

Prepared by FEHR & PEERS

100 Pringle Avenue Suite 600 Walnut Creek, CA 94596

December 2019

LBRE Replacement Building Local Access and Circulation Study

Prepared for: Stanford Department of Project Management

LBRE Replacement Building Local Access and Circulation Study

Prepared for:

Stanford Department of Project Management

December 2019

WC19-3607.00

FEHR PEERS

Table of Contents

1. Introduction	1
1.1 Background	1
1.2 Project Description	1
New LBRE Building	1
Equipment Shed	2
Maintenance Vehicle Parking	2
2. Existing Conditions	4
2.1 Study Area Roadway Network	4
2.2 Peak Hour Traffic Volumes	6
2.3 Intersection Operations	6
3. Project Evaluation	10
3.1 Project Traffic Estimates	10
Trip Generation	10
Trip Distribution and Assignment	11
3.2 Existing With Project Intersection Operations	12
3.3 Pedestrian and Bicycle Access and Circulation	15
3.4 Transit Access	15
3.5 Service, Delivery and Emergency Access	15
4. Findings	17

Appendices

A -- Traffic Counts

B -- LOS Results

List of Figures

Figure 1:	Project Site Plan
Figure 2:	Study Area5
Figure 3:	Existing Peak Hour Intersection Traffic Volumes, Lane Configurations and Traffic Controls7
Figure 4:	Existing Peak Hour Bicycle and Pedestrian Volumes8
Figure 5:	Project Trip Assignment
Figure 6:	Existing with Project Peak Hour Intersection Traffic Volumes, Lane Configurations and Traffic Controls
List of	Tables
Table 1	Unsignalized Intersection LOS Criteria6
Table 2	Existing Intersection Levels of Service9
	Estimated Project Peak Hour Trip Generation11
Table 4	Existing with Project Intersection Level of Service12

1. Introduction

1.1 Background

This report presents the local access and circulation characteristics of the proposed LBRE Replacement Building (Project). This report has been prepared because the Project proposes a re-distribution of 73,000 square feet of floor space from the DAPER Development District to the West Campus Development District. GUP Conditions of Approval D.5.a and D.6 require that an environmental assessment of such projects be prepared, and GUP Condition of Approval G.11 describes the scope of traffic studies for the environmental assessment. Further detail on the scope and methodology of the studies is provided in the memorandum of understanding (MOU) entitled *Scoping of Project-Specific Transportation Studies under Stanford GUP Condition of Approval G.11*.

The Project site plan is shown in Figure 1.

1.2 Project Description

The Project, located in the West Campus Development District in the vicinity of Fremont Road and Electioneer Road, includes three components: a new 73,000 square-foot building to serve as a consolidated facility for the on-campus Lands, Buildings and Real Estate (LBRE) functions; a shed to house large equipment; and a conversion of a portion of the Searsville parking lot to maintenance vehicle storage.

New LBRE Building

Facility

The new building will house the majority of the on-campus LBRE employees. The new facility will foster a diverse set of job functions including office, shop technicians, field technicians, grounds teams, warehouse, and event services. The new building and program will replace LBRE's four large buildings and the several small ancillary buildings on Bonair Siding, which are nearing the end of their useful life. The new building will be constructed on a vacant site west of Fremont Road and south of the Central Energy Facility. It will be three stories above grade and will include eight loading docks for trucks, as well as space for two to three tractor-trailer (large-wheelbase) trucks along the north edge of the building. The site includes a turnaround at the northwest corner for vehicle and truck maneuvering.



Uses

Desk employees will relocate from Bonair Siding buildings to work in the new building, and shops/grounds/trades workers will check in at the beginning of the work day (typically 5:30 - 6:00 AM) and check out at the end of the work day (2:30 - 3:00 PM).

The building will also facilitate on-campus logistics for the Buildings, Grounds and Maintenance (BGM) group, including event set-up and take-down and other activities. These trips currently occur at the Bonair Siding site, and would relocate to the new building. These trips are all internal to the campus. An estimated 20 to 25 round trips per day are typically made.

Approximately 5 to 15 external deliveries per day to the LBRE facilities warehouse typically occur at the Bonair site, and these would relocate to the new building. Most deliveries are in single-unit trucks, with occasional large tractor-trailer truck deliveries.

Equipment Shed

A shed to house heavy equipment will be constructed on the western portion of Electioneer Road, with access/egress to be provided both from Fremont Road and Electioneer Road; through traffic on Electioneer Road will no longer be allowed. A turn-around at the new western end of Electioneer Road will serve drivers needing to turn around as well as trucks/heavy equipment access and egress. The construction of the shed will eliminate up to 55 parking spaces on Electioneer Road.

Maintenance Vehicle Parking

A portion of Searsville Lot (L22) will be converted from commuter and resident parking to maintenance vehicle storage. The estimated loss of parking spaces is 250 spaces out of the total 611-space parking supply.



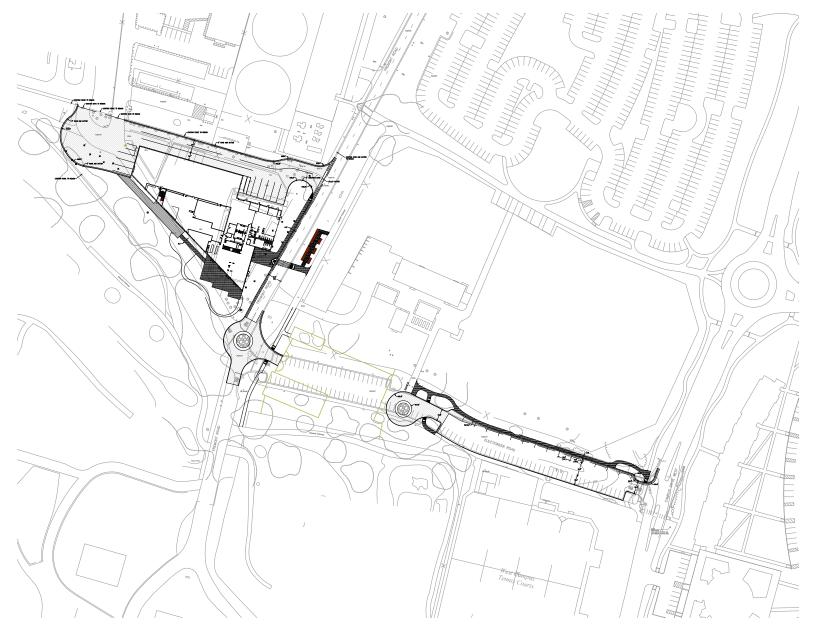




Figure 1





2. Existing Conditions

2.1 Study Area Roadway Network

Figure 2 shows the study area, which focuses on the internal campus network immediately surrounding the Project site. A description of the study area roadways is provided below.

Fremont Road is a two-lane roadway providing access between Searsville Road and Electioneer Road, and access to the Central Energy Facility, the Stanford Educational Farm, and the Stanford Equestrian Center. A sidewalk is provided along the Central Energy Facility frontage between Searsville Road and the main building entrance, and a striped pedestrian crossing is provided between the building's northeast access point and the Searsville parking lot.

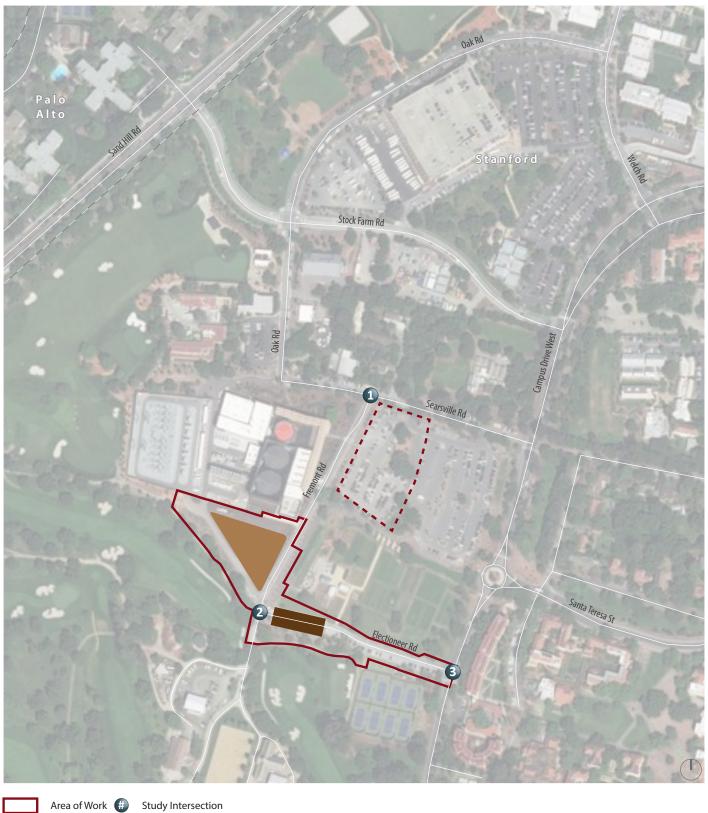
Electioneer Road connects Campus Drive West to Fremont Road, with perpendicular on-street parking on one or both sides. The West Campus Tennis Courts are accessed from Electioneer Road. A sidewalk is provided along the south side of the roadway.

Searsville Road connects Campus Drive West to Oak Road, providing access to the Searsville parking lot via two driveways. A sidewalk is provided along the south side of the roadway adjacent to the Searsville parking lot. Perpendicular parking is provided along portions of the north side of the roadway.

This study includes AM and PM peak hour intersection operations evaluation for the following three oncampus intersections immediately adjacent to the Project site. Note that the effect of the Project on the external intersections studied in the 2000 General Use Permit EIR is provided in the separate report, *LBRE Replacement Facility GUP EIR Intersection Evaluation*.

- 1. Fremont Road/Searsville Road
- 2. Fremont Road/Electioneer Road
- 3. Campus Drive West/Electioneer Road









LBRE Replacement Building



Equipment Shed



Convert to Fleet Parking (Approximate)



Figure 2

2.2 Peak Hour Traffic Volumes

AM and PM two-hour peak period counts of vehicles, bicyclists and pedestrians were conducted at the study intersections on Wednesday, May 29, 2019, when spring quarter was in regular session. **Figure 3** shows the AM and PM peak hour vehicle turning movements, and **Figure 4** shows the peak hour bicycle turning movements and pedestrian crossings.

2.3 Intersection Operations

The study intersections include all-way stop-controlled and side-street stop-controlled intersections. For this analysis, the *Highway Capacity Manual 6th Edition (HCM 6)* methodology is used. For stop-controlled intersections, this method defines intersection operations by the average control delay per vehicle (measured in seconds) for each stop-controlled movement. This incorporates delay associated with deceleration, acceleration, stopping, and moving up in the queue. For side-street stop-controlled intersections, the delay and LOS are typically reported for the worst stop-controlled movement or approach, along with the average delay and LOS for the entire intersection. The Synchro 10 analysis software is used to calculate the LOS for stop-controlled intersections.

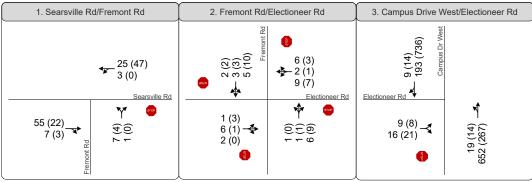
Table 1 summarizes the relationship between delay and LOS for unsignalized intersections.

	TABLE 1 UNSIGNALIZED INTERSECTION LOS CRITERIA	A
Level of Service	Description	Average Control Delay Per Vehicle (Seconds)
Α	Little or no traffic delays	<u><</u> 10.0
В	Short traffic delays	> 10.0 to 15.0
С	Average traffic delays	> 15.0 to 25.0
D	Long traffic delays	> 25.0 to 35.0
E	Very long traffic delays	> 35.0 to 50.0
F	Extreme traffic delays with intersection capacity exceeded	> 50.0

Source: Highway Capacity Manual – Special Report 209 (Transportation Research Board, 2010).







XX (YY) AM (PM) Peak Hour Traffic Volumes 👨 Stop Sign

Study Intersection

_____A

Area of Work



LBRE Replacement Building



Equipment Shed

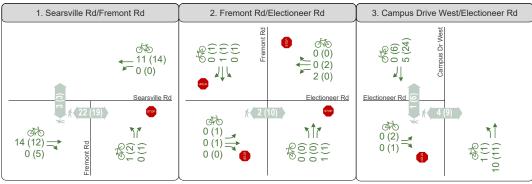


Convert to Fleet Parking (Approximate)









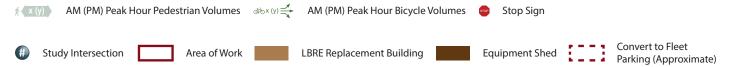


Figure 4

Table 2 shows the existing service levels at the internal study intersections. Currently, all intersections operate well during the AM and PM peak hours.

EXISTING INTER	TABLE 2 SECTION LEVELS	OF SERVIC	Ε	
Intersection	Traffic Control	Peak	Exis	ting
intersection	Traffic Control	Hour	Delay	LOS
Fremont Road/Searsville Road	Side-Street Stop	AM	1.0 (9.0)	A (A)
1. Fremont Road/Searsville Road	Side-Street Stop	PM	0.5 (8.9)	A (A)
2 Francost Board/Flortion or Board	All May Chan	AM	7.3	А
2. Fremont Road/Electioneer Road	All-Way Stop	PM	7.0	Α
2.6 P. W. (5)	6: 1 6: 16:	AM	0.5 (13.0)	A (B)
3. Campus Drive West/Electioneer Road	Side-Street Stop	PM	0.6 (16.8)	A (C)

Side-street stop-controlled intersection LOS is reported as: overall intersection delay and LOS (worst-case movement or approach delay and LOS).

All-way stop controlled intersection LOS is reported for the overall intersection, based on average delay per vehicle. Source: Fehr & Peers, December 2019.



3. Project Evaluation

3.1 Project Traffic Estimates

Trip Generation

As described in Section 1.2, the project will relocate employees and activities from Bonair Siding to the Project site on Fremont Road. The components of the Project and associated effect on AM and PM peak hour trip generation are described below.

- The relocation of the desk employees to the Project site is not expected to affect AM and PM peak hour traffic volumes in the study area, because commuter traffic patterns within the campus are based on parking supplies and locations. The Project will reduce the permit parking supply in the local access and circulation study area by 305 spaces. Therefore, the Project's effect on commuter traffic in the local access and circulation study area is expected to be a net reduction. However, for the purposes of the Existing With Project traffic analysis, no reduction was taken because maintenance vehicles may travel locally during peak hours.
- The relocation of the shop workers check-in/check-out location will also not affect peak hour trip generation in the local access and circulation study area, because the shop workers begin and end their workday at 5:30 6:00 AM and 2:30 3:00 PM, respectively, which is well before the AM and PM peak hours of the adjacent streets (8:00 9:00 AM and 5:00 6:00 PM).
- The relocation of the BGM logistics activities is expected to generate 20 to 25 round trips within the local access and circulation study area per day. These trips would not leave campus, but rather travel between the Project site and various on-campus destinations. Note that these trips are not new trips, but rather relocated trips from the Bonair Siding facilities to the Project site. Using the high end of 25 round trips per day, a peak hour estimate would be 25 divided by 8 hours per day, or 6 round trips per peak hour.
- The relocation of the external deliveries to the LBRE facilities warehouse from Bonair Siding to the Project site would result in an estimated 5 to 15 external deliveries per day. Note that these trips are not new trips to the campus, but rather relocated trips from the Bonair Siding facilities warehouse to the Project site. Using the high end of 15 round trips per day, a peak hour estimate would be 15 divided by 8 hours per day, or 2 round trips per peak hour.
- The new heavy equipment shed on Electioneer Road will be used for storage, and equipment will be moved to and from work locations as needed. These movements are not expected to occur within the peak commute hours on a regular basis. In addition, the loss of up to 55 parking spaces on Electioneer Road will reduce traffic entering and exiting Electioneer Road at Campus



Drive West; however, for the purposes of the Existing With Project traffic analysis, no reduction was taken.

• The elimination of 250 parking spaces in the Searsville parking lot to provide space for maintenance vehicle storage will reduce traffic related to commuter and resident parking trips at the Searsville parking lot driveways. Some maintenance vehicle movements between the storage lot and oncampus destinations will occur, including, potentially, trips during the peak commute hours. It is expected that the net change in peak commute hour traffic at the lot driveways will be a reduction in traffic; however, for the purposes of the Existing With Project traffic analysis, no reduction was taken.

Table 3 summarizes the estimated Project trip generation based on the above discussion. As noted above, this is considered a conservative estimate, as several components of the Project are expected to reduce traffic in the local access and circulation study area, but these reductions were not included in the estimate.

	ESTIM	IATED PR	T OJECT PI	ABLE 3 EAK HOU	R TRIP G	ENERATI	ON		
T. 4 T	Al	M Peak Ho	our	Pi	M Peak Ho	ur		Daily	
Truck Type	ln	Out	Total	ln	Out	Total	In	Out	Total
Event Box Trucks (Internal to Campus)	6	6	12	6	6	12	50	50	100
Logistics Deliveries (External)	2	2	4	2	2	4	15	15	30
Total	8	8	16	8	8	16	65	65	130

Trip Distribution and Assignment

The internal campus trips were assumed to travel to and from the LBRE Replacement Building site via Searsville Road. The external delivery trips may continue to use the gateways to campus that they currently use, with some re-routing to the Stock Farm Road gateway due to its proximity to the Project site. For the purposes of this analysis, the external truck trips were assumed to travel to and from the LBRE Replacement Building site via Stock Farm Road – Oak Road – Searsville Road – Fremont Road (entering), and the reverse to leave campus.

Figure 5 shows the Project trip assignment based on the above assumptions, and **Figure 6** shows the Existing With Project intersection peak hour turning movements.



3.2 Existing With Project Intersection Operations

Table 4 presents the study area intersection levels of service with the Project. Because the Project is estimated to generate very low net new peak hour traffic in the study area, the intersection service levels would remain good with the Project.

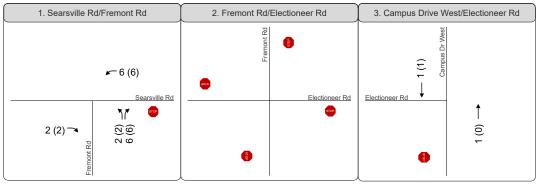
EXIST	TABLE 4 EXISTING WITH PROJECT INTERSECTION LEVEL OF SERVICE														
Intersection	Control ¹	Peak	Existing C	onditions		ith Project itions									
		Hour	Delay ²	LOS	Delay ²	LOS									
1. Fremont Road/Searsville	Side-Street	AM	1.0 (9.0)	A (A)	1.9 (9.1)	A (A)									
Road	Stop	PM	0.5 (8.9)	A (A)	1.6 (9.0)	A (A)									
2. Fremont	All-Way Stop	AM	7.3	A	7.3	A									
Road/Electioneer Road		PM	7.0	A	7.0	A									
3. Campus Drive	Side-Street	AM	0.5 (13.0)	A (B)	0.5 (13.0)	A (B)									
West/Electioneer Road	Stop	PM	0.6 (16.8)	A (C)	0.6 (16.8)	A (C)									

Side-street stop-controlled intersection LOS is reported as: overall intersection delay and LOS (worst-case movement or approach delay and LOS).

All-way stop controlled LOS is reported for the overall intersection, based on average delay per vehicle. Source: Fehr & Peers, December 2019.

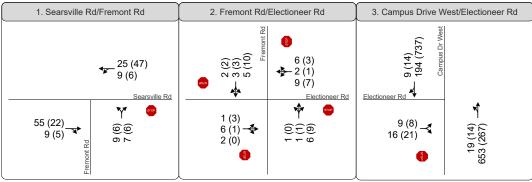




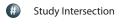








XX (YY) AM (PM) Peak Hour Traffic Volumes 👴 Stop Sign





Area of Work



LBRE Replacement Building



Equipment Shed



Convert to Fleet Parking (Approximate)





3.3 Pedestrian and Bicycle Access and Circulation

Pedestrian access to the Project site is provided by the walkways along the east side of Fremont Road and the west side of Fremont Road along the Central Energy Facility (CEF) site, the south side of Searsville Road, the south side of the Searsville parking lot, and the east side of Oak Road. These pathways connect the site to the larger campus pedestrian network. The Project design includes a pedestrian path along the west side of Fremont Road along the Project frontage, connecting to the existing path adjacent to the Central Energy Facility. A striped crosswalk will be provided to facilitate walking between the new building and the set-back path along the east side of Fremont Road.

Bicyclists travelling to and from the Project site would use the bicycle lanes on Campus Drive West and Stock Farm Road, and share the road with vehicles on Searsville Road, Oak Road and Fremont Road. Traffic volumes on Searsville Road and Fremont Road are low (refer to Figure 3), and are expected to remain low with the Project.

3.4 Transit Access

The Project site is located a short walk (a quarter to a third of a mile) from several Marguerite stops:

- Stops for the SLAC and Oak Creek Apartments (OCA) routes are located on Oak Road north of Fremont Road and on Searsville Road west of Campus Drive
- Stops for the Hospital Direct route are located on Campus Drive at Searsville Road
- Stops for the SLAC, O, N, SE, and OCA lines are located on Santa Teresa Street east of Campus Drive

The O and N routes can be used by employees and shop workers commuting by Caltrain, VTA or SamTrans. In addition, the X and Y routes are located nearby on Santa Teresa Street at Via Ortega. Transit access for the Project site is considered adequate, given the proximity to Marguerite stops.

3.5 Service, Delivery and Emergency Access

Service and delivery vehicle access to the Project site would be provided via the building's driveway on Fremont Road. Vehicles will use the Project driveway to access the loading docks, and generally will be able to enter the driveway head-in and exit head-out. Larger vehicles requiring extra turn-around space can use the 50-foot radius turnaround located at the end of the driveway, northwest of the building, or they can back out onto Fremont Road if needed (this is not expected to be a frequent occurrence). The Project plan set includes truck maneuvering diagrams that demonstrate how the site can accommodate up to a WB-67 truck.



Emergency vehicle access would be provided via Searsville Road to Fremont Road. Fremont Road is a 24-foot curb-to-curb roadway, meeting the 20-foot minimum fire lane width. In addition, the Project site driveway will be 32 feet wide at its connection with Fremont Road, transitioning to a 24-foot travel way and a 12-foot parallel truck parking lane adjacent to the north edge of the building. The driveway terminates in a 50-foot radius turnaround, effectively providing fire lane access along the building's north and west faces.



4. Findings

The traffic operations analysis indicates that the intersections in the immediate vicinity of the Project site would continue to operate well, at LOS C or better. No intersection improvements are needed to serve the traffic changes with the Project. The Project design incorporates pedestrian improvements to facilitate pedestrian trips between the new building and other campus destinations. Bicycle access to the site will be via Fremont Road. Several Marguerite stops are located within a quarter-mile to a third of a mile from the Project site. The site has been designed to accommodate a WB-67 truck, both on the LBRE Replacement Building site and to and from the Electioneer Road shed. Emergency access is adequately provided via Fremont Road.



Appendix A: Traffic Counts

San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name: 1AM FINAL Site Code: 00000001 Start Date: 5/29/2019

Page No : 1

Groups Printed- Lights - Buses - Trucks

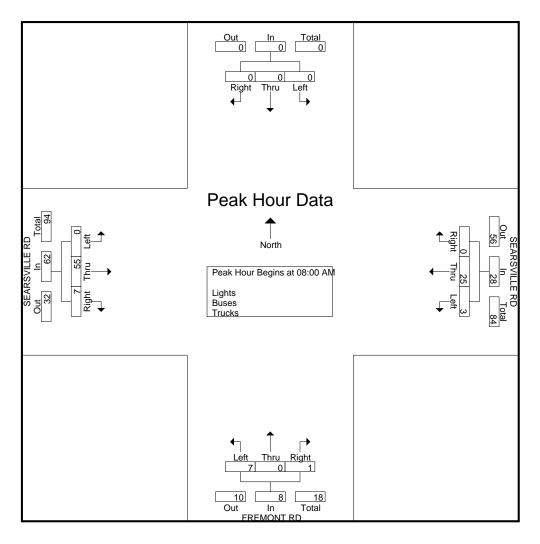
										u Ligii	<u> </u>		- 1 4 0 1				0545	201/11			
									LE RD)			EMON				_	_	LE RD)	
		Sc	uthbo	und			W	estbo	<u>und</u>			N	orthbo	und			E	astbou	<u>und</u>		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	0	0	0	0	0	0	3	1	1	5	1	0	1	3	5	1	10	0	1	12	22
07:15 AM	0	0	0	0	0	0	3	2	1	6	1	0	3	3	7	3	16	0	1	20	33
07:30 AM	0	0	0	0	0	0	2	0	1	3	1	0	2	9	12	2	9	0	2	13	28
07:45 AM	0	0	0	0	0	0	2	0	1	3	0	0	1	5	6	4	17	0	1	22	31_
Total	0	0	0	0	0	0	10	3	4	17	3	0	7	20	30	10	52	0	5	67	114
08:00 AM	0	0	0	0	0	0	7	1	0	8	1	0	1	2	4	4	14	0	0	18	30
08:15 AM	0	0	0	0	0	0	7	0	0	7	0	0	3	2	5	1	12	0	0	13	25
08:30 AM	0	0	0	0	0	0	5	0	0	5	0	0	1	11	12	2	14	0	3	19	36
08:45 AM	0	0	0	0	0	0	6	2	0	8	0	0	2	7	9	0	15	0	0	15	32
Total	0	0	0	0	0	0	25	3	0	28	1	0	7	22	30	7	55	0	3	65	123
Grand Total	0	0	0	0	0	0	35	6	4	45	4	0	14	42	60	17	107	0	8	132	237
Apprch %	0	0	0	0		0	77.8	13.3	8.9		6.7	0	23.3	70		12.9	81.1	0	6.1		
Total %	0	0	0	0	0	0	14.8	2.5	1.7	19	1.7	0	5.9	17.7	25.3	7.2	45.1	0	3.4	55.7	
Lights	0	0	0	0	0	0	35	5	4	44	4	0	13	42	59	16	104	0	8	128	231
% Lights	0	0	0	0	0	0	100	83.3	100	97.8	100	0	92.9	100	98.3	94.1	97.2	0	100	97	97.5
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	0	0	3	3
% Buses	0	0	0	0	0	0	0	0	0	0	Ō	0	0	0	0	0	2.8	0	0	2.3	1.3
Trucks	0	0	0	0	0	0	0	1	0	1	0	0	1	0	1	1	0	0	0	1	3
% Trucks	0	0	0	0	0	0	0	16.7	0	2.2	0	0	7.1	0	1.7	5.9	0	0	0	0.8	1.3

					5	SEARS\	/ILLE R	RD.		FREM	ONT RE)		SEARS\	/ILLE F	RD	
		South	bound			West	bound			North	bound			East	bound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	Peak Hour Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																
Peak Hour for E	Entire In	tersection	n Begi	ns at 08:0	00 AM												
08:00 AM	0	0	0	0	0	7	1	8	1	0	1	2	4	14	0	18	28
08:15 AM	0	0	0	0	0	7	0	7	0	0	3	3	1	12	0	13	23
08:30 AM	0	0	0	0	0	5	0	5	0	0	1	1	2	14	0	16	22
08:45 AM	0	0	0	0	0	6	2	8	0	0	2	2	0	15	0	15	25
Total Volume	0	0	0	0	0	25	3	28	1	0	7	8	7	55	0	62	98
% App. Total	0	0	0		0	89.3	10.7		12.5	0	87.5		11.3	88.7	0		
PHF	.000	.000	.000	.000	.000	.893	.375	.875	.250	.000	.583	.667	.438	.917	.000	.861	.875

San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name : 1AM FINAL Site Code : 00000001 Start Date : 5/29/2019

Page No : 2



San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name: 1PM FINAL Site Code: 00000001 Start Date: 5/29/2019

Page No : 1

Groups Printed- Lights - Buses - Trucks

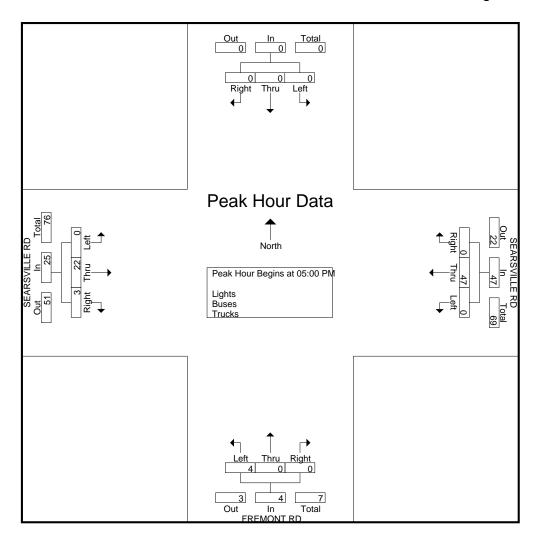
SEARSVILLE RD SEARSVILLE RD Northbound SEARSVILLE RD SEARSVILLE RD Northbound SEARSVILLE RD SE		T									u- Ligiti	15 - Du						CEAE	261/11	LEDD		
Start Time Right Thru Left Peds App. Total Right<			_								'										'	
04:00 PM 0 0 0 0 0 0 0 9 2 2 13 0 0 1 6 7 3 2 0 1 6 26 04:15 PM 0 0 0 0 0 0 0 7 0 0 7 2 0 1 6 9 4 8 0 0 12 28 04:30 PM 0 0 0 0 0 0 0 9 1 1 1 11 0 0 0 0 6 6 1 2 0 0 3 20 04:45 PM 0 0 0 0 0 0 0 6 2 0 8 0 0 1 4 5 2 4 0 0 6 19					und					und												
04:15 PM 0 0 0 0 0 0 0 7 0 0 7 2 0 1 6 9 4 8 0 0 12 28 04:30 PM 0 0 0 0 0 0 0 9 1 1 1 11 0 0 0 6 6 1 2 0 0 3 20 04:45 PM 0 0 0 0 0 0 0 6 2 0 8 0 0 1 4 5 2 4 0 0 6 19		Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:30 PM 0 0 0 0 0 0 0 9 1 1 11 0 0 0 6 6 1 2 0 0 3 20 04:45 PM 0 0 0 0 0 0 6 2 0 8 0 0 1 4 5 2 4 0 0 6 19		0	0	0	0	0	0	9	2	2	13	0	0	1	6	7	3	2	0	1	6	26
04:45 PM 0 0 0 0 0 0 6 2 0 8 0 0 1 4 5 2 4 0 0 6 19		0	0	0	0	0	0	7	0	0	7	2	0	1	6	9	4	8	0	0	12	28
	04:30 PM	0	0	0	0	0	0	9	1	1	11	0	0	0	6	6	1	2	0	0	3	20
Total 0 0 0 0 0 0 31 5 3 39 2 0 3 22 27 10 16 0 1 27 93	04:45 PM	0	0	0	0	0	0	6	2	0	8	0	0	1	4	5	2	4	0	0	6	19
	Total	0	0	0	0	0	0	31	5	3	39	2	0	3	22	27	10	16	0	1	27	93
05:00 PM 0 0 0 0 0 0 12 0 0 12 0 0 1 4 5 0 8 0 1 9 26	05:00 PM	0	0	0	0	0	0	12	0	0	12	0	0	1	4	5	0	8	0	1	9	26
05:15 PM 0 0 0 0 0 0 15 0 0 15 0 0 0 5 5 1 5 0 1 7 27	05:15 PM	0	0	0	0	0	0	15	0	0	15	0	0	0	5	5	1	5	0	1	7	27
05:30 PM 0 0 0 0 0 0 0 11 0 0 11 0 0 1 4 5 1 2 0 0 3 19	05:30 PM	0	0	0	0	0	0	11	0	0	11	0	0	1	4	5	1	2	0	0	3	19
05:45 PM 0 0 0 0 0 0 0 9 0 0 9 0 0 2 6 8 1 7 0 1 9 26	05:45 PM	0	0	0	0	0	0	9	0	0	9	0	0	2	6	8	1	7	0	1	9	26
Total 0 0 0 0 0 0 47 0 0 47 0 0 4 19 23 3 22 0 3 28 98	Total	0	0	0	0	0	0	47	0	0	47	0	0	4	19	23	3	22	0	3	28	98
																					•	
Grand Total 0 0 0 0 0 0 78 5 3 86 2 0 7 41 50 13 38 0 4 55 191	Grand Total	0	0	0	0	0	0	78	5	3	86	2	0	7	41	50	13	38	0	4	55	191
Apprch % 0 0 0 0 0 0 90.7 5.8 3.5 4 0 14 82 23.6 69.1 0 7.3	Apprch %	0	0	0	0		0	90.7	5.8	3.5		4	0	14	82		23.6	69.1	0	7.3		
Total % 0 0 0 0 0 0 0 40.8 2.6 1.6 45 1 0 3.7 21.5 26.2 6.8 19.9 0 2.1 28.8		0	0	0	0	0	0	40.8	2.6	1.6	45	1	0	3.7	21.5	26.2	6.8	19.9	0	2.1	28.8	
Lights 0 0 0 0 0 0 76 5 3 84 2 0 7 41 50 13 34 0 4 51 185	Lights	0	0	0	0	0	0	76		3	84	2	0	7	41		13	34	0	4		185
% Lights 0 0 0 0 0 0 97.4 100 100 97.7 100 0 100 100 100 89.5 0 100 92.7 96.9		0	0	0	0	0	0	97.4	100	100	97.7	100	0	100	100	100	100	89.5	0	100	92.7	
Buses 0 0 0 0 0 0 2 0 0 2 0 0 0 0 0 0 4 0 0 4 6		0		0			0						0									
% Buses 0 0 0 0 0 0 0 2.6 0 0 2.3 0 0 0 0 0 0 10.5 0 0 7.3 3.1		0	Ô	Ô	-	-	0	_	-	Ô			-	-	-	-	_	-	-	Ô	-	_
Trucks 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0		0			_					_								0	0	0
% Trucks 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0		0	0	0	0	-	0	0	0	0	0	_	0	-	-	-	_	-	-	0	0	0

					5	SEARS\	/ILLE R	RD		FREM	ONT RE)	5	SEARS\	/ILLE R	.D	
		South	bound			West	bound			North	bound			Eastl	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	lysis Fro	om 04:0	0 PM to	05:45 P	M - Peal	k 1 of 1											
Peak Hour for E	Entire In	tersection	n Begi	ns at 05:0	00 PM												
05:00 PM	0	0	0	0	0	12	0	12	0	0	1	1	0	8	0	8	21
05:15 PM	0	0	0	0	0	15	0	15	0	0	0	0	1	5	0	6	21
05:30 PM	0	0	0	0	0	11	0	11	0	0	1	1	1	2	0	3	15
05:45 PM	0	0	0	0	0	9	0	9	0	0	2	2	1	7	0	8	19
Total Volume	0	0	0	0	0	47	0	47	0	0	4	4	3	22	0	25	76
% App. Total	0	0	0		0	100	0		0	0	100		12	88	0		
PHF	.000	.000	.000	.000	.000	.783	.000	.783	.000	.000	.500	.500	.750	.688	.000	.781	.905

San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name: 1PM FINAL Site Code: 00000001 Start Date: 5/29/2019

Page No : 2



San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name : 2AM FINAL Site Code : 00000002

Start Date : 5/29/2019

Page No : 1

Groups Printed- Lights - Buses - Trucks

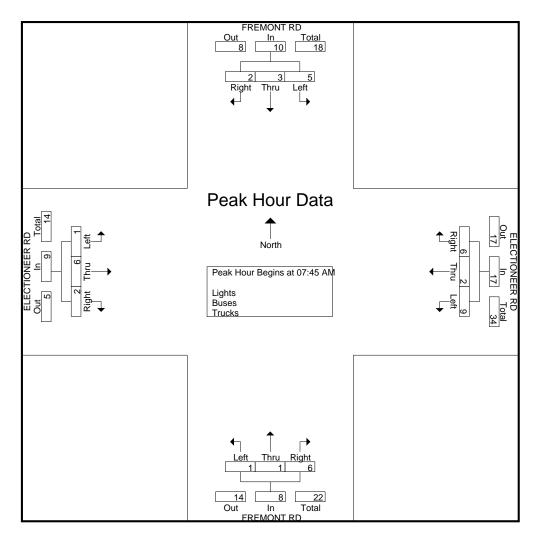
		EDI	EMON	TDD					ER RI	a- Lign	15 - Du		EMON				EL EC:	TIONE	ER RI	`	
										J										,	
			outhbo					estbo					orthbo					<u>astbo</u> ı			
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	0	0	1	0	1	2	0	0	0	2	0	0	0	0	0	0	1	0	0	1	4
07:15 AM	1	3	3	0	7	1	0	0	0	1	0	0	0	0	0	0	2	3	0	5	13
07:30 AM	0	0	2	0	2	3	1	0	0	4	0	0	0	0	0	0	0	0	0	0	6
07:45 AM	0	1_	3	0	4	1	1_	0	0	2	1	0	0	0	1	0	0	0	0	0	7_
Total	1	4	9	0	14	7	2	0	0	9	1	0	0	0	1	0	3	3	0	6	30
08:00 AM	1	1	2	0	4	1	0	2	0	3	3	0	0	0	3	0	1	1	0	2	12
08:15 AM	0	0	0	0	0	2	1	5	0	8	2	1	0	0	3	1	2	0	0	3	14
08:30 AM	1	1	0	0	2	2	0	2	0	4	0	0	1	2	3	1	3	0	0	4	13
08:45 AM	1	0	1	0	2	2	0	1	0	3	0	1	0	0	1	0	1	0	0	1	7
Total	3	2	3	0	8	7	1	10	0	18	5	2	1	2	10	2	7	1	0	10	46
Grand Total	4	6	12	0	22	14	3	10	0	27	6	2	1	2	11	2	10	4	0	16	76
Apprch %	18.2	27.3	54.5	0		51.9	11.1	37	0		54.5	18.2	9.1	18.2		12.5	62.5	25	0		
Total %	5.3	7.9	15.8	0	28.9	18.4	3.9	13.2	0	35.5	7.9	2.6	1.3	2.6	14.5	2.6	13.2	5.3	0	21.1	
Lights	4	6	9	0	19	13	1	10	0	24	6	2	0	2	10	1	5	3	0	9	62
% Lights	100	100	75	0	86.4	92.9	33.3	100	0	88.9	100	100	0	100	90.9	50	50	75	0	56.2	81.6
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Buses	Ó	Ö	Ō	Ö	0	Ö	Ó	Ö	Ö	Ö	Ö	Ö	0	0	Ö	Ö	Ö	Ō	0	0	Ó
Trucks	0	0	3	0	3	1	2	0	0	3	0	0	1	0	1	1	5	1	0	7	14
% Trucks	0	0	25	Ō	13.6	7.1	66.7	Ō	Ö	11.1	Ō	Ō	100	0	9.1	50	50	25	Ö	43.8	18.4
		-		-				-	-			-		-					-		

		FREMO	ONT RE)	El	LECTIO	NEER	RD		FREMO	ONT RD)	EI	LECTIO	NEER	RD	
		South	bound			Westl	oound			North	bound			Easth	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	our Analysis From 07:00 AM to 08:45 AM - Peak 1 of 1																
Peak Hour for E	Entire Int	tersection	on Begii	ns at 07:4	15 AM												
07:45 AM	0	1	3	4	1	1	0	2	1	0	0	1	0	0	0	0	7
08:00 AM	1	1	2	4	1	0	2	3	3	0	0	3	0	1	1	2	12
08:15 AM	0	0	0	0	2	1	5	8	2	1	0	3	1	2	0	3	14
08:30 AM	1	1	0	2	2	0	2	4	0	0	1	1	1	3	0	4	11_
Total Volume	2	3	5	10	6	2	9	17	6	1	1	8	2	6	1	9	44
% App. Total	20	30	50		35.3	11.8	52.9		75	12.5	12.5		22.2	66.7	11.1		
PHF	.500	.750	.417	.625	.750	.500	.450	.531	.500	.250	.250	.667	.500	.500	.250	.563	.786

San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name : 2AM FINAL Site Code : 00000002 Start Date : 5/29/2019

Page No : 2



San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name : 2PM FINAL Site Code : 00000002 Start Date : 5/29/2019

Page No : 1

Groups Printed- Lights - Buses - Trucks

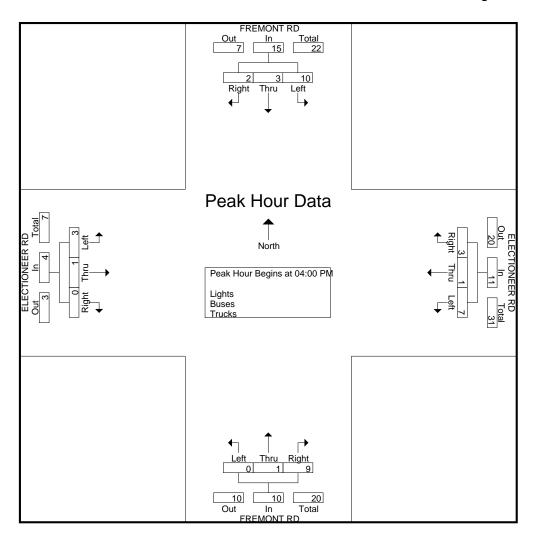
		FRE	MON	T RD					ER R	D Ligit	.o <u>Du</u>		MON				FLFC	TIONE	ER RI	ר	
			outhbo					estbo		_			orthbo				_	astbou		_	
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	0	1	2	0	3	0	0	3	0	3	0	0	0	0	0	0	0	2	0	2	8
04:15 PM	0	2	4	0	6	2	0	1	0	3	1	0	0	0	1	0	1	0	0	1	11
04:30 PM	1	0	2	0	3	0	0	0	0	0	1	0	0	0	1	0	0	1	0	1	5
04:45 PM	1	0	2	0	3	1	1	3	0	5	7	1	0	0	8	0	0	0	0	0	16
Total	2	3	10	0	15	3	1	7	0	11	9	1	0	0	10	0	1	3	0	4	40
05.00.514			•	•	•		•		•			•	•				•	•			
05:00 PM	0	0	0	0	0	1	0	1	2	4	2	0	0	1	3	0	2	0	0	2	9
05:15 PM	0	0	1	0	1	1	0	1	0	2	2	0	0	0	2	0	0	0	0	0	5
05:30 PM	0	1	2	0	3	1	0	1	0	2	0	0	0	0	0	0	0	0	0	0	5
05:45 PM	0	0	1	0	1	0	0	0	0	0	3	2	0	0	5	0	0	0	0	0	6
Total	0	1	4	0	5	3	0	3	2	8	7	2	0	1	10	0	2	0	0	2	25
	ı																				
Grand Total	2	4	14	0	20	6	1	10	2	19	16	3	0	1	20	0	3	3	0	6	65
Apprch %	10	20	70	0		31.6	5.3	52.6	10.5		80	15	0	5		0	50	50	0		
Total %	3.1	6.2	21.5	0	30.8	9.2	1.5	15.4	3.1	29.2	24.6	4.6	0	1.5	30.8	0	4.6	4.6	0	9.2	
Lights	2	4	14	0	20	6	1	10	2	19	16	3	0	1	20	0	3	3	0	6	65
% Lights	100	100	100	0	100	100	100	100	100	100	100	100	0	100	100	0	100	100	0	100	100
Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Buses	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
% Trucks	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0

		FREMO	ONT RE)	El	ECTIO	NEER	RD		FREMO	ONT RE)	Е	LECTIO	NEER	RD	
		South	bound			West	oound			North	bound			Eastl	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	lysis Fro	m 04:0	0 PM to	05:45 P	M - Peal	< 1 of 1											
Peak Hour for E	Entire Int	tersection	on Begi	ns at 04:0	00 PM												
04:00 PM	0	1	2	3	0	0	3	3	0	0	0	0	0	0	2	2	8
04:15 PM	0	2	4	6	2	0	1	3	1	0	0	1	0	1	0	1	11
04:30 PM	1	0	2	3	0	0	0	0	1	0	0	1	0	0	1	1	5
04:45 PM	1	0	2	3	1	1	3	5	7	1	0	8	0	0	0	0	16
Total Volume	2	3	10	15	3	1	7	11	9	1	0	10	0	1	3	4	40
% App. Total	13.3	20	66.7		27.3	9.1	63.6		90	10	0		0	25	75		
PHF	.500	.375	.625	.625	.375	.250	.583	.550	.321	.250	.000	.313	.000	.250	.375	.500	.625

San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name : 2PM FINAL Site Code : 00000002 Start Date : 5/29/2019

Page No : 2



San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name: 3AM FINAL Site Code: 00000003

Start Date : 5/29/2019

Page No : 1

Groups Printed- Lights - Buses - Trucks

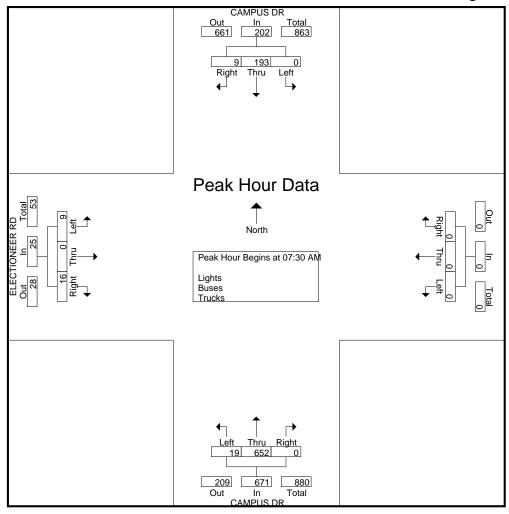
							G	roups	Printe	a- Ligni	S - Bu										ì
			MPUS									_	MPUS						ER RI	י	i
		Sc	<u>uthbo</u>	<u>und</u>			W	<u>estbo</u>	<u>und</u>			N	<u>orthbo</u>	und			E;	<u>astbou</u>	<u>ınd</u>		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
07:00 AM	1	27	0	0	28	0	0	0	0	0	0	117	2	0	119	0	0	2	0	2	149
07:15 AM	1	34	0	0	35	0	0	0	0	0	0	170	4	0	174	0	0	2	0	2	211
07:30 AM	1	58	0	0	59	0	0	0	0	0	0	175	5	0	180	3	0	1	0	4	243
07:45 AM	3	74	0	0	77	0	0	0	0	0	0	138	6	2	146	5	0	2	1	8	231
Total	6	193	0	0	199	0	0	0	0	0	0	600	17	2	619	8	0	7	1	16	834
																					1
MA 00:80	0	35	0	0	35	0	0	0	0	0	0	168	6	0	174	4	0	2	0	6	215
08:15 AM	5	26	0	0	31	0	0	0	0	0	0	171	2	2	175	4	0	4	0	8	214
08:30 AM	2	29	1	0	32	0	0	0	0	0	0	159	2	3	164	1	0	3	0	4	200
08:45 AM	3	25	0	0	28	0	0	0	0	0	0	171	3	2	176	2	0	2	0	4	208
Total	10	115	1	0	126	0	0	0	0	0	0	669	13	7	689	11	0	11	0	22	837
Grand Total	16	308	1	0	325	0	0	0	0	0	0	1269	30	9	1308	19	0	18	1	38	1671
Apprch %	4.9	94.8	0.3	0		0	0	0	0		0	97	2.3	0.7		50	0	47.4	2.6		i
Total %	1	18.4	0.1	0	19.4	0	0	0	0	0	0	75.9	1.8	0.5	78.3	1.1	0	1.1	0.1	2.3	i
Lights	10	292	1	0	303	0	0	0	0	0	0	1254	30	9	1293	17	0	11	1	29	1625
% Lights	62.5	94.8	100	0	93.2	0	0	0	0	0	0	98.8	100	100	98.9	89.5	0	61.1	100	76.3	97.2
Buses	0	12	0	0	12	0	0	0	0	0	0	6	0	0	6	0	0	0	0	0	18
% Buses	0	3.9	0	0	3.7	0	0	0	0	0	0	0.5	0	0	0.5	0	0	0	0	0	1.1
Trucks	6	4	0	0	10	0	0	0	0	0	0	9	0	0	9	2	0	7	0	9	28
% Trucks	37.5	1.3	0	0	3.1	0	0	0	0	0	0	0.7	0	0	0.7	10.5	0	38.9	0	23.7	1.7

		CAMP	US DR							CAMP	US DR		E	LECTIO	NEER	RD	
		South	bound			Westl	oound			North	bound			Eastb	ound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	lysis Fro	om 07:0	0 AM to	08:45 AI	M - Peal	(1 of 1											
Peak Hour for E	Entire In	tersection	on Begii	ns at 07:3	30 AM												
07:30 AM	1	58	o o	59	0	0	0	0	0	175	5	180	3	0	1	4	243
07:45 AM	3	74	0	77	0	0	0	0	0	138	6	144	5	0	2	7	228
08:00 AM	0	35	0	35	0	0	0	0	0	168	6	174	4	0	2	6	215
08:15 AM	5	26	0	31	0	0	0	0	0	171	2	173	4	0	4	8	212
Total Volume	9	193	0	202	0	0	0	0	0	652	19	671	16	0	9	25	898
% App. Total	4.5	95.5	0		0	0	0		0	97.2	2.8		64	0	36		
PHF	.450	.652	.000	.656	.000	.000	.000	.000	.000	.931	.792	.932	.800	.000	.563	.781	.924

San Jose, CA (408) 622-4787 tdsbay@cs.com

> File Name : 3AM FINAL Site Code : 00000003 Start Date : 5/29/2019

Page No : 2



San Jose, CA (408) 622-4787 tdsbay@cs.com

File Name : 3PM FINAL Site Code : 00000003

Start Date : 5/29/2019

Page No : 1

Groups Printed- Lights - Buses - Trucks

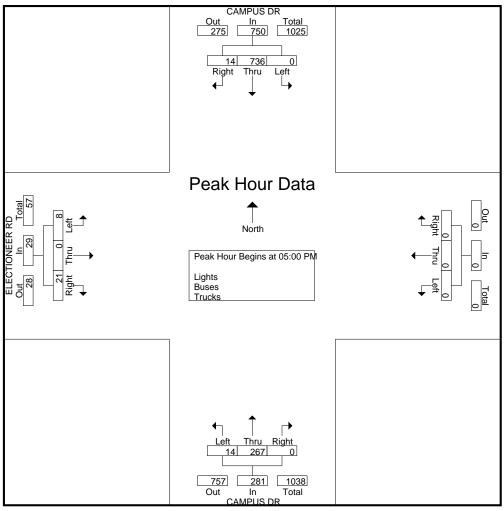
						_		ioups	FIIIILE	u- Lign	19 - Du										i
		CA	MPUS	S DR								CA	MPUS	S DR			ELEC	TIONE	ER RI)	
		Sc	uthbo	und			W	estbo	und			No	orthbo	und			E	astbou	ınd		
Start Time	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Right	Thru	Left	Peds	App. Total	Int. Total
04:00 PM	0	139	0	0	139	0	0	0	0	0	0	55	7	2	64	1	0	2	0	3	206
04:15 PM	5	158	0	0	163	0	0	0	0	0	0	57	2	0	59	5	0	4	1	10	232
04:30 PM	3	194	0	0	197	0	0	0	0	0	0	63	0	1	64	3	0	3	0	6	267
04:45 PM	4	177	0	0	181	0	0	0	0	0	0	68	4	1_	73	9	0	1	1	11	265
Total	12	668	0	0	680	0	0	0	0	0	0	243	13	4	260	18	0	10	2	30	970
05:00 PM	4	181	0	0	185	0	0	0	0	0	0	58	3	2	63	6	0	1	3	10	258
05:15 PM	6	184	0	0	190	0	0	0	0	0	0	73	2	1	76	4	0	1	1	6	272
05:30 PM	3	196	0	0	199	0	0	0	0	0	0	51	6	4	61	5	0	4	2	11	271
05:45 PM	1	175	0	0	176	0	0	0	0	0	0	85	3	2	90	6	0	2	0	8	274
Total	14	736	0	0	750	0	0	0	0	0	0	267	14	9	290	21	0	8	6	35	1075
Grand Total	26	1404	0	0	1430	0	0	0	0	0	0	510	27	13	550	39	0	18	8	65	2045
Apprch %	1.8	98.2	0	0		0	0	0	0		0	92.7	4.9	2.4		60	0	27.7	12.3		
Total %	1.3	68.7	0	0	69.9	0	0	0	0	0	0	24.9	1.3	0.6	26.9	1.9	0	0.9	0.4	3.2	
Lights	25	1388	0	0	1413	0	0	0	0	0	0	503	27	13	543	39	0	18	8	65	2021
% Lights	96.2	98.9	0	0	98.8	0	0	0	0	0	0	98.6	100	100	98.7	100	0	100	100	100	98.8
Buses	0	10	0	0	10	0	0	0	0	0	0	5	0	0	5	0	0	0	0	0	15
% Buses	0	0.7	0	0	0.7	0	0	0	0	0	0	1	0	0	0.9	0	0	0	0	0	0.7
Trucks	1	6	0	0	7	0	0	0	0	0	0	2	0	0	2	0	0	0	0	0	9
% Trucks	3.8	0.4	0	0	0.5	0	0	0	0	0	0	0.4	0	0	0.4	0	0	0	0	0	0.4

		CAMP	US DR							CAMP	US DR		El	ECTIO	NEER	RD	
		South	bound			West	oound			North	bound			Eastl	oound		
Start Time	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Right	Thru	Left	App. Total	Int. Total
Peak Hour Ana	llysis Fro	om 04:0	0 PM to	05:45 PI	M - Peal	< 1 of 1											
Peak Hour for E	Entire In	tersection	n Begi	ns at 05:0	00 PM												
05:00 PM	4	181	0	185	0	0	0	0	0	58	3	61	6	0	1	7	253
05:15 PM	6	184	0	190	0	0	0	0	0	73	2	75	4	0	1	5	270
05:30 PM	3	196	0	199	0	0	0	0	0	51	6	57	5	0	4	9	265
05:45 PM	1	175	0	176	0	0	0	0	0	85	3	88	6	0	2	8	272
Total Volume	14	736	0	750	0	0	0	0	0	267	14	281	21	0	8	29	1060
% App. Total	1.9	98.1	0		0	0	0		0	95	5		72.4	0	27.6		
PHF	.583	.939	.000	.942	.000	.000	.000	.000	.000	.785	.583	.798	.875	.000	.500	.806	.974

San Jose, CA (408) 622-4787 tdsbay@cs.com

> File Name : 3PM FINAL Site Code : 00000003 Start Date : 5/29/2019

Page No : 2



Appendix B: LOS Results

Intersection						
Int Delay, s/veh	1					
	•			==		
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations					¥	
Traffic Vol, veh/h	55	7	3	25	7	1
Future Vol, veh/h	55	7	3	25	7	1
Conflicting Peds, #/hr	0	0	0	0	0	3
	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	63	8	3	28	8	1
Major/Minor NA	laior1		/laior2		Minor1	
	lajor1		Major2			70
Conflicting Flow All	0	0	71	0	101	70
Stage 1	-	-	-	-	67	-
Stage 2	-	-	-	-	34	-
Critical Hdwy	-	-	4.13	-	6.43	6.23
Critical Hdwy Stg 1	-	-	-	-	5.43	-
Critical Hdwy Stg 2	-	-	-	-	5.43	-
Follow-up Hdwy	-	-	2.227	-	3.527	
Pot Cap-1 Maneuver	-	-	1523	-	895	990
Stage 1	-	-	-	-	953	-
Stage 2	-	-	-	-	986	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1523	-	893	987
Mov Cap-2 Maneuver	-	-	-	-	893	-
Stage 1	-	-	-	-	951	-
Stage 2	_	_	_	_	986	-
2.030 =						
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		9	
HCM LOS					Α	
Minor Lane/Major Mvmt	1	NBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		904	-		1523	-
HCM Lane V/C Ratio		0.01			0.002	
HCM Control Delay (s)		9	-	-	7.4	-
HCM Lane LOS		A		-	7.4 A	-
HCM 95th %tile Q(veh)		0	-		0	
HOW BOTH WITH MICHAEL		U	-	-	U	-

Intersection			
Intersection Delay, s/veh	7.3		
Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	6	2	9	2	6	1	1	6	5	3	2
Future Vol, veh/h	1	6	2	9	2	6	1	1	6	5	3	2
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	20	20	20	20	20	20	20	20	20	20	20	20
Mvmt Flow	1	8	3	11	3	8	1	1	8	6	4	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.3			7.3			6.9			7.4		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	12%	11%	53%	50%	
Vol Thru, %	12%	67%	12%	30%	
Vol Right, %	75%	22%	35%	20%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	8	9	17	10	
LT Vol	1	1	9	5	
Through Vol	1	6	2	3	
RT Vol	6	2	6	2	
Lane Flow Rate	10	11	22	13	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.011	0.013	0.025	0.015	
Departure Headway (Hd)	3.881	4.184	4.182	4.284	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	923	857	858	837	
Service Time	1.902	2.201	2.196	2.304	
HCM Lane V/C Ratio	0.011	0.013	0.026	0.016	
HCM Control Delay	6.9	7.3	7.3	7.4	
HCM Lane LOS	А	Α	Α	Α	
HCM 95th-tile Q	0	0	0.1	0	

Intersection						
Int Delay, s/veh	0.5					
		EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	À	4.0		4	Ą.	
Traffic Vol, veh/h	9	16	19	652	193	9
Future Vol, veh/h	9	16	19	652	193	9
Conflicting Peds, #/hr	4	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	92	92	92	92	92	92
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	10	17	21	709	210	10
NA : (NA)		_				
	Minor2		Major1		//ajor2	
Conflicting Flow All	970	215	220	0	-	0
Stage 1	215	-	-	-	-	-
Stage 2	755	-	-	-	-	-
Critical Hdwy	6.43	6.23	4.13	-	-	-
Critical Hdwy Stg 1	5.43	-	-	_	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.327	2.227	-	-	-
Pot Cap-1 Maneuver	280	822	1343	-	-	-
Stage 1	818	-	-	-	-	-
Stage 2	462	-	_	-	-	-
Platoon blocked, %				-	_	_
Mov Cap-1 Maneuver	273	822	1343	_	_	_
Mov Cap-1 Maneuver	273	022	1070	_	_	
Stage 1	797	-	_	_	<u>-</u>	-
<u> </u>		-	-	-	-	-
Stage 2	462	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	13		0.2		0	
HCM LOS	В		V			
					055	05-
Minor Lane/Major Mvm	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		1343	-	2.5	-	-
HCM Lane V/C Ratio		0.015	-	0.057	-	-
HCM Control Delay (s)		7.7	0	13	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-

1: Fremont Rd & Oak Rd/Searsville Rd

Intersection						
Int Delay, s/veh	0.5					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	1	LDI	VVDL	4	¥	NDIX
Traffic Vol, veh/h	22	3	0	47	4	0
Future Vol, veh/h	22	3	0	47	4	0
	0	0	0	0	0	3
Conflicting Peds, #/hr						
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	-	-	-	0	-
Veh in Median Storage,		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	5	5	5	5	5	5
Mvmt Flow	24	3	0	52	4	0
Major/Minor N	lajor1	N	Major2		Minor1	
Conflicting Flow All	0	0	27	0	78	29
Stage 1	-	_	-	-	26	-
Stage 2	_		_	_	52	-
		-	4.15		6.45	6.25
Critical Hdwy	-	-	4.15	-		
Critical Hdwy Stg 1	-	-	-	-	5.45	-
Critical Hdwy Stg 2	-	-	-	-	5.45	-
Follow-up Hdwy	-	-	2.245	-	3.545	
Pot Cap-1 Maneuver	-	-	1568	-	917	1037
Stage 1	-	-	-	-	989	-
Stage 2	-	-	-	-	963	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1568	-	917	1034
Mov Cap-2 Maneuver	-	_	-	-	917	-
Stage 1	-	-	-	-	989	-
Stage 2	_	_	_	_	963	_
Olugo Z					500	
Approach	EB		WB		NB	
HCM Control Delay, s	0		0		8.9	
HCM LOS					Α	
Minor Lane/Major Mvmt	N	NBLn1	EBT	EBR	WBL	WBT
	ľ					
Capacity (veh/h)		917	-		1568	-
HCM Lane V/C Ratio		0.005	-	-	-	-
HCM Control Delay (s)		8.9	-	-	0	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0	-	-	0	-

Intersection			
Intersection Delay, s/veh	7	 _	
Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	1	0	7	1	3	0	1	9	10	3	2
Future Vol, veh/h	3	1	0	7	1	3	0	1	9	10	3	2
Peak Hour Factor	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3
Mvmt Flow	5	2	0	11	2	5	0	2	15	16	5	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB				NB		SB		
Opposing Approach	WB			EB				SB		NB		
Opposing Lanes	1			1				1		1		
Conflicting Approach Left	SB			NB				EB		WB		
Conflicting Lanes Left	1			1				1		1		
Conflicting Approach Right	NB			SB				WB		EB		
Conflicting Lanes Right	1			1				1		1		
HCM Control Delay	7.2			7.1				6.5		7.2		
HCM LOS	Α			Α				Α		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	75%	64%	67%	
Vol Thru, %	10%	25%	9%	20%	
Vol Right, %	90%	0%	27%	13%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	10	4	11	15	
LT Vol	0	3	7	10	
Through Vol	1	1	1	3	
RT Vol	9	0	3	2	
Lane Flow Rate	16	6	18	24	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.016	0.008	0.02	0.027	
Departure Headway (Hd)	3.472	4.185	3.99	4.059	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	1032	857	899	884	
Service Time	1.488	2.202	2.005	2.073	
HCM Lane V/C Ratio	0.016	0.007	0.02	0.027	
HCM Control Delay	6.5	7.2	7.1	7.2	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0	0	0.1	0.1	

Intersection						
Int Delay, s/veh	0.6					
	EDI	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥			र्स	\$	
Traffic Vol, veh/h	8	21	14	267	736	14
Future Vol, veh/h	8	21	14	267	736	14
Conflicting Peds, #/hr	9	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	, # 0	-	_	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	8	22	14	275	759	14
IVIVIII(I IOW	U	22	17	210	100	17
Major/Minor	Minor2	I	Major1	N	Major2	
Conflicting Flow All	1078	766	773	0	-	0
Stage 1	766	_	-	-	-	-
Stage 2	312	-	-	_	_	_
Critical Hdwy	6.43	6.23	4.13	_	_	_
Critical Hdwy Stg 1	5.43	-	-	_	_	_
Critical Hdwy Stg 2	5.43	_	_	_	_	_
Follow-up Hdwy	3.527	3.327	2.227		_	
Pot Cap-1 Maneuver	241	401	838	-		-
			030	-	-	-
Stage 1	457	-	-	-	-	-
Stage 2	740	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	236	401	838	-	-	-
Mov Cap-2 Maneuver	236	-	-	-	-	-
Stage 1	448	-	-	-	-	-
Stage 2	740	-	-	-	-	-
, and the second						
A mara a ab	ED		ND		O.D.	
Approach	EB		NB		SB	
HCM Control Delay, s	16.8		0.5		0	
HCM LOS	С					
Minor Lane/Major Mvm	nt	NBL	NRT	EBLn1	SBT	SBR
	IL					SDR
Capacity (veh/h)		838	-		-	-
HCM Lane V/C Ratio		0.017		0.089	-	-
		() /	0	16.8	_	-
HCM Control Delay (s)		9.4				
HCM Control Delay (s) HCM Lane LOS HCM 95th %tile Q(veh)		9.4 A 0.1	A	C 0.3	-	-

1: Fremont Rd & Oak Rd/Searsville Rd

Intersection						
Int Delay, s/veh	1.9					
	EBT	EBR	WBL	WBT	NBL	NBR
		EBK	WBL			NDK
Lane Configurations	†	0	0	↑	**	7
Traffic Vol, veh/h	55	9	9	25	9	7
Future Vol, veh/h	55	9	9	25	9	7
Conflicting Peds, #/hr	0	0	0	0	0	3
•	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	
Storage Length	-	-	-	-	0	-
Veh in Median Storage, #		-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	88	88	88	88	88	88
Heavy Vehicles, %	16	16	16	16	16	16
Mvmt Flow	63	10	10	28	10	8
Major/Minor M	nior1		Major?		Minor1	
	ajor1		Major2			74
Conflicting Flow All	0	0	73	0	116	71
Stage 1	-	-	-	-	68	-
Stage 2	-	-	-	-	48	-
Critical Hdwy	-	-	4.26	-	6.56	6.36
Critical Hdwy Stg 1	-	-	-	-	5.56	-
Critical Hdwy Stg 2	-	-	-	-	5.56	-
Follow-up Hdwy	-	-	2.344	-	3.644	3.444
Pot Cap-1 Maneuver	-	-	1442	-	848	954
Stage 1	-	-	-	-	920	-
Stage 2	-	-	-	-	940	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1442	-	842	951
Mov Cap-2 Maneuver	_	_	-	_	842	-
Stage 1				_	914	_
•	_				940	_
Stage 2	-	-	-	-	940	-
Approach	EB		WB		NB	
HCM Control Delay, s	0		2		9.1	
HCM LOS	•		_		A	
					, \	
Minor Lane/Major Mvmt	1	VBLn1	EBT	EBR	WBL	WBT
Capacity (veh/h)		886	-		1442	-
HCM Lane V/C Ratio		0.021	-	-	0.007	-
HCM Control Delay (s)		9.1	-	-	7.5	-
HCM Lane LOS		Α	-	-	Α	-
HCM 95th %tile Q(veh)		0.1	-	-	0	-
		• • •			•	

Intersection			
Intersection Delay, s/veh	7.3		
Intersection LOS	Α		

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	1	6	2	9	2	6	1	1	6	5	3	2
Future Vol, veh/h	1	6	2	9	2	6	1	1	6	5	3	2
Peak Hour Factor	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79	0.79
Heavy Vehicles, %	20	20	20	20	20	20	20	20	20	20	20	20
Mvmt Flow	1	8	3	11	3	8	1	1	8	6	4	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB			NB			SB		
Opposing Approach	WB			EB			SB			NB		
Opposing Lanes	1			1			1			1		
Conflicting Approach Left	SB			NB			EB			WB		
Conflicting Lanes Left	1			1			1			1		
Conflicting Approach Right	NB			SB			WB			EB		
Conflicting Lanes Right	1			1			1			1		
HCM Control Delay	7.3			7.3			6.9			7.4		
HCM LOS	Α			Α			Α			Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	12%	11%	53%	50%	
Vol Thru, %	12%	67%	12%	30%	
Vol Right, %	75%	22%	35%	20%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	8	9	17	10	
LT Vol	1	1	9	5	
Through Vol	1	6	2	3	
RT Vol	6	2	6	2	
Lane Flow Rate	10	11	22	13	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.011	0.013	0.025	0.015	
Departure Headway (Hd)	3.881	4.184	4.182	4.284	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Сар	923	857	858	837	
Service Time	1.902	2.201	2.196	2.304	
HCM Lane V/C Ratio	0.011	0.013	0.026	0.016	
HCM Control Delay	6.9	7.3	7.3	7.4	
HCM Lane LOS	А	Α	Α	Α	
HCM 95th-tile Q	0	0	0.1	0	

Intersection						
Int Delay, s/veh	0.5					
Movement	EDI	EBR	NDI	NPT	CPT	CDD
	EBL	EBK	NBL	NBT	SBT	SBR
Lane Configurations	M		10	4	\$	_
Traffic Vol, veh/h	9	16	19	653	194	9
Future Vol, veh/h	9	16	19	653	194	9
Conflicting Peds, #/hr	4	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	_	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage		_	_	0	0	_
Grade, %	0	_	_	0	0	_
Peak Hour Factor	92	92	92	92	92	92
	3	3	3	3	3	3
Heavy Vehicles, %						
Mvmt Flow	10	17	21	710	211	10
Major/Minor	Minor2		Major1	I.	/lajor2	
Conflicting Flow All	972	216	221	0	-	0
	216	210				
Stage 1			-	-	-	-
Stage 2	756	-	-	-	-	-
Critical Hdwy	6.43	6.23	4.13	-	-	-
Critical Hdwy Stg 1	5.43	-	-	-	-	-
Critical Hdwy Stg 2	5.43	-	-	-	-	-
Follow-up Hdwy	3.527	3.327	2.227	-	-	-
Pot Cap-1 Maneuver	279	821	1342	-	-	-
Stage 1	818	-	-	-	-	-
Stage 2	462	_	_	_	_	_
Platoon blocked, %				_	_	_
Mov Cap-1 Maneuver	272	821	1342	_	_	_
Mov Cap-2 Maneuver	272	-	-	-	-	-
Stage 1	797	-	-	-	-	-
Stage 2	462	-	-	-	-	-
Approach	EB		NB		SB	
HCM Control Delay, s	13		0.2		0	
HCM LOS	В					
Minor Lane/Major Mvn	nt	NBL	NRT	EBLn1	SBT	SBR
						אומט
Capacity (veh/h)		1342	-	475	-	-
HCM Lane V/C Ratio		0.015		0.057	-	-
HCM Control Delay (s)		7.7	0	13	-	-
HCM Lane LOS		Α	Α	В	-	-
HCM 95th %tile Q(veh)	0	-	0.2	-	-
-						

1: Fremont Rd & Oak Rd/Searsville Rd

Intersection						
Int Delay, s/veh	1.6					
Movement	EBT	EBR	WBL	WBT	NBL	NBR
Lane Configurations	î,			4	¥	
Traffic Vol, veh/h	22	5	6	47	6	6
Future Vol, veh/h	22	5	6	47	6	6
Conflicting Peds, #/hr	0	0	0	0	0	3
Sign Control	Free	Free	Free	Free	Stop	Stop
RT Channelized	-	None	-	None	-	None
Storage Length	-	-	-	-	0	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	90	90	90	90	90	90
Heavy Vehicles, %	21	21	21	21	21	21
Mvmt Flow	24	6	7	52	7	7
					•	
N. 4 . 1 . 1 . 1 . 1 . 1 . 1 . 1 . 1 . 1						
	Major1		Major2		Minor1	
Conflicting Flow All	0	0	30	0	93	30
Stage 1	-	-	-	-	27	-
Stage 2	-	-	-	-	66	-
Critical Hdwy	-	-	4.31	-	6.61	6.41
Critical Hdwy Stg 1	-	-	-	-	5.61	-
Critical Hdwy Stg 2	-	-	-	-	5.61	-
Follow-up Hdwy	-	-	2.389	-	3.689	3.489
Pot Cap-1 Maneuver	-	-	1468	-	863	992
Stage 1	-	-	-	-	949	-
Stage 2	-	-	-	-	911	-
Platoon blocked, %	-	-		-		
Mov Cap-1 Maneuver	-	-	1468	-	859	989
Mov Cap-2 Maneuver	-	-	_	-	859	-
Stage 1	-	-	-	-	944	-
Stage 2	_	-	_	-	911	-
g 						
Approach	EB		WB		NB	
HCM Control Delay, s	0		0.8		9	
HCM LOS					Α	
Minor Lane/Major Mvn	nt t	NBLn1	EBT	EBR	WBL	WBT
	it I	919		-	1468	
Capacity (veh/h) HCM Lane V/C Ratio			-			-
		0.015	-		0.005	-
HCM Lang LOS		9	-	-	7.5	0
HCM Lane LOS HCM 95th %tile Q(veh	١	A 0	-	-	A 0	Α
ncivi yain %ille U(ven	1	U	-	-	U	-

Intersection		
Intersection Delay, s/veh	7	
Intersection LOS	Α	

Movement	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations		4			4			4			4	
Traffic Vol, veh/h	3	1	0	7	1	3	0	1	9	10	3	2
Future Vol, veh/h	3	1	0	7	1	3	0	1	9	10	3	2
Peak Hour Factor	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62	0.62
Heavy Vehicles, %	3	3	3	3	3	3	3	3	3	3	3	3
Mvmt Flow	5	2	0	11	2	5	0	2	15	16	5	3
Number of Lanes	0	1	0	0	1	0	0	1	0	0	1	0
Approach	EB			WB				NB		SB		
Opposing Approach	WB			EB				SB		NB		
Opposing Lanes	1			1				1		1		
Conflicting Approach Left	SB			NB				EB		WB		
Conflicting Lanes Left	1			1				1		1		
Conflicting Approach Right	NB			SB				WB		EB		
Conflicting Lanes Right	1			1				1		1		
HCM Control Delay	7.2			7.1				6.5		7.2		
HCM LOS	Α			Α				Α		Α		

Lane	NBLn1	EBLn1	WBLn1	SBLn1	
Vol Left, %	0%	75%	64%	67%	
Vol Thru, %	10%	25%	9%	20%	
Vol Right, %	90%	0%	27%	13%	
Sign Control	Stop	Stop	Stop	Stop	
Traffic Vol by Lane	10	4	11	15	
LT Vol	0	3	7	10	
Through Vol	1	1	1	3	
RT Vol	9	0	3	2	
Lane Flow Rate	16	6	18	24	
Geometry Grp	1	1	1	1	
Degree of Util (X)	0.016	0.008	0.02	0.027	
Departure Headway (Hd)	3.472	4.185	3.99	4.059	
Convergence, Y/N	Yes	Yes	Yes	Yes	
Cap	1032	857	899	884	
Service Time	1.488	2.202	2.005	2.073	
HCM Lane V/C Ratio	0.016	0.007	0.02	0.027	
HCM Control Delay	6.5	7.2	7.1	7.2	
HCM Lane LOS	Α	Α	Α	Α	
HCM 95th-tile Q	0	0	0.1	0.1	

Intersection	_					
Int Delay, s/veh	0.6					
Movement	ĘDI.	EDD	NDI	NDT	CDT	CDD
Movement	EBL	EBR	NBL	NBT	SBT	SBR
Lane Configurations	¥	•		4	♣	
Traffic Vol, veh/h	8	21	14	268	737	14
Future Vol, veh/h	8	21	14	268	737	14
Conflicting Peds, #/hr	9	0	0	0	0	0
Sign Control	Stop	Stop	Free	Free	Free	Free
RT Channelized	-	None	-	None	-	None
Storage Length	0	-	-	-	-	-
Veh in Median Storage	e, # 0	-	-	0	0	-
Grade, %	0	-	-	0	0	-
Peak Hour Factor	97	97	97	97	97	97
Heavy Vehicles, %	3	3	3	3	3	3
Mvmt Flow	8	22	14	276	760	14
			11	_10	. 00	
Major/Minor	Minor2	l	Major1	N	/lajor2	
Conflicting Flow All	1080	767	774	0		0
Stage 1	767	-	_	-	_	-
Stage 2	313	_	_	_	_	_
Critical Hdwy	6.43	6.23	4.13	_	_	_
Critical Hdwy Stg 1	5.43	0.23	₹.13	_	_	_
	5.43		-		-	
Critical Hdwy Stg 2		2 227	2 227	-		-
Follow-up Hdwy		3.327		-	-	-
Pot Cap-1 Maneuver	240	401	837	-	-	-
Stage 1	457	-	-	-	-	-
Stage 2	739	-	-	-	-	-
Platoon blocked, %				-	-	-
Mov Cap-1 Maneuver	235	401	837	-	-	-
Mov Cap-2 Maneuver	235	-	-	-	-	-
Stage 1	448	-	-	-	-	-
Stage 2	739	_	-	_	-	_
J	. 00					
Approach	EB		NB		SB	
HCM Control Delay, s	16.8		0.5		0	
HCM LOS	С					
, <u></u>						
Minor Lane/Major Mvn	nt	NBL	NBT	EBLn1	SBT	SBR
Capacity (veh/h)		837	-		-	-
HCM Lane V/C Ratio		0.017	-	0.089	-	-
HCM Control Delay (s)	9.4	0	16.8	-	-
HCM Lane LOS		Α	Α	С	-	-
HCM 95th %tile Q(veh)	0.1	_	0.3	_	-
	7	J .,		3.0		