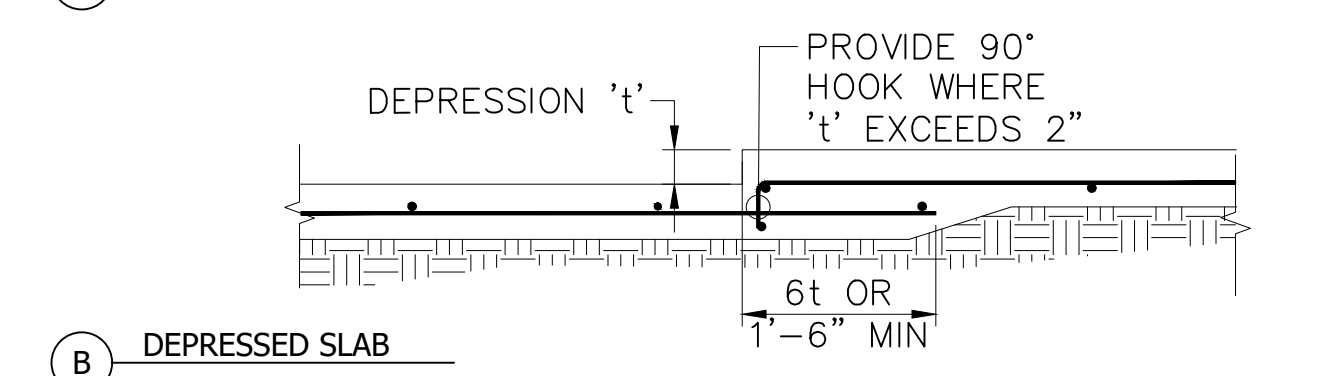
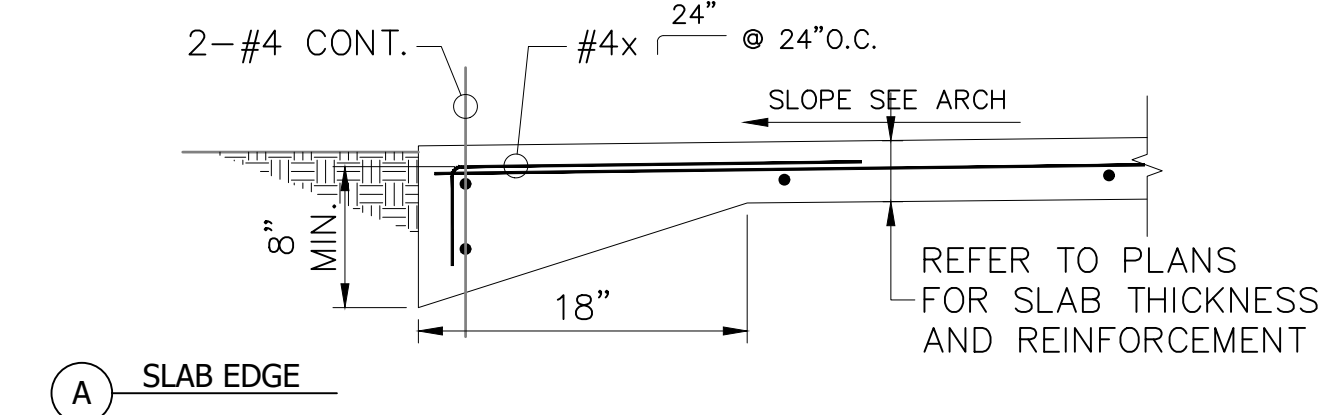


NO DIGGING FOR PIPE TRENCH PARALLEL TO FOOTING BELOW THIS LINE (STEP FOOTING DOWN AS REQ'D)

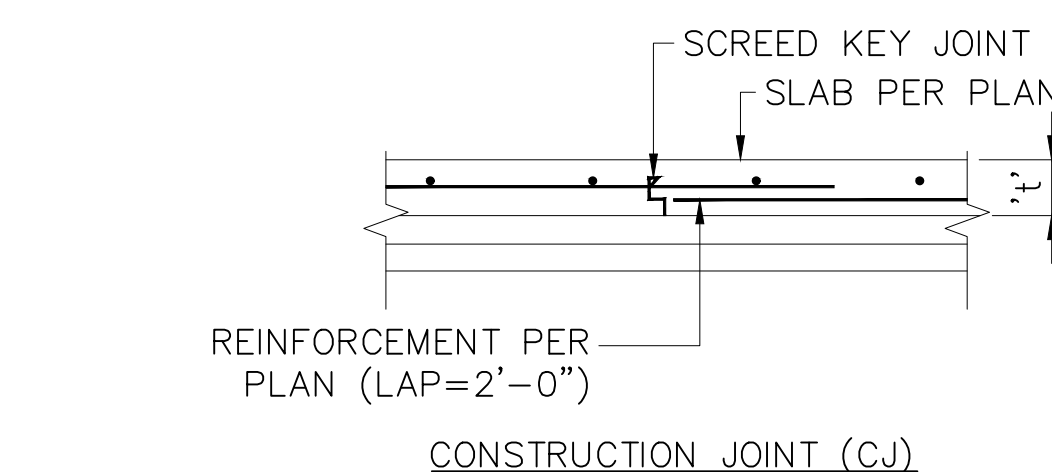
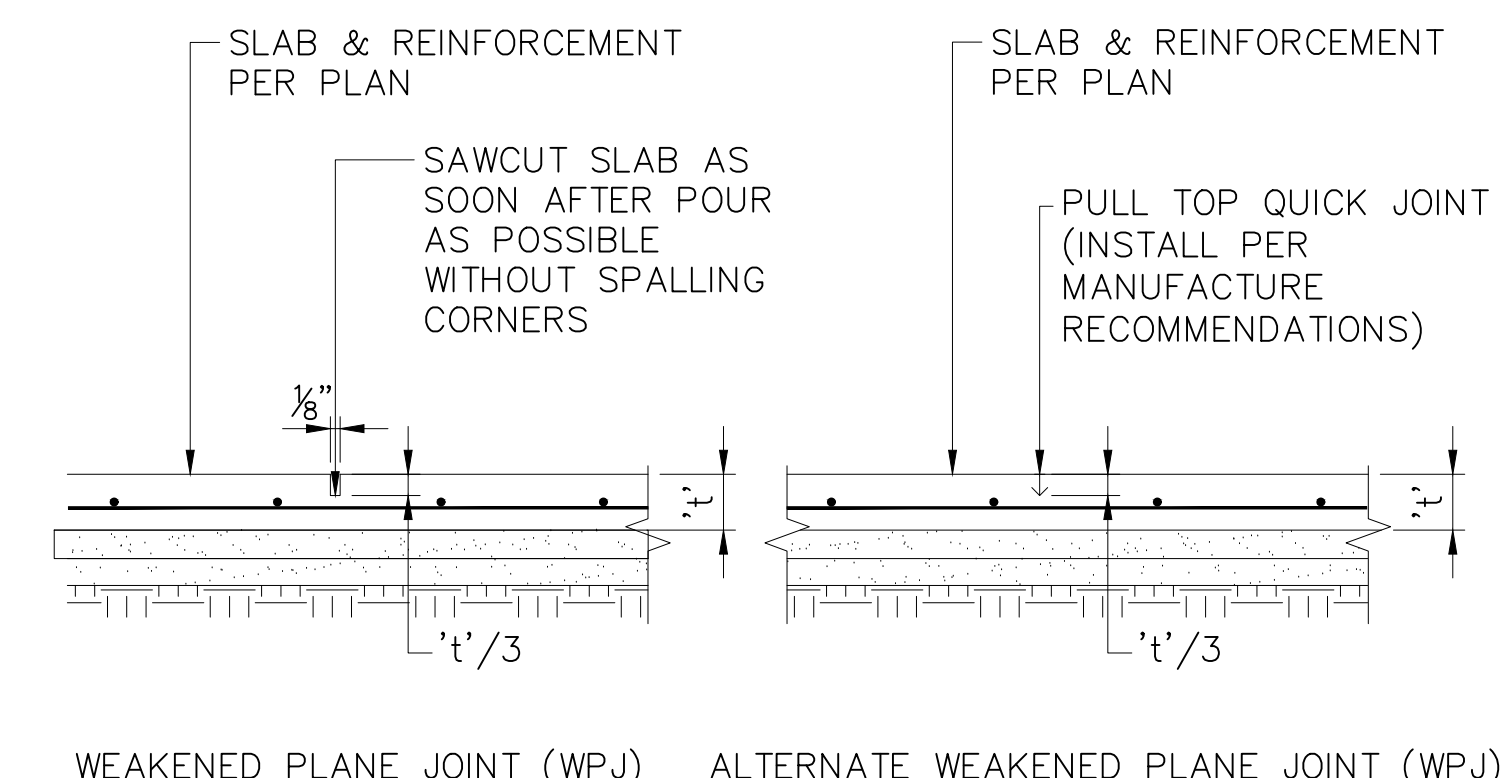


NOTE :
1. WHERE PIPES ARE MORE THAN 2'-0" BELOW BOTTOM OF FOOTING, STEP DOWN PER TYPICAL FOOTING STEP DETAIL.
2. PLACE CONCRETE FILL AROUND SLEEVES BEFORE POURING FOOTINGS AND/OR PADS. FILL SHALL BE THE SAME WIDTH AS FOOTING AND FULL DEPTH OF PIPE TRENCH.
3. THIS DETAIL APPLIES ALL CONT. FOOTINGS AND PAD FOOTING

5 TYP. CONT. FTG/PAD FTG @ PIPE/CONDUIT Scale: NTS



1 TYP. DEPRESSED SLAB AND SLAB EDGE Scale: NTS

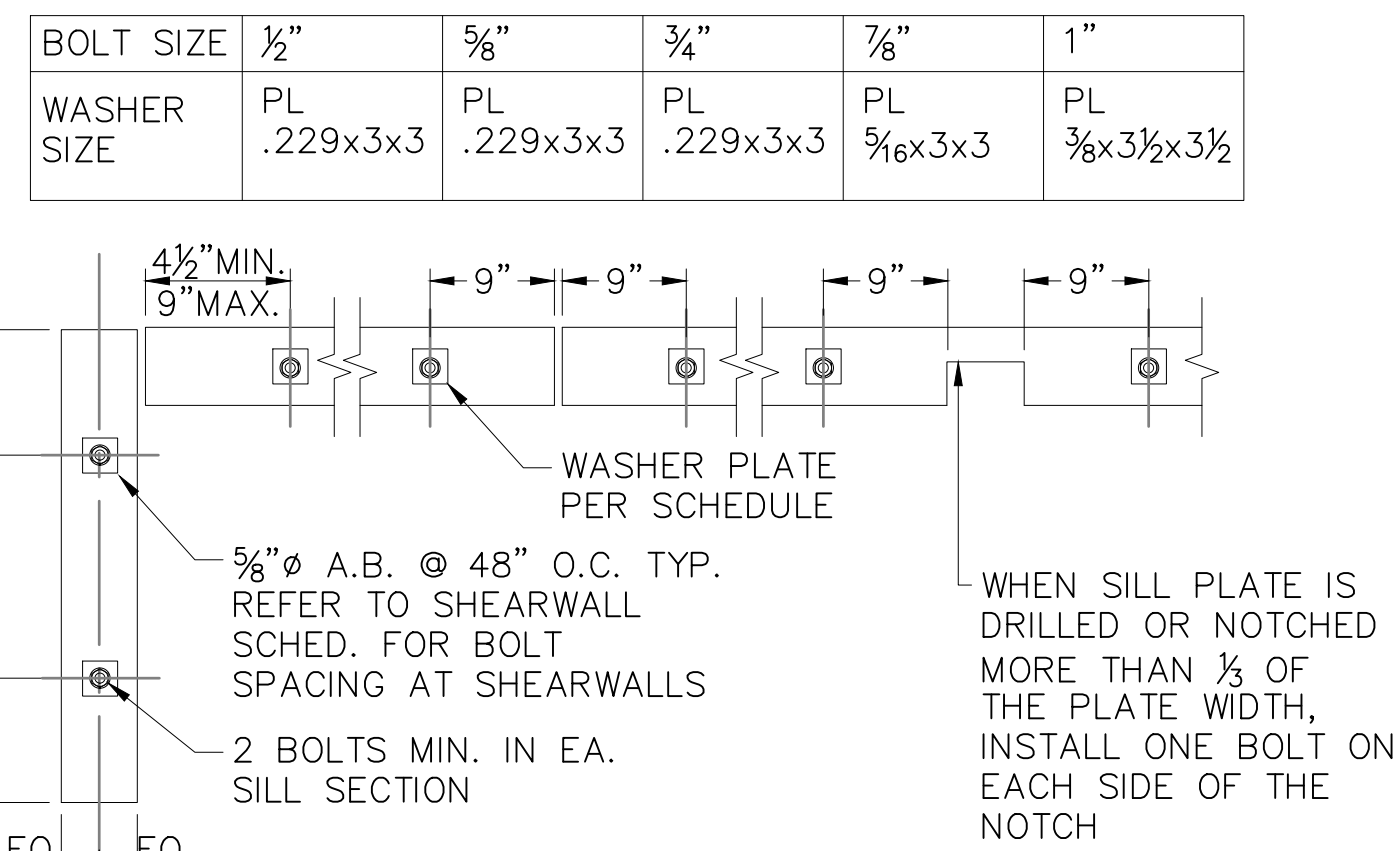


NOTES:
1) JOINT SPACING SHALL NOT EXCEED 24 x THE SLAB THICKNESS IN BOTH DIRECTIONS PANELS BOUNDED BY SAW CUTS SHALL BE AS CLOSE TO SQUARE AS POSSIBLE, WITH A "1½: 1" MAX. ASPECT RATIO.
2) CURING COMPOUND SHALL BE APPLIED IN TWO COATS IN OPPOSITE DIRECTIONS WITH AN EXTRA APPLICATION ON ALL SAW CUT JOINTS COMPOUND TO BE APPLIED WITHIN TWO HOUR OF FINISHING OPERATIONS.
3) UNDERGROUND UTILITIES, PIPING, CONDUIT, ETC. SHALL BE PLACED BELOW THE SAND PIPING AND CONDUITS SHALL NOT BE PLACE IN THE PLANE OF THE SLAB.

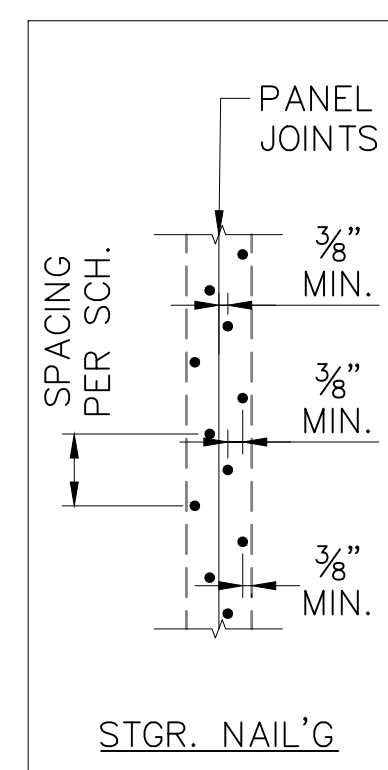
2 TYP. SLAB-ON-GRADE JOINTS DETAIL Scale: NTS

TYPICAL DETAILS

22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120



4 TYP. SILL PLATE DETAIL



FOR TYPE 3 & 4 SHEAR WALLS (DOUBLE SIDE SHEATHING):

1. SHEATHING ON ONE SIDE OF THE SHEAR WALL MUST BE NAILED COMPLETELY BEFORE THE FRAME INSPECTION. SHEATHING ON THE OTHER SIDE MUST BE INSTALLED AND INSPECTED PRIOR TO INSTALLATION OF WALL SURFACE COVERING.
2. SEE HOLDOWN DETAIL FOR ADDITIONAL HD POST SIZE REQUIREMENTS..

BOLT DIA. 'D'	$\frac{1}{2}" \phi$	$\frac{5}{8}" \phi$	$\frac{3}{4}" \phi$	$\frac{7}{8}" \phi$	$1" \phi$	$1\frac{1}{8}" \phi$
MIN. EMBED. VERT. ANCHOR BOLTS	7"	7"	7"	8"	9"	10"
MIN. EMBED. HORZ. ANCHOR BOLTS	5"	5"	5"	6"	7"	8"

NOTE:
USE 'J' BOLTS U.O.N.
ON PLANS/DETAILS

A PLAN VIEW (SLAB-ON-GRADE)

STUB POST
MATCH & ALIGN
POST ABOVE

PLATE WASHER
P 3x3x $\frac{3}{8}$ "

COUPLER NUT

SCHEDULE NOTES:

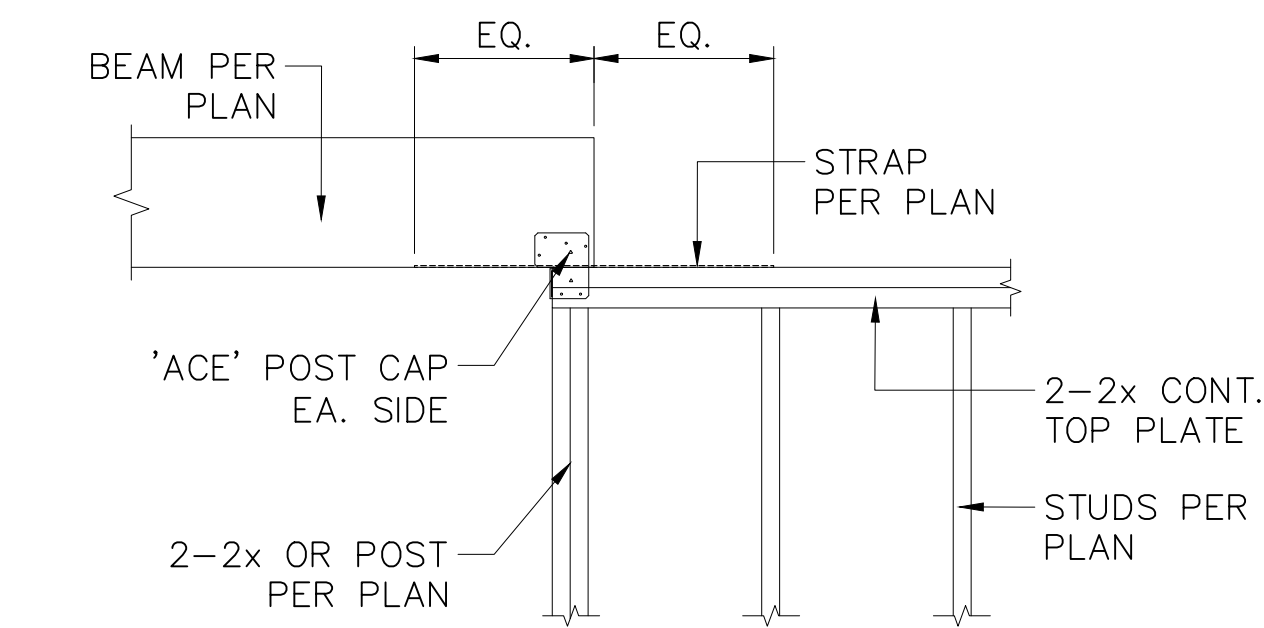
1. ALL HOLDOWN ANCHOR BOLTS SHALL BE SECURELY FASTENED IN PLACE PRIOR TO PLACING CONCRETE.
2. PROVIDE 3" COVER AT EARTH FORMED SURFACES, AT ALL HD. A.B.'S.
3. BASED ON LARR #5720 VALUES REDUCED BY 25

C SECTION VIEW (RAISED FLOOR)

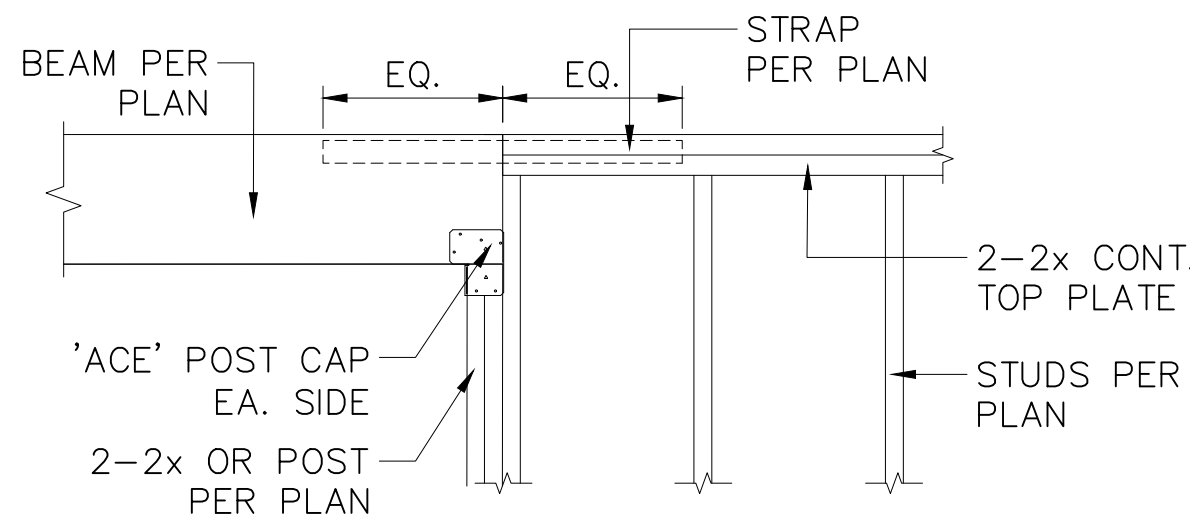
SCHEDULE NOTES:

1. ALL HOLDOWN ANCHOR BOLTS SHALL BE SECURELY FASTENED IN PLACE PRIOR TO PLACING CONCRETE.
2. PROVIDE 3" COVER AT EARTH FORMED SURFACES, AT ALL HD. A.B.'S.
3. BASED ON LARR #25720 VALUES REDUCED BY 25 PERCENT.
4. HD ANCHOR BOLT TO BE @CENTER LINE OF STUD WALL.

1 TYP. HOLDOWN @ FOUNDATION



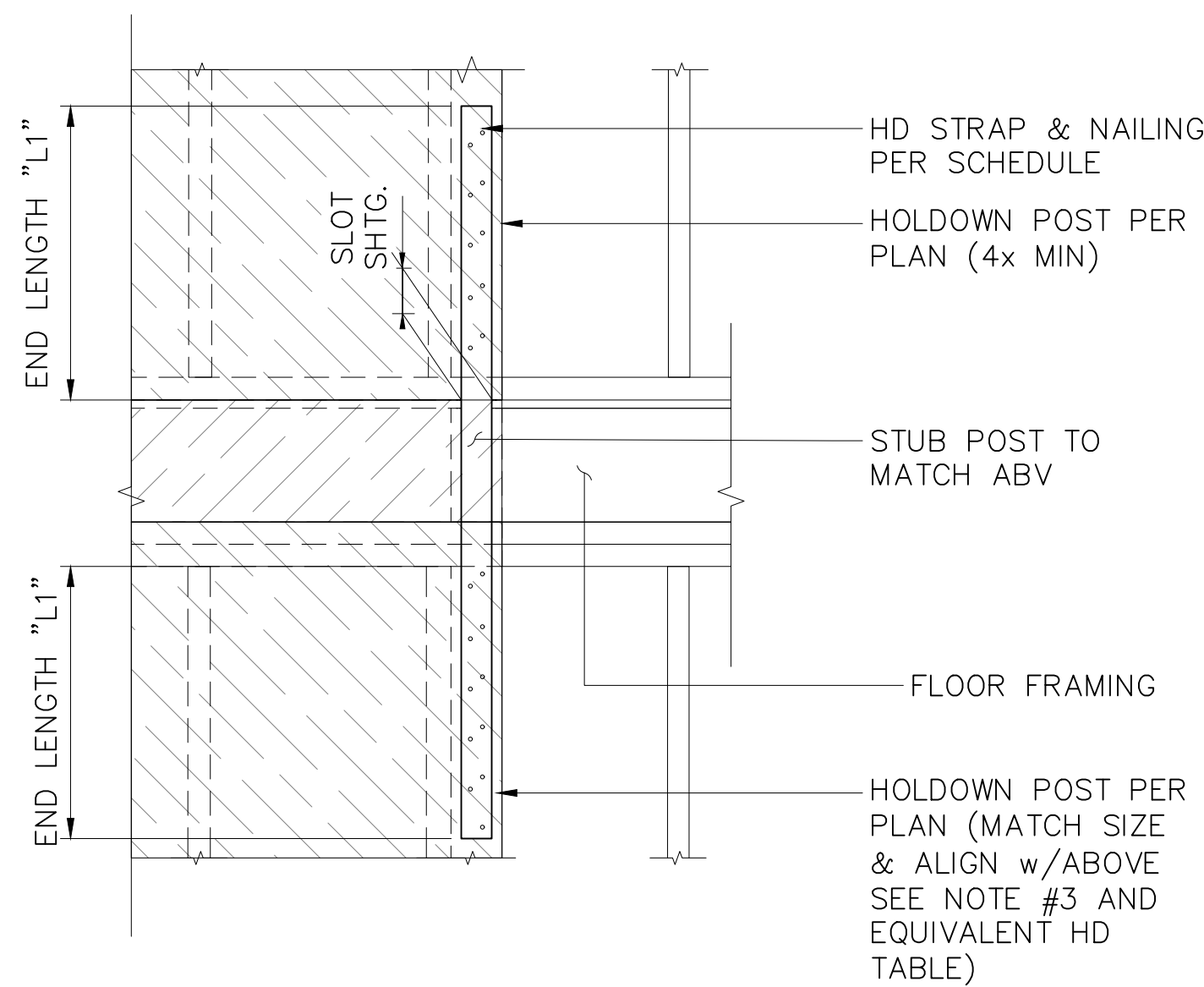
A BM ABOVE WALL TOP PLATES



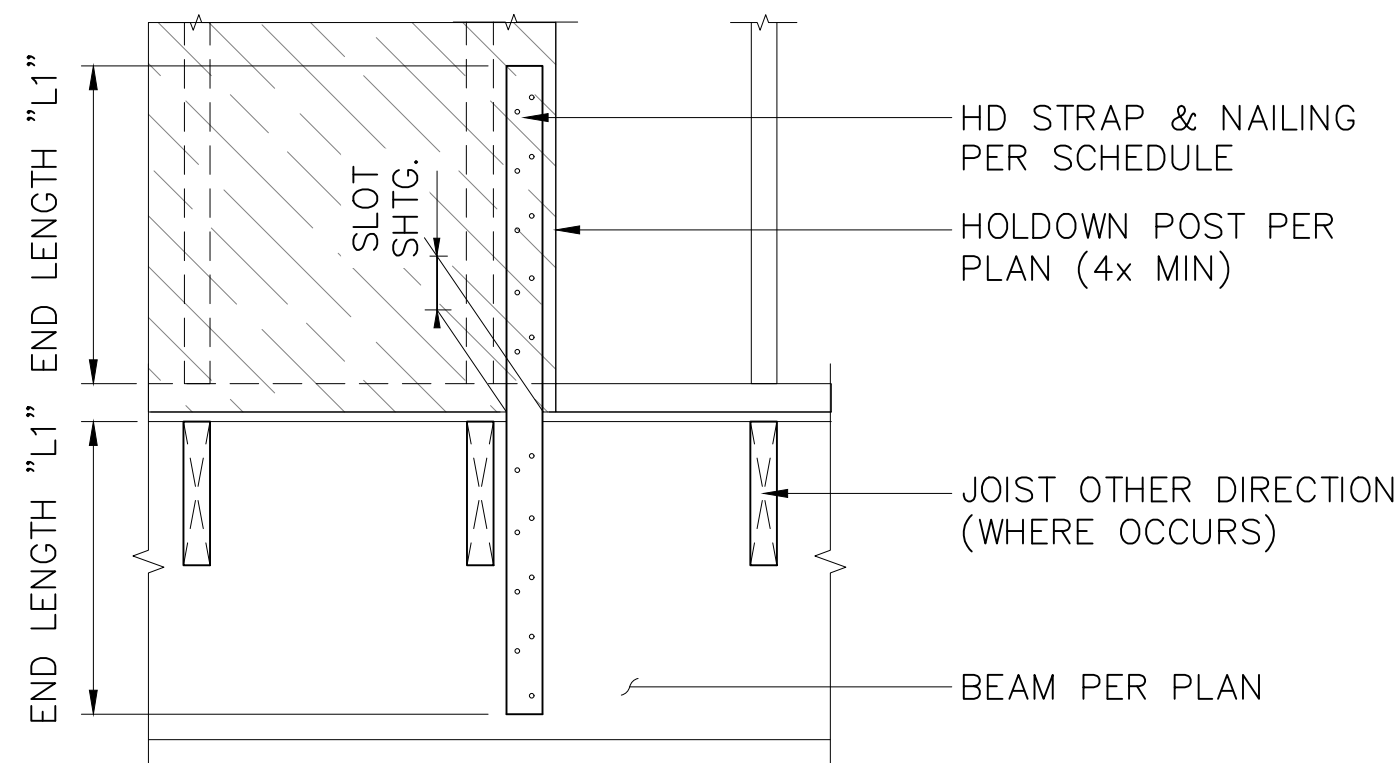
B BM FLUSH w/ WALL TOP PLATES

6 TYPICAL DRAG DETAIL @ OPENING

Scale: NTS



A TYPICAL STRAP HOLDOWN

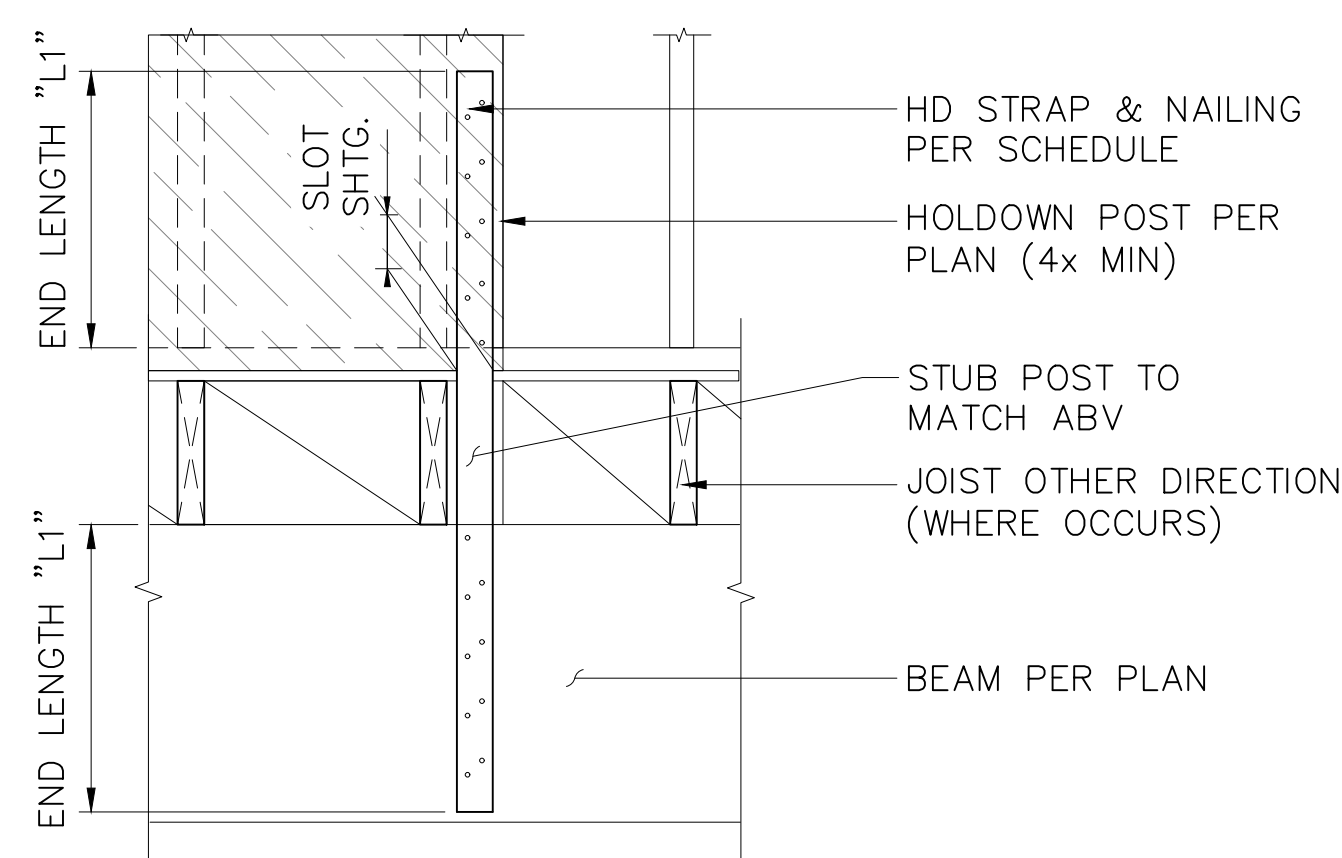


B TYPICAL STRAP HOLDOWN TO BEAM

NOTE: IF END LENGTH "L1" IS GRATER THAN BEAM DEPTH. CAREFULLY BEND STRAP AROUND BOTTOM OF BEAM AND NAIL

STRAP HOLDOWN SCHEDULE (PER SIMPSON C-C-2017, INSTALLATION SEE DETAIL 7/ S1.2)					
STRAP TYPE	EQUIVALENT HD @ FND	END LENGTH "L1"	NAILS REQ. WITHIN END LENGTH "L1"	ALLOW. TENSION (KIPS)	L.A. TENSION (KIPS)
CS18	HDU2	10"	(8) 10d	1.37	1.03
CS16	HDU2	12"	(10) 10d	1.71	1.27
(2) CS18	HDU2	10"	(16) 10d	2.74	2.05
(2) CS16	HDU4	16"	(20) 10d	3.41	2.58
CMST16	HDU5	20"	(25) 16d SINKERS	4.59	3.44
CMST14	HDU8	30"	(33) 16d	6.49	4.86
CMST12	HDU11	39"	(43) 16d	9.22	6.92
(2) MST60	HDU14	N/A	(68) 16d	13.46	10.10

* USE 2½" LONG NAIL MINIMUM FOR STRAP HOLDOWN

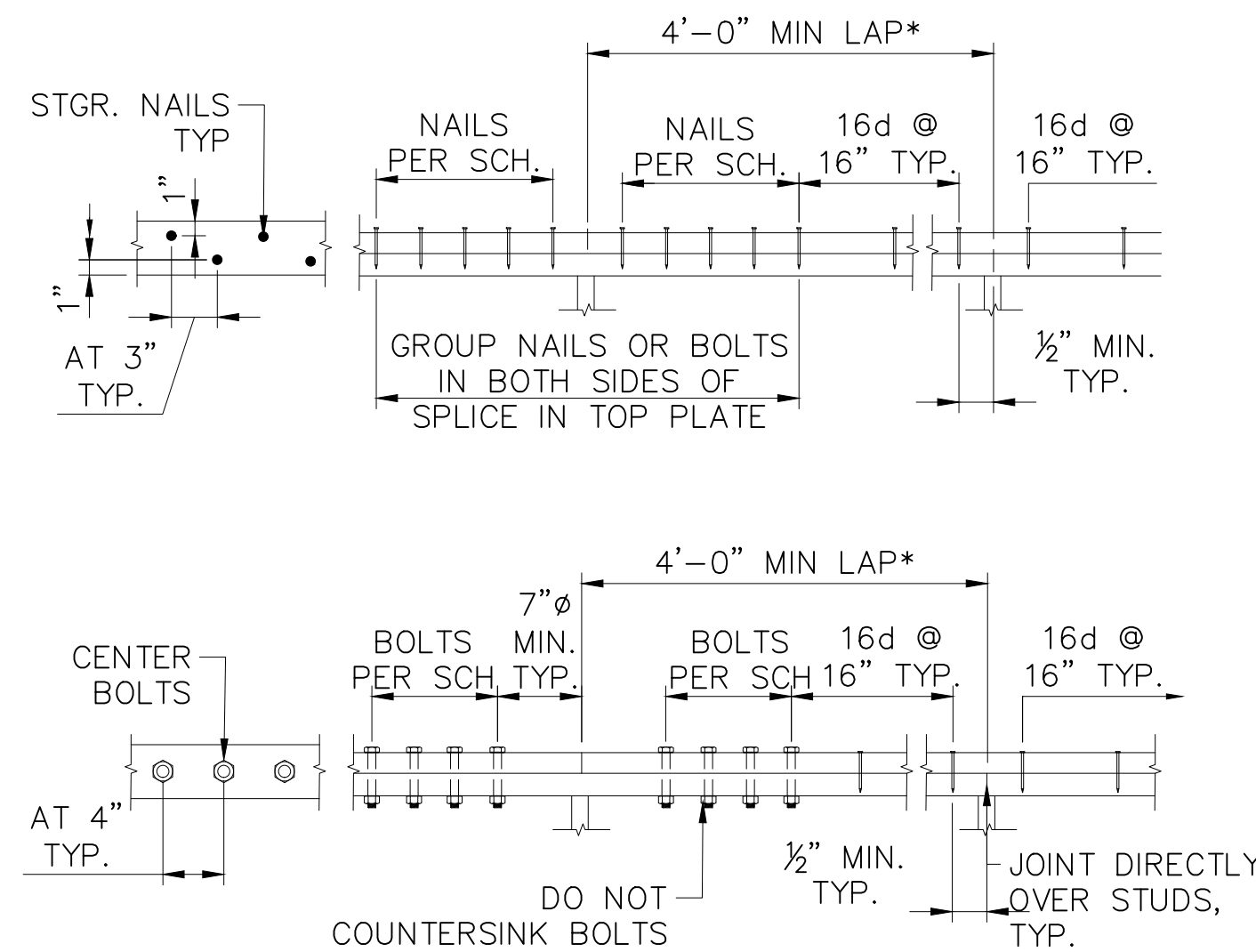


C TYPICAL STRAP HOLDOWN TO DROP BEAM

NOTE :
1. BASED ON LARR #25713 AND ICC #ESR 2105.
2. HOLDOWN IS REQ'D AT FOUNDATION EA. FLOOR TO FLOOR STRAP/POST.
3. SEE EQUIVALENT HD IN TABLE FOR HD'S TO FOUNDATION
4. INSTALL STRAP OVER SHEATHING. SHEATHING EDGE NAILING REQ'D IN ADDITION TO STRAP NAILING.

7 TYPICAL STRAP HOLDOWN DETAILS

Scale: NTS



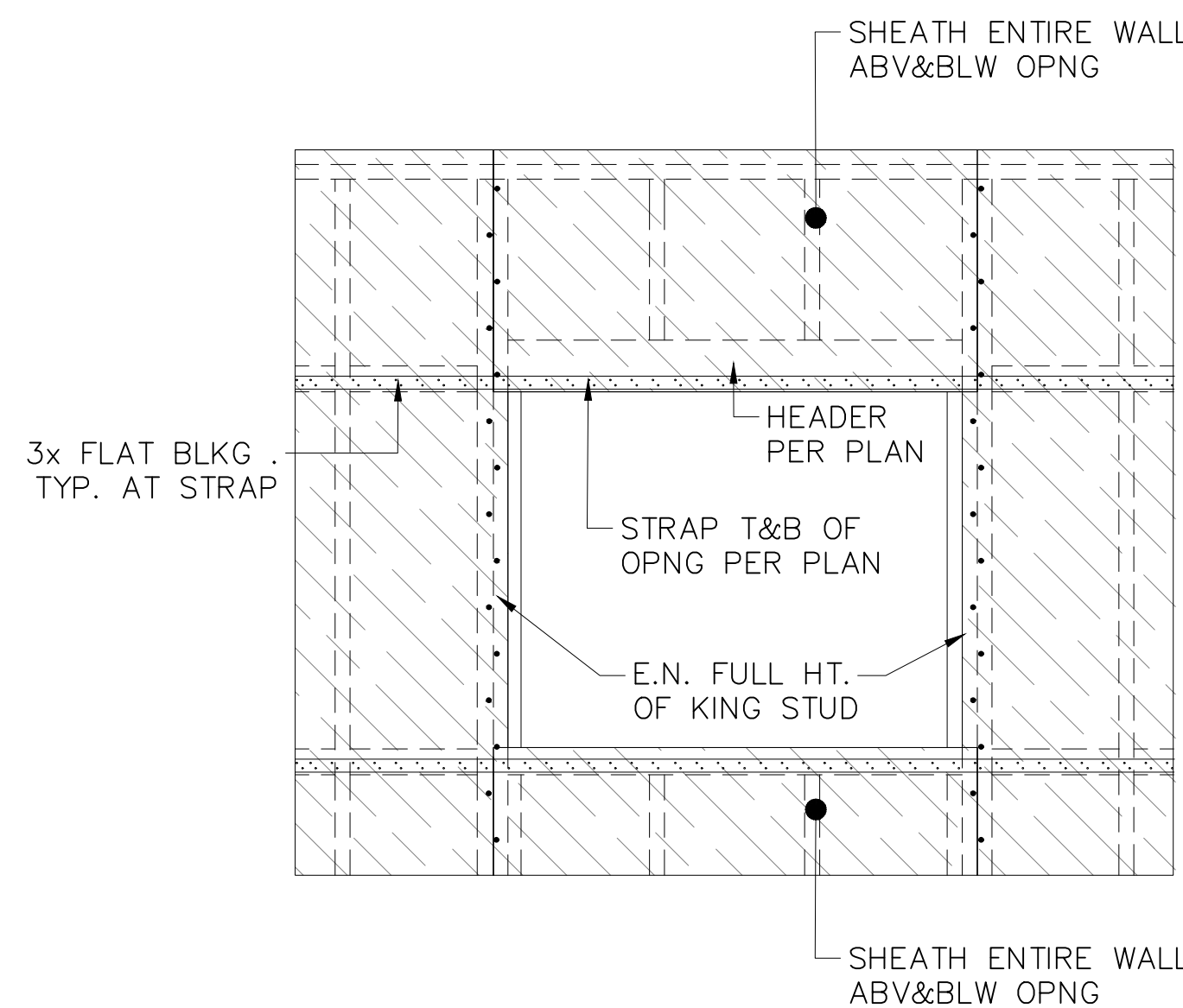
MARK	MIN.PL THK	NAILS	DESIGN LOAD (LB)	MARK	MIN.PL THK	BOLTS	DESIGN LOAD (LB)
(6)	2x	(6)16d	1,250	(22)*	2x	(22)16d	3,780/4,600
(8)	2x	(8)16d	1,670	(24)*	2x	(24)16d	3,780/5,010
(10)	2x	(10)16d	2,090	(B1)	2x	(6)½"φ	3,780/5,800
(12)	2x	(12)16d	2,510	(B2)	2x	(8)½"φ	3,780/7,590
(14)	2x	(14)16d	2,920	(B3)	2x	(8)¾"φ	3,780/7,590
(16)	2x	(16)16d	3,340	(B4)	3x6	(6)¾"φ	11,600
(18)	2x	(18)16d	3,760	(B5)	3x6	(8)¾"φ	12,650
(20)*	2x	(20)16d	3,780/4,180				

* INDICATE 6'-0" MIN LAP LENGTH @ SPLICE

DESIGN LOAD = x4 PLATE/x6 PLATE

4 TOP PLATE SPLICE DESIGN TABLE

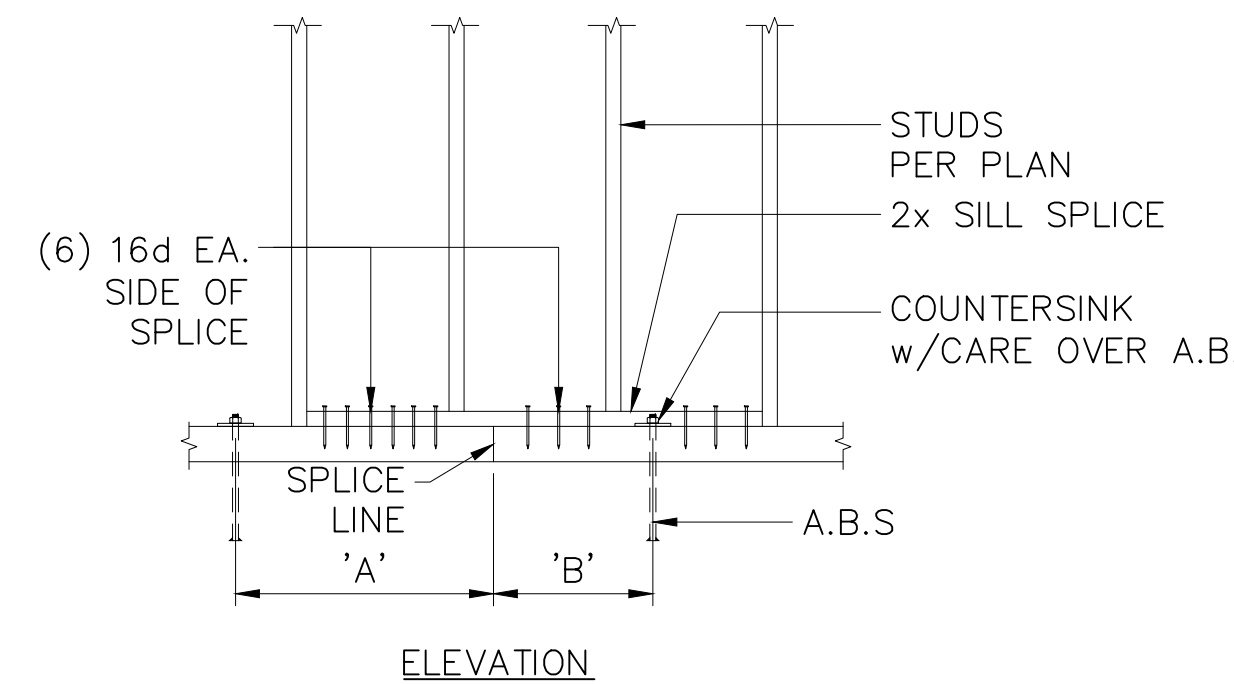
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NOTE:
1. INSTALL STRAP OVER SHEATHING.
2. E.N. ON TOP PLATES NOT SHOWN FOR CLARITY

5 TYP STRAP @ SHEAR WALL OPNG

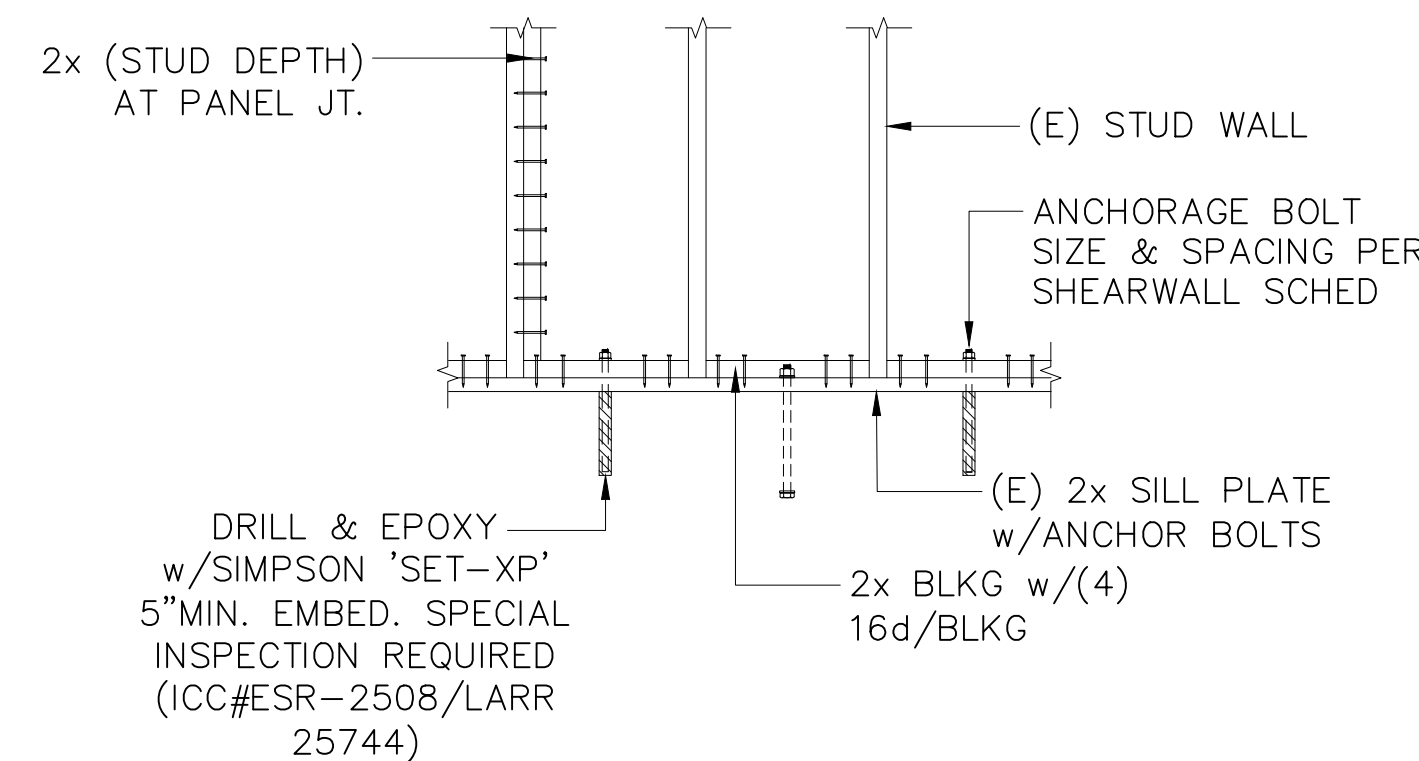
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NOTE: THIS DETAIL IS REQUIRED WHEN EITHER 'A' OR 'B' IS GREATER THAN 9".

1 SPECIAL COND. AT SILL PLATE SPLICE

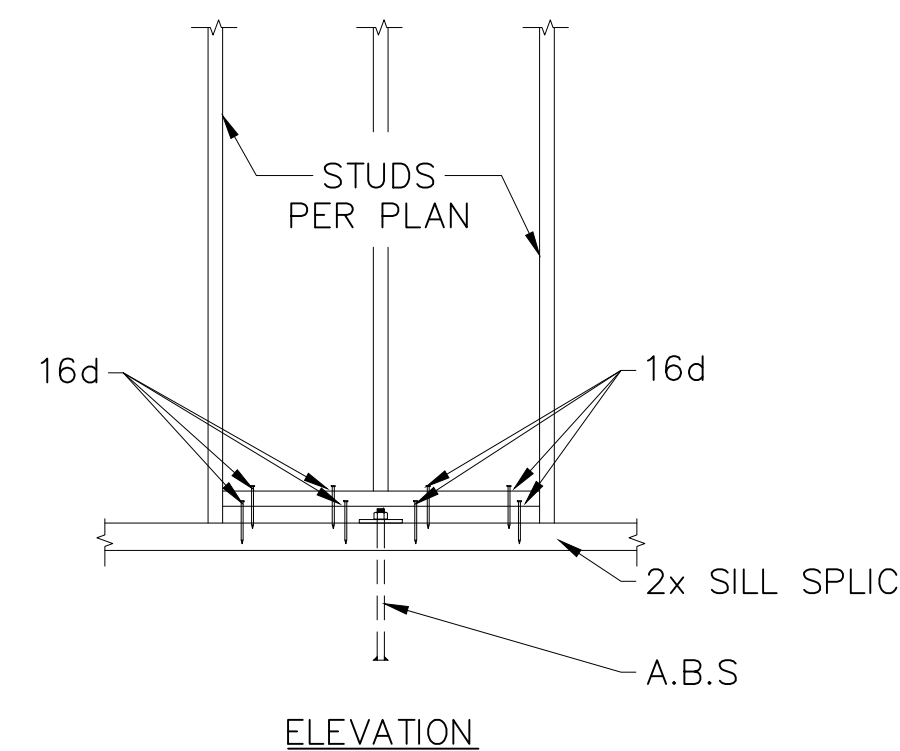
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NOTES: THIS DETAIL TO BE USED ONLY WHERE 3x SILL IS REQD. AS INDICATED ON SHEARWALL DETAILS AND SCHEDULE.

2 TYP. SILL PLATE REINFORCEMENT

Scale: NTS



3 TYP. STUD OCCURS OVER SILL BOLT

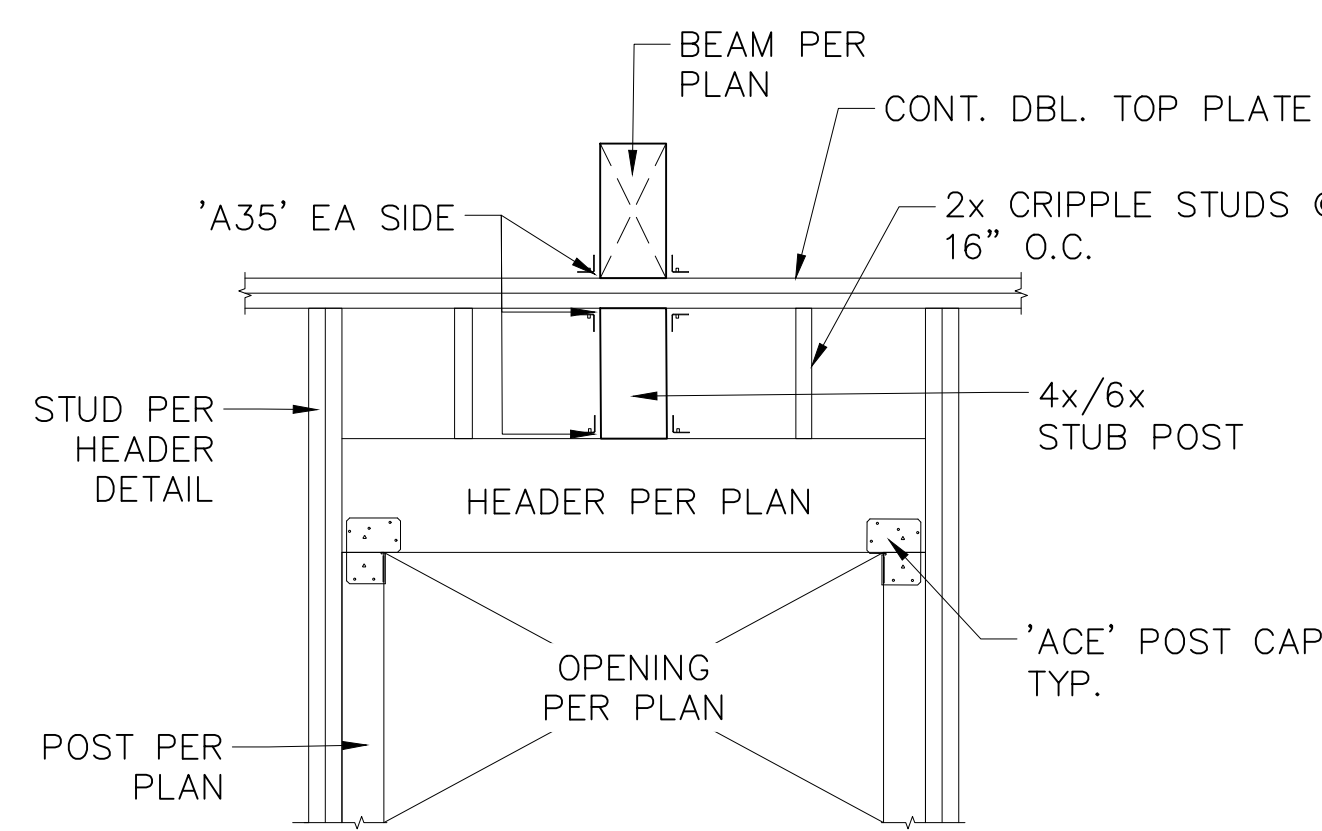
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TYPICAL DETAILS

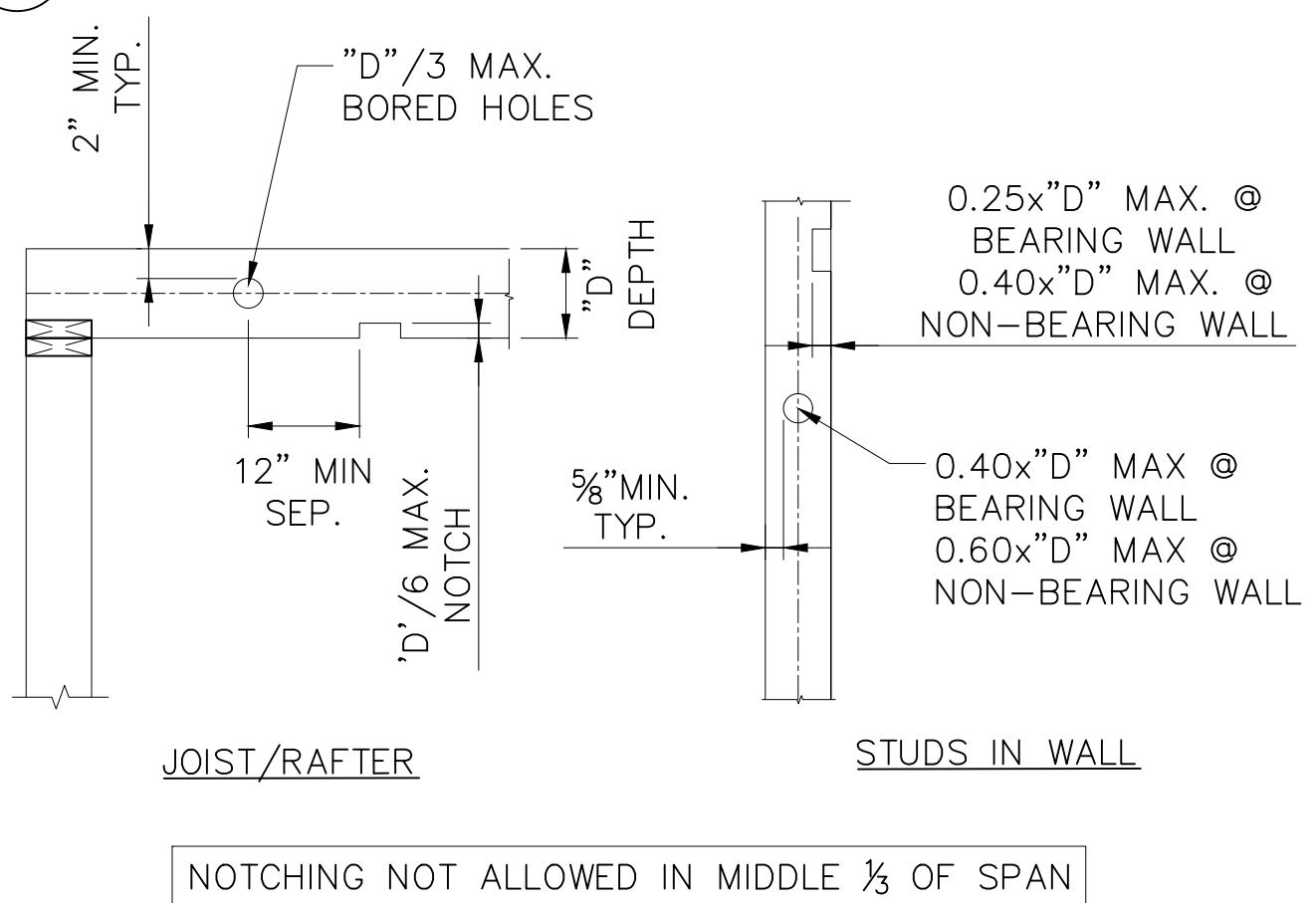
22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120

DATE: 12.17.2019
SCALE: AS SHOWN
DESIGN BY: AZ
DRAWN BY: AZ
REVIEWED BY: SP
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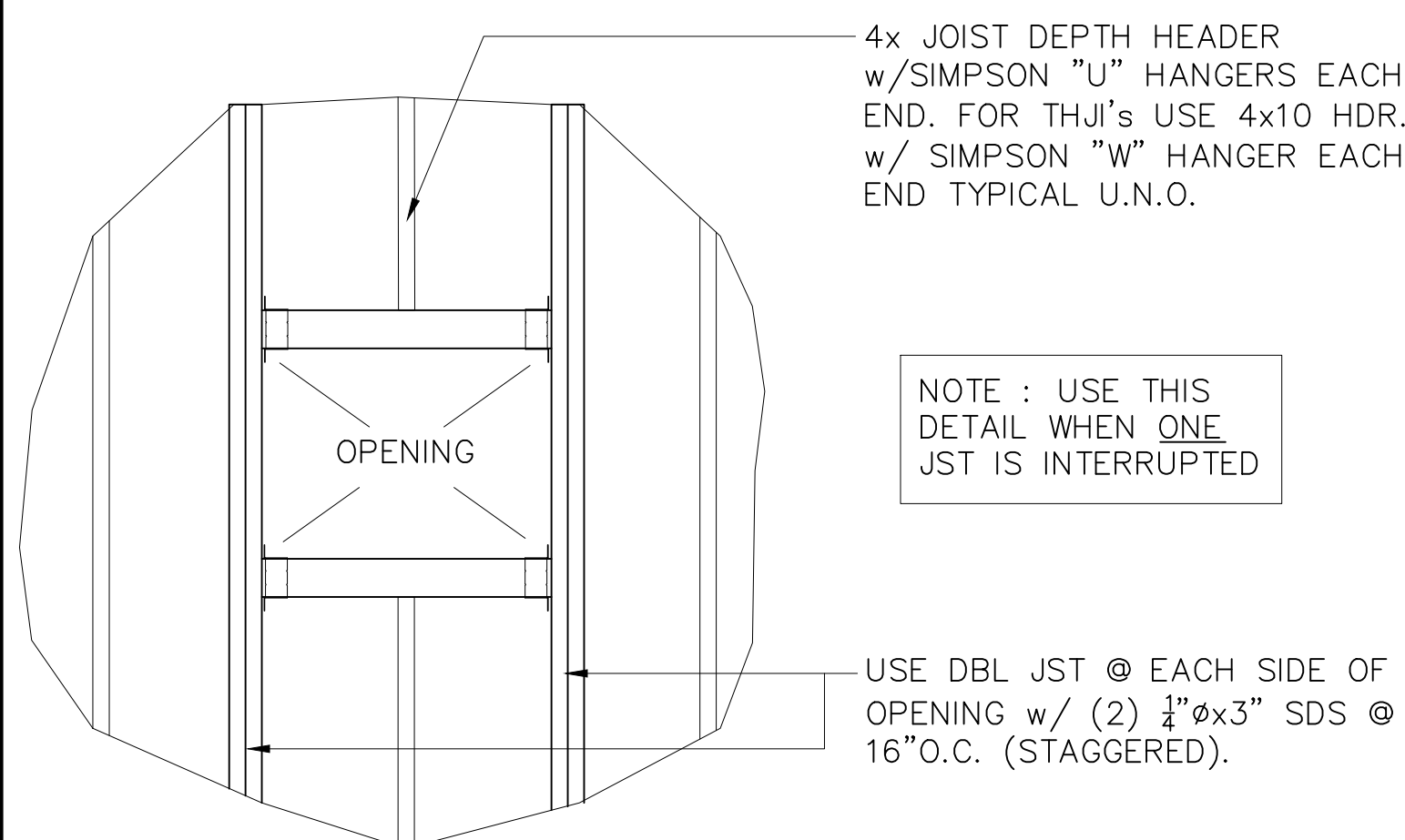
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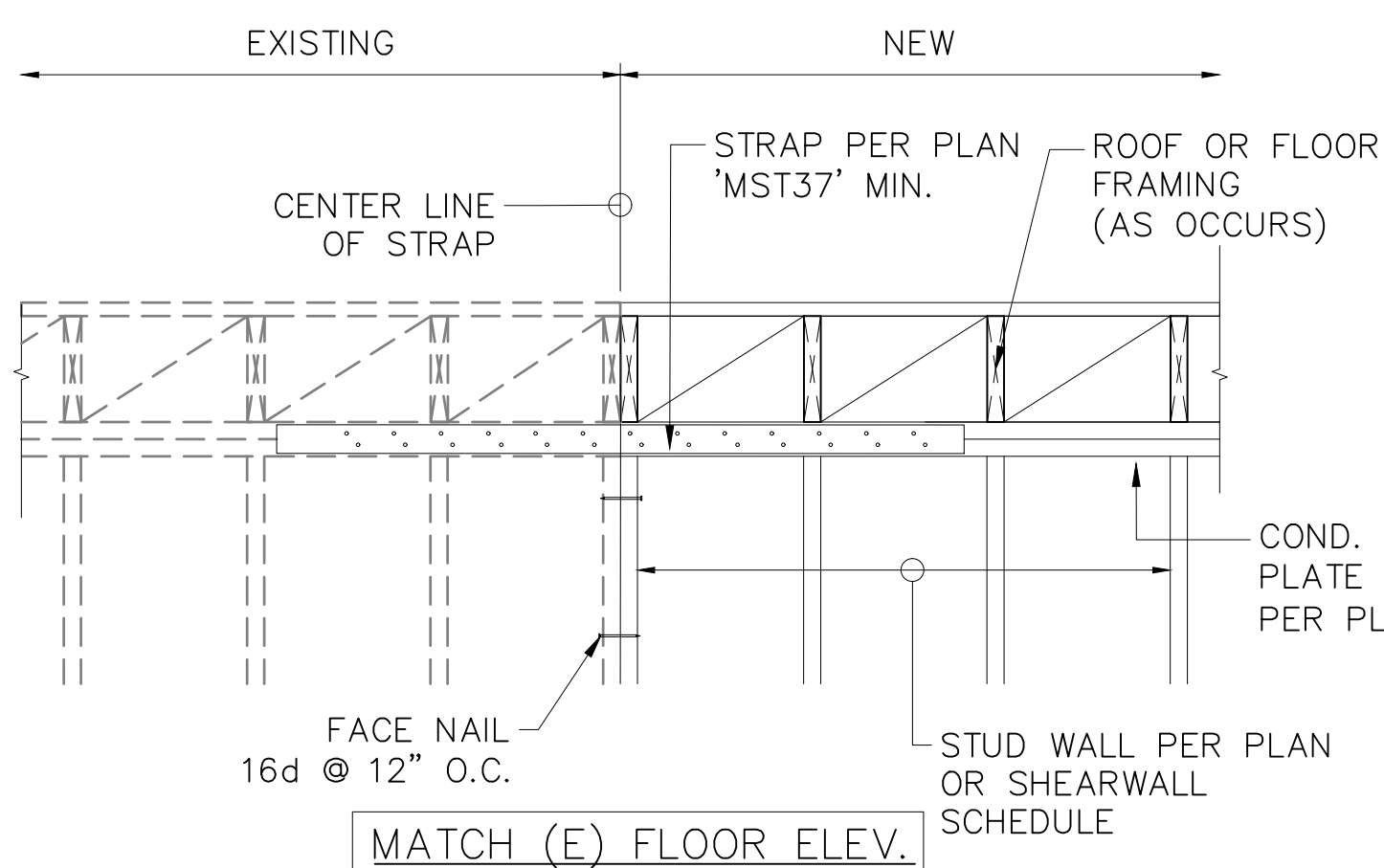
6 TYP. GRAVITY BEAM @ OPENING Scale: NTS



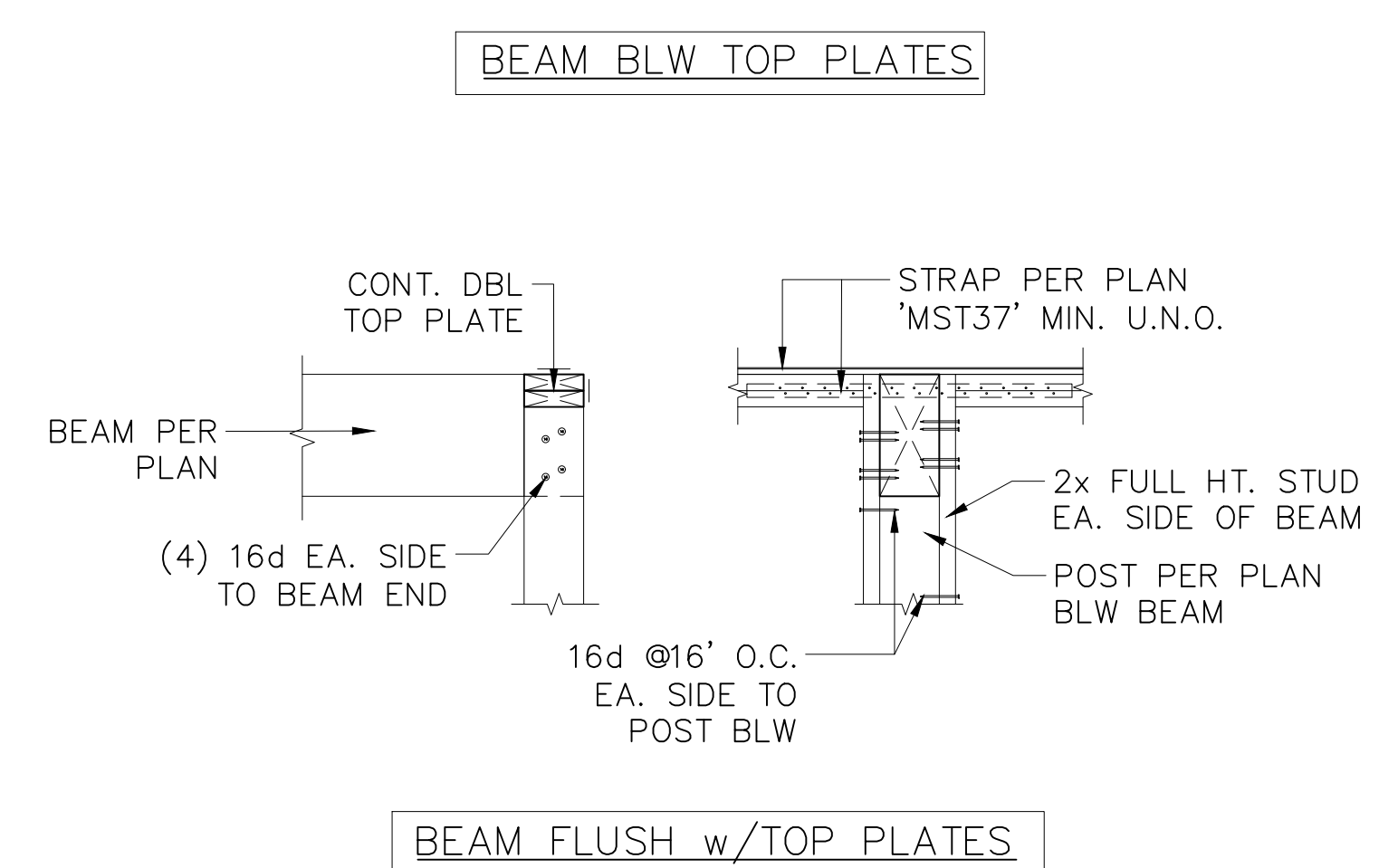
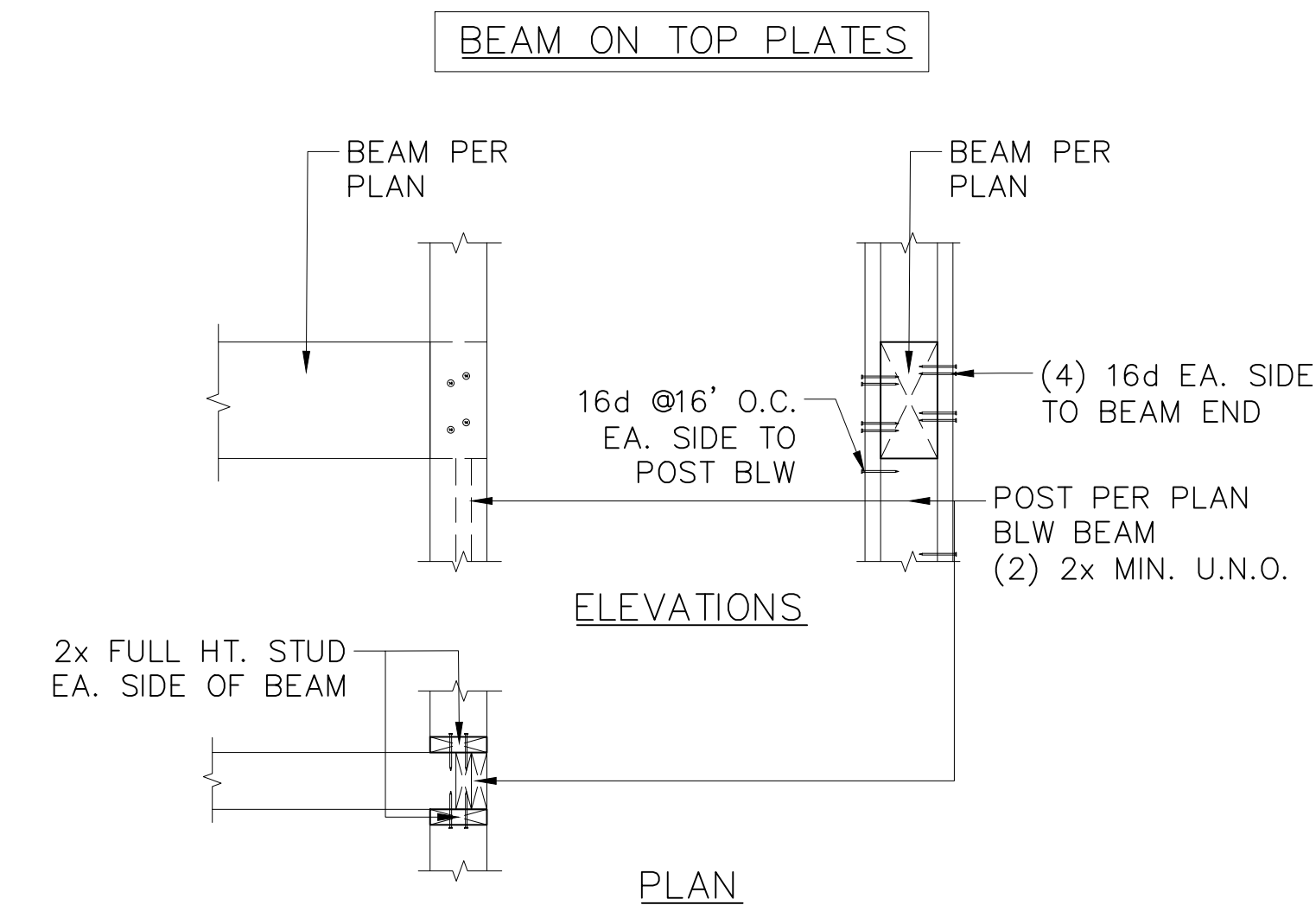
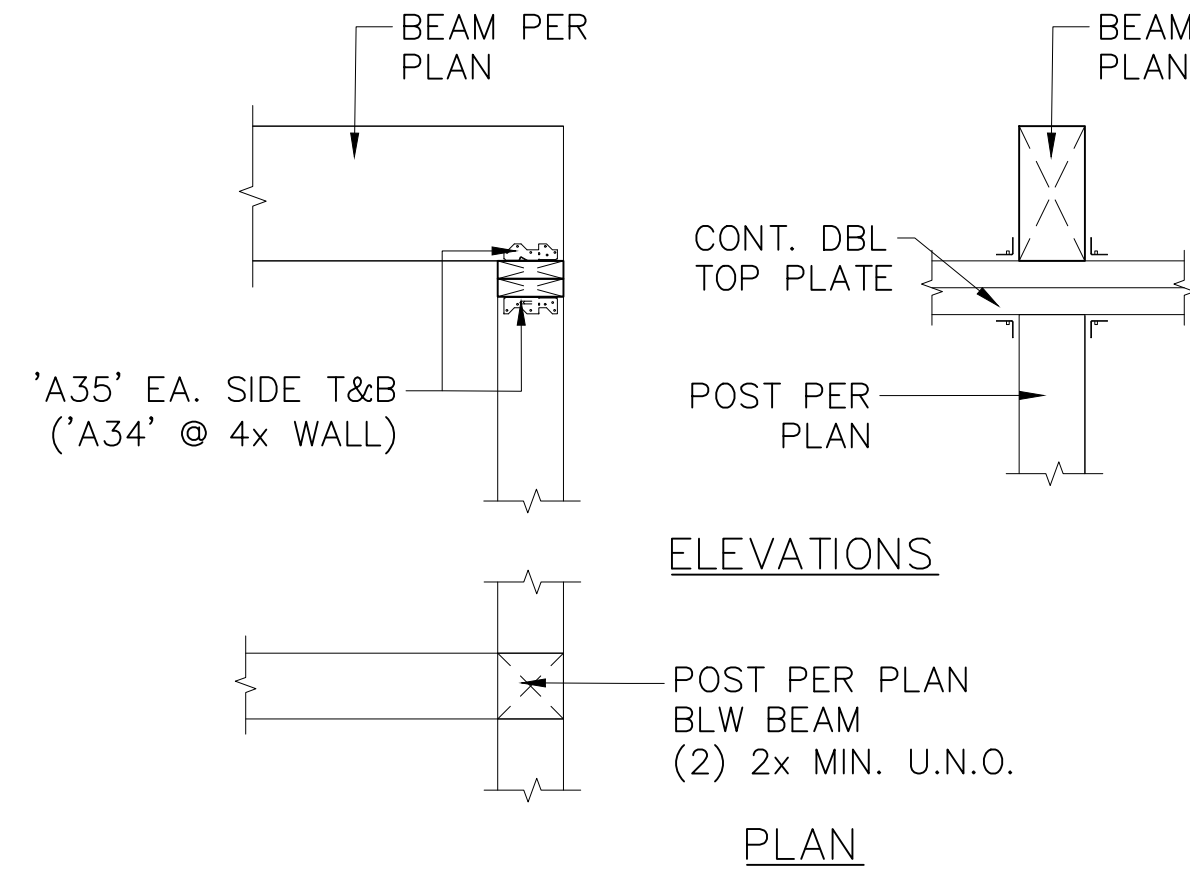
7 TYP. BORING AND NOTCHING DETAIL Scale: NTS



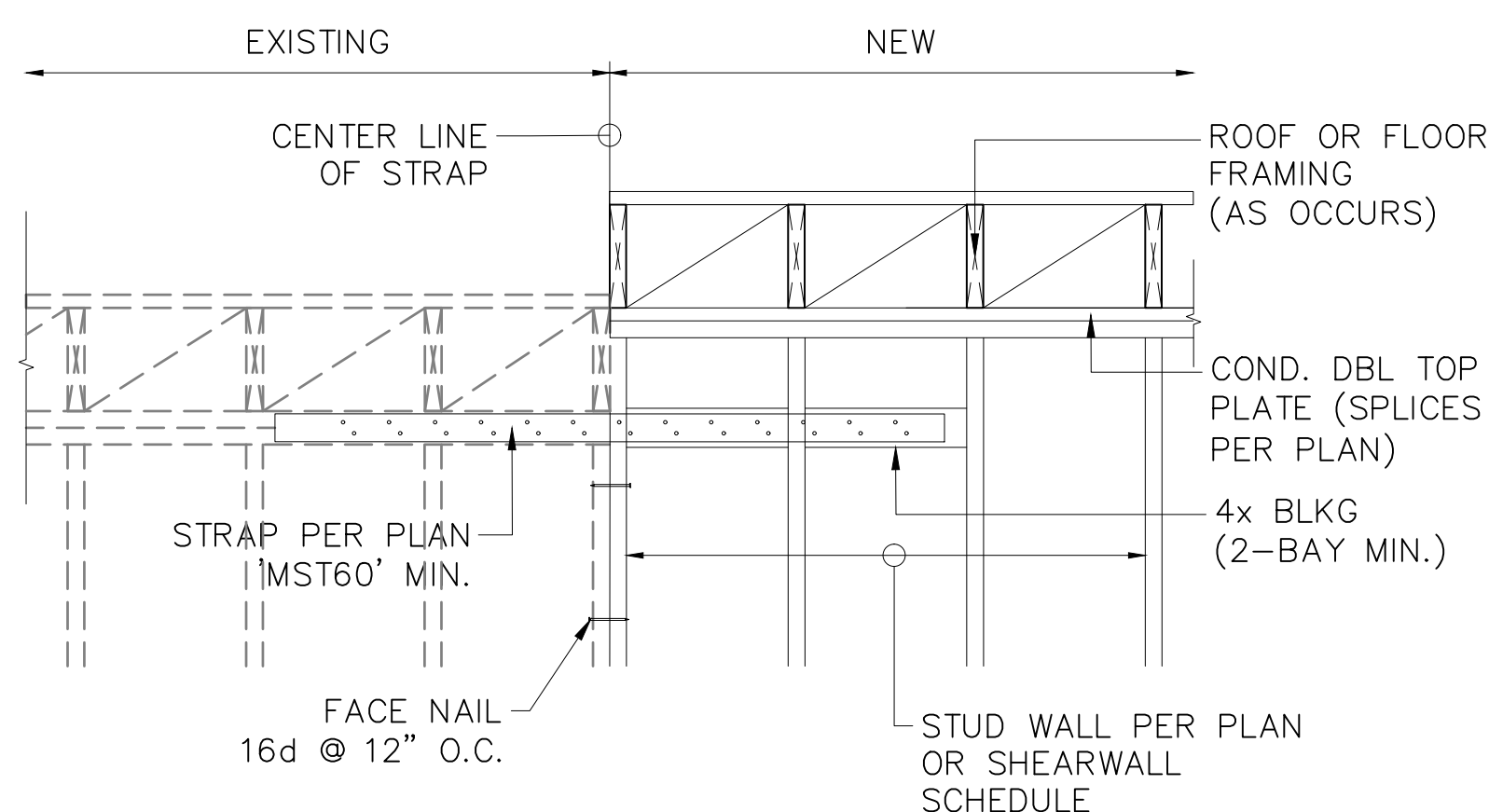
8 TYP. OPENING @ FLOOR OR ROOF Scale: NTS



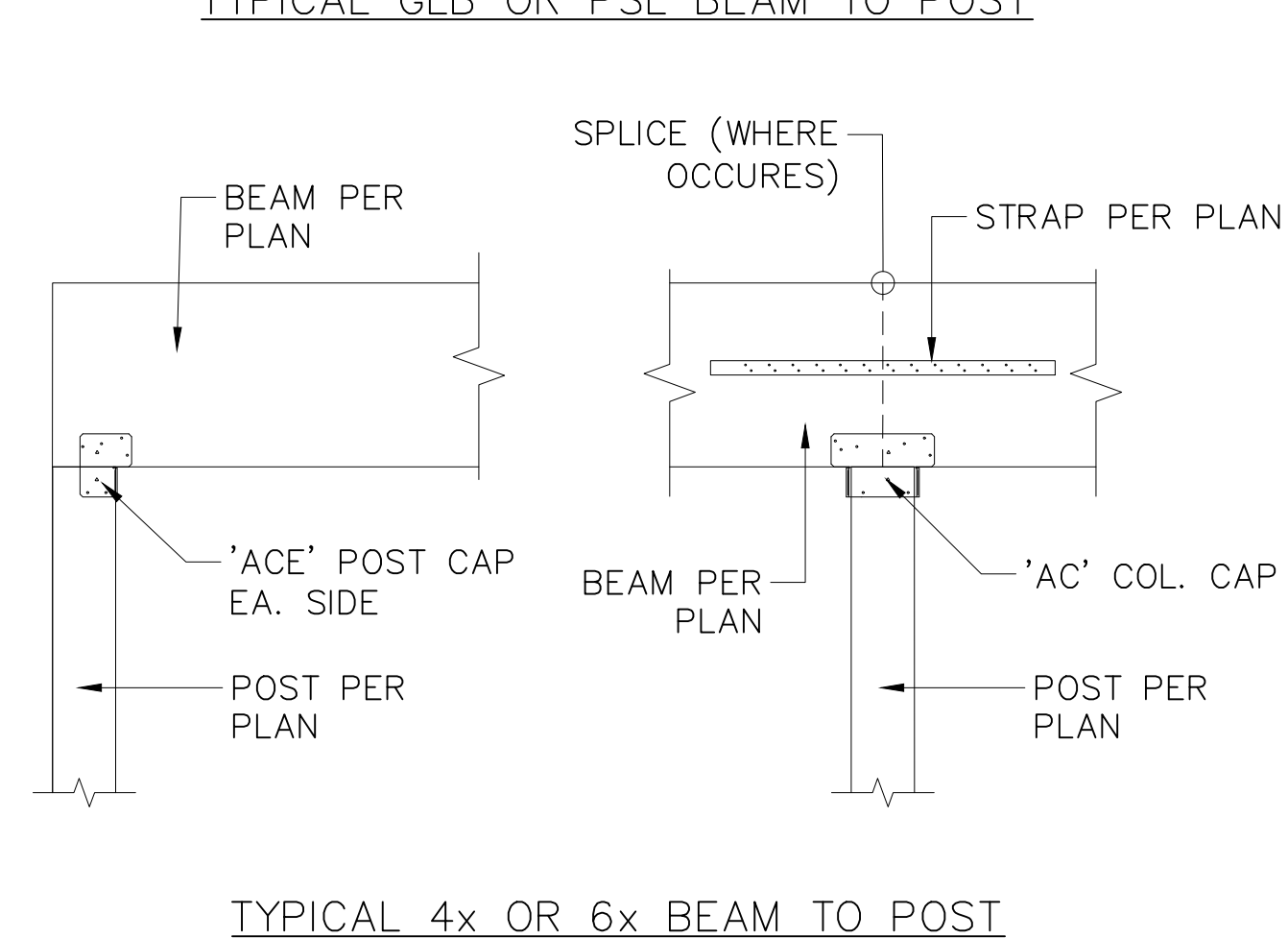
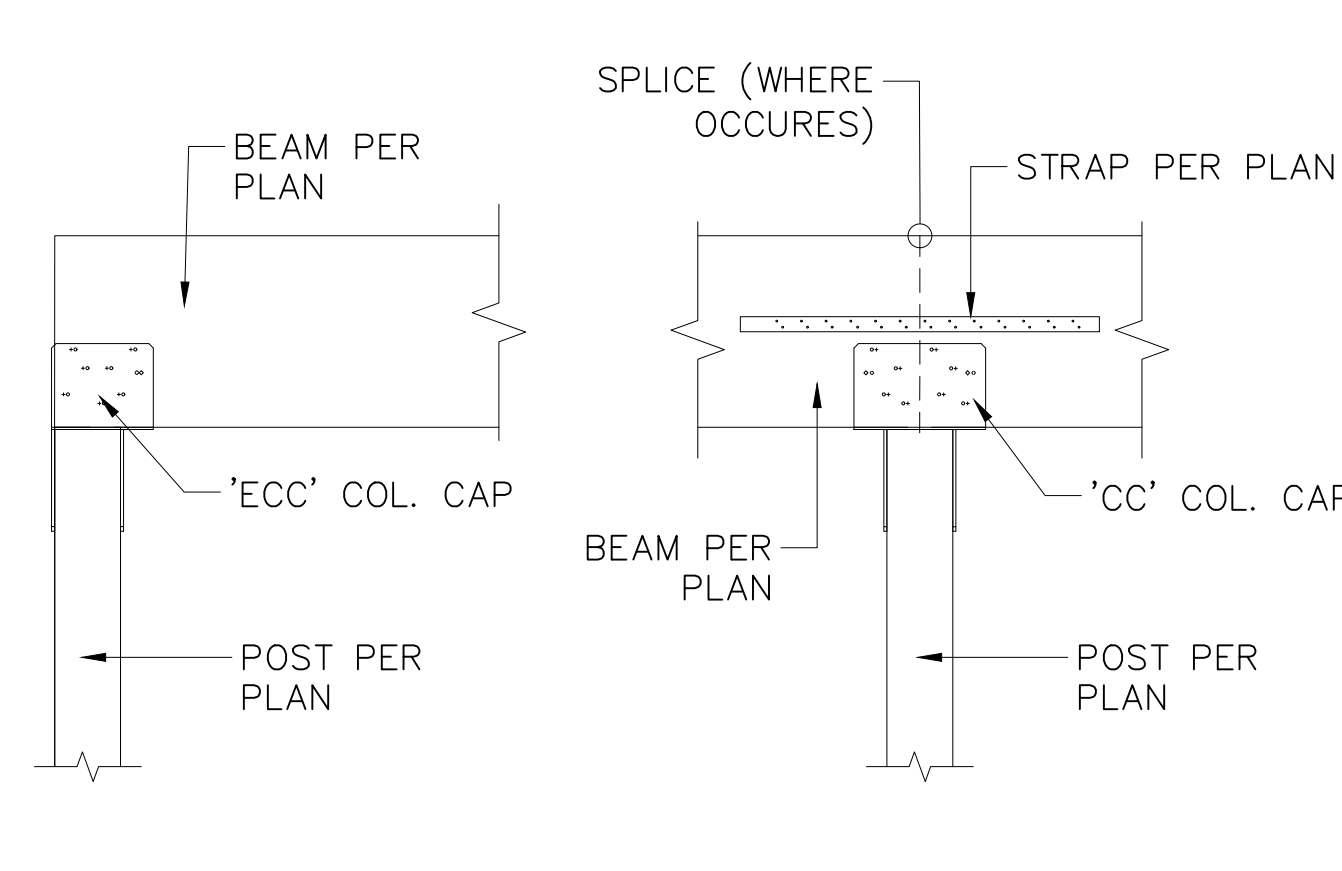
9 SPECIAL COND. AT SILL PLATE SPLICE Scale: NTS



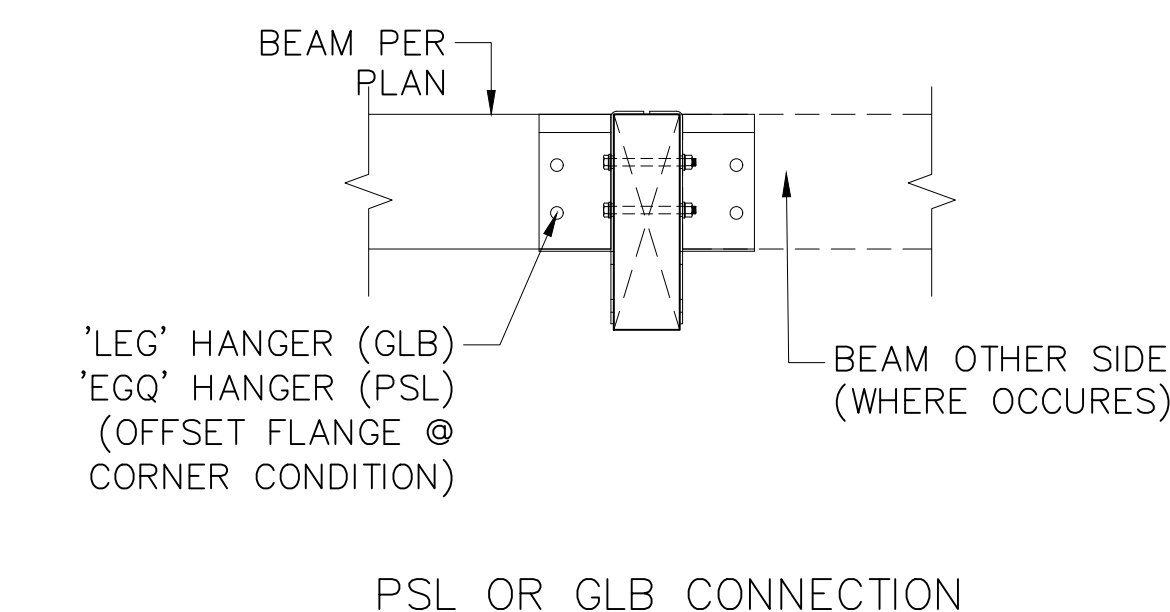
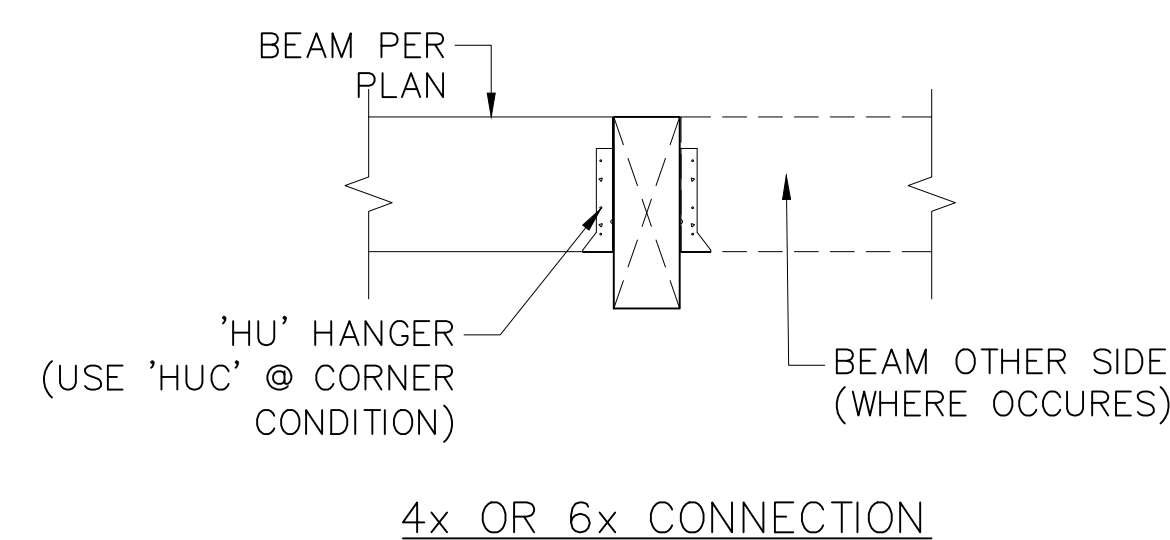
5 TYP. BEAM SUPPORT @ STUD WALL Scale: NTS



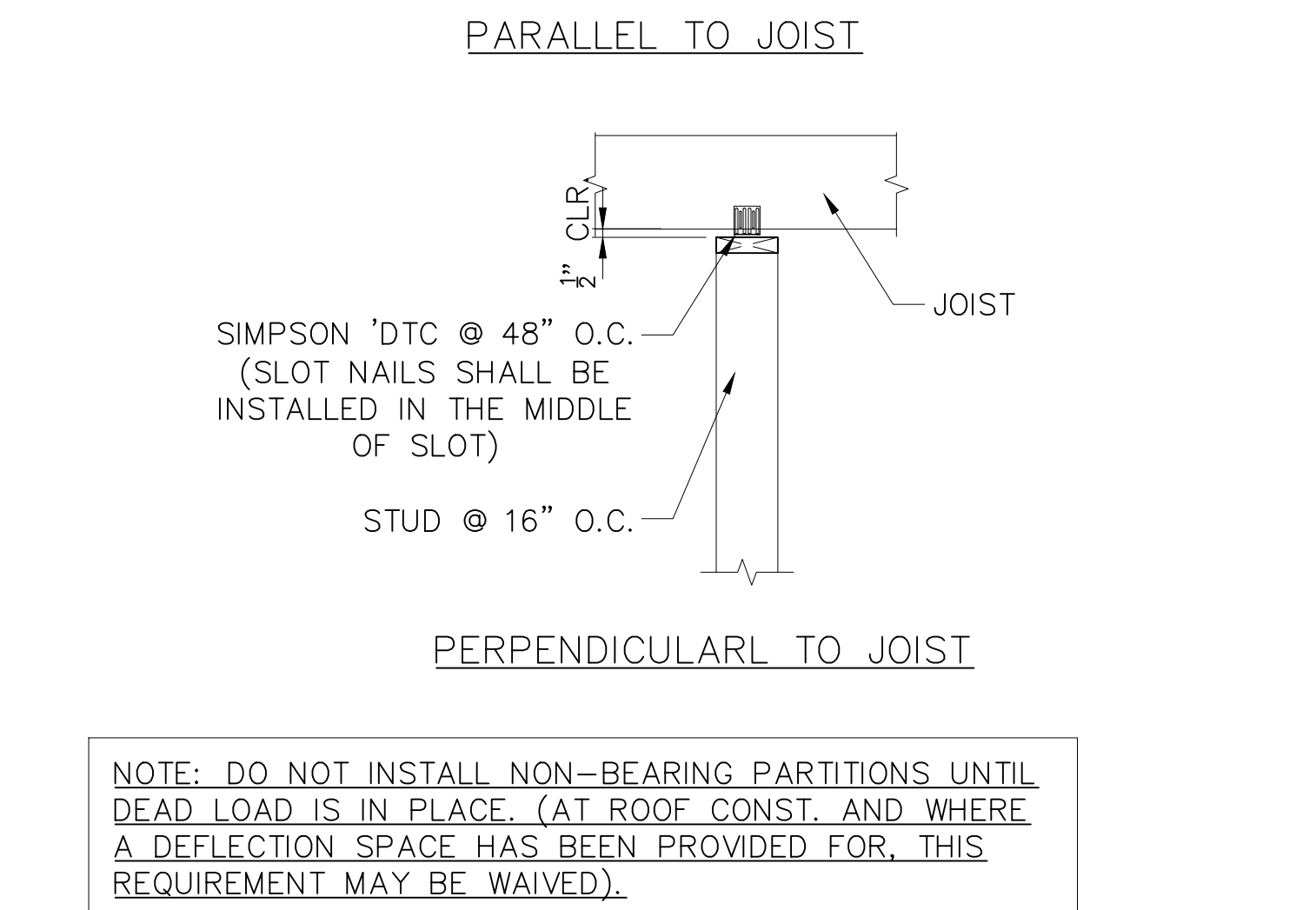
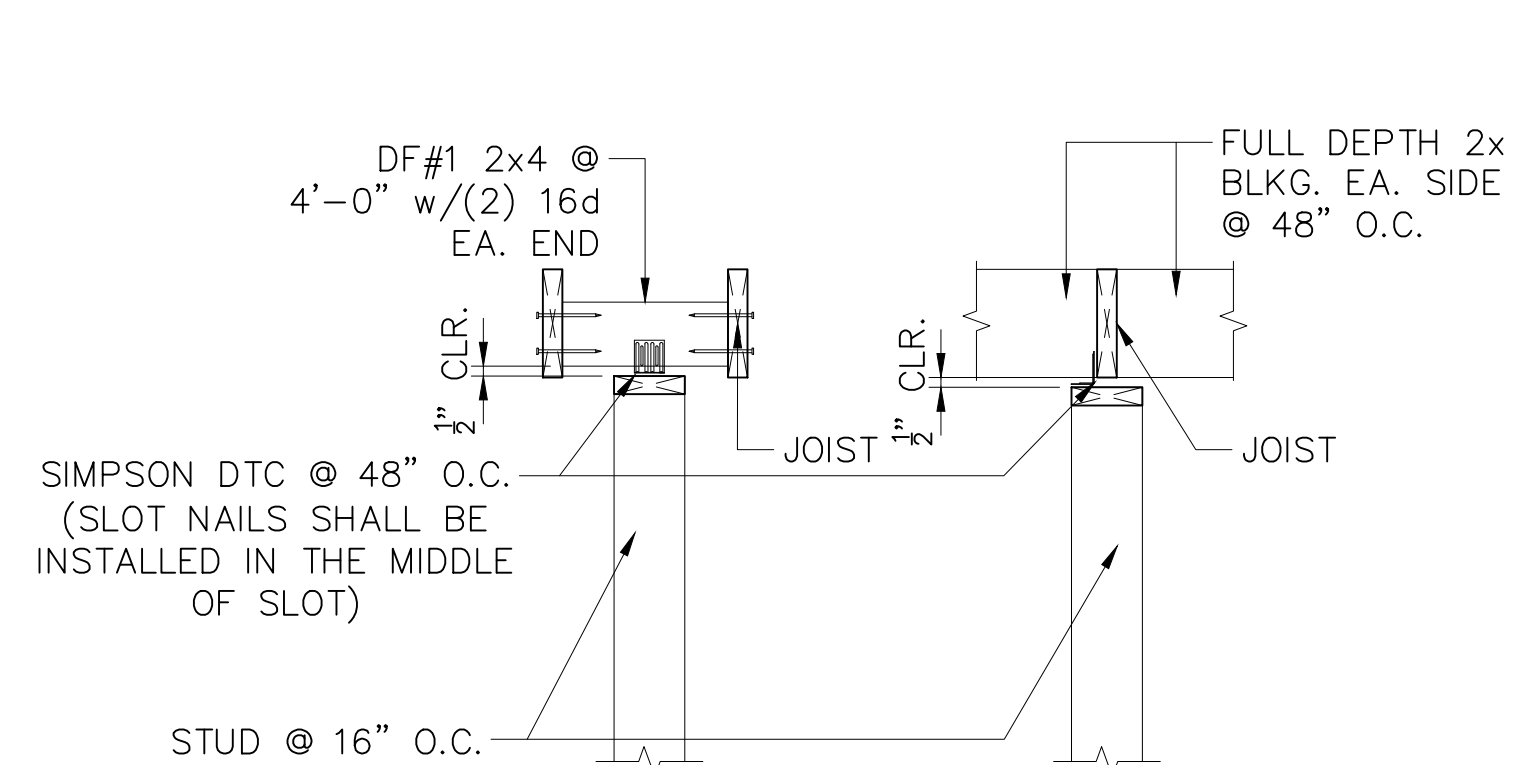
5 TYP. BEAM SUPPORT @ STUD WALL Scale: NTS



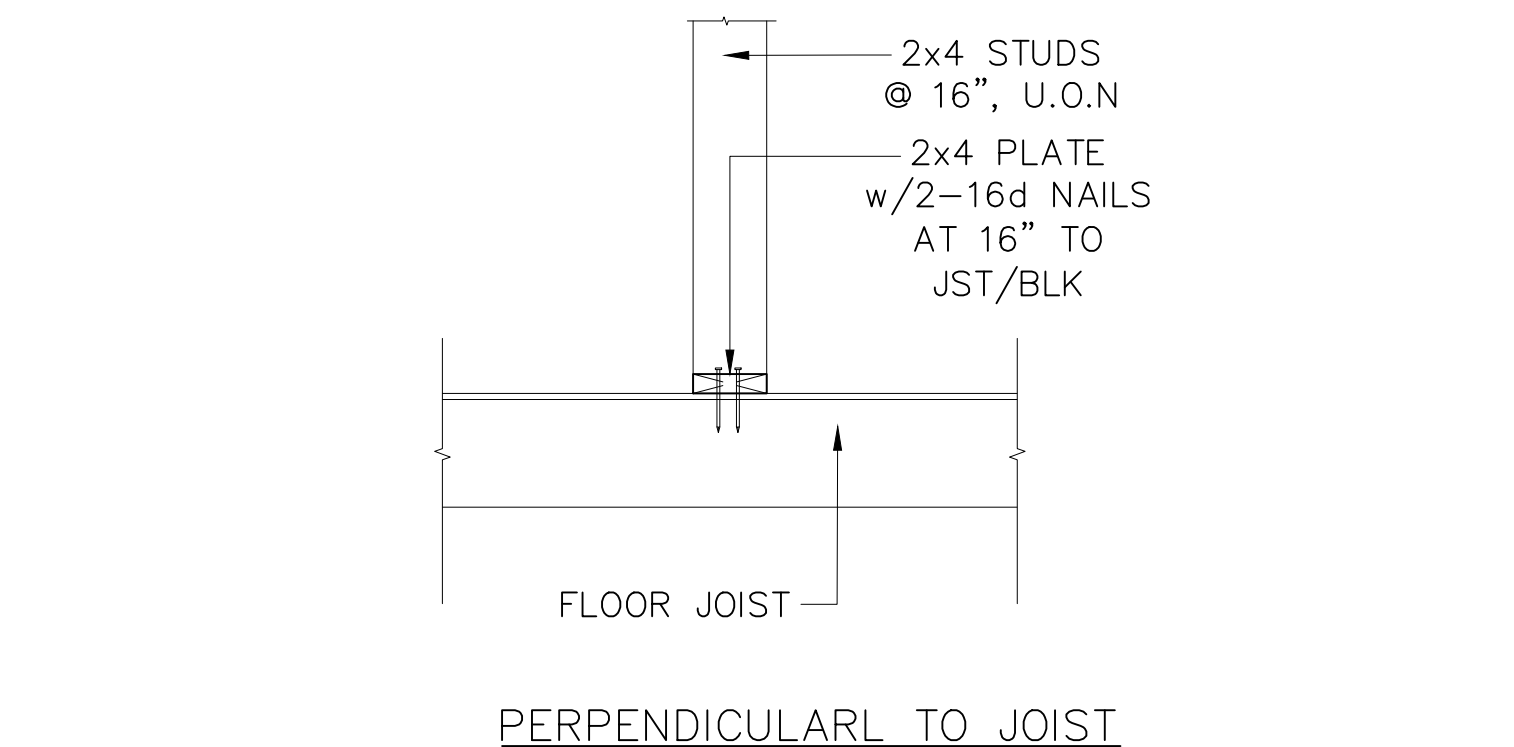
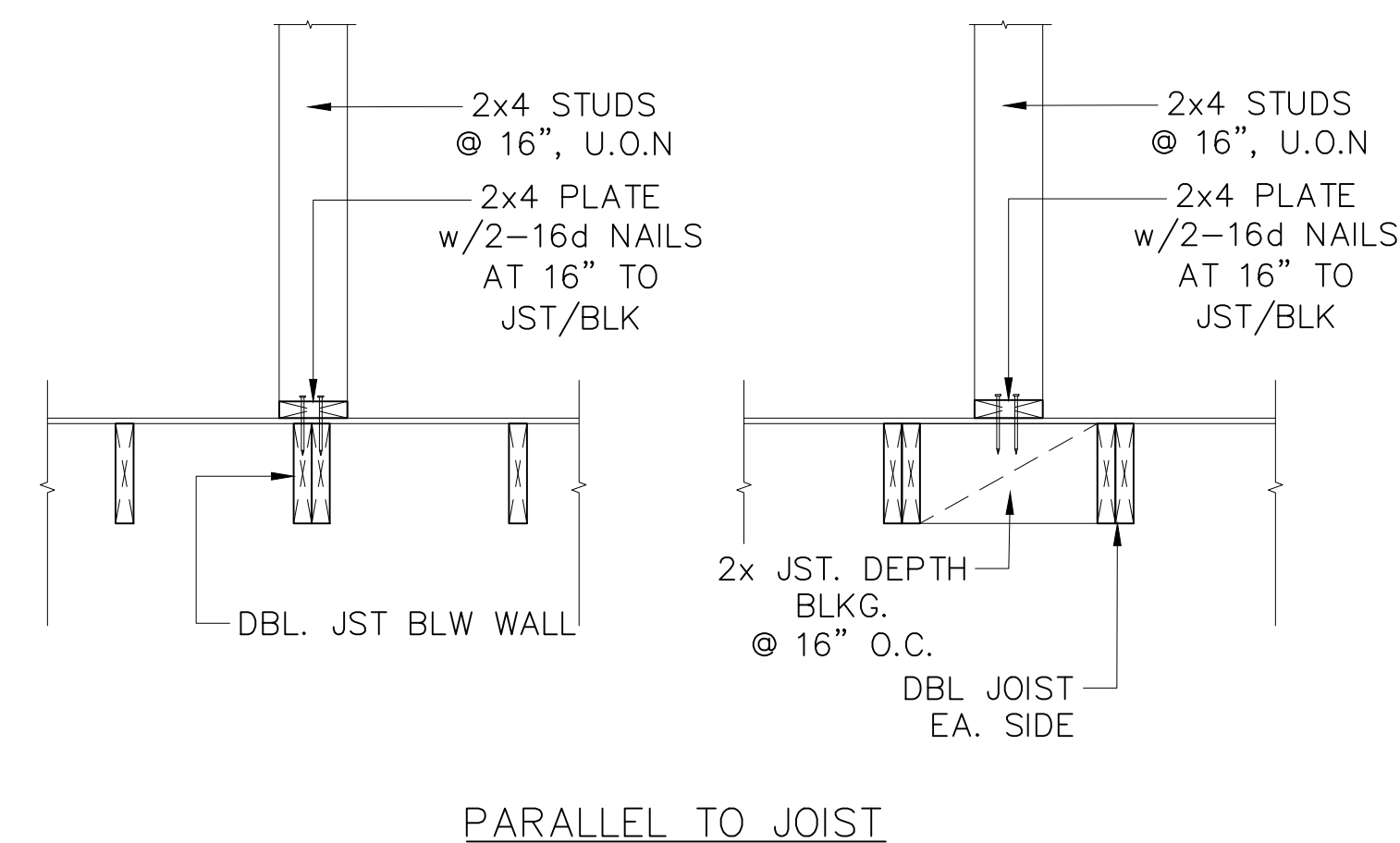
3 TYP. BEAM TO POST CONNECTION Scale: NTS



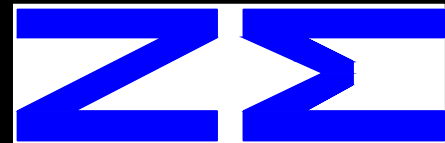
4 TYP. BEAM TO BEAM CONNECTION Scale: NTS



1 TYP. NON-BEARING PARTITIONS TOP CONN. Scale: NTS



2 TYP. NON-BEARING PARTITIONS SILL CONN. Scale: NTS



ZENITH ENGINEERS, INC
22320 Foothill Blvd,
Suite 600 Hayward, CA 94541
www.zenithengineers.com

REV:

REV:



FOUNDATION PLAN

22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120

DATE: 12.17.2019

SCALE: AS SHOWN

DESIGN BY: AZ

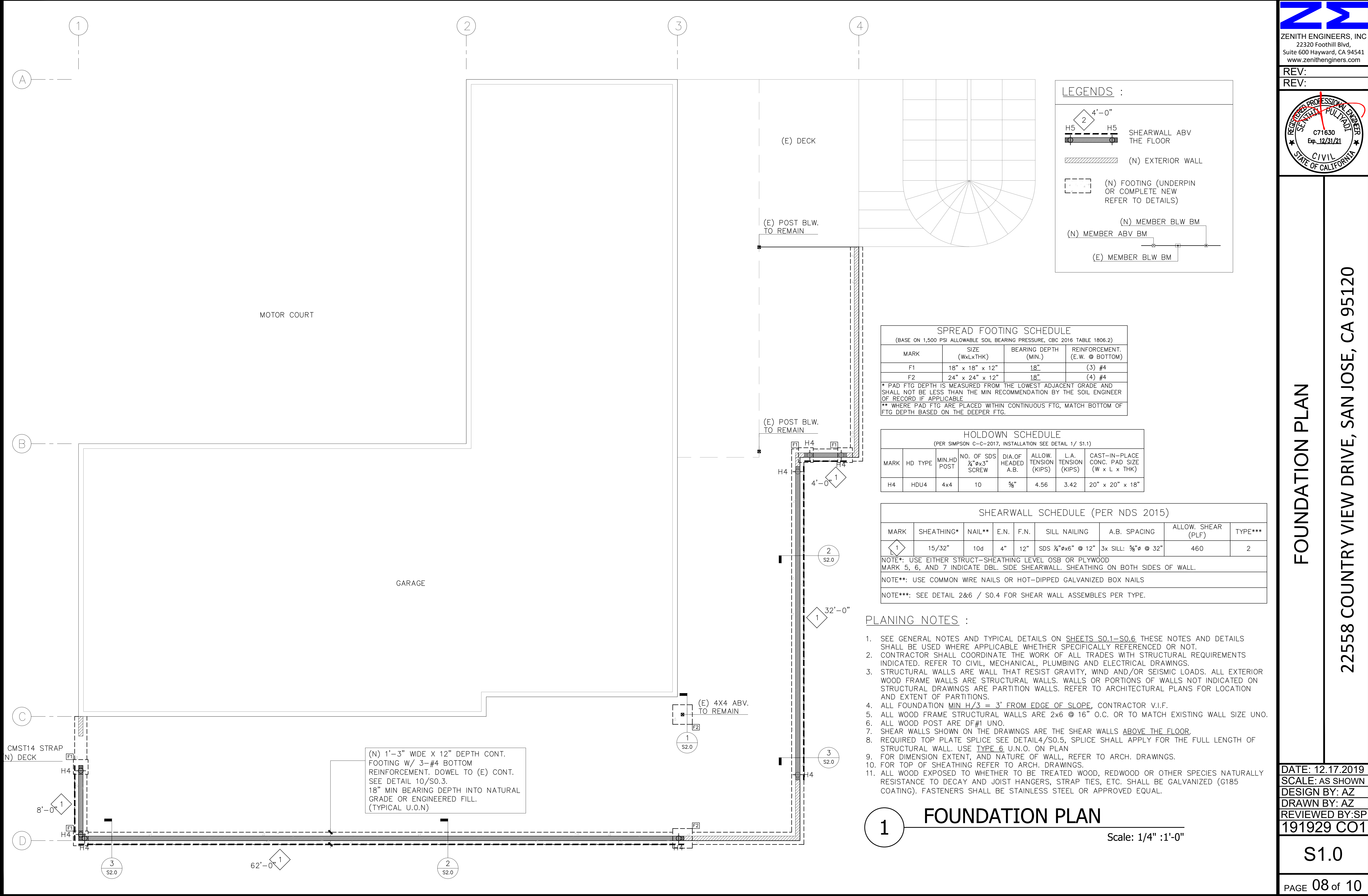
DRAWN BY: AZ

REVIEWED BY: SP

191929 CO1

S1.0

PAGE 08 of 10





REV:

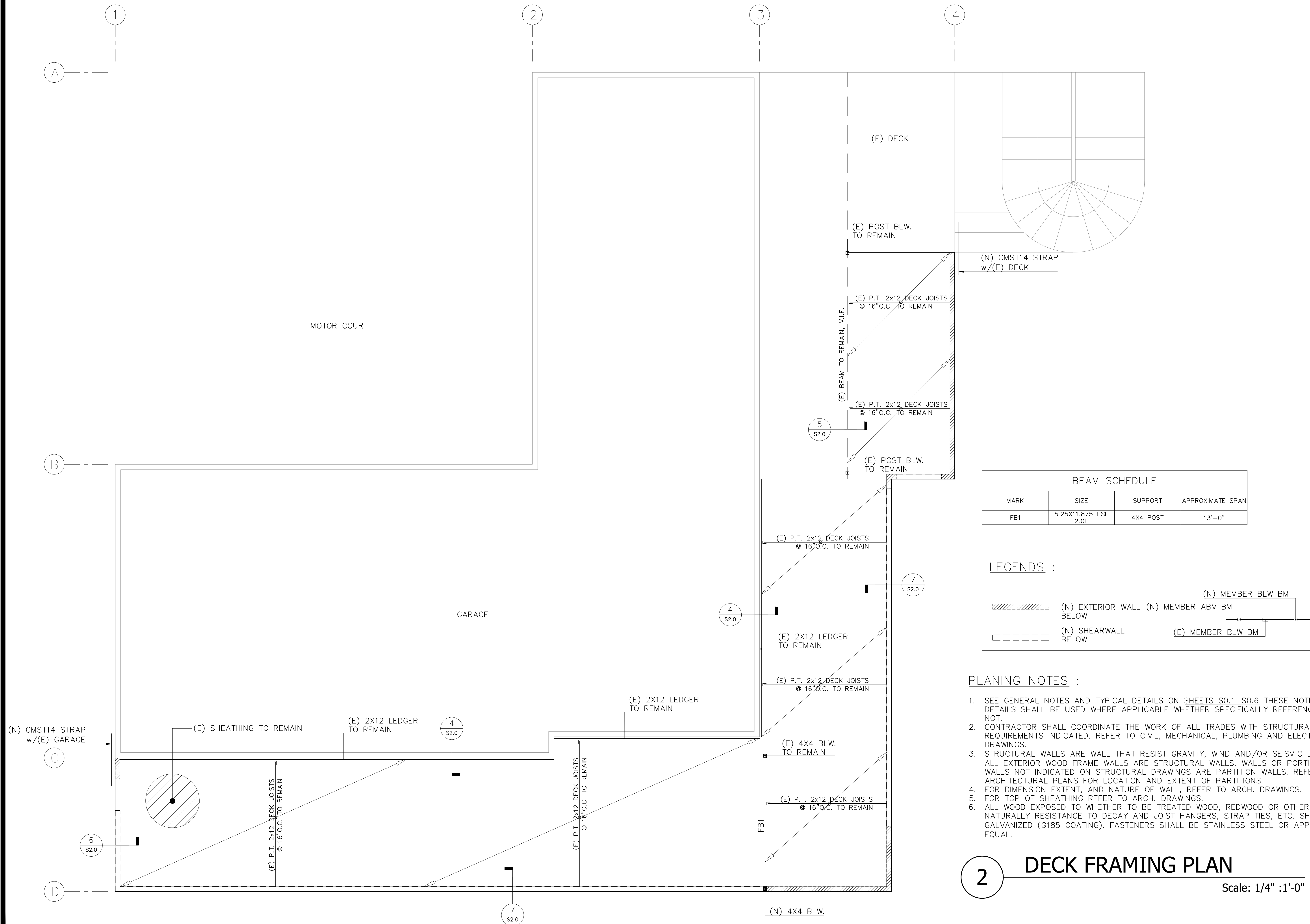


22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120

191929 CO1

S1.1

PAGE 09 of 10



1. SEE GENERAL NOTES AND TYPICAL DETAILS ON SHEETS S0.1-S0.6 THESE NOTES AND DETAILS SHALL BE USED WHERE APPLICABLE WHETHER SPECIFICALLY REFERENCED OR NOT.
2. CONTRACTOR SHALL COORDINATE THE WORK OF ALL TRADES WITH STRUCTURAL REQUIREMENTS INDICATED. REFER TO CIVIL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS.
3. STRUCTURAL WALLS ARE WALL THAT RESIST GRAVITY, WIND AND/OR SEISMIC LOADS. ALL EXTERIOR WOOD FRAME WALLS ARE STRUCTURAL WALLS. WALLS OR PORTIONS OF WALLS NOT INDICATED ON STRUCTURAL DRAWINGS ARE PARTITION WALLS. REFER TO ARCHITECTURAL PLANS FOR LOCATION AND EXTENT OF PARTITIONS.
4. FOR DIMENSION EXTENT, AND NATURE OF WALL, REFER TO ARCH. DRAWINGS.
5. FOR TOP OF SHEATHING REFER TO ARCH. DRAWINGS.
6. ALL WOOD EXPOSED TO WHETHER TO BE TREATED WOOD, REDWOOD OR OTHER SPECIES NATURALLY RESISTANCE TO DECAY AND JOIST HANGERS, STRAP TIES, ETC. SHALL BE GALVANIZED (G185 COATING). FASTENERS SHALL BE STAINLESS STEEL OR APPROVED EQUAL.

2 DECK FRAMING PLAN

Scale: 1/4" : 1'-0"

	1		2		3		4
	5		6		7		8
	9						

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REV:
REV:

REGISTERED PROFESSIONAL ENGINEER
SEVENTH DISTRICT
C71630
Exp. 12/31/21
CIVIL
STATE OF CALIFORNIA

STRUCTURAL DETAILS

22558 COUNTRY VIEW DRIVE, SAN JOSE, CA 95120

DATE: 12.17.2019
SCALE: AS SHOWN
DESIGN BY: AZ
DRAWN BY: AZ
REVIEWED BY: SP
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S2.0

PAGE 10 of 10