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# NWC #5 FACILITY

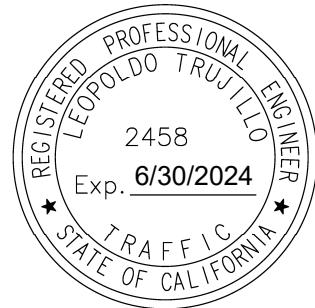
## TRAFFIC IMPACT ANALYSIS

### DRAFT REPORT

SANTA CLARA COUNTY, CALIFORNIA

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## **EXECUTIVE SUMMARY**

The proposed NWC #5 project involves the establishment of a storage and maintenance area for portable toilets south the intersection of Monterey Road and California Avenue in San Martin, Santa Clara County, California. Although the study property does not have frontage on either street, it does have a driveway off Monterey Road.

This report is a revision of the previous report dated November 14, 2022. This updated report was prepared due to comments received from the Santa Clara County Roads and Airports dated January 18, 2023.

### **Study Network**

AM and PM peak periods were analyzed at the following two intersections:

1. Monterey Road / California Avenue; and
2. Monterey Road / San Martin Avenue.

In addition, the project driveway was analyzed.

Traffic operations for the following analysis scenarios were analyzed, as requested by Santa Clara County Roads and Airports Department:

- Existing Conditions
- Existing Plus Project Conditions
- Cumulative Without Project Conditions
- Cumulative Plus Project Conditions

### **Existing Conditions**

This traffic study uses a hybrid approach to estimate traffic volumes for this study. Whereas volumes at Monterey Road / California Avenue was approximated from counts prior to the COVID-19 pandemic (2018), new traffic counts (2022) were performed at the Monterey Road / San Martin Avenue intersection. The Monterey / California intersection was adjusted up to Year 2022 volumes using growth rates (3% per year, or 12% total) derived from data published by the Santa Clara County Roads and Airports Department. The Monterey / San Martin intersection was subsequently adjusted upwards to be more reflective of the adjusted volumes at the Monterey / California intersection. By more reflective, the adjustments are meant to approximate the volumes at the Monterey / California intersection, not match them, taking into account the other intersections and driveways between these two intersections.

The two study intersections operate below their level of service standard under Existing Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM) LOS E (PM).  
Monterey Road / San Martin Avenue – LOS E (AM).

### **Existing Plus Project Conditions**

The unique use and operations of this project do not correspond well with any typical land uses. Therefore, the project trip generation will not be based on rates from *Trip Generation Manual*, 10<sup>th</sup> Edition, published by the Institute of Traffic Engineers in September 2017. Instead, a custom trip generation has been prepared for the project, based on information provided by the project applicant and other assumptions.

The project is estimated to generate 48 daily trips, with 5 trips (1 in, 4 out) during the AM peak hour and 4 trips (3 in, 1 out) during the PM peak hour.

The project driveway would be located approximately 490 feet south of the Monterey Road / California Avenue intersection.

The two study intersections operate below their level of service standard under Existing Plus Project Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM) LOS E (PM).
2. Monterey Road / San Martin Avenue – LOS E (AM).

Neither intersection meets the significant local adverse effect criteria; hence no improvements are required under Existing Plus Project conditions.

The project driveway would operate at LOS A (AM) and LOS C (PM) under Existing Plus Project conditions, which is better than its level of service standard.

### **Cumulative Without Project Conditions**

The traffic volume growth under the Cumulative Without Project conditions were derived in two ways. First, traffic growth projected by Fehr & Peers and further expanded by Pinnacle Traffic Engineering, was derived from traffic volumes in the Pinnacle Traffic Engineering traffic study. This traffic growth is the equivalent of 1.2% per year for 18 years (21.6% total) for mainline traffic on Monterey Road, plus 0.6% per year for 18 years (10.8% total) on California Avenue and San Martin Avenue.

Second, traffic volumes from adjacent approved and proposed development was also added to the study intersections. This specifically includes the Cordoba Center (to be located on Monterey Road north of California Avenue).

The two study intersections operate below their level of service standard under Cumulative Without Project Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM, PM).
2. Monterey Road / San Martin Avenue – LOS F (AM).

There are no planned pedestrian or bicycle facility improvements in the study area.

### **Cumulative Plus Project Conditions**

The two study intersections operate below their level of service standard under Cumulative Plus Project Conditions:

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## *NWC #5 Facility Traffic Impact Analysis*

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1. Monterey Road / California Avenue – Side-Street LOS F (AM, PM).
2. Monterey Road / San Martin Avenue – LOS F (AM).

Neither intersection meets the significant local adverse effect criteria; hence no improvements are required under Cumulative Plus Project conditions.

The project driveway would operate at LOS A (AM) and LOS C (PM) under Cumulative Plus Project conditions, which is better than its level of service standard.

### **Sight Distance at Public Street Intersection**

Sight distance was evaluated at the Monterey Road / California Avenue intersection for eastbound California Avenue looking to the north and south on Monterey Road. A speed of 55 mph was used for this evaluation, 5 more than the speed limit of 50 mph on Monterey Road. The available sight distance to the north and south are both below the Caltrans preferred sight distance standard but above the minimum sight distance standard. It is therefore concluded that the available sight distance at the Monterey Road / California Avenue intersection is adequate.

### **Vehicle Queuing at Public Street Intersection**

Vehicle queues were evaluated at the Monterey Road / San Martin Avenue intersection for the southbound Monterey left turn lane and the westbound San Martin right turn lane. Both the 2000 Highway Capacity Manual methodology (stop-controlled intersection) and the Poisson method identified in the VTA guidelines (with signalization) were used to quantify the queue lengths. This will minimize the effect of removal of the northbound Monterey left turn to residents and businesses in the area.

The westbound San Martin right turn lane is approximately 170 feet long, excluding the railroad crossing. The PM peak hour queue does not exceed this amount. However, the AM queue length is more than two times this length, extending to nearly Lincoln Avenue. This is due to traffic diverting from US 101 to avoid morning congestion through Morgan Hill. It is conjectured that if the freeway congestion is removed, this traffic would no longer divert, thus reducing the vehicle queues to more reasonable ranges. Therefore, no improvements are deemed necessary at the Monterey Road / San Martin Avenue intersection to address the projected queues.

### **Site Circulation Analysis**

Project site circulation will be more than adequate for vehicles. The paved area provides direct access from the project driveway to the employee and trailer parking spaces and the proposed toilet wash down area. Employee parking spaces are up against the northeastern edge of the paved parking area. The trailer parking spaces are in the western part of the parking area with access aisles on all four sides. The employee bathroom is located on the eastern edge of the pavement, away from the travel path. All this allows for good access within the parking area.

No members of the public will be allowed on the project site. Therefore, there is no need for commercial parking spaces in addition to those parking spaces provided.

Neither pedestrian or bicycle facilities are required to connect to the project site, as the project is not anticipated to generate either pedestrian or bicycle trips.

## NWC #5 Facility Traffic Impact Analysis

The driveway access is proposed to be gated. The gate will be closed outside of working hours.

The truck turning template shows that trucks can easily turn into the project site with the proposed driveway design. Outbound truck traffic has similar results. These truck turning templates indicate that the existing shoulder on Monterey Road is not sufficient as an acceleration or deceleration lane. The project should construct acceleration and deceleration lanes for the driveway on Monterey Road.

Onsite circulation would also be adequate. Trucks circulating around the paved area of the central project site would not encroach upon any of the parking spaces, nor any of the proposed or retained on-site buildings. Although the template assumes a counterclockwise circulation around the project site, the reverse (clockwise) circulation would also be adequate. No additional improvements would be required.

### **Site Access Analysis**

Operations of the project driveway would all be better than the County standard of LOS D under all analyzed scenarios. No additional improvements are required due to level of service.

A sight distance evaluation was performed at the project driveway with Monterey Road. A speed of 55 mph was used for this evaluation, 5 more than the speed limit of 50 mph on Monterey Road. The available sight distance to the north is slightly below the applicable Caltrans minimum sight distance standard for this driveway, while the sight distance to the south exceeds this standard. Although the sight distance to the north is limited by the Monterey Road / California Avenue intersection, the fact that left turns are not allowed out of the project driveway will minimize the discrepancy such that improvements are not required.

The infrequent use of the project driveway by project traffic will result in minimal queuing. All queues will be stored in the long driveway off Monterey.

### **Project Vehicle Miles Traveled**

Although the Santa Clara Valley Transportation Agency (VTA) has established a process to determine if properties could have a significant impact to VMT, it does not have a category similar to the study project. The State VMT guidelines are presented in *Technical Advisory on Evaluating Transportation Impacts in CEQA*, State of California Governor's Office of Planning and Research, December 2018. In the state publication, it is established that projects generating less than 110 daily trips are exempt from any further VMT analysis. As the project generates only 48 daily trips, it is determined that the study project qualifies for this exemption. There is no need for further VMT analysis.

### **Summary of Recommendations**

See Chapter 11 for a list of recommendations made in this report.

### **References**

See Chapter 12 for a list of references and contacts used when preparing this report.

# 1 INTRODUCTION

## 1.1 Project Description

The proposed NWC #5 project involves the establishment of a storage and maintenance area for portable toilets and fences south of the intersection of Monterey Road and California Avenue in San Martin, Santa Clara County, California. Although the study property does not have frontage on either street, it does have a driveway off Monterey Road.

This report is a revision of the previous report dated November 14, 2022. This updated report was prepared due to removal of a previously approved project under the Cumulative Conditions analysis.

Vehicular, pedestrian, bicycle and transit circulation issues were evaluated at the project site and the immediately surrounding street network. The locations of the project site and study area are indicated on **Exhibit 1**. The site plan is shown on **Exhibit 2**.

## 1.2 Scope of Work

This report addresses the following topics:

- Existing vehicular, pedestrian and bicycle circulation at the project access and the study street network.
- Assessment of potential adverse effects to vehicular, pedestrian, bicycle, and transit circulation due to the project, and recommendations to minimize or alleviate those adverse effects.
- Assessment of potential cumulative traffic adverse effects.
- Site circulation analysis, including truck turning templates.
- Site access analysis, including evaluation of driveway access alternatives.
- Vehicle queuing on Monterey Road, California Avenue and the two potential project driveway options.
- Vehicle Miles Traveled (VMT) evaluation.

The study scope of work, study network and analysis scenarios of this analysis were determined in consultation with Santa Clara County Roads and Airports Department and Santa Clara County Planning and Development Department staff.

## 1.3 Study Network

The AM and PM peak periods were analyzed at the following two intersections:

1. Monterey Road / California Avenue; and
2. Monterey Road / San Martin Avenue.

#### **1.4 In addition, the project driveway was analyzed. Analysis Scenarios**

Traffic operations for the following analysis scenarios were analyzed, as requested by Santa Clara County Roads and Airports Department:

- Existing Conditions
- Existing Plus Project Conditions
- Cumulative Without Project Conditions
- Cumulative Plus Project Conditions

Improvements to offset adverse effects created by the proposed project are recommended where warranted.

#### **1.5 Traffic Operation Evaluation Methodologies**

Intersection traffic operations were evaluated using level of service (LOS). LOS is a qualitative description of an intersection's operations, ranging from LOS A to LOS F. Level of Service "A" represents free flow uncongested traffic conditions. Level of Service "F" represents highly congested traffic conditions with unacceptable delay to vehicles at intersections. The intermediate levels of service represent incremental levels of congestion and delay between these two extremes. LOS descriptions for each type of existing traffic control at the study intersections (i.e., signal) are included as **Appendix A**.

Intersection traffic operations were evaluated using the Traffix traffic analysis software (Version 8) using the 2000 Highway Capacity Manual (HCM) methodologies and the VTA *Traffic Level of Service Guidelines*. The average delay is then correlated to a level of service. When using the HCM 2000 methods for the analysis of signalized intersections, the overall intersection delay is used to determine LOS.

#### **1.6 Level of Service Standards**

The Santa Clara County level of service objective is LOS D. This applies to overall conditions (signalized and all-way stop control intersections) and side-street conditions (one- and two-way stop control intersections).

#### **1.7 Modeling of Right Turn on Red (RTOR)**

All the signalized study intersections allow right turns on red (RTOR), which generally reduce the overall intersection delay, thus improving the overall intersection level of service. There are several options to model right turns on red with different traffic analysis software packages, but the only method prescribed by the HCM for modeling RTOR is to reduce the input volumes to account for vehicles turning right on red. Where an exclusive right turn lane movement runs concurrent with a protected left turn phase from the cross street, the HCM allows for the right turn volume to be reduced by the number of simultaneous left turners. However, the length of the right turn lane affects the number of vehicles that can turn right on red. This is because a short right turn lane can result in right turning vehicles being trapped in the queue with vehicles in the through

lane. For the purposes of this analysis, it is assumed that no vehicles would be able to turn right on red at any of the study intersections.

### **1.8 Significance Criteria**

Two different significance criteria are used to assess the impacts and adverse effects of this project – one for environmental impacts and one for local adverse effects. The environmental impacts refer to impacts assessed per the California Environmental Quality Act (CEQA) guidelines, while the local adverse effects are assessed relative to capacity and the Santa Clara County level of service standard. The following significance criteria are used in this study:

#### **1.8.1 Environmental (CEQA)**

Senate Bill (SB) 743 requires that, starting July 2020, transportation impacts for projects per the California Environmental Quality Act (CEQA) be based on a project's Vehicle Miles Traveled (VMT), rather than level of service. The publication *Technical Advisory on Evaluating Transportation Impacts in CEQA*, State of California Governor's Office of Planning and Research, December 2018, suggests that a significant environmental (CEQA) VMT threshold for commercial/retail project be a maintaining of the current retail VMT for the region, although agencies are allowed to adopt their own customized thresholds. As of this writing, Santa Clara County has not established either a VMT standard or significance threshold for VMT analysis. Rather, VTA has created a site evaluation tool, vmttool.vta.org, which allows quantification of VMT for specific land use types in Santa Clara County. The proposed project does not fit any of the specific land use types allowed by VTA's tool, therefore VMT is estimated based on project information. See Chapter 10 for more information.

#### **1.8.2 Local**

SB 743 also allows local jurisdictions to, separate from CEQA significance analysis, assess local adverse effects associated with their own adopted level of service standards.

For the purposes of this analysis, adverse effects on intersection operations are defined in the following situations:

##### *Signalized Intersection:*

- Traffic increases from a proposed project would cause the overall operations at a signalized intersection to fall below LOS D with the addition of project vehicle trips to baseline conditions; OR
- An intersection already operating at LOS E or F under baseline conditions would experience an increase of average critical delay by 4.0 seconds or more AND an increase in the critical V/C ratio of 0.010 or more; OR
- An intersection already operating at LOS E or F under baseline conditions would experience a decrease in average critical delay AND an increase in critical V/C ratio of 0.010 or more.

**One-Way Stop Control Intersection:**

- Traffic increases from a proposed project would cause the side-street operations at a side-street stop sign-controlled intersection to fall below LOS D with the addition of project vehicle trips, compared to baseline conditions; OR
- An intersection already operating at LOS E or F under baseline conditions would experience an increase of average critical delay by 4.0 seconds or more.

## 2 EXISTING TRAFFIC CONDITIONS

This chapter evaluates Existing traffic conditions and includes a description of the project setting.

### 2.1 Existing Traffic Network

The project site is located just south of the corner of California Avenue and Monterey Road in the community of San Martin, between Morgan Hill and Gilroy in Santa Clara County. The site is bordered by rural residential and largely undeveloped properties.

The site is closest to California Avenue and Monterey Road. Other roadways serving the study area include San Martin Avenue. A brief description of each roadway can be found below.

**California Avenue** is a two-lane, east-west roadway in northern San Martin, connecting Monterey Road and Santa Teresa Boulevard. California Avenue primarily provides access to the adjacent rural residential properties. The roadway has little to no paved shoulders. The posted speed limit on California Avenue is 40 miles per hour (mph).

**Monterey Road** is a four-lane, north-south roadway in Santa Clara County, extending between Gilroy and San Jose through San Martin and Morgan Hill. It is the “main street” of all three cities, having once served as US 101 prior to the construction of the US 101 freeway through these communities. In the project vicinity, Monterey Road generally has wide, paved shoulders, especially near California Avenue. The posted speed limit on Monterey Road is 50 mph north of Roosevelt Avenue and 45 mph south of Roosevelt Avenue. (The intersection with Roosevelt Avenue is located approximately 1,770 feet south of California Avenue.)

**San Martin Avenue** is a two-lane, east-west roadway in central San Martin. It provides access to the commercial district of the community. It also provides regional access to the area via its interchange with US 101 and its connections to Monterey Road and Santa Teresa Boulevard. San Martin Avenue generally has little to no shoulder at its far eastern and western ends, but shoulders are wide enough to the central business district to allow for on-street parking. Trucks over 7 tons are prohibited on San Martin Avenue. The posted speed limit on San Martin Avenue is 35 mph west of Colony Avenue, 25 mph between Colony Avenue and Llagas Avenue, and 35 mph east of Llagas Avenue.

### 2.2 Existing Pedestrian Network

The study area is mostly rural in nature. Within the study area, neither California Avenue nor Monterey Road have sidewalks. San Martin Avenue only has sidewalks between Monterey Road and Llagas Avenue through the commercial district of San Martin. Marked crosswalks are only present at the Monterey Road / San Martin Avenue intersection.

### 2.3 Existing Bicycle Network

There are four types of bicycle facilities defined by Caltrans. Each type is described below:

1. Bike path (Class I) – A completely separate right-of-way designed for the exclusive use of bicycle and pedestrian traffic with crossflow minimized.
2. Bike lane (Class II) – A striped lane for one-way bike travel on a street or highway, typically including signs placed along the street segment.
3. Bike route (Class III) – Provides a shared use with pedestrian or motor vehicle traffic. Typically, these facilities are city streets with signage designating the segment for Bike Route without additional striping or facilities.
4. Separated Bikeways (Class IV) – A bikeway for the exclusive use of bicycles and includes a separation between the bikeway and the through vehicular traffic. The separation may include, but is not limited to, grade separation, flexible posts, inflexible posts, inflexible barriers, or on-street parking.

There are no formal bicycle lanes in the study area, although the shoulders on Monterey Road are of sufficient width to allow bicycle travel separated from vehicle traffic. Bicycles on both California Avenue and San Martin Avenue must travel in the through lanes with vehicle traffic.

### 2.4 Existing Transit Service

The Santa Clara Valley Transportation Authority (VTA) provides transit service to the study area. The following routes service the area:

- Route 68 – San Jose Diridon Station – Gilroy Transit Center . Service every 15-30 minutes on weekdays and every 20-30 minutes on Saturdays and Sundays.
- Route 287 – Live Oak High School – Monterey and San Martin. This route provides service once a day when school is in session. It only operates in the southbound direction, leaving Live Oak High School at 3:42 PM.
- Route 568 – Gilroy Transit Center to San Jose Diridon. Service every 30-60 minutes on weekdays only. This route runs largely along Route 68, but skips some bus stops, thus serving like an express route.

Bus stops near the project site are at the following locations:

- Monterey Road / California Avenue (Route 68; both directions)
- Monterey Road / Roosevelt Avenue (Route 68; both directions)
- Monterey Road / San Martin Avenue (Routes 68, 287 (southbound only), 568; both directions)

Caltrain provides limited commuter rail service to San Martin via a rail station located on Monterey Road just north of San Martin Avenue. This station is only service by six trips per weekday – three during the AM (northbound only) and three during the PM (southbound only). No weekend service is provided to the San Martin station.

## 2.5 Existing Conditions Traffic Circulation

### 2.5.1 Vehicle Circulation

In March 2020, the Santa Clara County Public Health Department instituted a shelter-in-place order for all of Santa Clara County, restricting operations and travel to/from offices, commercial businesses, and recreational activities. This order was in response to the COVID-19 pandemic occurring within the County during the Year 2020. As a result, traffic activity throughout the county was significantly reduced from typical conditions, precluding the usual collection of peak period traffic volumes at the four study intersections.

As the pandemic and its restrictions eased in 2022, traffic in the County began to return to near-normal conditions. Although most companies fully reopened for work, some tech companies chose to keep all or some of their staff working from home. Therefore, traffic volumes returned to just almost normal levels across the county.

This traffic study uses a hybrid approach to estimate traffic volumes for this study. Whereas volumes at Monterey Road / California Avenue was approximated from counts prior to the COVID-19 pandemic, new traffic counts were performed at the Monterey Road / San Martin Avenue intersection. The methodology used for collecting each intersection is described below.

Existing peak hour traffic volumes at the Monterey Road / California Avenue intersection was approximated in the following manner:

1. Existing traffic volumes from recent pre-COVID traffic studies in the area – including the Cordoba Center and other available area traffic studies – were reviewed to obtain pre-COVID-19 existing volumes (Year 2018) at this study intersection. These volumes are depicted in **Exhibit 3A**.
2. A growth factor was derived to convert the recent volumes from Step 1 to the equivalent of Year 2022 volumes (without the effects of the COVID-19 shelter-in-place order by Santa Clara County). This growth factor was derived using available Santa Clara County volumes near the project vicinity, as published by the Santa Clara County Roads and Airports Department.
3. The growth factor from Step 2 – 3% per year for 4 years, or 12% total – was applied to the volumes from Step 1 to approximate Year 2022 volumes.

The approximated existing volumes at this intersection were also used to project through volumes at the project driveway on Monterey Road as analyzed under Existing Plus Project and Cumulative Plus Project conditions.

New counts (March 2022) were also performed at the intersection of Monterey Road / San Martin Avenue during the peak AM (7:00 – 9:00 AM) and PM (4:00 – 6:00 PM) periods. From these, the peak one-hour AM and PM periods were identified. **Exhibit 3A** contains the peak hour volumes. The counts at this intersection can be found in **Appendix B**.

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## *NWC #5 Facility Traffic Impact Analysis*

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A comparison of the Monterey Road / San Martin Avenue volumes to the adjusted Monterey Road / California Avenue found the Monterey / San Martin volumes are lower. This indicates that Year 2022 volumes are closer but not fully representative of what volumes would be in a post-COVID-19 world. Therefore, the collected volumes were adjusted upwards to be more reflective of the adjusted volumes at Monterey / California. By more reflective, the adjustments are meant to approximate the volumes at the Monterey / California intersection, not match them, taking into account the other intersections and driveways between these intersections.

**Exhibit 3B** depicts the approximated/adjusted Year 2022 peak hour turning movement volumes for the study intersections under Existing Conditions.

Existing intersection levels of service are summarized on **Exhibit 4**. The LOS calculation sheets for Existing conditions can be found in **Appendix C**.

Both primary study intersections operate below the level of service standards under Existing Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM), E (PM); and
2. Monterey Road / San Martin Avenue – LOS E (AM).

Note: The poor level of service at Intersection #2 – Monterey Road / San Martin Avenue – is primarily due to the high westbound right turn movement. This movement is well over 300 vehicles in the AM, considerably higher than the reverse movement (i.e., southbound Monterey left turn) in either the AM or PM peak hours. The higher than expected San Martin westbound right movement may be because of vehicles diverting off northbound US 101 and onto northbound Monterey Road instead. Northbound US 101 is typically congested through Morgan Hill during the AM – see **Appendix G**.

### **2.5.2 Pedestrian Circulation**

There is very little pedestrian activity near the study project. Observations in October 2020 found no pedestrian activity at the Monterey Road / California Avenue intersection. This is primary due to the rural nature of the area immediately surrounding the project site.

### **2.5.3 Bicycle Circulation**

There is very little bicycle activity near the study project. Observations in October 2020 found no pedestrian activity at the Monterey Road / California Avenue intersection. This is primary due to the rural nature of the area immediately surrounding the project site.

## 3 EXISTING PLUS PROJECT CONDITIONS

### 3.1 Project Description

The project involves the establishment of a storage and maintenance area for portable toilets. These toilets would be deployed to construction sites and area festivals as needed. A total of 10 employees would work at the facility. In addition, 12 trucks would be based at the facility, which would both tow the toilets to and from their destinations and resupply the deployed toilets as necessary (e.g., toilet paper, etc.). When not deployed, the toilets would be stored on site. Maintenance and cleaning supplies would also be stored on site, for serving of the toilets after return to the project site. However, pumping out of the toilets would occur elsewhere, not on site.

Operations of the project site would be roughly 5:00 AM – 7:00 PM on weekdays. Daily site activity would be the following, as proposed by the project applicant:

1. Employees would arrive between 5:40 – 6:00 AM. All employees would be deployed on the company trucks.
2. Truck activity would be split into two delivery trucks – which would deliver and return the portable toilets – and ten route trucks – which would service deployed portable toilets.
  - a. The delivery trucks would leave the site between 6:15 – 8:00 AM, deploying and picking up portable toilets as necessary, and returning to the project site between 1:00- 2:00 PM. The delivery trucks would again leave the project site for more deliveries and picking up between 3:00 – 4:00 PM, returning to the project site again between 7:00 – 8:00 PM.
  - b. The route trucks would leave the project site between 6:15 – 10:00 AM. Trucks would remain in the field throughout the remainder of the morning and afternoon, serving deployed toilets as necessary, eventually returning to the site between 4:00 – 8:00 PM.
3. Employees would then leave the site to return home between 8:00 – 8:20 PM.

The service area for the project site will be all of Santa Clara County and the northern Monterey Bay Area. This will augment an existing facility located in Pittsburg, in northeastern Contra Costa County.

The project site is largely vacant but does include multiple existing buildings that are currently unused. The project site was once part of a seasonal Christmas tree farm that is now closed.

### 3.2 Project Trip Generation

The unique use and operations of this project do not correspond well with any typical land uses. Therefore, the project trip generation will not be based on rates from *Trip Generation Manual*, 10<sup>th</sup> Edition, published by the Institute of Traffic Engineers in September 2017. Instead, a custom trip generation has been prepared for the project; this trip generation is shown on **Exhibit 5**.

## *NWC #5 Facility Traffic Impact Analysis*

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The customized trip generation estimate was derived based on the following information and assumptions:

1. The trip generation estimate represents project traffic on a typical weekday at full employment and full deployment of trucks, i.e., all 10 employees work and all 10 trucks are deployed.
2. To be conservative, it is assumed that all employees drive themselves to and from the project site. No employees are assumed to carpool together or be dropped off/picked up by other drivers.
3. The project operations described in Section 3.1.1 indicate that the employees of the project would arrive at and depart from the project site outside of the street peak hours. (According to the Pinnacle Traffic Engineering traffic study for the Cordoba Center, the peak hours of the Monterey Road / California Avenue intersection are approximately 7:00 – 8:00 AM and 4:30 – 5:30 PM.) Therefore, the employees would generate no trips during either peak hour.
4. Only some of the truck trips would occur during the AM and PM peak hours. Based on the proposed departure and arrival times of the two truck types (delivery and route), it is anticipated that 50% of the delivery trucks would travel during the AM peak hour near the project site and no delivery trucks would travel during the PM peak hour. It is also anticipated that 25% of the total outbound route trucks (i.e., to the field) and 25% of the inbound route trucks (i.e., back to the project site) occur during the AM and PM peak hours, respectively.
5. It is assumed that two deliveries occur to the project site on an average day. This may include US mail, cleaning materials and other supplies. To be conservative, it is assumed that these deliveries occur during the peak hours – one during the AM peak hour and one during the PM peak hour.
6. It is assumed that there are no visitors to the project site, i.e., only the employees, trucks and delivery vehicles visit and depart from the project site.

**Exhibit 5** summarizes the project trip activity. The project is estimated to generate 48 daily trips, with 5 trips (1 in, 4 out) during the AM peak hour and 4 trips (3 in, 1 out) during the PM peak hour.

### **3.3 Project Access**

As shown on the site plan in **Exhibit 2**, the project is proposing a driveway directly on Monterey Road. This driveway would be located approximately 490 feet south of the Monterey Road / California Avenue intersection. This analysis summary on **Exhibit 4** identifies the operations of the two existing study intersections and the proposed project driveway.

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## *NWC #5 Facility Traffic Impact Analysis*

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The proposed driveway would be a right-in, right out (RIRO) driveway. This means that there would only be right turns in and right turns out. No left turns in or out of the driveway would be allowed. This dynamic means that all inbound traffic would come from southbound Monterey Road and all outbound traffic would travel from southbound Monterey Road to eastbound San Martin Avenue.

### **3.4 Project Trip Distribution and Assignment**

**Exhibit 6** depicts the trip distribution for the project. This distribution was derived based on the anticipated market area of the project, as virtually all of the trips to and from the project site during the AM and PM peak hours would be the project trucks. According to the project applicant, the project site would service an approximately 30-mile radius from San Martin. This would include all of Santa Clara County, all of Santa Cruz County, northern San Benito County, and northern Monterey County. For this reason, all outbound truck traffic would use eastbound San Martin Avenue to reach US 101, while all inbound traffic would use southbound Monterey Road north of the project site. This project would augment the existing NWC facility located in Pittsburg, in northeastern Contra Costa County, hence expansion of the project service area is not anticipated. The trip distribution was combined with the trip generation to derive the project trip assignment depicted on **Exhibit 7**.

### **3.5 Existing Plus Project Condition Traffic Circulation**

#### **3.5.1 Vehicle Circulation**

The trip assignment (**Exhibit 7**) was added to the adjusted existing traffic volumes (**Exhibit 3B**) to create the Existing Plus Project volumes depicted on **Exhibit 8**.

Existing Plus Project condition intersection levels of service are summarized on **Exhibit 4**. The LOS calculation sheets for Existing Plus Project conditions can be found in **Appendix D**.

The project driveway would operate at LOS A (AM) and LOS C (PM) under Existing Plus Project conditions, which is better than its level of service standard.

Both primary study intersections would operate below their level of service standards under Existing Plus Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM), E (PM); and
2. Monterey Road / San Martin Avenue – LOS E (AM).

Below is a detailed discussion of all intersections that operate below their relative level of service standards and potential improvements.

1. Monterey Road / California Avenue: Under Existing Plus Project conditions, the project would increase side-street delay at this intersection between 0.2 – 1.0 seconds. Based on the significance criteria in Section 1.8.2, the project would not represent a significant local adverse effect at this intersection. No improvements are required.

2. Monterey Road / San Martin Avenue: Under Existing Plus Project conditions, the project would increase average delay at this intersection by 0.8 seconds. Based on the significance criteria in Section 1.8.2, the project would not represent a significant local adverse effect at this intersection. No improvements are required.

### **3.5.2 Pedestrian Circulation**

The project would not increase pedestrian traffic in the study area. Therefore, the project would not represent a significant local adverse effect to pedestrian circulation.

### **3.5.3 Bicycle Circulation**

The project would not increase bicycle traffic in the study area. Therefore, the project would not represent a significant local adverse effect to bicycle circulation.

### **3.5.4 Transit Circulation**

The project is not expected to increase transit demand. The closest bus stop to the site – southbound Monterey Road just north of California Avenue, would provide adequate access to the project site. However, the bus stop in the other direction – northbound Monterey Road, north of California Avenue – would require crossing Monterey Road, a free-flowing, uncontrolled, four-lane and high-speed roadway. These attributes would likely discourage potential use of transit by employees of the project. Therefore, the project would not represent a significant local adverse effect to transit service demand.

## 4 CUMULATIVE WITHOUT PROJECT CONDITIONS

This section describes the analysis results under Cumulative Without Project conditions. The Cumulative Without Project traffic condition is defined as traffic conditions at roughly the Year 2035.

### 4.1 *Derivation of Cumulative Without Project Condition Traffic Volumes*

The traffic volume growth under the Cumulative Without Project conditions was derived using growth rates confirmed from multiple sources. This includes traffic growth projected by Fehr & Peers in its Cordoba Center traffic study and as further expanded by Pinnacle Traffic Engineering in its supplemental Cordoba Center traffic study. This traffic growth is the equivalent of 1.2% per year for 18 years (21.6% total) for mainline traffic on Monterey Road, plus 0.6% per year for 18 years (10.8% total) on California Avenue and San Martin Avenue. The traffic growth, as noted by Fehr & Peers, is derived from the City of Morgan Hill General Plan buildout forecasts developed by Hexagon Transportation Consultants in 2015.

In addition to the growth rates, traffic volumes from adjacent approved and proposed development was also added to the study intersections. This specifically includes the Cordoba Center (to be located on Monterey Road north of California Avenue). The volumes for this project are derived from the Existing Plus Project volume forecasts in the aforementioned Pinnacle Traffic Engineering traffic study.

The total cumulative growth is composed of both the future growth projections and the trips from the adjacent development project. This growth was added to the adjusted Existing volumes in **Exhibit 3B** to create the Cumulative Without Project volumes in **Exhibit 9**.

Note that the Pinnacle Traffic Engineering traffic study primarily evaluated the operations and queuing potential for a new southbound Monterey U-turn movement at the Monterey Road / California Avenue intersection. This U-turn lane is assumed to be in place for Cumulative Without and Cumulative Plus Project conditions. The study project would not be responsible for implementation of this U-turn lane.

### 4.2 *Cumulative Without Traffic Conditions*

#### 4.2.1 *Vehicle Circulation*

Cumulative Without Project conditions AM and PM intersection levels of service are summarized on **Exhibit 4**. The LOS calculation sheets for Cumulative Without Project traffic conditions can be found in **Appendix E**.

The following primary intersections would operate below their level of service standards under Cumulative Without Project Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM, PM).
2. Monterey Road / San Martin Avenue – LOS F (AM).

#### **4.2.2 Pedestrian Circulation**

There are no planned pedestrian facility improvements in the study area.

#### **4.2.3 Bicycle Circulation**

There are no planned bicycle facility improvements in the study area.

## 5 CUMULATIVE PLUS PROJECT CONDITIONS

This section describes the analysis results under Cumulative Plus Project traffic conditions, which combines both Cumulative Without Project conditions with traffic from the study project.

### 5.1 Derivation of Cumulative Plus Project Condition Traffic Volumes

The project trip assignment (**Exhibit 7**) was combined with the Cumulative Without Project condition volumes to create the Cumulative Plus Project volumes depicted on **Exhibit 10**.

### 5.2 Cumulative Plus Project Traffic Conditions

#### 5.2.1 Vehicle Circulation

Cumulative Plus Project AM and PM intersection levels of service are summarized on **Exhibit 4**. The LOS calculation sheets for Cumulative Plus Project traffic conditions can be found in **Appendix F**.

The project driveway would operate at LOS A (AM) and LOS C (PM) under Cumulative Plus Project conditions, which is better than its level of service standard.

Both primary study intersections would operate below their level of service standards under Cumulative Plus Conditions:

1. Monterey Road / California Avenue – Side-Street LOS F (AM, PM); and
2. Monterey Road / San Martin Avenue – LOS F (AM).

Below is a detailed discussion of all intersections that operate below their relative level of service standards and potential improvements.

1. Monterey Road / California Avenue: Under Cumulative Plus Project conditions, the project would increase side-street delay at this intersection between 0.8 – 1.2 seconds. Based on the significance criteria in Section 1.8.2, the project would not represent a significant local adverse effect at this intersection. No improvements are required.
2. Monterey Road / San Martin Avenue: Under Cumulative Plus Project conditions, the project would increase average delay at this intersection by 1.2 seconds. Based on the significance criteria in Section 1.8.2, the project would not represent a significant local adverse effect at this intersection. No improvements are required.

#### 5.2.2 Pedestrian Circulation

Pedestrian activity is not anticipated to increase significantly under Cumulative Plus Project conditions. Therefore, the project would not represent a significant cumulative local adverse effect to pedestrian circulation.

### **5.2.3 Bicycle Circulation**

Bicycle activity is not anticipated to increase significantly under Cumulative Plus Project conditions. Thus, the project would not represent a significant cumulative local adverse effect to bicycle circulation.

### **5.2.4 Transit Circulation**

Similar to Existing Plus Project conditions, very few employees of the project site would use transit. Implementation of the operation improvement at Monterey Road / California Avenue – signalization – may slightly increase use of transit. However, even if all of the future employees of the project would switch to transit, this would only amount to 10 employees, split over the entire 12-hour workday of the project. As such, the project would not represent a significant cumulative local adverse effect to transit circulation.

## 6 SIGHT DISTANCE AT PUBLIC STREET INTERSECTION

Note: This chapter discusses only the sight distance available at the Monterey Road / California Avenue intersection. See Chapter 9 – Site Access Analysis – for the sight distance evaluation at the proposed project driveways.

Sight distance was evaluated at the Monterey Road / California Avenue intersection, more specifically for eastbound California Avenue looking to the north and south on Monterey Road. For this intersection, which is an intersection of public roads, both the Caltrans stopping sight and corner sight distance standards would apply. The available sight distance must meet at least the corner sight distance, if not both standards.

The available vehicle sight distance was evaluated, using Caltrans sight distance standards. Sight distance was measured from 15 feet back from the traveled way of the major street (i.e., edge of nearest through lane). This places the measurement location right at the stop bar.

The posted speed limit on Monterey Road is 50 mph. To be conservative, a speed of 55 mph was used for the sight distance evaluation. For 55 mph on a four-lane roadway, Caltrans sight distance standards require a minimum stopping sight distance of 500 feet and a preferred sight distance (also known the “corner sight distance”) of approximately 1,043 feet. **Appendix H** contains the calculation of the applicable sight distance standards.

To the north on Monterey Road (i.e., towards Morgan Hill), the available sight distance from eastbound California Drive is approximately 850 feet. This is below the preferred sight distance standard but above the minimum sight distance standard. To the south (i.e., towards San Martin Avenue), the available sight distance is about 1,000 feet, which also is below the preferred but above the minimum Caltrans standards. Sight distance on California Avenue is limited to the north by the horizontal curvature of the roadway and is not limited to the south.

It is therefore concluded that the available sight distance at the Monterey Road / California Avenue intersection is adequate.

## 7 VEHICLE QUEUING AT PUBLIC STREET INTERSECTION

Note: This chapter discusses only the vehicle queuing at the Monterey Road / San Martin Avenue intersection. See Chapter 9 – Site Access Analysis – for the vehicle queuing evaluation at the proposed project driveways.

**Exhibit 11** summarizes the vehicle queuing evaluation at the study intersections. The queues were evaluated on the exclusive turn lanes at the following study intersection:

1. Monterey Road / San Martin Avenue:
  - a. Southbound Monterey left turn lane
  - b. Westbound San Martin right turn lane

The queues were quantified using the 2000 Highway Capacity Manual methodology (stop-controlled intersection) and the Poisson method identified in the VTA guidelines (with signalization). **Appendix I** contains the detailed 95th percentile queue length calculations.

The southbound Monterey left turn lane is currently 115 feet long. The projected vehicle queue for this movement varies from 150 to 200 feet, regardless of time period or if the project is included. The queue capacity for this lane is limited by the northbound left turn at Burbank Avenue. It is thus recommended that the northbound Monterey left turn lane at Burbank Avenue be removed so that the southbound Monterey left turn at San Martin Avenue can be extended. The southbound left turn lane can thus be extended to 200 feet, which is adequate for the queue length under Existing Plus Project & Cumulative Plus Project conditions. It is also recommended that the southbound Monterey left turn transition into the existing two-way left turn. This will minimize the effect of removal of the northbound Monterey left turn to residents and businesses in the area.

The westbound San Martin right turn lane is approximately 170 feet long, excluding the railroad crossing. During the PM, the queue does not exceed that length. However, the AM queue length is more than two times this length, extending to nearly Lincoln Avenue. The long queue is due to the large AM volume of over 300 vehicles, nearly reaching 400 vehicles under Cumulative conditions. As discussed earlier, this high volume is likely due to diversions off US 101 to avoid morning congestion through Morgan Hill. It is conjectured that if the freeway congestion is removed, this traffic would no longer divert the freeway through San Martin in the AM, thus reducing the vehicle queues to more reasonable ranges. Therefore, no improvements are deemed necessary at the Monterey Road / San Martin Avenue intersection to address the projected queues.

## 8 SITE CIRCULATION ANALYSIS

This section summarizes the site internal circulation analysis.

### 8.1 Vehicle, Pedestrian and Bicycle Circulation

As shown on the project site plan in **Exhibit 2**, less than a quarter of the project site will be paved. This pavement will be used for parking of vehicles and trailers – 13 employee and 10 trailer parking spaces will be provided. One of the employee spaces is an ADA space. Also, a vacant on-site building will be removed and replaced with an employee bathroom.

Project site circulation will be more than adequate for vehicles. The paved area provides direct access from the project driveway to the employee and trailer parking spaces and the proposed toilet wash down area. The employee spaces are up against the northeastern edge of the paved parking area. The trailer parking spaces are in the western part of the parking area with access aisles on all four sides. The employee bathroom is located on the eastern edge of the pavement, away from the travel path. All this allows for good access within the parking area.

No members of the public will be allowed on the project site. Therefore, there is no need for commercial parking spaces in addition to those parking spaces provided.

Neither pedestrian or bicycle facilities are required to connect to the project site, as the project is not anticipated to generate either pedestrian or bicycle trips.

The driveway access is proposed to be gated. The gate will be closed outside of working hours.

### 8.2 Truck Turning Templates

**Exhibit 12** depicts a truck turning template for project on-site circulation, as prepared by Hanna-Brunetti. Project vehicles will be a mixture of pick-up trucks and a trailer loaded with toilets. The WB-50 (WB-15 metric) semi-trailer was used to approximate the path swept by a project truck towing toilets into the project site, as well as circulating around on the paved area.

The truck turning template shows that trucks can easily turn into the project site with the proposed driveway design. Outbound truck traffic has similar results. These truck turning templates also indicate that the existing shoulder on Monterey Road is not sufficient as an acceleration or deceleration lane. The project should construct acceleration and deceleration lanes for the driveway on Monterey Road.

Onsite circulation would also be adequate. Trucks circulating around the paved area of the central project site would not encroach upon any of the parking spaces, nor any of the proposed or retained on-site buildings. Although the template assumes a counterclockwise circulation around the project site, the reverse (clockwise) circulation would also be adequate. No additional improvements would be required.

## 9 SITE ACCESS ANALYSIS

This section summarizes the site access, including operations of the project driveway operations.

### 9.1 Driveway Operations

As discussed previously and shown on **Exhibit 4**, operations of the project driveway would all be better than the County standard of LOS D. No improvements are required due to level of service.

### 9.2 Driveway Sight Distance

Sight distance was evaluated for the project driveway on Monterey Road (“Monterey”). Only the Caltrans minimum stopping sight distance standard applies at private driveways. The available sight distance must meet this standard.

The sight distance was measured from 15 feet back from the traveled way of the major street (i.e., edge of nearest through lane).

The posted speed limit on Monterey Road is 50 mph. To be conservative, a speed of 55 mph was used for the sight distance evaluation. For 55 mph on a four-lane roadway, Caltrans sight distance standard standards require a minimum stopping sight distance of 500 feet. **Appendix H** contains the calculation of the applicable sight distance standards at the project driveway.

The available sight distance to the north from the eastbound Monterey driveway is 490 feet, while the available sight distance to the south is 800 feet. The available sight distance to the north is slightly below the applicable Caltrans minimum sight distance standard for this driveway, while the sight distance to the south exceeds this standard. Although the sight distance to the north is limited by the Monterey Road / California Avenue intersection, the fact that left turns are not allowed out of the project driveway will minimize the discrepancy such that improvements are not required.

### 9.3 Driveway Queuing

**Exhibit 11** summarizes the vehicle queuing evaluation at the Monterey Road / San Martin Avenue intersection.

The infrequent use of the project driveway by project traffic will result in minimal queuing. All queues will be stored in the long driveway off Monterey. No improvements are required.

## 10 PROJECT VEHICLE MILES TRAVELED

This section summarizes the calculation of the project Vehicle Miles Traveled (VMT).

As described in Section 1.5.1 of this report, Senate Bill (SB) 743 is changing the CEQA Guidelines statewide. SB 743 requires that, starting July 2020, transportation impacts for projects per the California Environmental Quality Act (CEQA) be based on a project's Vehicle Miles Traveled (VMT), rather than level of service. The changes to CEQA guidelines will replace congestion-based metrics, such as auto delay and level of service, with Vehicle Miles Traveled (VMT) as the basis for determining significant impacts under the California Environmental Quality Act (CEQA), unless the guidelines provide specific exceptions.

Santa Clara Valley Transportation Agency (VTA), on behalf of all of Santa Clara County, has established a process to determine if properties could have a significant impact to VMT. However, as the project is not easily categorized as either housing or employment, the project's VMT is not easily determined. The State VMT guidelines are presented in *Technical Advisory on Evaluating Transportation Impacts in CEQA*, State of California Governor's Office of Planning and Research, December 2018. In the state publication, it is established that projects generating less than 110 daily trips are exempt from any further VMT analysis. Per the trip generation table on **Exhibit 5**, the project generates only 48 daily trips. It is thus determined that the study project qualifies for this exemption. There is no need for further VMT analysis.

## **11 SUMMARY OF RECOMMENDATIONS**

Below is a summary of the recommended improvements in this traffic impact analysis report.

1. Extend the storage of the southbound Monterey left turn at San Martin Avenue to 200 feet by eliminating the northbound Monterey left turn at Burbank Avenue. At the end of the new southbound left turn, transition into the existing Monterey two-way left turn lane.
2. Add an acceleration lane and a deceleration lane on Monterey Road above and below the project driveway.

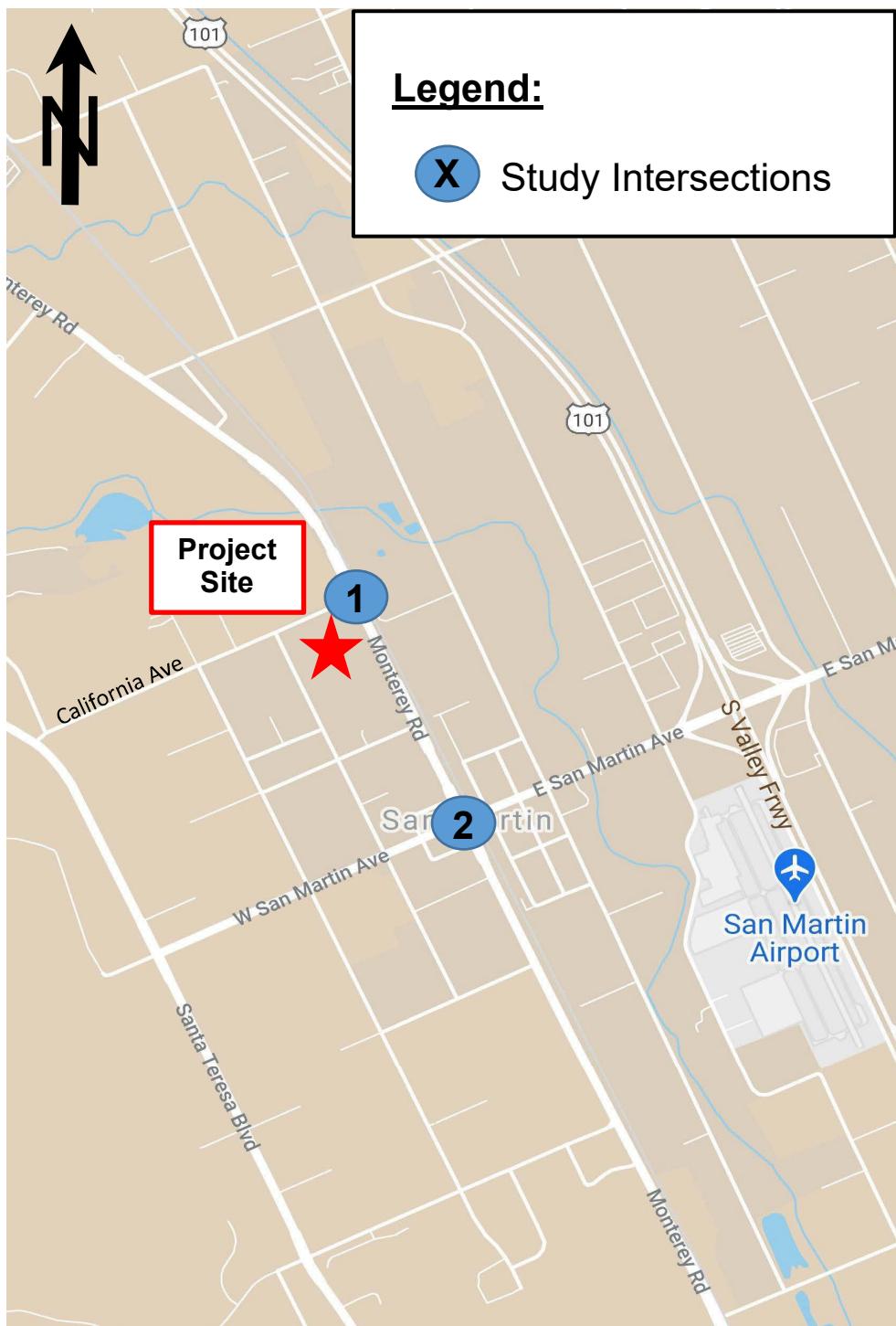
## 12 REFERENCES

### 12.1 List of References

1. *2000 Highway Capacity Manual*, Transportation Research Board, 2000.
2. *Traffic Level of Service Analysis Guidelines*, Santa Clara County Transportation Authority Congestion Management Program, Updated June 2003.
3. *Santa Clara County 2030 General Plan*, Santa Clara County, Adopted June 2012.
4. *Technical Advisory on Evaluating Transportation Impacts in CEQA*, State of California Governor's Office of Planning and Research, December 2018.
5. Santa Clara Valley Transportation Authority web site, <http://www.vta.org>. Accessed April 7, 2022.
6. Caltrain web site, <http://www.caltrain.com>. Accessed April 7, 2022.
7. *Cordoba Center Project; Santa Clara County (San Martin), California – Supplemental Traffic Analysis Material*, Pinnacle Traffic Engineering, November 26, 2018.
8. *Trip Generation Manual*, 10<sup>th</sup> Edition, Institute of Transportation Engineers, September 2017.
9. *Transportation Analysis for Cordoba Center in San Martin*, Fehr & Peers, April 28, 2017.
10. *Morgan Hill 2035 DEIR*, Placeworks, January 13, 2016.
11. *Heart of the Valley RV Resort Traffic Impact Study*, Santa Clara, California, Hatch Mott MacDonald, July 24, 2015.
12. *Technical Advisory on Evaluating Transportation Impacts in CEQA*, State of California Governor's Office of Planning and Research, December 2018.
13. Santa Clara County VMT calculation tool web site, <https://vmttool.vta.org>. Accessed May 19, 2022.

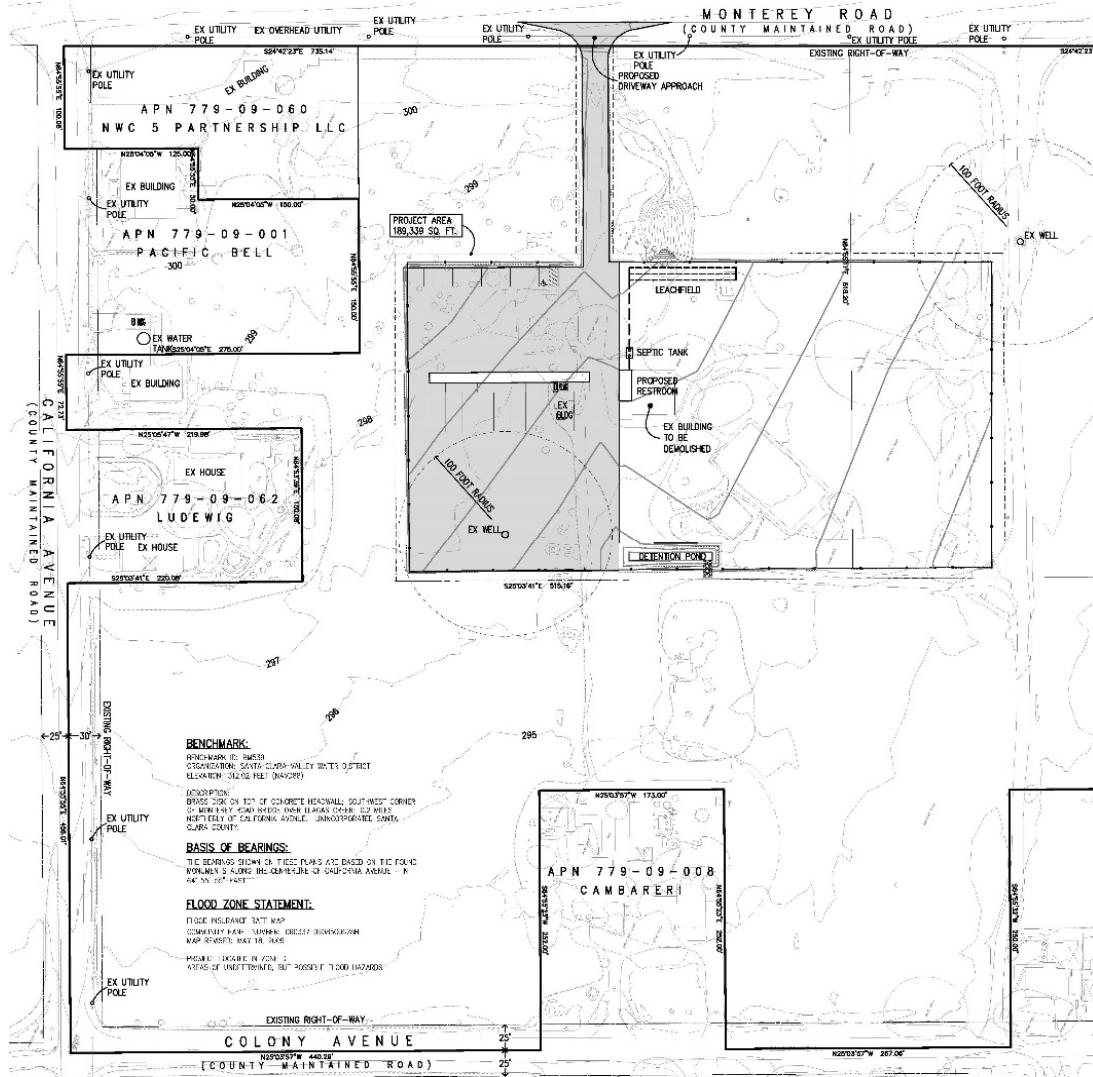
### 12.2 List of Contacts

1. Amanda Musy-Verdel, Hanna – Brunetti, Gilroy, California.
2. Mark Perry, Northwest Cascade, Inc., Puyallup, Washington.
3. Leo Camacho, Santa Clara County Roads and Airports Department, San Jose, California.
4. Joanna Wilk, Santa Clara County Planning and Development Department, San Jose, California.



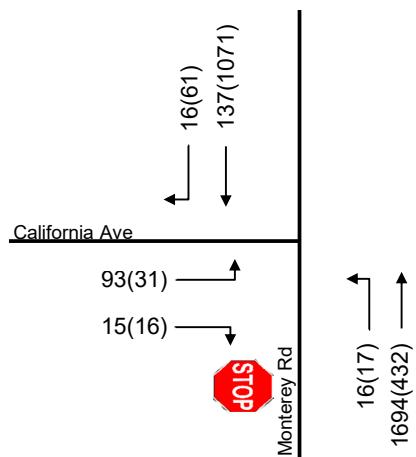
Basemap Source: Google Maps, 2020.

**Exhibit 1**  
**Project Location Map**  
**and Study Area**

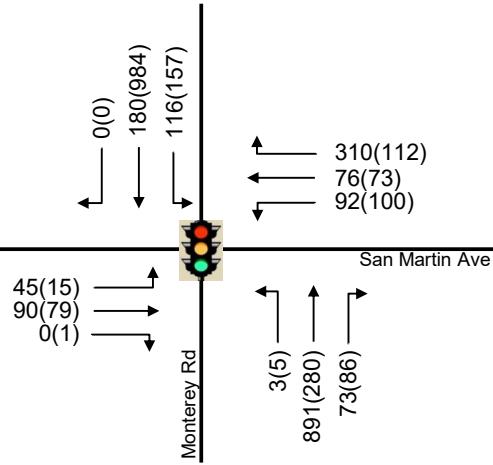


Source: Hanna - Brunetti, February 2022.

1. Monterey Road / California Avenue



2. Monterey Road / San Martin Avenue



3. Project Driveway / Monterey Road

N/A

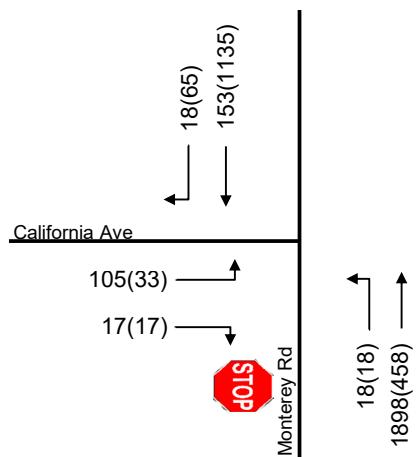
(Intersection does not exist under this scenario)

Legend:

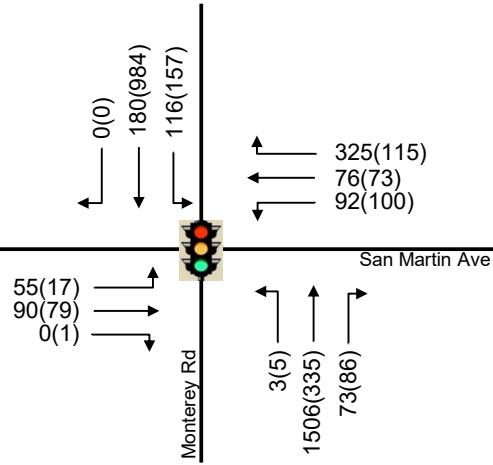
XXX(XXX) = AM Peak Hour Volume (PM Peak Hour Volume)

**Exhibit 3A**  
**Existing Conditions**  
**(Raw) Peak Hour Volumes**

1. Monterey Road / California Avenue



2. Monterey Road / San Martin Avenue



3. Project Driveway / Monterey Road

N/A  
(Intersection does not exist under this scenario)

Legend:

XXX(XXX) = AM Peak Hour Volume (PM Peak Hour Volume)

**Exhibit 3B**  
**Existing Conditions (Adjusted)**  
**Peak Hour Volumes**

N-S Street	E-W Street	Existing Lane Configuration	Existing Intersection Control	LOS Standard	Peak Hour	Existing Conditions		Existing Plus Project Conditions		Cumulative Without Project Conditions		Cumulative Plus Project Conditions	
						Delay	LOS	Delay	LOS	Delay	LOS	Delay	LOS
1 Monterey Road	California Avenue	NB 1-L, 2-T SB 1-T, 1-T/R EB 1-L/R	One-Way Stop	D	AM	56.0	F	57.0	F	205.0	F	205.8	F
					PM	39.1	E	39.3	E	122.9	F	124.1	F
2 Monterey Road	San Martin Avenue	NB 1-L, 2-T, 1-R SB 1-L, 1-T, 1-T/R EB 1-L/T/R WB 2-L, 1-T, 1-R	Signal	D	AM	62.3	E	63.1	E	113.3	F	114.5	F
					PM	22.1	C+	22.1	C+	25.5	C	25.5	C
3 Monterey Road	Project Driveway	N/A	N/A	D	AM	N/A		9.7	A	N/A		9.9	A
					PM			16.8	C			23.1	C

Notes:

1. L, T, R = Left, Through, Right.
  2. NB, SB, EB, WB = Left, Through, Right, Northbound, Southbound, Eastbound, Westbound.
  3. \* = Delay exceeds 3000 seconds
  4. Overall and side-street level of service standards for Santa Clara County are LOS D.
  5. N/A = Not Applicable. This intersection or control type does not exist under this scenario.
  6. For signalized and all-way stop intersection analysis, delay is average overall delay in seconds per vehicle (sec/veh).
- For one- and two-way stop intersections, delays are side-street approach operations, also in seconds per vehicle (sec/veh).
7. Analysis performed using 2000 Highway Capacity Manual methodologies.
  8. Level of service calculations can be found in **Appendices C-F**.
  9. LOS highlighted in red indicates intersection operating below level of service standard.
  10. Delays highlighted in bold indicate intersection impacts.
  11. Levels of service with recommended improvements noted under "With Improvements".

## Exhibit 4

### Intersection Levels of Service

PROPOSED USE	UNITS	DAILY TRIPS	WEEKDAY					
			AM PEAK HOUR			PM PEAK HOUR		
			PEAK HOUR TRIPS	% OF ADT	TRIPS IN OUT	PEAK HOUR TRIPS	% OF ADT	TRIPS IN OUT
A. Employees	10	20	0	0%	0 0	0	0%	0 0
B. Trucks								
1. Delivery Trucks	2	8	1	13%	0 1	0	0%	0 0
2. Route Trucks	8	16	2	13%	0 2	2	13%	2 0
C. Deliveries	2	4	2		1 1	2		1 1
<b>D. Total</b>		<b>48</b>	<b>5</b>		<b>1 4</b>	<b>4</b>		<b>3 1</b>

Notes:

General:

1. Hours of Operation: 5:00 AM - 7:00 PM (Weekdays)

2. A trip is defined here as a journey from Point A to Point B.

Employees:

3. Number of Employees: 10 people  
 4. Employee Vehicle Occupancy: 1 employee/vehicle (estimated)  
 5. Employee Daily Trip Rate: 2 trips/employee (estimated)

6. Percentage of Employees arriving/departing during peak hours (estimate):  
 AM: 0% in, 0% out  
 PM: 0% in, 0% out

7. Percentage of Employees being dropped off by non-employees (estimate):

AM: 0%  
 PM: 0%

Trucks:

8. Trucks transport portable toilets to and from the field, as well as service deployed portable toilets.  
 9. Once they have left the project site, trucks will travel to multiple locations prior to returning back to the project site.  
 10. Two types of truck trips will occur daily:

- a. Delivery Trucks: Deliver and return individual portable toilets.  
 b. Route Trucks: Service deployed portable toilets (fresh water, toilet paper, etc.)

11. Number of Total Trucks stored on site:

- a. Delivery Trucks: 2 trucks  
 b. Route Trucks: 10 trucks

12. Number of Daily Deployed Trucks:

- a. Delivery Trucks: 2 trucks  
 b. Route Trucks: 8 trucks

13. Number of Daily Truck Trips:

- a. Delivery Trucks: 4 trips (2 in, 2 out)  
 b. Route Trucks: 2 trips (1 in, 1 out)

14. Truck Trips occurring in each peak hour (estimate):

Delivery	Route
----------	-------

AM: 50%	25%
---------	-----

PM: 0%	25%
--------	-----

Delivery	Route
----------	-------

15. Truck Directional Split: AM: In: 0% Out: 100%  
 PM: In: 0% Out: 100%

Visitors:

16. Daily visitors: 0 visitors

Deliveries:

17. Deliveries include US Mail, overnight deliveries, etc.

18. Number of Daily Deliveries to site (assumed): 2 delivery

19. One Delivery = 2 trips

20. Delivery Trips occurring in each peak hour (estimate): AM: 100%  
 PM: 100%

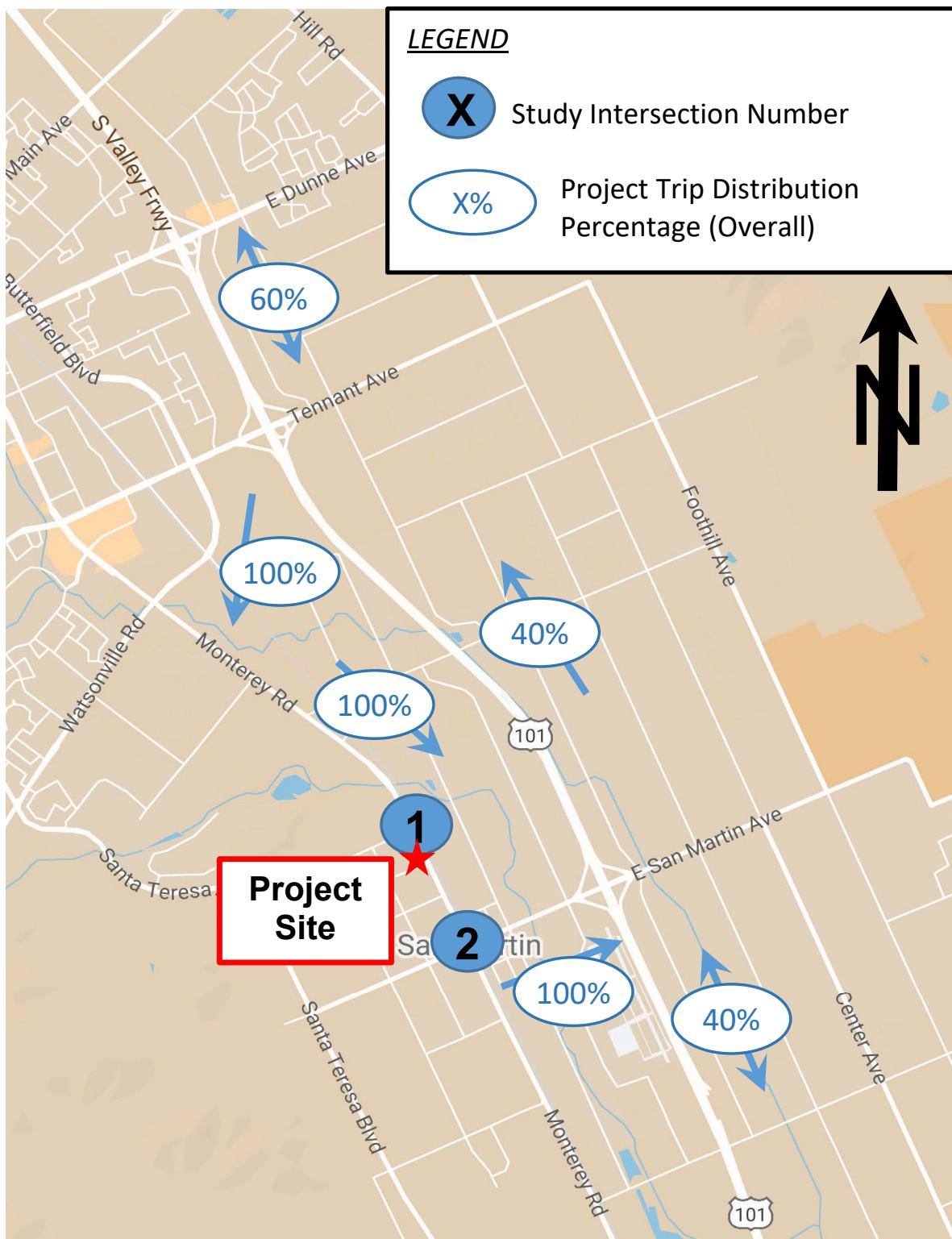
21. Delivery Directional Split:

AM: In: 50% Out: 50%
----------------------

PM: In: 50% Out: 50%
----------------------

## Exhibit 5

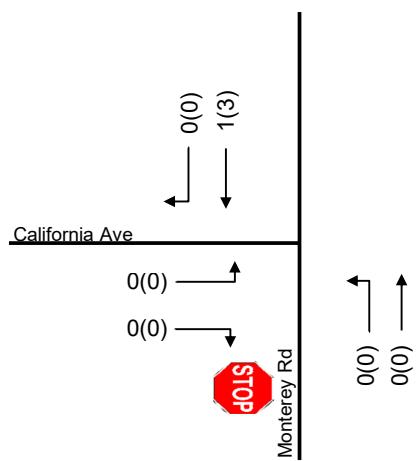
### Project Trip Generation



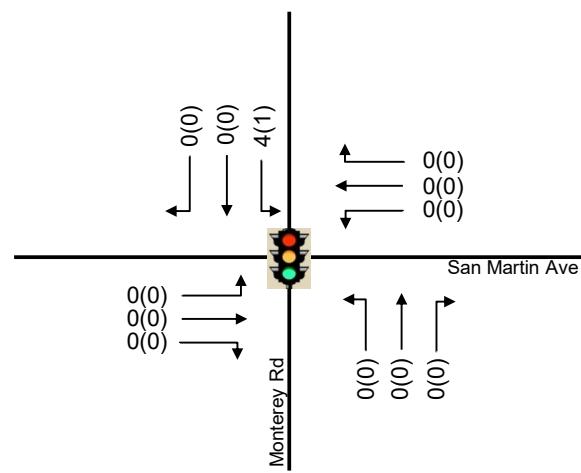
Basemap Source: Google Maps, 2022.

**Exhibit 6**  
**Project Trip**  
**Distribution**

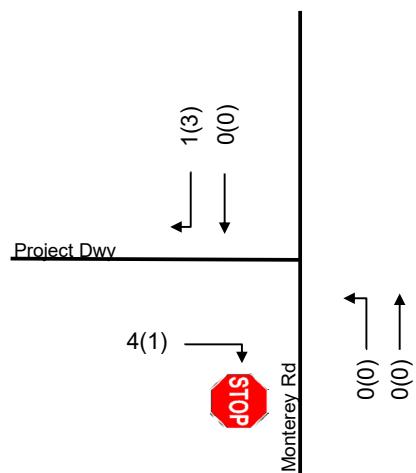
1. Monterey Road / California Avenue



2. Monterey Road / San Martin Avenue



3. Project Driveway / Monterey Road

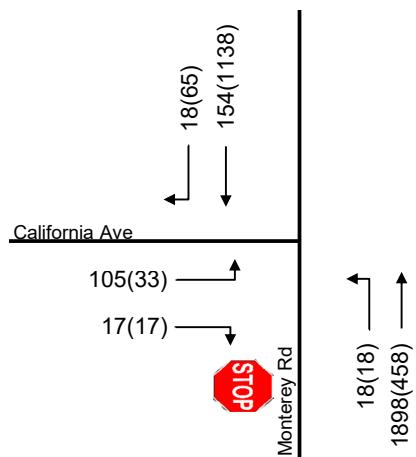


Legend:

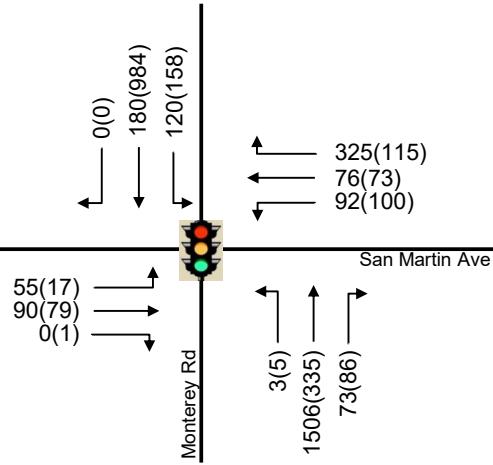
XX(XX) = AM Peak Hour Trips (PM Peak Hour Trips)

**Exhibit 7**  
**Project Trip**  
**Assignment**

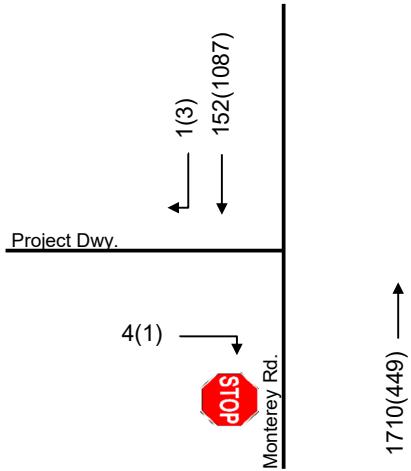
1. Monterey Road / California Avenue



2. Monterey Road / San Martin Avenue



3. Project Driveway / Monterey Road

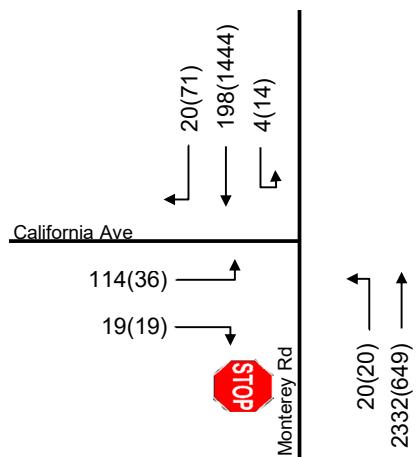


Legend:

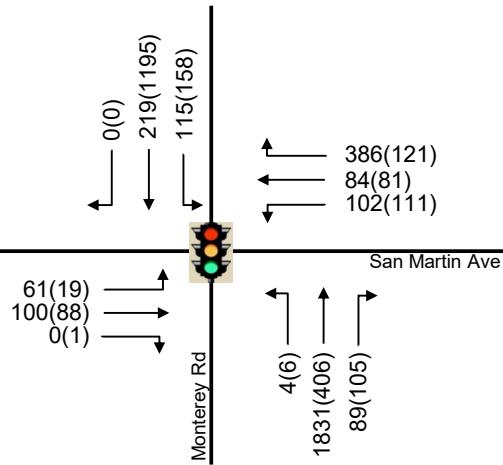
XXX(XXX) = AM Peak Hour Volume (PM Peak Hour Volume)

**Exhibit 8**  
**Existing Plus Project Conditions**  
**Peak Hour Volumes**

1. Monterey Road / California Avenue



2. Monterey Road / San Martin Avenue



3. Project Driveway / Monterey Road

N/A

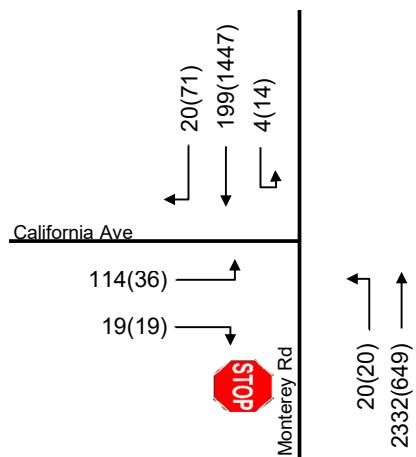
(Intersection does not exist under this scenario)

Legend:

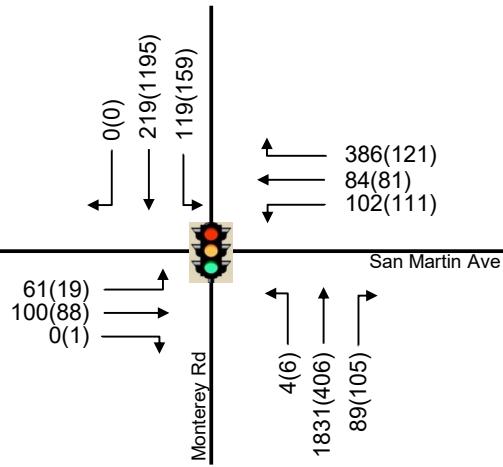
XXX(XXX) = AM Peak Hour Volume (PM Peak Hour Volume)

**Exhibit 9**  
**Cumulative w/o Project Conditions**  
**Peak Hour Volumes**

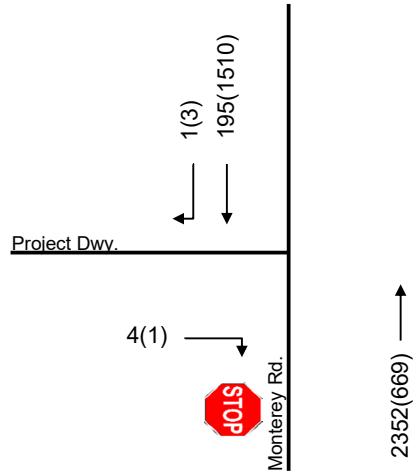
1. Monterey Road / California Avenue



2. Monterey Road / San Martin Avenue



3. Project Driveway / Monterey Road



Legend:

XXX(XXX) = AM Peak Hour Volume (PM Peak Hour Volume)

**Exhibit 10**  
**Cumulative Plus Project Conditions**  
**Peak Hour Volumes**

N-S Street	E-W Street	Intersection Control	Peak Hour	Existing Conditions		Existing Plus Project Conditions		Cumulative Without Project Conditions		Cumulative With Project Conditions	
				95th Percentile (Design) Queue Lengths (per lane - feet)							
				SB L	WB R	SB L	WB R	SB L	WB R	SB L	WB R
2 Monterey Road	San Martin Avenue	Signal	AM	175	400	200	400	175	475	175	475
			PM	150	125	150	125	150	125	150	125
		Available Storage (feet)		115	170	115	170	115	170	115	170

Notes:

1. L, T, R = Left, Through, Right.
2. NB, SB, EB, WB = Left, Through, Right, Northbound, Southbound, Eastbound, Westbound.
3. The 95th Percentile (Design) vehicle queue lengths represent the maximum queue lengths in each peak hour for use in roadway design.
4. Queue lengths estimated using the Poisson approximation method, as documented by the Santa Clara Valley Transportation Authority (VTA).
5. Vehicle queue calculations can be found in **Appendix I**. These queue calculations assume a typical vehicle length of 25 feet.
6. \* = Queue lengths are per lane for both turn lanes.
7. Available queue storage is measured from Google Earth.
8. Queues highlighted in red indicates queues longer than available storage lengths (including block lengths).

**Exhibit 11**  
**Intersection**  
**Vehicle Queues**



Source: Hanna - Brunetti, June 2022.

**Exhibit 12**  
**Project Site**  
**Truck Turning Template**

## **Appendix A**

### **Level of Service Descriptions**

## APPENDIX A1

### LEVEL OF SERVICE (LOS) DESCRIPTION

### SIGNALIZED INTERSECTIONS

The capacity of an urban street is related primarily to the signal timing and the geometric characteristics of the facility as well as to the composition of traffic on the facility. Geometrics are a fixed characteristic of a facility. Thus, while traffic composition may vary somewhat over time, the capacity of a facility is generally a stable value that can be significantly improved only by initiating geometric improvements. A traffic signal essentially allocates time among conflicting traffic movements that seek to use the same space. The way in which time is allocated significantly affects the operation and the capacity of the intersection and its approaches.

The methodology for signalized intersection is designed to consider individual intersection approaches and individual lane groups within approaches. A lane group consists of one or more lanes on an intersection approach. The outputs from application of the method described in the HCM 2000 are reported on the basis of each lane. For a given lane group at a signalized intersection, three indications are displayed: green, yellow and red. The red indication may include a short period during which all indications are red, referred to as an all-red interval and the yellow indication forms the change and clearance interval between two green phases.

The methodology for analyzing the capacity and level of service must consider a wide variety of prevailing conditions, including the amount and distribution of traffic movements, traffic composition, geometric characteristics, and details of intersection signalization. The methodology addresses the capacity, LOS, and other performance measures for lane groups and the intersection approaches and the LOS for the intersection as a whole.

Capacity is evaluated in terms of the ratio of demand flow rate to capacity (v/c ratio), whereas LOS is evaluated on the basis of control delay per vehicle (in seconds per vehicle). The methodology does not take into account the potential impact of downstream congestion on intersection operation, nor does the methodology detect and adjust for the impacts of turn-pocket overflows on through traffic and intersection operation.

#### **LEVEL OF SERVICE (LOS) CRITERIA FOR SIGNALIZED INTERSECTIONS**

(Reference: Traffic Level of Service Guidelines, Santa Clara County Transportation Authority - Congestion Management Program, Updated June 2003)

<b>Level of Service</b>	<b>Average Control Delay (seconds / vehicle)</b>
<b>A</b>	<b>delay ≤ 10.0</b>
<b>B+</b>	<b>10.0 ≤ delay ≤ 12.0</b>
<b>B</b>	<b>12.0 ≤ delay ≤ 18.0</b>
<b>B-</b>	<b>18.0 ≤ delay ≤ 20.0</b>
<b>C+</b>	<b>20.0 ≤ delay ≤ 23.0</b>
<b>C</b>	<b>23.0 ≤ delay ≤ 32.0</b>
<b>C-</b>	<b>32.0 ≤ delay ≤ 35.0</b>
<b>D+</b>	<b>35.0 ≤ delay ≤ 39.0</b>
<b>D</b>	<b>39.0 ≤ delay ≤ 51.0</b>
<b>D-</b>	<b>51.0 ≤ delay ≤ 55.0</b>
<b>E+</b>	<b>55.0 ≤ delay ≤ 60.0</b>
<b>E</b>	<b>60.0 ≤ delay ≤ 75.0</b>
<b>E-</b>	<b>75.0 ≤ delay ≤ 80.0</b>
<b>F</b>	<b>delay &gt; 80.0</b>

## APPENDIX A2

### LEVEL OF SERVICE (LOS) DESCRIPTION UNSIGNALIZED INTERSECTIONS WITH TWO-WAY STOP CONTROL (TWSC)

TWSC intersections are widely used and stop signs are used to control vehicle movements at such intersections. At TWSC intersections, the stop-controlled approaches are referred to as the minor street approaches; they can be either public streets or private driveways. The intersection approaches that are not controlled by stop signs are referred to as the major street approaches. A three-leg intersection is considered to be a standard type of TWSC intersection if the single minor street approach (i.e. the stem of the T configuration) is controlled by a stop sign. Three-leg intersections where two of the three approaches are controlled by stop signs are a special form of unsignalized intersection control.

At TWSC intersections, drivers on the controlled approaches are required to select gaps in the major street flow through which to execute crossing or turning maneuvers on the basis of judgment. In the presence of a queue, each driver on the controlled approach must use some time to move into the front-of-queue position and prepare to evaluate gaps in the major street flow. Capacity analysis at TWSC intersections depends on a clear description and understanding of the interaction of drivers on the minor or stop-controlled approach with drivers on the major street. Both gap acceptance and empirical models have been developed to describe this interaction.

Thus, the capacity of the controlled legs is based on three factors:

- the distribution of gaps in the major street traffic stream;
- driver judgment in selecting gaps through which to execute the desired maneuvers; and
- the follow-up time required by each driver in a queue.

The delay experienced by a motorist is made up of a number of factors that relate to control, geometrics, traffic and incidents. Total delay is the difference between the travel time actually experienced and the reference travel time that would result during base conditions, in the absence of incident, control, traffic or geometric delay. Average control delay for any particular minor movement is a function of the capacity of the approach and the degree of saturation and referred to as level of service.

### LEVEL OF SERVICE (LOS) CRITERIA FOR TWSC INTERSECTIONS

(Reference 2000 Highway Capacity Manual)

Level of Service	Control Delay (seconds / vehicle)
A	0 - 10
B	>10 - 15
C	>15 - 25
D	>25 - 35
E	>35 - 50
F	>50

## Appendix B

Traffic Volume

Data Collection

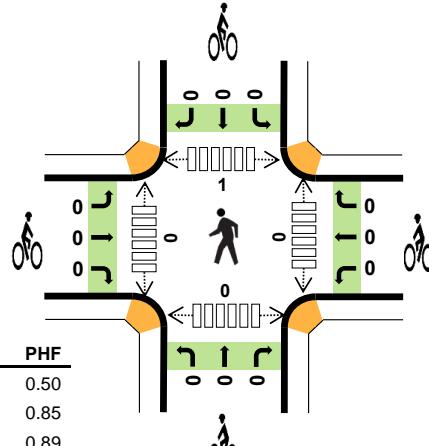
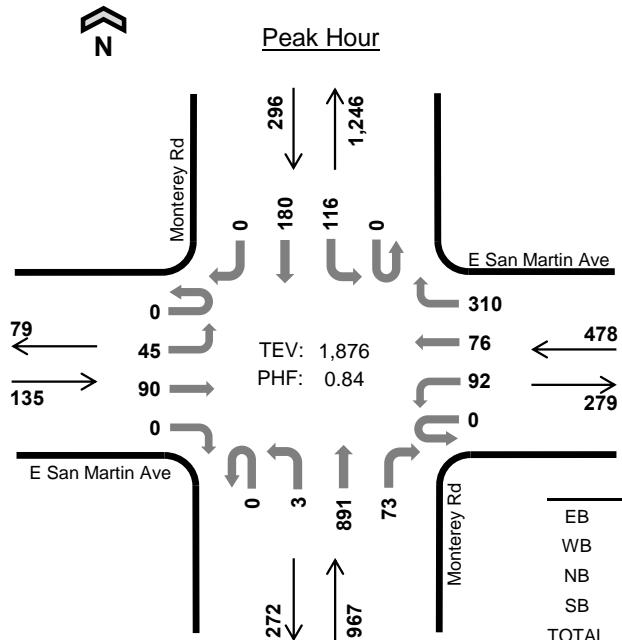
# Monterey Rd E San Martin Ave



Date: 03/30/2022

Count Period: 7:00 AM to 9:00 AM

Peak Hour: 7:00 AM to 8:00 AM



## Two-Hour Count Summaries

Interval Start	E San Martin Ave				E San Martin Ave				Monterey Rd				Monterey Rd				15-min Total	Rolling One Hour					
	Eastbound		Westbound		Northbound		Southbound		UT		LT		TH		RT		UT		LT		TH		RT
7:00 AM	0	10	7	0	0	17	9	65	0	1	220	7	0	8	35	0	379	0					
7:15 AM	0	10	18	0	0	14	10	87	0	0	252	21	0	22	30	0	464	0					
7:30 AM	0	18	49	0	0	26	24	85	0	1	227	23	0	57	50	0	560	0					
7:45 AM	0	7	16	0	0	35	33	73	0	1	192	22	0	29	65	0	473	1,876					
8:00 AM	0	2	16	0	0	20	12	30	0	3	138	17	0	19	59	0	316	1,813					
8:15 AM	0	5	14	0	0	20	12	29	0	2	120	16	0	28	51	0	297	1,646					
8:30 AM	0	6	17	0	0	20	12	37	0	2	97	23	1	18	47	0	280	1,366					
8:45 AM	0	6	16	0	0	18	13	21	0	1	112	21	0	20	54	0	282	1,175					
Count Total	0	64	153	0	0	170	125	427	0	11	1,358	150	1	201	391	0	3,051	0					
Peak Hour	All	0	45	90	0	0	92	76	310	0	3	891	73	0	116	180	0	1,876	0				
	HV	0	1	0	0	0	4	3	8	0	0	37	4	0	3	14	0	74	0				
	HV%	-	2%	0%	-	4%	4%	3%	-	0%	4%	5%	-	3%	8%	-	4%	0					

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	1	2	10	2	15	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	6	12	6	24	0	0	0	0	0	0	0	1	0	1
7:30 AM	0	5	10	3	18	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	2	9	6	17	0	0	0	0	0	0	0	0	0	0
8:00 AM	1	7	9	6	23	0	0	0	0	0	0	1	0	0	1
8:15 AM	1	5	14	7	27	0	0	0	0	0	0	0	1	0	1
8:30 AM	0	7	18	7	32	0	0	0	0	0	0	0	0	0	0
8:45 AM	2	4	15	4	25	0	0	0	0	0	0	0	1	0	1
Count Total	5	38	97	41	181	0	0	0	0	0	0	1	3	0	4
Peak Hour	1	15	41	17	74	0	0	0	0	0	0	0	1	0	1

Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	E San Martin Ave				E San Martin Ave				Monterey Rd				Monterey Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	1	0	0	0	0	1	1	0	0	9	1	0	0	2	0	15	0
7:15 AM	0	0	0	0	0	2	1	3	0	0	10	2	0	2	4	0	24	0
7:30 AM	0	0	0	0	0	1	0	4	0	0	9	1	0	1	2	0	18	0
7:45 AM	0	0	0	0	0	1	1	0	0	0	9	0	0	0	6	0	17	74
8:00 AM	0	0	1	0	0	3	1	3	0	1	7	1	0	1	5	0	23	82
8:15 AM	0	0	1	0	0	2	1	2	0	0	9	5	0	2	5	0	27	85
8:30 AM	0	0	0	0	0	4	0	3	0	1	9	8	0	1	6	0	32	99
8:45 AM	0	0	2	0	0	3	1	0	0	0	9	6	0	1	3	0	25	107
Count Total	0	1	4	0	0	16	6	16	0	2	71	24	0	8	33	0	181	0
Peak Hour	0	1	0	0	0	4	3	8	0	0	37	4	0	3	14	0	74	0
Two-Hour Count Summaries - Bikes																		
Interval Start	E San Martin Ave				E San Martin Ave				Monterey Rd				Monterey Rd				15-min Total	Rolling One Hour
	Eastbound				Westbound				Northbound				Southbound					
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT			
7:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
7:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
7:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
7:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																		

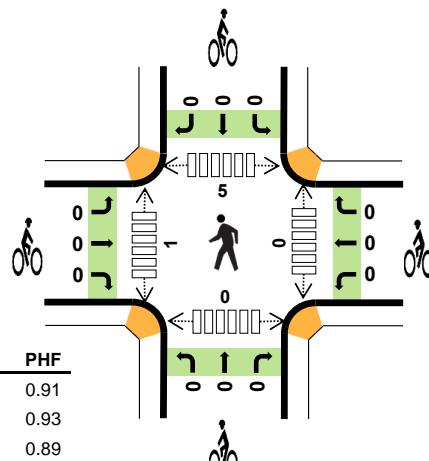
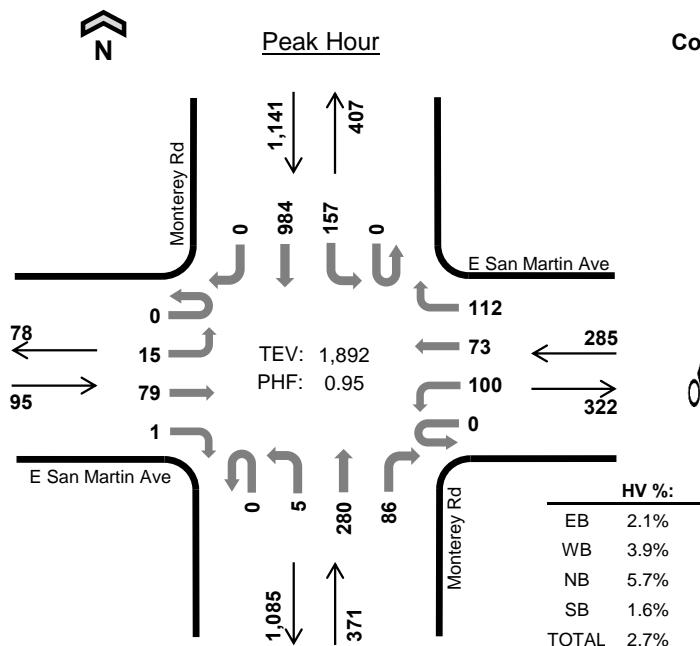
# Monterey Rd E San Martin Ave



Date: 03/30/2022

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:30 PM to 5:30 PM

**Two-Hour Count Summaries**

Interval Start	E San Martin Ave				E San Martin Ave				Monterey Rd				Monterey Rd				15-min Total	Rolling One Hour	
	Eastbound		Westbound		Northbound		Southbound		UT		LT		TH		RT				
4:00 PM	0	6	22	0	0	31	19	32	0	4	72	16	0	35	187	0	424	0	
4:15 PM	0	1	14	0	0	26	18	35	0	0	69	25	0	36	217	1	442	0	
4:30 PM	0	6	13	0	0	32	14	25	0	2	69	27	0	43	250	0	481	0	
4:45 PM	0	4	21	1	0	25	22	30	0	1	81	22	0	42	249	0	498	1,845	
5:00 PM	0	3	23	0	0	26	22	19	0	0	56	21	0	44	245	0	459	1,880	
5:15 PM	0	2	22	0	0	17	15	38	0	2	74	16	0	28	240	0	454	1,892	
5:30 PM	0	3	26	0	0	14	17	22	0	0	65	19	0	31	236	0	433	1,844	
5:45 PM	0	3	12	0	0	12	24	18	0	0	65	9	1	36	188	0	368	1,714	
Count Total	0	28	153	1	0	183	151	219	0	9	551	155	1	295	1,812	1	3,559	0	
Peak Hour	All	0	15	79	1	0	100	73	112	0	5	280	86	0	157	984	0	1,892	0
	HV	0	0	2	0	0	3	1	7	0	0	15	6	0	3	15	0	52	0
	HV%	-	0%	3%	0%	-	3%	1%	6%	-	0%	5%	7%	-	2%	2%	-	3%	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	3	7	7	5	22	0	0	0	0	0	0	2	0	0	2
4:15 PM	0	5	2	7	14	0	0	0	0	0	0	0	2	0	2
4:30 PM	0	3	8	3	14	0	0	0	0	0	0	0	1	0	1
4:45 PM	0	5	3	6	14	0	0	0	0	0	0	0	1	0	1
5:00 PM	2	1	4	7	14	0	0	0	0	0	0	0	2	0	2
5:15 PM	0	2	6	2	10	0	0	0	0	0	0	1	1	0	2
5:30 PM	2	1	3	8	14	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	4	2	6	0	0	0	0	0	0	0	0	0	0
Count Total	7	24	37	40	108	0	0	0	0	0	0	3	7	0	10
Peak Hour	2	11	21	18	52	0	0	0	0	0	0	1	5	0	6

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	E San Martin Ave				E San Martin Ave				Monterey Rd				Monterey Rd				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
4:00 PM	0	0	3	0	0	2	0	5	0	0	5	2	0	1	4	0	22	0		
4:15 PM	0	0	0	0	0	2	0	3	0	0	1	1	0	1	6	0	14	0		
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>4</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>14</b>	<b>0</b>		
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>4</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>1</b>	<b>0</b>	<b>3</b>	<b>3</b>	<b>0</b>	<b>14</b>	<b>64</b>		
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>7</b>	<b>0</b>	<b>14</b>	<b>56</b>		
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>10</b>	<b>52</b>		
5:30 PM	0	0	2	0	0	0	0	1	0	0	2	1	0	1	7	0	14	52		
5:45 PM	0	0	0	0	0	0	0	0	0	0	2	2	0	1	1	0	6	44		
Count Total	0	0	7	0	0	7	1	16	0	0	25	12	0	7	33	0	108	0		
<b>Peak Hour</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>1</b>	<b>7</b>	<b>0</b>	<b>0</b>	<b>15</b>	<b>6</b>	<b>0</b>	<b>3</b>	<b>15</b>	<b>0</b>	<b>52</b>	<b>0</b>		
Two-Hour Count Summaries - Bikes																				
Interval Start	E San Martin Ave				E San Martin Ave				Monterey Rd				Monterey Rd				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
4:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
4:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
5:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
5:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
<b>Peak Hour</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

## Appendix C

Intersection  
Level of Service  
Calculations

Existing  
Conditions

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Monterey Rd / California Ave

Average Delay (sec/veh): 3.2 Worst Case Level Of Service: F[ 56.8]

Street Name:	Monterey Rd	California Ave		
Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	1 0 2 0 1	0 0 1! 0 0	0 0 0 0 0

## Volume Module:AM Peak Hour

Base Vol:	18	1898	0	0	153	18	105	0	17	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	18	1898	0	0	153	18	105	0	17	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
PHF Volume:	19	2041	0	0	165	19	113	0	18	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	19	2041	0	0	165	19	113	0	18	0	0	0

## Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	xxxxx	xxxx	xxxxx
FollowUpTim:	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	xxxxx	xxxx	xxxxx

## Capacity Module:

Cnflict Vol:	184	xxxx	xxxxx	xxxx	xxxx	xxxxx	1224	2244	82	xxxx	xxxx	xxxxx
Potent Cap.:	1388	xxxx	xxxxx	xxxx	xxxx	xxxxx	171	41	961	xxxx	xxxx	xxxxx
Move Cap.:	1388	xxxx	xxxxx	xxxx	xxxx	xxxxx	170	41	961	xxxx	xxxx	xxxxx
Volume/Cap:	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	0.67	0.00	0.02	xxxx	xxxx	xxxx

## Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	7.6	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT											
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	192	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	4.2	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	56.8	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	F	*	*	*	*
ApproachDel:	xxxxxx		xxxxxx					56.8		xxxxxx		
ApproachLOS:	*		*					F		*		

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec):	120	Critical Vol./Cap.(X):	1.001
Loss Time (sec):	16	Average Delay (sec/veh):	62.3
Optimal Cycle:	180	Level Of Service:	E

Street Name:	Monterey Rd			San Martin Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Protected	Protected	Split Phase	Split Phase		
Rights:	Include	Include	Include	Include		
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10		
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0		
Lanes:	1 0 2 0 1	1 0 1 1 0	0 1 0 0 0	2 0 1 0 1		

## Volume Module:AM Peak Hour

Base Vol:	3 1506	73	116	180	0	55	90	0	92	76	325
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	3 1506	73	116	180	0	55	90	0	92	76	325
User Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	0.84 0.84	0.84	0.84 0.84	0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84
PHF Volume:	4 1793	87	138	214	0	65	107	0	110	90	387
Reduct Vol:	0 0	0	0 0	0	0 0	0 0	0 0	0 0	0 0	0 0	0 0
Reduced Vol:	4 1793	87	138	214	0	65	107	0	110	90	387
PCE Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
MLF Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FinalVolume:	4 1793	87	138	214	0	65	107	0	110	90	387

## Saturation Flow Module:

Sat/Lane:	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900
Adjustment:	0.92 1.00	0.92 0.92	0.92 0.97	0.92 0.92	0.95 0.95	0.95 0.95	0.92 0.92	0.83 1.00	0.83 1.00	0.83 1.00	0.92 0.92
Lanes:	1.00 2.00	1.00 1.00	1.00 2.00	1.00 2.00	0.00 0.00	0.38 0.62	0.62 0.00	2.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Final Sat.:	1750 3800	1750 1750	1750 3700	1750 3700	0 0	683 1117	1117 0	3150 3150	1900 1900	1900 1900	1750 1750

## Capacity Analysis Module:

Vol/Sat:	0.00 0.47	0.05 0.08	0.06 0.00	0.10 0.10	0.10 0.00	0.03 0.03	0.05 0.05	0.22 0.22
Crit Moves:	****	****	****	****	****	****	****	****
Green Time:	7.0 56.5	56.5 9.5	38.8 0.0	11.5 11.5	0.0 26.5	26.5 26.5	26.5 26.5	26.5 26.5
Volume/Cap:	0.04 1.00	0.11 1.00	0.18 0.00	1.00 1.00	0.00 0.00	0.16 0.16	0.22 0.22	1.00 1.00
Uniform Del:	53.3 31.7	31.7 55.3	29.1 0.0	54.3 54.3	0.0 37.7	38.2 38.2	46.8 46.8	46.8 46.8
IncremntDel:	0.1 21.6	0.1 76.9	0.1 0.0	68.9 68.9	0.0 0.1	0.3 0.3	46.1 46.1	46.1 46.1
InitQueueDel:	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0
Delay Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	0.00 0.00	1.00 1.00	1.00 1.00	1.00 1.00
Delay/Veh:	53.5 53.3	53.3 17.7	29.2 132.2	29.2 123.1	123 0.0	37.8 37.8	38.5 38.5	92.8 92.8
User DelAdj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
AdjDel/Veh:	53.5 53.3	53.3 17.7	29.2 132.2	29.2 123.1	123 0.0	37.8 37.8	38.5 38.5	92.8 92.8
LOS by Move:	D- D-	B F	C A	F F	F A	D+ D+	D+ D+	F
HCM2kAvgQ:	0 41	2 7	3 0	11 11	11 0	2 2	3 3	21 21

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Monterey Rd / California Ave

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: E[ 39.1]

Street Name:	Monterey Rd			California Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign		
Rights:	Include	Include	Include	Include		
Lanes:	1 0 2 0 0	1 0 2 0 1	0 0 1! 0 0	0 0 0 0 0		

## Volume Module:PM Peak Hour

Base Vol:	18	458	0	0	1135	65	33	0	17	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	18	458	0	0	1135	65	33	0	17	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
PHF Volume:	19	477	0	0	1182	68	34	0	18	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	19	477	0	0	1182	68	34	0	18	0	0	0

## Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	xxxxx	xxxx	xxxxx
FollowUpTim:	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	xxxxx	xxxx	xxxxx

## Capacity Module:

Cnflict Vol:	1250	xxxx	xxxxx	xxxx	xxxx	xxxxx	1458	1697	591	xxxx	xxxx	xxxxx
Potent Cap.:	553	xxxx	xxxxx	xxxx	xxxx	xxxxx	120	92	450	xxxx	xxxx	xxxxx
Move Cap.:	553	xxxx	xxxxx	xxxx	xxxx	xxxxx	117	88	450	xxxx	xxxx	xxxxx
Volume/Cap:	0.03	xxxx	xxxx	xxxx	xxxx	xxxx	0.29	0.00	0.04	xxxx	xxxx	xxxx

## Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	11.7	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
LOS by Move:	B	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT											
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	156	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	1.4	xxxxx	xxxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	39.1	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	E	*	*	*	*
ApproachDel:	xxxxxx		xxxxxx					39.1		xxxxxx		
ApproachLOS:	*		*					E		*		

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec):	70	Critical Vol./Cap.(X):	0.495
Loss Time (sec):	16	Average Delay (sec/veh):	22.1
Optimal Cycle:	53	Level Of Service:	C+

Street Name:	Monterey Rd	San Martin Ave
--------------	-------------	----------------

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R

Control:	Protected	Protected	Split Phase	Split Phase
Rights:	Include	Include	Include	Include
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0
Lanes:	1 0 2 0 1	1 0 1 1 0	0 0 1! 0 0	2 0 1 0 1

## Volume Module:PM Peak Hour

Base Vol:	5 335	86 157	984 0	17 79	1 100	73 115
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	5 335	86 157	984 0	17 79	1 100	73 115
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	0.95 0.95	0.95 0.95	0.95 0.95	0.95 0.95	0.95 0.95	0.95 0.95
PHF Volume:	5 353	91 165	1036 0	18 83	1 105	77 121
Reduc Vol:	0 0	0 0	0 0	0 0	0 0	0 0
Reduced Vol:	5 353	91 165	1036 0	18 83	1 105	77 121
PCE Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
MLF Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FinalVolume:	5 353	91 165	1036 0	18 83	1 105	77 121

## Saturation Flow Module:

Sat/Lane:	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900
Adjustment:	0.92 1.00	0.92 0.97	0.92 0.92	0.92 0.92	0.92 0.92	0.83 1.00	0.92 0.92
Lanes:	1.00 2.00	1.00 2.00	0.00 0.00	0.18 0.81	0.01 0.01	2.00 1.00	1.00 1.00
Final Sat.:	1750 3800	1750 3700	0 0	307 1425	18 18	3150 1900	1750 1750

## Capacity Analysis Module:

Vol/Sat:	0.00 0.09	0.05 0.09	0.28 0.00	0.06 0.06	0.06 0.06	0.03 0.04	0.07 ****
Crit Moves:	****	****	****	****	****	****	****
Green Time:	7.0 20.0	20.0 14.0	27.0 0.0	10.0 10.0	10.0 10.0	10.0 10.0	10.0 10.0
Volume/Cap:	0.03 0.32	0.18 0.47	0.73 0.00	0.41 0.41	0.41 0.41	0.23 0.28	0.48 0.48
Uniform Del:	28.4 19.7	18.8 24.7	18.3 0.0	27.3 27.3	27.3 27.3	26.6 26.6	26.8 27.6
IncremntDel:	0.1 0.2	0.2 1.0	1.9 0.0	1.1 1.1	1.1 1.1	0.3 0.3	0.6 1.5
InitQueueDel:	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0
Delay Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Delay/Veh:	28.5 19.9	19.0 25.7	20.2 0.0	28.4 28.4	28.4 28.4	26.9 26.9	27.4 29.1
User DelAdj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
AdjDel/Veh:	28.5 19.9	19.0 25.7	20.2 0.0	28.4 28.4	28.4 28.4	26.9 26.9	27.4 29.1
LOS by Move:	C B-	B- C	C+ A	C C	C C	C C	C C
HCM2kAvgQ:	0 3	2 3	10 0	3 3	3 3	1 2	3 3

Note: Queue reported is the number of cars per lane.

## Appendix D

Intersection

Level of Service

Calculations

Existing Plus Project

Conditions

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Monterey Rd / California Ave

Average Delay (sec/veh): 3.2 Worst Case Level Of Service: F[ 57.0]

Street Name:	Monterey Rd	California Ave		
Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	1 0 2 0 1	0 0 1! 0 0	0 0 0 0 0

Volume Module:AM Peak Hour

Base Vol:	18	1898	0	0	154	18	105	0	17	0	0	0
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	18	1898	0	0	154	18	105	0	17	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
PHF Volume:	19	2041	0	0	166	19	113	0	18	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	19	2041	0	0	166	19	113	0	18	0	0	0

Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx	6.8	6.5	6.9	xxxxx	xxxx	xxxxx
FollowUpTim:	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	xxxxx	xxxx	xxxxx

Capacity Module:

Cnflict Vol:	185	xxxx	xxxxx	xxxx	xxxx	xxxxx	1225	2245	83	xxxx	xxxx	xxxxx
Potent Cap.:	1387	xxxx	xxxxx	xxxx	xxxx	xxxxx	171	41	960	xxxx	xxxx	xxxxx
Move Cap.:	1387	xxxx	xxxxx	xxxx	xxxx	xxxxx	169	41	960	xxxx	xxxx	xxxxx
Volume/Cap:	0.01	xxxx	xxxx	xxxx	xxxx	xxxx	0.67	0.00	0.02	xxxx	xxxx	xxxx

Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	7.6	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
LOS by Move:	A	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT											
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	191	xxxxx	xxxx	xxxx	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	4.2	xxxxx	xxxx	xxxx	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	57.0	xxxxx	xxxxx	xxxx	xxxxx
Shared LOS:	*	*	*	*	*	*	*	F	*	*	*	*
ApproachDel:	xxxxxx		xxxxxx					57.0		xxxxxx		
ApproachLOS:	*		*					F		*		

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec):	120	Critical Vol./Cap.(X):	1.004
Loss Time (sec):	16	Average Delay (sec/veh):	63.1
Optimal Cycle:	180	Level Of Service:	E

Street Name:	Monterey Rd			San Martin Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Protected	Protected	Split Phase	Split Phase		
Rights:	Include	Include	Include	Include		
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10		
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0		
Lanes:	1 0 2 0 1	1 0 1 1 0	0 1 0 0 0	2 0 1 0 1		

## Volume Module:AM Peak Hour

Base Vol:	3 1506	73	120	180	0	55	90	0	92	76	325
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	3 1506	73	120	180	0	55	90	0	92	76	325
User Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	0.84 0.84	0.84	0.84 0.84	0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84
PHF Volume:	4 1793	87	143	214	0	65	107	0	110	90	387
Reduct Vol:	0 0	0	0 0	0	0 0	0 0	0 0	0 0	0 0	0 0	0 0
Reduced Vol:	4 1793	87	143	214	0	65	107	0	110	90	387
PCE Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
MLF Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FinalVolume:	4 1793	87	143	214	0	65	107	0	110	90	387

## Saturation Flow Module:

Sat/Lane:	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900
Adjustment:	0.92 1.00	0.92	0.92 0.97	0.92	0.95 0.95	0.95	0.92 0.83	1.00	0.83 1.00	0.92	
Lanes:	1.00 2.00	1.00	1.00 2.00	0.00	0.38 0.62	0.62	0.00 2.00	1.00	2.00 1.00	1.00	
Final Sat.:	1750 3800	1750	1750 3700	0	683 1117	1117	0 3150	1900	3150 1900	1750	

## Capacity Analysis Module:

Vol/Sat:	0.00 0.47	0.05	0.08 0.06	0.00	0.10 0.10	0.00	0.03 0.05	0.22
Crit Moves:	****		****		****		****	
Green Time:	7.0 56.3	56.3	9.8 38.9	0.0	11.5 11.5	0.0	26.4 26.4	26.4
Volume/Cap:	0.04 1.00	0.11	1.00 0.18	0.00	1.00 1.00	0.00	0.16 0.22	1.00
Uniform Del:	53.3 31.8	17.8	55.1 29.1	0.0	54.3 54.3	0.0	37.8 38.3	46.8
IncremntDel:	0.1 22.5	0.1	75.0 0.1	0.0	69.9 69.9	0.0	0.1 0.3	47.1
InitQueueDel:	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0
Delay Adj:	1.00 1.00	1.00	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00
Delay/Veh:	53.5 54.3	17.8	130.1 29.2	0.0	124.2 124	0.0	37.9 38.6	93.9
User DelAdj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
AdjDel/Veh:	53.5 54.3	17.8	130.1 29.2	0.0	124.2 124	0.0	37.9 38.6	93.9
LOS by Move:	D- D-	B F	C A	F F	A A	D+ D+	D+ F	
HCM2kAvgQ:	0 41	2 7	3 0	11 11	0 0	2 3	21	

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Monterey Rd / Project Dwy S

Average Delay (sec/veh): 0.0 Worst Case Level Of Service: A[ 9.7]

Street Name:	Monterey Rd				Project Dwy S																	
Approach:	North Bound		South Bound		East Bound		West Bound															
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R							
Control:	Uncontrolled				Uncontrolled				Stop Sign				Stop Sign									
Rights:	Include				Include				Include				Include									
Lanes:	0	0	2	0	0	0	0	1	1	0	0	0	0	0	1	0	0	0	0	0	0	0

## Volume Module:AM Peak Hour

Base Vol:	0	1710	0	0	152	1	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
Initial Bse:	0	1710	0	0	152	1	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	
PHF Adj:	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	0.92	
PHF Volume:	0	1859	0	0	165	1	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	1859	0	0	165	1	0	0	4	0	0	0	0	0	0	0	0	0	0	0	0	0

## Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	8.4	xxxxx	xxxx	xxxxx
FollowUpTim:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	4.1	xxxxx	xxxx	xxxxx

## Capacity Module:

Cnflict Vol:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	83	xxxx	xxxx	xxxxx
Potent Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	767	xxxx	xxxx	xxxxx
Move Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	767	xxxx	xxxx	xxxxx
Total Cap:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	199	52	xxxxx	74	118	xxxxx
Volume/Cap:	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	xxxx	0.01	xxxx	xxxx	xxxxx

## Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	0.0	xxxx	xxxx	xxxxx			
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	9.7	xxxxx	xxxx	xxxxx			
LOS by Move:	*	*	*	*	*	*	*	*	A	*	*	*			
Movement:	LT	-	LTR	-	RT	LT	-	LTR	-	RT	LT	-	LTR	-	RT
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx			
SharedQueue:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx			
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx			
Shared LOS:	*	*	*	*	*	*	*	*	*	*	*	*			
ApproachDel:	xxxxx		xxxxxx						9.7	xxxxxx					
ApproachLOS:	*		*						A			*			

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Monterey Rd / California Ave

Average Delay (sec/veh): 1.3 Worst Case Level Of Service: E[ 39.3]

Street Name: Monterey Rd California Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 1 0 2 0 1 0 0 1! 0 0 0 0 0 0 0 0 0 0

Volume Module: PM Peak Hour

Base Vol: 18 458 0 0 1138 65 33 0 17 0 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 18 458 0 0 1138 65 33 0 17 0 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96

PHF Volume: 19 477 0 0 1185 68 34 0 18 0 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 19 477 0 0 1185 68 34 0 18 0 0 0 0 0

Critical Gap Module:

Critical Gp: 4.1 xxxx xxxx xxxx xxxx xxxx 6.8 6.5 6.9 xxxx xxxx xxxx

FollowUpTim: 2.2 xxxx xxxx xxxx xxxx xxxx 3.5 4.0 3.3 xxxx xxxx xxxx

Capacity Module:

Cnflct Vol: 1253 xxxx xxxx xxxx xxxx xxxx 1461 1700 593 xxxx xxxx xxxx

Potent Cap.: 551 xxxx xxxx xxxx xxxx xxxx 120 91 449 xxxx xxxx xxxx

Move Cap.: 551 xxxx xxxx xxxx xxxx xxxx 117 88 449 xxxx xxxx xxxx

Volume/Cap: 0.03 xxxx xxxx xxxx xxxx 0.30 0.00 0.04 xxxx xxxx xxxx

Level Of Service Module:

2Way95thQ: 0.1 xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Control Del: 11.8 xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

LOS by Move: B \* \* \* \* \* \* \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx 156 xxxx xxxx xxxx xxxx

SharedQueue:xxxxx xxxx xxxx xxxx xxxx xxxx 1.4 xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx xxxx xxxx xxxx xxxx xxxx 39.3 xxxx xxxx xxxx xxxx

Shared LOS: \* \* \* \* \* \* \* E \* \* \* \*

ApproachDel: xxxxxx xxxxxx 39.3 xxxxxx

ApproachLOS: \* \* E \*

Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec): 70 Critical Vol./Cap.(X): 0.495  
 Loss Time (sec): 16 Average Delay (sec/veh): 22.1  
 Optimal Cycle: 53 Level Of Service: C+

Street Name:	Monterey Rd			San Martin Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Protected	Protected	Split Phase	Split Phase		
Rights:	Include	Include	Include	Include		
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10		
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0		
Lanes:	1 0 2 0 1	1 0 1 1 0	0 0 1! 0 0	2 0 1 0 1		

## Volume Module:PM Peak Hour

Base Vol:	5 335	86	158	984	0	17	79	1	100	73	115
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
Initial Bse:	5 335	86	158	984	0	17	79	1	100	73	115
User Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
PHF Adj:	0.95 0.95	0.95	0.95 0.95	0.95	0.95 0.95	0.95	0.95 0.95	0.95	0.95 0.95	0.95	0.95
PHF Volume:	5 353	91	166	1036	0	18	83	1	105	77	121
Reduced Vol:	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0
Reduced Vol:	5 353	91	166	1036	0	18	83	1	105	77	121
PCE Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
MLF Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
FinalVolume:	5 353	91	166	1036	0	18	83	1	105	77	121

## Saturation Flow Module:

Sat/Lane:	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900
Adjustment:	0.92 1.00	0.92	0.92 0.97	0.92	0.92 0.92	0.92	0.92 0.83	1.00	0.92 0.92	0.92	0.92
Lanes:	1.00 2.00	1.00	1.00 2.00	0.00	0.18 0.81	0.01	2.00 1.00	1.00	1.00 1.00	1.00	1.00
Final Sat.:	1750 3800	1750	1750 3700	0	307 1425	18	3150 1900	1750			

## Capacity Analysis Module:

Vol/Sat:	0.00 0.09	0.05	0.10 0.28	0.00	0.06 0.06	0.06	0.03 0.04	0.07
Crit Moves:	****	****	****	****	****	****	****	****
Green Time:	7.0 20.0	20.0	14.0 27.0	0.0	10.0 10.0	10.0	10.0 10.0	10.0
Volume/Cap:	0.03 0.32	0.18	0.48 0.73	0.00	0.41 0.41	0.41	0.23 0.28	0.48
Uniform Del:	28.4 19.7	18.8	24.8 18.3	0.0	27.3 27.3	27.3	26.6 26.8	27.6
IncremntDel:	0.1 0.2	0.2	1.0 1.9	0.0	1.1 1.1	1.1	0.3 0.6	1.5
InitQueuDel:	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0
Delay Adj:	1.00 1.00	1.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00
Delay/Veh:	28.5 19.9	19.0	25.8 20.2	0.0	28.4 28.4	28.4	26.9 27.4	29.1
User DelAdj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
AdjDel/Veh:	28.5 19.9	19.0	25.8 20.2	0.0	28.4 28.4	28.4	26.9 27.4	29.1
LOS by Move:	C B-	B-	C C+	A	C C	C	C C	C
HCM2kAvgQ:	0 3	2	3 10	0	3 3	3	1 2	3

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Monterey Rd / Project Dwy S

Average Delay (sec/veh): 0.0 Worst Case Level Of Service: C[ 16.8]

Street Name: Monterey Rd Project Dwy S

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include

Lanes: 0 0 2 0 0 0 0 1 1 0 0 0 0 0 1 0 0 0 0 0 0

Volume Module: PM Peak Hour

Base Vol: 0 449 0 0 1087 3 0 0 1 0 0 0 0 0 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 0 449 0 0 1087 3 0 0 1 0 0 0 0 0 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Volume: 0 488 0 0 1182 3 0 0 1 0 0 0 0 0 0 0 0 0

Reduct Vol: 0

FinalVolume: 0 488 0 0 1182 3 0 0 1 0 0 0 0 0 0 0 0 0 0 0

Critical Gap Module:

Critical Gp:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 8.4 xxxx xxxx xxxx

FollowUpTim:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 4.1 xxxx xxxx xxxx

Capacity Module:

Cnflct Vol: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 592 xxxx xxxx xxxx

Potent Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 306 xxxx xxxx xxxx

Move Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 306 xxxx xxxx xxxx

Total Cap: xxxx xxxx xxxx xxxx xxxx 134 136 xxxx 368 237 xxxx

Volume/Cap: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.00 xxxx xxxx xxxx

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx 16.8 xxxx xxxx xxxx

LOS by Move: \* \* \* \* \* \* \* \* \* \* C \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx

SharedQueue:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxx xxxxxx 16.8 xxxxxx

ApproachLOS: \* \* C \*

Note: Queue reported is the number of cars per lane.

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## Appendix E

Intersection

Level of Service

Calculations

Cumulative Without Project

Conditions

Level Of Service Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

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Intersection #1 Monterey Rd / California Ave

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Average Delay (sec/veh): 10.2 Worst Case Level Of Service: F[205.0]

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Street Name:	Monterey Rd	California Ave		
Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	1 0 2 0 1	0 0 1! 0 0	0 0 0 0 0

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Volume Module:AM Peak Hour

Base Vol:	20 2332	0 4 198	20 114 0 19	0 0 0 0
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	20 2332	0 4 198	20 114 0 19	0 0 0 0
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	0.93 0.93 0.93	0.93 0.93 0.93	0.93 0.93 0.93	0.93 0.93 0.93
PHF Volume:	22 2508	0 4 213	22 123 0 20	0 0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0 0	0 0 0 0
FinalVolume:	22 2508	0 4 213	22 123 0 20	0 0 0 0

---

Critical Gap Module:

Critical Gp:	4.1 xxxx xxxx	4.1 xxxx xxxx	6.8 6.5	6.9 xxxx xxxx xxxx
FollowUpTim:	2.2 xxxx xxxx	2.2 xxxx xxxx	3.5 4.0	3.3 xxxx xxxx xxxx

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Capacity Module:

Cnflct Vol:	234 xxxx xxxx	2508 xxxx xxxx	1518 2772	106 xxxx xxxx xxxx
Potent Cap.:	1330 xxxx xxxx	178 xxxx xxxx	110 19	927 xxxx xxxx xxxx
Move Cap.:	1330 xxxx xxxx	178 xxxx xxxx	106 18	927 xxxx xxxx xxxx
Volume/Cap:	0.02 xxxx xxxx	0.02 xxxx xxxx	1.15 0.00	0.02 xxxx xxxx xxxx

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Level Of Service Module:

2Way95thQ:	0.0 xxxx xxxx	0.1 xxxx xxxx	xxxx xxxx xxxx	xxxx xxxx xxxx
Control Del:	7.8 xxxx xxxx	25.7 xxxx xxxx	xxxx xxxx xxxx	xxxx xxxx xxxx
LOS by Move:	A * * D *	*	*	*
Movement:	LT - LTR - RT			
Shared Cap.:	xxxx xxxx xxxx	xxxx xxxx xxxx	xxxx 122	xxxx xxxx xxxx
SharedQueue:	xxxx xxxx xxxx	xxxx xxxx xxxx	8.8 xxxx xxxx	xxxx xxxx xxxx
Shrd ConDel:	xxxx xxxx xxxx	xxxx xxxx xxxx	205 xxxx xxxx	xxxx xxxx xxxx
Shared LOS:	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	205.0	xxxxxx
ApproachLOS:	*	*	F	*

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Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec): 120 Critical Vol./Cap.(X): 1.178  
 Loss Time (sec): 16 Average Delay (sec/veh): 113.3  
 Optimal Cycle: 180 Level Of Service: F

Street Name:	Monterey Rd			San Martin Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Protected	Protected	Split Phase	Split Phase		
Rights:	Include	Include	Include	Include		
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10		
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0		
Lanes:	1 0 2 0 1	1 0 1 1 0	0 1 0 0 0	2 0 1 0 1		

## Volume Module:AM Peak Hour

Base Vol:	4 1831	89	115	219	0	61	100	0	102	84	386
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	4 1831	89	115	219	0	61	100	0	102	84	386
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84	0.84
PHF Volume:	5 2180	106	137	261	0	73	119	0	121	100	460
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0	0 0 0
Reduced Vol:	5 2180	106	137	261	0	73	119	0	121	100	460
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	5 2180	106	137	261	0	73	119	0	121	100	460

## Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	
Adjustment:	0.92	1.00	0.92	0.92	0.97	0.92	0.95	0.95	0.92	0.83	1.00	0.92
Lanes:	1.00	2.00	1.00	1.00	2.00	0.00	0.38	0.62	0.00	2.00	1.00	1.00
Final Sat.:	1750	3800	1750	1750	3700	0	682	1118	0	3150	1900	1750

## Capacity Analysis Module:

Vol/Sat:	0.00	0.57	0.06	0.08	0.07	0.00	0.11	0.11	0.00	0.04	0.05	0.26
Crit Moves:	****	****	****	****	****	****	****	****	****	****	****	****
Green Time:	7.0	58.4	58.4	8.0	39.1	0.0	10.8	10.8	0.0	26.7	26.7	26.7
Volume/Cap:	0.05	1.18	0.12	1.18	0.22	0.00	1.18	1.18	0.00	0.17	0.24	1.18
Uniform Del:	53.4	30.8	16.8	56.0	29.4	0.0	54.6	54.6	0.0	37.7	38.2	46.6
IncremntDel:	0.2	86.1	0.1	139.1	0.1	0.0	126.5	126	0.0	0.1	0.3	103.7
InitQueuDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	0.00	1.00	1.00	1.00
Delay/Veh:	53.5	117	16.9	195.1	29.5	0.0	181.0	181	0.0	37.8	38.5	150.3
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	53.5	117	16.9	195.1	29.5	0.0	181.0	181	0.0	37.8	38.5	150.3
LOS by Move:	D-	F	B	F	C	A	F	F	A	D+	D+	F
HCM2kAvgQ:	0	62	2	9	3	0	14	14	0	2	3	31

Note: Queue reported is the number of cars per lane.

Level Of Service Computation Report  
2000 HCM Unsignalized Method (Base Volume Alternative)

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Intersection #1 Monterey Rd / California Ave

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Average Delay (sec/veh): 3.2 Worst Case Level Of Service: F[122.9]

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Street Name:	Monterey Rd	California Ave		
Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 0	1 0 2 0 1	0 0 1! 0 0	0 0 0 0 0

---

Volume Module: PM Peak Hour

Base Vol:	20 649 0	14 1444 71	36 0 19	0 0 0
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	20 649 0	14 1444 71	36 0 19	0 0 0
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	0.96 0.96 0.96	0.96 0.96 0.96	0.96 0.96 0.96	0.96 0.96 0.96
PHF Volume:	21 676 0	15 1504 74	38 0 20	0 0 0
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
FinalVolume:	21 676 0	15 1504 74	38 0 20	0 0 0

---

Critical Gap Module:

Critical Gp:	4.1 xxxx xxxx	4.1 xxxx xxxx	6.8 6.5	6.9 xxxx xxxx xxxx
FollowUpTim:	2.2 xxxx xxxx	2.2 xxxx xxxx	3.5 4.0	3.3 xxxx xxxx xxxx

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Capacity Module:

Cnflict Vol:	1578 xxxx xxxx	676 xxxx xxxx	1913 2251	752 xxxx xxxx xxxx
Potent Cap.:	413 xxxx xxxx	911 xxxx xxxx	60 41	353 xxxx xxxx xxxx
Move Cap.:	413 xxxx xxxx	911 xxxx xxxx	57 38	353 xxxx xxxx xxxx
Volume/Cap:	0.05 xxxx xxxx	0.02 xxxx xxxx	0.66 0.00	0.06 xxxx xxxx xxxx

---

Level Of Service Module:

2Way95thQ:	0.2 xxxx xxxx	0.0 xxxx xxxx	xxxx xxxx xxxx	xxxx xxxx xxxx
Control Del:	14.2 xxxx xxxx	9.0 xxxx xxxx	xxxx xxxx xxxx	xxxx xxxx xxxx
LOS by Move:	B * *	A * *	* * *	* * *
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx xxxx xxxx	xxxx xxxx xxxx	xxxx 80	xxxx xxxx xxxx
SharedQueue:	xxxx xxxx xxxx	xxxx xxxx xxxx	3.4 xxxx xxxx xxxx	xxxx xxxx xxxx
Shrd ConDel:	xxxx xxxx xxxx	xxxx xxxx xxxx	123 xxxx xxxx xxxx	xxxx xxxx xxxx
Shared LOS:	*	*	*	*
ApproachDel:	xxxxxx	xxxxxx	122.9	xxxxxx
ApproachLOS:	*	*	F	*

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Note: Queue reported is the number of cars per lane.

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Level Of Service Computation Report  
2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

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Cycle (sec): 70 Critical Vol./Cap.(X): 0.588  
Loss Time (sec): 16 Average Delay (sec/veh): 25.5  
Optimal Cycle: 53 Level Of Service: C

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Street Name:	Monterey Rd			San Martin Ave											
Approach:	North Bound		South Bound		East Bound		West Bound								
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R
Control:	Protected			Protected			Split Phase			Split Phase					
Rights:	Include			Include			Include			Include					
Min. Green:	7	10	10	7	10	10	10	10	10	10	10	10	10	10	
Y+R:	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	4.0	
Lanes:	1	0	2	0	1	1	0	1	0	0	0	2	0	1	0

Volume Module: PM Peak Hour

Base Vol:	6	406	105	158	1195	0	19	88	1	111	81	121
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	6	406	105	158	1195	0	19	88	1	111	81	121
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95	0.95
PHF Volume:	6	427	111	166	1258	0	20	93	1	117	85	127
Reducut Vol:	0	0	0	0	0	0	0	0	0	0	0	0
Reduced Vol:	6	427	111	166	1258	0	20	93	1	117	85	127
PCE Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
MLF Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
FinalVolume:	6	427	111	166	1258	0	20	93	1	117	85	127

Saturation Flow Module:

Sat/Lane:	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Adjustment:	0.92	1.00	0.92	0.92	0.97	0.92	0.92	0.92	0.92	0.83	1.00	0.92
Lanes:	1.00	2.00	1.00	1.00	2.00	0.00	0.18	0.81	0.01	2.00	1.00	1.00
Final Sat.:	1750	3800	1750	1750	3700	0	308	1426	16	3150	1900	1750

Capacity Analysis Module:

Vol/Sat:	0.00	0.11	0.06	0.10	0.34	0.00	0.06	0.06	0.06	0.04	0.04	0.07
Crit Moves:	****			****		****		****		****		****
Green Time:	7.0	20.0	20.0	14.0	27.0	0.0	10.0	10.0	10.0	10.0	10.0	10.0
Volume/Cap:	0.04	0.39	0.22	0.48	0.88	0.00	0.45	0.45	0.45	0.26	0.31	0.51
Uniform Del:	28.5	20.1	19.1	24.8	20.0	0.0	27.5	27.5	27.5	26.7	26.9	27.7
IncremntDel:	0.1	0.2	0.2	1.0	6.7	0.0	1.3	1.3	1.3	0.3	0.7	1.7
InitQueuDel:	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0	0.0
Delay Adj:	1.00	1.00	1.00	1.00	1.00	0.00	1.00	1.00	1.00	1.00	1.00	1.00
Delay/Veh:	28.5	20.4	19.3	25.8	26.8	0.0	28.8	28.8	28.8	27.0	27.6	29.5
User DelAdj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
AdjDel/Veh:	28.5	20.4	19.3	25.8	26.8	0.0	28.8	28.8	28.8	27.0	27.6	29.5
LOS by Move:	C	C+	B-	C	C	A	C	C	C	C	C	C
HCM2kAvgQ:	0	4	2	3	14	0	3	3	3	2	2	3

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Note: Queue reported is the number of cars per lane.

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Note: Queue reported is the number of cars per lane.

## Appendix F

Intersection

Level of Service

Calculations

Cumulative Plus Project

Conditions

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Monterey Rd / California Ave

Average Delay (sec/veh): 10.2 Worst Case Level Of Service: F[205.8]

Street Name: Monterey Rd California Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 1 0 2 0 1 0 0 1! 0 0 0 0 0 0 0 0 0 0 0

Volume Module: AM Peak Hour

Base Vol: 20 2332 0 4 199 20 114 0 19 0 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 20 2332 0 4 199 20 114 0 19 0 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93 0.93

PHF Volume: 22 2508 0 4 214 22 123 0 20 0 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 22 2508 0 4 214 22 123 0 20 0 0 0 0 0

Critical Gap Module:

Critical Gp: 4.1 xxxx xxxx 4.1 xxxx xxxx 6.8 6.5 6.9 xxxx xxxx xxxx

FollowUpTim: 2.2 xxxx xxxx 2.2 xxxx xxxx 3.5 4.0 3.3 xxxx xxxx xxxx

Capacity Module:

Cnflct Vol: 235 xxxx xxxx 2508 xxxx xxxx 1519 2773 107 xxxx xxxx xxxx

Potent Cap.: 1329 xxxx xxxx 178 xxxx xxxx 110 19 926 xxxx xxxx xxxx

Move Cap.: 1329 xxxx xxxx 178 xxxx xxxx 106 18 926 xxxx xxxx xxxx

Volume/Cap: 0.02 xxxx xxxx 0.02 xxxx xxxx 1.15 0.00 0.02 xxxx xxxx xxxx

Level Of Service Module:

2Way95thQ: 0.0 xxxx xxxx 0.1 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Control Del: 7.8 xxxx xxxx 25.7 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

LOS by Move: A \* \* D \* \* \* \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx 122 xxxx xxxx xxxx xxxx

SharedQueue:xxxxx xxxx xxxx xxxx xxxx xxxx 8.8 xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx xxxx xxxx xxxx xxxx xxxx 206 xxxx xxxx xxxx xxxx

Shared LOS: \* \* \* \* \* \* \* F \* \* \* \* \*

ApproachDel: xxxxxx xxxxxx 205.8 xxxxxx

ApproachLOS: \* \* F \*

Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec): 120 Critical Vol./Cap.(X): 1.181  
 Loss Time (sec): 16 Average Delay (sec/veh): 114.5  
 Optimal Cycle: 180 Level Of Service: F

Street Name:	Monterey Rd			San Martin Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Protected	Protected	Split Phase	Split Phase		
Rights:	Include	Include	Include	Include		
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10		
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0		
Lanes:	1 0 2 0 1	1 0 1 1 0	0 1 0 0 0	2 0 1 0 1		

## Volume Module:AM Peak Hour

Base Vol:	4 1831	89	119	219	0	61	100	0	102	84	386
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
Initial Bse:	4 1831	89	119	219	0	61	100	0	102	84	386
User Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
PHF Adj:	0.84 0.84	0.84	0.84 0.84	0.84	0.84 0.84	0.84	0.84 0.84	0.84	0.84 0.84	0.84	0.84
PHF Volume:	5 2180	106	142	261	0	73	119	0	121	100	460
Reduct Vol:	0 0 0	0	0 0	0	0 0	0	0 0	0	0 0	0	0
Reduced Vol:	5 2180	106	142	261	0	73	119	0	121	100	460
PCE Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
MLF Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00
FinalVolume:	5 2180	106	142	261	0	73	119	0	121	100	460

## Saturation Flow Module:

Sat/Lane:	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900 1900	1900	1900
Adjustment:	0.92 1.00	0.92	0.92 0.97	0.92	0.95 0.95	0.92	0.83 1.00	0.92	0.92 1.00	0.92	0.92
Lanes:	1.00 2.00	1.00	1.00 2.00	0.00	0.38 0.62	0.00	2.00 1.00	1.00	1.00 1.00	1.00	1.00
Final Sat.:	1750 3800	1750	1750 3700	0	682 1118	0	3150 1900	1750			

## Capacity Analysis Module:

Vol/Sat:	0.00 0.57	0.06	0.08 0.07	0.00	0.11 0.11	0.00	0.04 0.05	0.26			
Crit Moves:	****	****	****		****	****	****	****			
Green Time:	7.0 58.3	58.3	8.2 39.1	0.0	10.8 10.8	0.0	26.7 26.7	26.7			
Volume/Cap:	0.05 1.18	0.12	1.18 0.22	0.00	1.18 1.18	0.00	0.17 0.24	1.18			
Uniform Del:	53.4 30.9	16.9	55.9 29.3	0.0	54.6 54.6	0.0	37.7 38.3	46.7			
IncremntDel:	0.2 87.4	0.1	138.9 0.1	0.0	127.7 128	0.0	0.1 0.3	104.9			
InitQueuDel:	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0			
Delay Adj:	1.00 1.00	1.00	1.00 1.00	0.00	1.00 1.00	0.00	1.00 1.00	1.00			
Delay/Veh:	53.5 118	17.0	194.8 29.4	0.0	182.3 182	0.0	37.9 38.6	151.6			
User DelAdj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00			
AdjDel/Veh:	53.5 118	17.0	194.8 29.4	0.0	182.3 182	0.0	37.9 38.6	151.6			
LOS by Move:	D- F	B F	C A	F F	A A	D+ D+	D+ D+	F			
HCM2kAvgQ:	0 62	2 9	3 0	14 14	0 0	2 2	3 3	31			

Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Monterey Rd / Project Dwy S

Average Delay (sec/veh): 0.0 Worst Case Level Of Service: A[ 9.9]

Street Name: Monterey Rd Project Dwy S

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include

Lanes: 0 0 2 0 0 0 0 1 1 0 0 0 0 0 1 0 0 0 0 0 0

Volume Module:AM Peak Hour

Base Vol: 0 2352 0 0 195 1 0 0 4 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 0 2352 0 0 195 1 0 0 4 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Volume: 0 2557 0 0 212 1 0 0 4 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 0 2557 0 0 212 1 0 0 4 0 0 0

Critical Gap Module:

Critical Gp:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 8.4 xxxx xxxx xxxx

FollowUpTim:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 4.1 xxxx xxxx xxxx

Capacity Module:

Cnflct Vol: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 107 xxxx xxxx xxxx

Potent Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 736 xxxx xxxx xxxx

Move Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 736 xxxx xxxx xxxx

Total Cap: xxxx xxxx xxxx xxxx xxxx xxxx 116 17 xxxx 26 52 xxxx

Volume/Cap: xxxx xxxx xxxx xxxx xxxx xxxx 0.01 xxxx xxxx xxxx

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx 9.9 xxxx xxxx xxxx

LOS by Move: \* \* \* \* \* \* \* \* A \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

SharedQueue:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxx xxxxxxxx 9.9 xxxxxxxx

ApproachLOS: \* \* A \*

Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1 Monterey Rd / California Ave

Average Delay (sec/veh): 3.2 Worst Case Level Of Service: F[124.1]

Street Name: Monterey Rd California Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include

Lanes: 1 0 2 0 0 1 0 2 0 1 0 0 1! 0 0 0 0 0 0 0 0 0 0

Volume Module: PM Peak Hour

Base Vol: 20 649 0 14 1447 71 36 0 19 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 20 649 0 14 1447 71 36 0 19 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96 0.96

PHF Volume: 21 676 0 15 1507 74 38 0 20 0 0 0 0

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 21 676 0 15 1507 74 38 0 20 0 0 0 0

Critical Gap Module:

Critical Gp: 4.1 xxxx xxxx 4.1 xxxx xxxx 6.8 6.5 6.9 xxxx xxxx xxxx

FollowUpTim: 2.2 xxxx xxxx 2.2 xxxx xxxx 3.5 4.0 3.3 xxxx xxxx xxxx

Capacity Module:

Cnflct Vol: 1581 xxxx xxxx 676 xxxx xxxx 1916 2254 754 xxxx xxxx xxxx

Potent Cap.: 412 xxxx xxxx 911 xxxx xxxx 59 41 352 xxxx xxxx xxxx

Move Cap.: 412 xxxx xxxx 911 xxxx xxxx 56 38 352 xxxx xxxx xxxx

Volume/Cap: 0.05 xxxx xxxx 0.02 xxxx xxxx 0.66 0.00 0.06 xxxx xxxx xxxx

Level Of Service Module:

2Way95thQ: 0.2 xxxx xxxx 0.0 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Control Del: 14.2 xxxx xxxx 9.0 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

LOS by Move: B \* \* A \* \* \* \* \* \* \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx 79 xxxx xxxx xxxx xxxx

SharedQueue:xxxxx xxxx xxxx xxxx xxxx xxxx 3.5 xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx xxxx xxxx xxxx xxxx xxxx 124 xxxx xxxx xxxx xxxx

Shared LOS: \* \* \* \* \* \* \* F \* \* \* \* \*

ApproachDel: xxxxxx xxxxxx 124.1 xxxxxx

ApproachLOS: \* \* F \*

Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #2 Monterey Rd / San Martin Ave

Cycle (sec): 70 Critical Vol./Cap.(X): 0.588  
 Loss Time (sec): 16 Average Delay (sec/veh): 25.5  
 Optimal Cycle: 53 Level Of Service: C

Street Name:	Monterey Rd			San Martin Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Protected	Protected	Split Phase	Split Phase		
Rights:	Include	Include	Include	Include		
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10		
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0		
Lanes:	1 0 2 0 1	1 0 1 1 0	0 0 1! 0 0	2 0 1 0 1		

## Volume Module:PM Peak Hour

Base Vol:	6 406 105	159 1195	0	19 88	1	111 81	121
Growth Adj:	1.00 1.00 1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
Initial Bse:	6 406 105	159 1195	0	19 88	1	111 81	121
User Adj:	1.00 1.00 1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
PHF Adj:	0.95 0.95 0.95	0.95 0.95	0.95	0.95 0.95	0.95	0.95 0.95	0.95
PHF Volume:	6 427 111	167 1258	0	20 93	1	117 85	127
Reduct Vol:	0 0 0	0 0	0	0 0	0	0 0	0
Reduced Vol:	6 427 111	167 1258	0	20 93	1	117 85	127
PCE Adj:	1.00 1.00 1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
MLF Adj:	1.00 1.00 1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
FinalVolume:	6 427 111	167 1258	0	20 93	1	117 85	127

## Saturation Flow Module:

Sat/Lane:	1900 1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900
Adjustment:	0.92 1.00 0.92	0.92 0.97	0.92 0.92	0.92 0.92	0.92 0.92	0.92 0.83	1.00 0.92
Lanes:	1.00 2.00 1.00	1.00 2.00	0.00 0.00	0.18 0.81	0.01 0.01	2.00 1.00	1.00 1.00
Final Sat.:	1750 3800 1750	1750 3700	0 308	1426 16	3150 3150	1900 1900	1750 1750

## Capacity Analysis Module:

Vol/Sat:	0.00 0.11 0.06	0.10 0.34	0.00	0.06 0.06	0.06	0.04 0.04	0.04 0.07
Crit Moves:	****	****	****	****	****	****	****
Green Time:	7.0 20.0 20.0	14.0 27.0	0.0	10.0 10.0	10.0	10.0 10.0	10.0
Volume/Cap:	0.04 0.39 0.22	0.48 0.88	0.00	0.45 0.45	0.45	0.26 0.31	0.51
Uniform Del:	28.5 20.1 19.1	24.8 20.0	0.0	27.5 27.5	27.5	26.7 26.9	27.7
IncremntDel:	0.1 0.2 0.2	1.0 6.7	0.0	1.3 1.3	1.3	0.3 0.7	1.7
InitQueuDel:	0.0 0.0 0.0	0.0 0.0	0.0	0.0 0.0	0.0	0.0 0.0	0.0
Delay Adj:	1.00 1.00 1.00	1.00 1.00	0.00	1.00 1.00	1.00	1.00 1.00	1.00
Delay/Veh:	28.5 20.4 19.3	25.8 26.8	0.0	28.8 28.8	28.8	27.0 27.6	29.5
User DelAdj:	1.00 1.00 1.00	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00
AdjDel/Veh:	28.5 20.4 19.3	25.8 26.8	0.0	28.8 28.8	28.8	27.0 27.6	29.5
LOS by Move:	C C+ B-	C C A	C C	C C C	C C C		
HCM2kAvgQ:	0 4 2	3 15 0	3 3	3 3 2	2 2 3		

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #3 Monterey Rd / Project Dwy S

Average Delay (sec/veh): 0.0 Worst Case Level Of Service: C [ 23.1 ]

Street Name: Monterey Rd Project Dwy S

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Uncontrolled Uncontrolled Stop Sign Stop Sign

Rights: Include Include Include Include

Lanes: 0 0 2 0 0 0 0 1 1 0 0 0 0 0 1 0 0 0 0 0 0

Volume Module: PM Peak Hour

Base Vol: 0 669 0 0 1510 3 0 0 1 0 0 0 0 0 0 0 0 0

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 0 669 0 0 1510 3 0 0 1 0 0 0 0 0 0 0 0 0

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92 0.92

PHF Volume: 0 727 0 0 1641 3 0 0 1 0 0 0 0 0 0 0 0 0

Reduct Vol: 0

FinalVolume: 0 727 0 0 1641 3 0 0 1 0 0 0 0 0 0 0 0 0

Critical Gap Module:

Critical Gp:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 8.4 xxxx xxxx xxxx xxxx

FollowUpTim:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 4.1 xxxx xxxx xxxx xxxx

Capacity Module:

Cnflct Vol: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 822 xxxx xxxx xxxx

Potent Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 200 xxxx xxxx xxxx

Move Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 200 xxxx xxxx xxxx

Total Cap: xxxx xxxx xxxx xxxx xxxx 65 70 xxxx 252 143 xxxx

Volume/Cap: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.01 xxxx xxxx xxxx

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx 23.1 xxxx xxxx xxxx

LOS by Move: \* \* \* \* \* \* \* \* \* C \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

SharedQueue:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* \* \* \* \* \* \* \* \* \* \* \*

ApproachDel: xxxxxx xxxxxxxx 23.1 xxxxxxxx

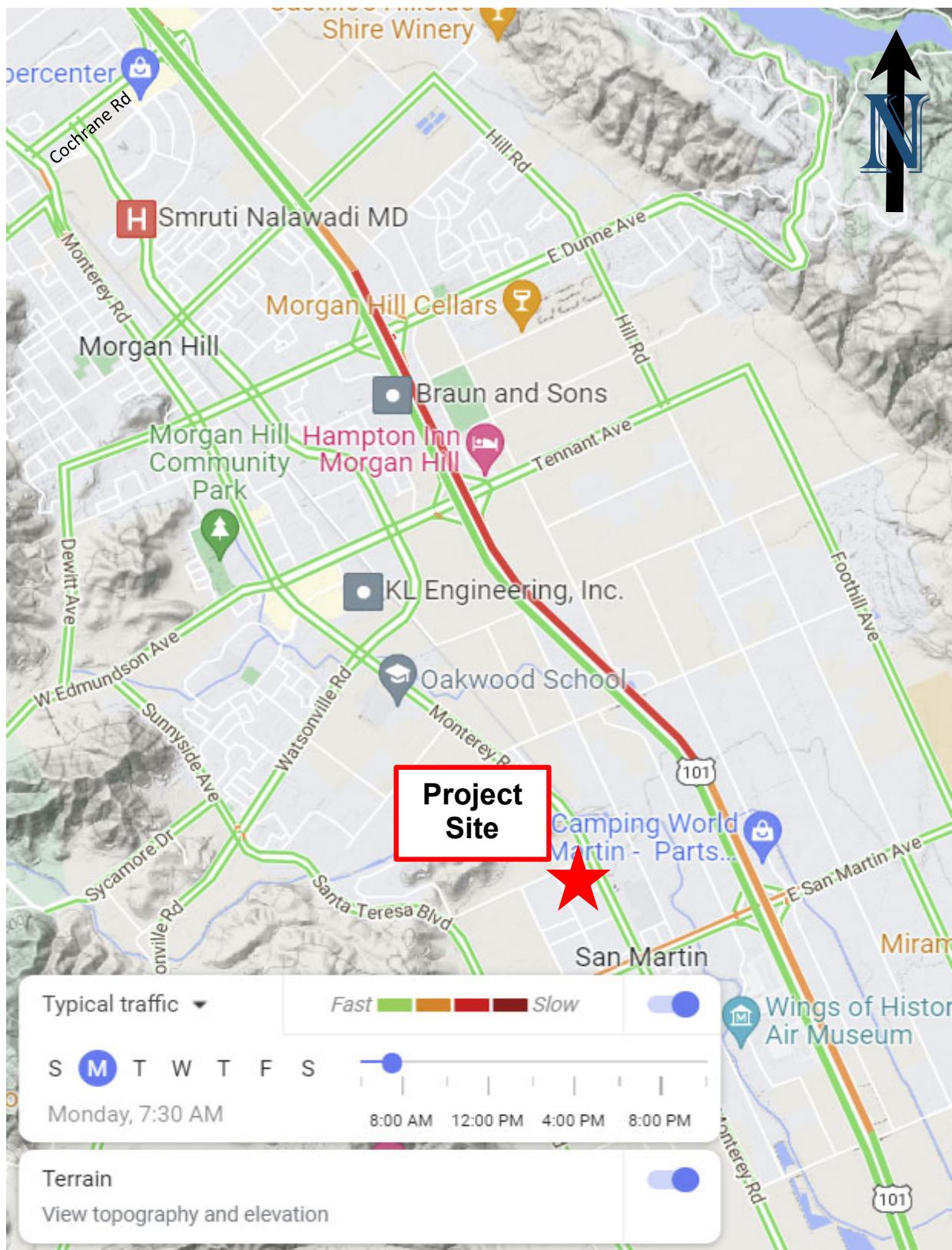
ApproachLOS: \* \* C \*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

## Appendix G

AM Vehicle  
Delay Documentation



Basemap Source: Google Maps, 2021.

## Appendix H

### Sight Distance Calculations

**Appendix H**  
**Sight Distance Evaluation**  
**Caltrans Method (Highway Design Manual)**

Project Name:	San Martin NWC #5
Location:	California Ave, Eastbound
Date:	10/6/20
Prepared by:	Jeff Waller

**A. Data:**

Vehicle Type:	Combination Truck
Type of Maneuver:	Left Turn
Number of Lanes Crossed	
When Making Maneuver:	2 lanes
Median Width:	12 feet (including left turn lanes)
Travel Speed:	
Northbound:	55 mph
Southbound:	55 mph
Grade:	
Stopped Approach:	0.00%

**B. Corner Sight Distance**

Sight Distance Standard:	
To/From North:	1,043 feet
To/From South:	1,043 feet

Available Sight Distance:

To/From North:	850 feet	Deficient
To/From South:	1,000 feet	Deficient

**C. Stopping Sight Distance**

Sight Distance Standard:	
To/From North:	500 feet
To/From South:	500 feet

Available Sight Distance:

To/From North:	850 feet	Adequate
To/From South:	1,000 feet	Adequate

**D. Overall Conclusion**

To/From North:	Adequate
To/From South:	Adequate

**Appendix H**  
**Sight Distance Evaluation**  
**Caltrans Method (Highway Design Manual)**

Project Name: San Martin NWC #5  
Location: Monterey Road Driveway, Eastbound  
Date: 10/6/20  
Prepared by: Jeff Waller

**A. Data:**

		<u>Applicable?</u>
Location Type:	Private Driveway	Corner No
Vehicle Type:	Combination Truck	Stopping Yes
Type of Maneuver:	Left Turn	
Number of Lanes Crossed		
When Making Maneuver:	1 lanes	
Median Width:	0 feet (including left turn lanes)	
Travel Speed:		
Northbound:	55 mph	
Southbound:	55 mph	
Grade:		
Stopped Approach:	0.00%	

**B. Corner Sight Distance**

Does not apply to  
Private Intersections  
and Driveways

**C. Stopping Sight Distance**

Sight Distance Standard:	
To/From North:	500 feet
To/From South:	500 feet

Available Sight Distance:

To/From North:	490 feet	Deficient
To/From South:	800 feet	Adequate

**D. Overall Conclusion**

To/From North:	Deficient
To/From South:	Adequate

## Appendix I

### Vehicle Queue

#### Calculations

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Southbound Left Turn  
 Analysis Period: AM Peak Hour

#### Existing Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	116
Volume (vphpl)	116
Average Queue (veh/ln)	3.87
Percentile	0.95
95% Queue (veh/ln)	7

#### Existing Plus Project Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	120
Volume (vphpl)	120
Average Queue (veh/ln)	4
Percentile	0.95
95% Queue (veh/ln)	8

Cumulative Probability	Queued Vehicles
0.021	0
0.102	1
0.258	2
0.459	3
0.654	4
0.805	5
0.902	6
0.956	7
0.982	8
0.993	9
0.998	10
0.999	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.018	0
0.092	1
0.238	2
0.433	3
0.629	4
0.785	5
0.889	6
0.949	7
0.979	8
0.992	9
0.997	10
0.999	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Southbound Left Turn  
 Analysis Period: PM Peak Hour

#### Existing Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	157
Volume (vphpl)	157
Average Queue (veh/ln)	3.05
Percentile	0.95
95% Queue (veh/ln)	6

#### Existing Plus Project Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	158
Volume (vphpl)	158
Average Queue (veh/ln)	3.07
Percentile	0.95
95% Queue (veh/ln)	6

Cumulative Probability	Queued Vehicles
0.047	0
0.192	1
0.412	2
0.636	3
0.807	4
0.911	5
0.964	6
0.987	7
0.996	8
0.999	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.046	0
0.189	1
0.408	2
0.632	3
0.803	4
0.909	5
0.963	6
0.987	7
0.996	8
0.999	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Southbound Left Turn  
 Analysis Period: AM Peak Hour

#### Cumulative Without Project Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	115
Volume (vphpl)	115
Average Queue (veh/ln)	3.83
Percentile	0.95
95% Queue (veh/ln)	7

#### Cumulative Plus Project Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	119
Volume (vphpl)	119
Average Queue (veh/ln)	3.97
Percentile	0.95
95% Queue (veh/ln)	7

Cumulative Probability	Queued Vehicles
0.022	0
0.105	1
0.264	2
0.467	3
0.662	4
0.811	5
0.906	6
0.958	7
0.983	8
0.994	9
0.998	10
0.999	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.019	0
0.094	1
0.243	2
0.439	3
0.635	4
0.790	5
0.892	6
0.951	7
0.980	8
0.992	9
0.997	10
0.999	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Southbound Left Turn  
 Analysis Period: PM Peak Hour

#### Cumulative Without Project Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	158
Volume (vphpl)	158
Average Queue (veh/ln)	3.07
Percentile	0.95
95% Queue (veh/ln)	6

#### Cumulative Plus Project Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	159
Volume (vphpl)	159
Average Queue (veh/ln)	3.09
Percentile	0.95
95% Queue (veh/ln)	6

Cumulative Probability	Queued Vehicles
0.046	0
0.189	1
0.408	2
0.632	3
0.803	4
0.909	5
0.963	6
0.987	7
0.996	8
0.999	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.046	0
0.186	1
0.403	2
0.627	3
0.800	4
0.907	5
0.962	6
0.986	7
0.995	8
0.999	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Westbound Right Turn  
 Analysis Period: AM Peak Hour

#### Existing Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	325
Volume (vphpl)	325
Average Queue (veh/ln)	10.83
Percentile	0.95
95% Queue (veh/ln)	16

#### Existing Plus Project Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	325
Volume (vphpl)	325
Average Queue (veh/ln)	10.83
Percentile	0.95
95% Queue (veh/ln)	16

Cumulative Probability	Queued Vehicles
0.000	0
0.000	1
0.001	2
0.006	3
0.017	4
0.042	5
0.086	6
0.155	7
0.247	8
0.359	9
0.480	10
0.600	11
0.707	12
0.797	13
0.866	14
0.916	15
0.950	16
0.972	17
0.985	18
0.992	19
0.996	20
0.998	21
0.999	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.000	0
0.000	1
0.001	2
0.006	3
0.017	4
0.042	5
0.086	6
0.155	7
0.247	8
0.359	9
0.480	10
0.600	11
0.707	12
0.797	13
0.866	14
0.916	15
0.950	16
0.972	17
0.985	18
0.992	19
0.996	20
0.998	21
0.999	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Westbound Right Turn  
 Analysis Period: PM Peak Hour

#### Existing Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	115
Volume (vphpl)	115
Average Queue (veh/ln)	2.24
Percentile	0.95
95% Queue (veh/ln)	5

#### Existing Plus Project Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	115
Volume (vphpl)	115
Average Queue (veh/ln)	2.24
Percentile	0.95
95% Queue (veh/ln)	5

Cumulative Probability	Queued Vehicles
0.106	0
0.345	1
0.612	2
0.811	3
0.923	4
0.973	5
0.992	6
0.998	7
0.999	8
1.000	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.106	0
0.345	1
0.612	2
0.811	3
0.923	4
0.973	5
0.992	6
0.998	7
0.999	8
1.000	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Westbound Right Turn  
 Analysis Period: AM Peak Hour

#### Cumulative Without Project Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	386
Volume (vphpl)	386
Average Queue (veh/ln)	12.87
Percentile	0.95
95% Queue (veh/ln)	19

#### Cumulative Plus Project Conditions

Cycle Length (sec)	120
Lanes	1
Volume (vph)	386
Volume (vphpl)	386
Average Queue (veh/ln)	12.87
Percentile	0.95
95% Queue (veh/ln)	19

Cumulative Probability	Queued Vehicles
0.000	0
0.000	1
0.000	2
0.001	3
0.004	4
0.012	5
0.028	6
0.058	7
0.106	8
0.175	9
0.263	10
0.366	11
0.477	12
0.587	13
0.688	14
0.775	15
0.845	16
0.897	17
0.935	18
0.961	19
0.977	20
0.987	21
0.993	22
0.996	23
0.998	24
0.999	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.000	0
0.000	1
0.000	2
0.001	3
0.004	4
0.012	5
0.028	6
0.058	7
0.106	8
0.175	9
0.263	10
0.366	11
0.477	12
0.587	13
0.688	14
0.775	15
0.845	16
0.897	17
0.935	18
0.961	19
0.977	20
0.987	21
0.993	22
0.996	23
0.998	24
0.999	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

### Vehicle Queue Estimates - Turning Movements

Intersection: 2. Monterey Road / San Martin Avenue  
 Movement: Westbound Right Turn  
 Analysis Period: PM Peak Hour

#### Cumulative Without Project Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	121
Volume (vphpl)	121
Average Queue (veh/ln)	2.35
Percentile	0.95
95% Queue (veh/ln)	5

#### Cumulative Plus Project Conditions

Cycle Length (sec)	70
Lanes	1
Volume (vph)	121
Volume (vphpl)	121
Average Queue (veh/ln)	2.35
Percentile	0.95
95% Queue (veh/ln)	5

Cumulative Probability	Queued Vehicles
0.095	0
0.319	1
0.583	2
0.789	3
0.910	4
0.967	5
0.990	6
0.997	7
0.999	8
1.000	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35

Cumulative Probability	Queued Vehicles
0.095	0
0.319	1
0.583	2
0.789	3
0.910	4
0.967	5
0.990	6
0.997	7
0.999	8
1.000	9
1.000	10
1.000	11
1.000	12
1.000	13
1.000	14
1.000	15
1.000	16
1.000	17
1.000	18
1.000	19
1.000	20
1.000	21
1.000	22
1.000	23
1.000	24
1.000	25
1.000	26
1.000	27
1.000	28
1.000	29
1.000	30
1.000	31
1.000	32
1.000	33
1.000	34
1.000	35



October 4, 2023

Amanda Musy-Verdel  
Hanna-Brunetti  
7651 Egleberry Street  
Gilroy, CA 95020

Re: Honey Bucket Facility, San Martin, CA – Additional County-Requested Analysis

Dear Amanda,

Per your request, this letter provides professional transportation planning services for the proposed storage of portable toilets at the corner of Monterey Road and California Avenue in San Martin, Santa Clara County, California (Honey Bucket facility). At a web conference for the project on September 18, 2023, Santa Clara County Roads and Airports staff requested an analysis of two intersections associated with the left turn restriping and movement prohibition at the intersection of Monterey Road and Burbank Avenue in San Martin. This improvement is required due to striping improvements at the Monterey Road / San Martin Avenue intersection recommended in the prior Honey Bucket facility traffic study – *NWC #5 Facility Traffic Impact Analysis*, TL Engineering, April 11, 2023 (TL Engineering report). **Appendix A** contains the restriping design. The following scope of work for this letter was provided by County Road and Airports staff.

The study area is depicted in **Exhibit 1**.

#### A. Existing and Project Traffic

The following intersections were analyzed in this study:

1. Monterey Road / Burbank Avenue;
2. Sewell Avenue / San Martin Avenue; and
3. Monterey Road / San Martin Avenue.

New traffic volume counts were performed at the Monterey / Burbank and Sewell / San Martin intersections on Wednesday, September 27, 2023 between 7:00 – 9:00 AM and 4:00 – 6:00 PM, including cars, trucks, buses, pedestrians, and bicyclists. From these counts, the peak one-hour AM and PM peak hour volumes were identified. **Appendix B** contains these intersection counts.

The traffic volumes at the Monterey / San Martin intersection were taken from the previous traffic analysis for the project prepared in April 2023.

The Existing condition volumes at the study intersections are depicted in **Exhibit 2**.

The project trip generation and assignment from the earlier TL Engineering report were assigned to the Monterey / Burbank and Sewel / San Martin intersections based on the project distribution in said report. The project trips at the Monterey / San Martin intersection are again taken from the earlier TL Engineering report.

The project trip assignment at all three intersections is depicted in **Exhibit 3**.

The project trip assignment at each of the three study intersections were added to the Existing volumes to create the Existing Plus Project volumes depicted in **Exhibit 4**.

#### **B. Traffic Reassignment with Movement Closure**

The earlier TL Engineering report concluded that the storage length for the southbound Monterey Road left turn lane at San Martin Avenue required lengthening. To provide this lengthening, the existing northbound Monterey Road left turn lane at Burbank Avenue will need to be closed and this movement prohibited. Such as closure and prohibition will require existing traffic to reroute onto northbound Sewell Avenue to reach Burbank Avenue. **Exhibit 5** depicts the anticipated traffic rerouting that will occur with the proposed closure.

Sewell and Burbank Avenues are primarily residential streets, although two commercial businesses are located adjacent to Burbank Avenue – Rocca's Market and A-1 Saw and Lawnmower. In total, 5 AM and 14 PM vehicles would need to reroute with the closure. All these vehicles were reassigned to turn at the northbound Monterey Road left turn lane at San Martin Avenue and use Sewell Avenue to reach Burbank Avenue.

**Exhibit 6** depicts the Existing Plus Project volumes with the closure, including the rerouted traffic.

#### **C. Analysis With and Without Closure**

**Exhibit 7** summarizes operations at the study intersections with and without the closure and reassignment. The level of service calculations can be found in **Appendix C**.

Operations with the closure include the lengthened southbound Monterey left turn lane at San Martin Avenue and the associated removal and traffic reassignment for the northbound Monterey left turn lane at Burbank Avenue. Operations without the closure include the existing intersection geometry at the Monterey Road / Burbank Avenue intersection.

Operations at the Monterey / Burbank and Sewell / San Martin intersections will continue to operate within acceptable conditions with the proposed lane closure and reassignment. The Monterey / San Martin intersection will also retain its existing levels of service with the closure and reassignment. The latter intersection will also retain the same delay during the PM peak hour as without the closure, as well as reduce its overall delay during the AM peak hour with the closure.

#### **D. Assessment of Potential Adverse Effects With Closure**

The beneficial operations of the study intersections with the proposed closure and traffic reassignment indicates that no additional intersection improvements will be required. in combination with the northbound Monterey left turn lane closure.

The September 2023 intersection counts found no pedestrian or bicycle traffic at the Sewell Avenue / San Martin Avenue intersection. The beneficial operations with the reassignment, as well as the actual reassigned traffic, will thus not affect pedestrian or bicycle circulation at this intersection. The operations at the Monterey Road / San Marin Avenue intersection will also not affect pedestrians or bicyclists.

At the Monterey Road / Burbank Avenue, the closure and removal of the northbound Monterey left turn movement will remove one of the potential vehicle/pedestrian and vehicle/bicycle conflict points at that intersection. While there are no pedestrians or bicyclists crossing Burbank Avenue during the AM peak hour, 4 pedestrians cross during the PM peak hour. The closure and removal of the northbound Monterey left turn lane will thus improve the safety of both pedestrian and bicycle activity at this intersection.

There are no bus stops on either Burbank or San Martin Avenues in the vicinity of Monterey Road. Bus stops are instead located on Monterey Road north and south of San Martin Avenue. A Caltrain rail station is also located on Monterey Road north of San Martin Avenue with vehicle access solely from Monterey Road. The proposed closure of the northbound Monterey left turn lane at Burbank Avenue will not affect access to either bus stop or the Caltrain station.

#### **E. Summary and Conclusion**

In summary, the proposed closure and relocation of the northbound Monterey Road left turn lane at Burbank Avenue will not have any negative effects on any of the study intersections nor on area transit routes. The closure will have a benefit to pedestrian and bicycle traffic crossing Burbank Avenue by removing one of the potential vehicle/pedestrian and vehicle/bicycle conflicts at this intersection. No additional intersection improvements are necessary due to the closure and reassignment.

Amanda Musy-Verdel  
October 4, 2023

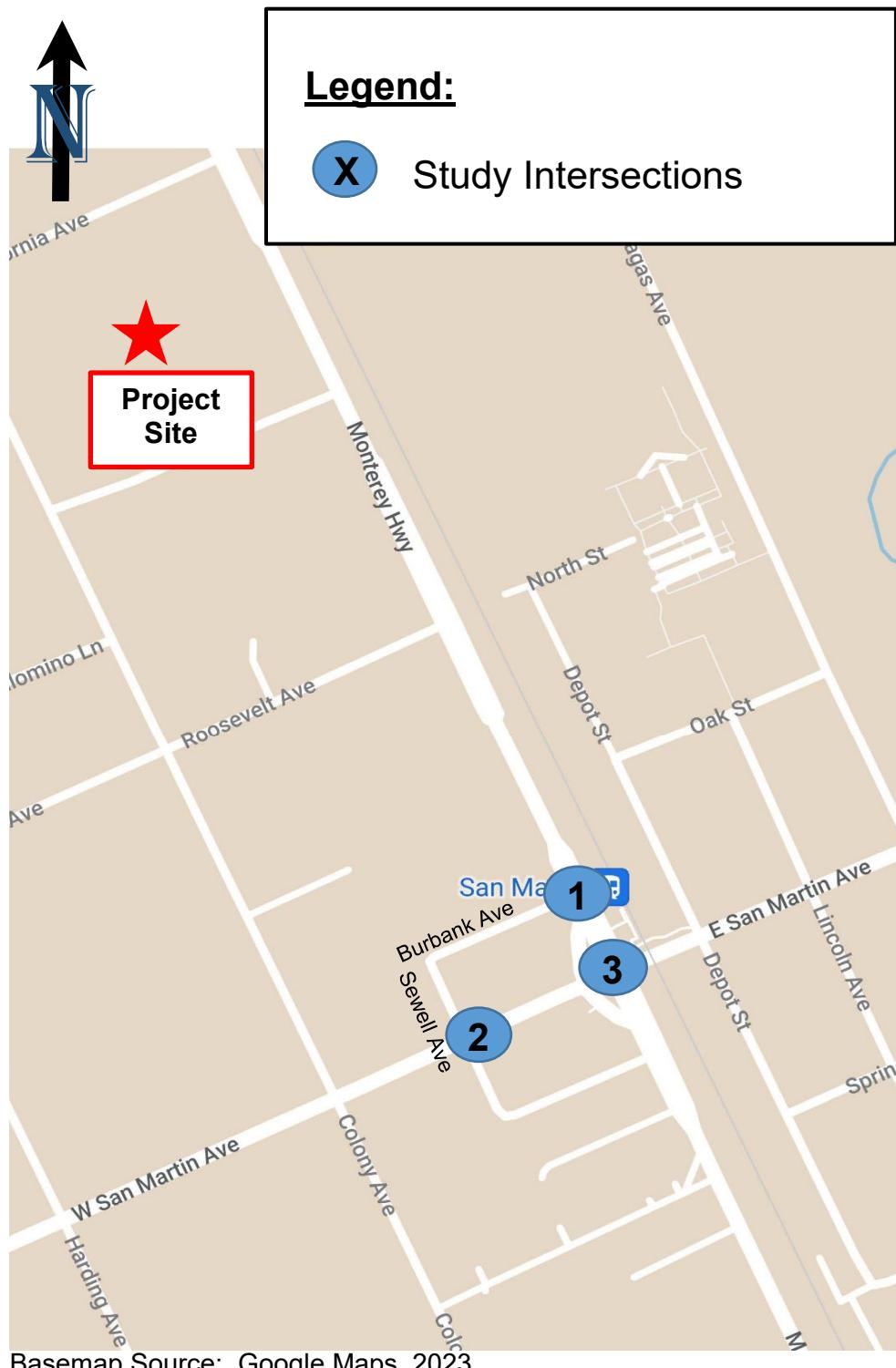
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If you have any questions regarding the contents of this letter or need additional information, please contact me at your convenience. Thank you for the opportunity to assist you with this project.

Respectfully submitted,



Jeff Waller, TE  
Owner / Senior Analyst  
T: (408) 607-1454  
E: [jeffwallerconsulting@gmail.com](mailto:jeffwallerconsulting@gmail.com)

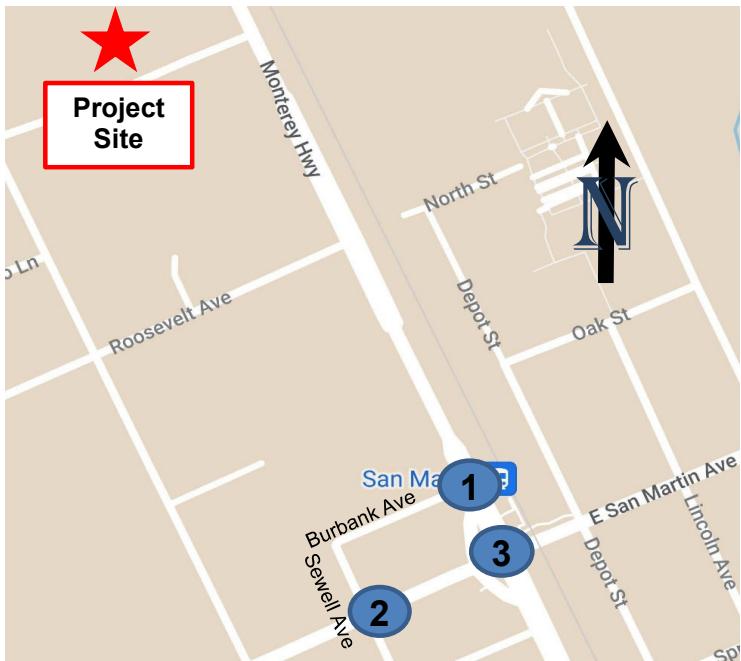


Basemap Source: Google Maps, 2023.

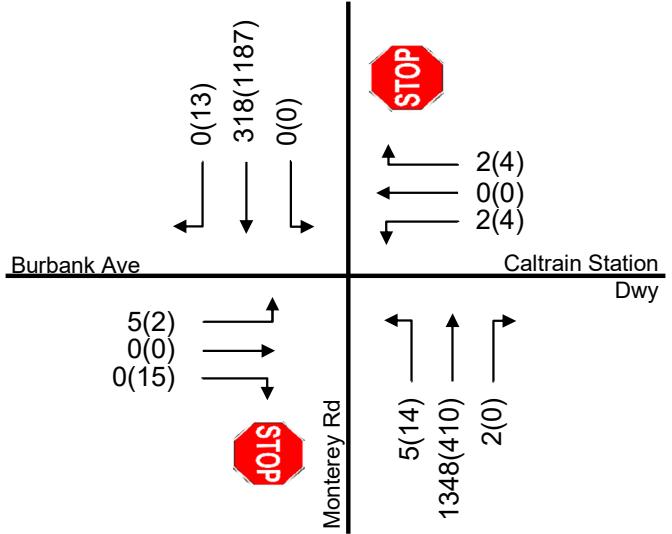
# **Jeff Waller Consulting**

## **Exhibit 1**

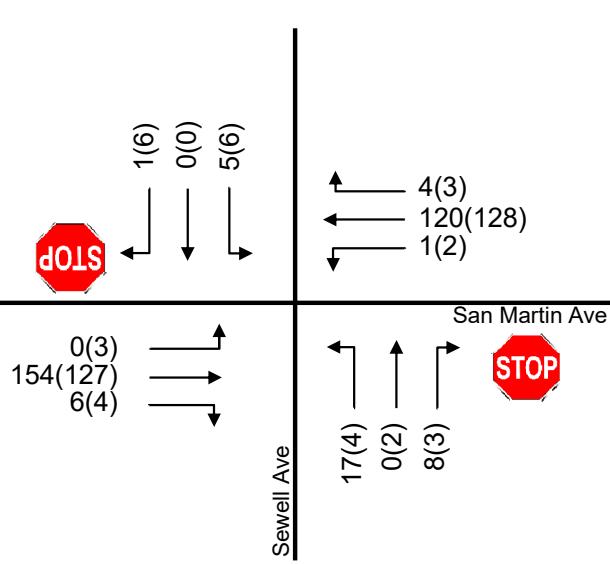
# **Project Location Map and Study Area**



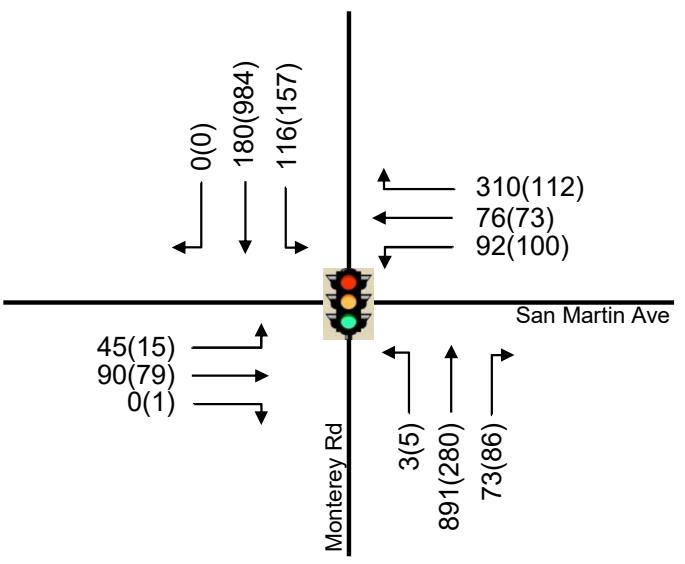
### 1. Monterey Road / Burbank Avenue

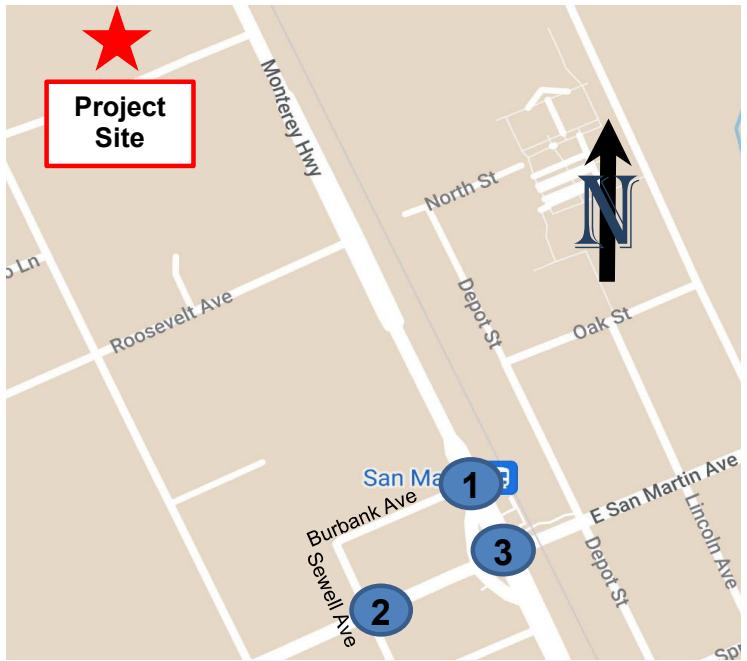


### 2. Sewell Ave / San Martin Ave

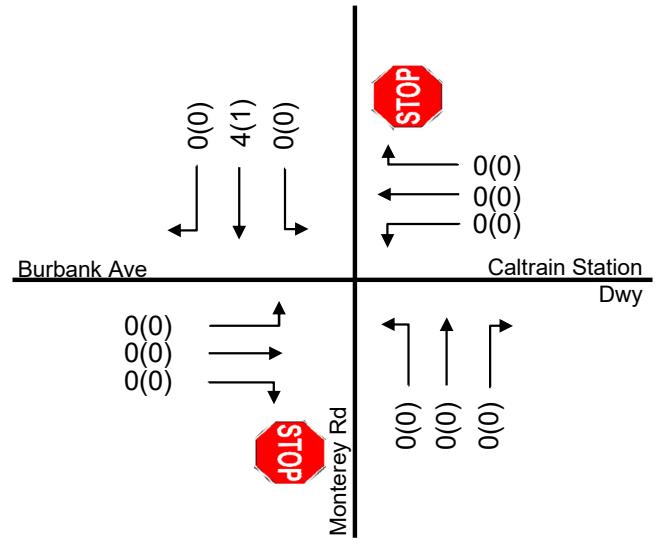


### 3. Monterey Road / San Martin Avenue

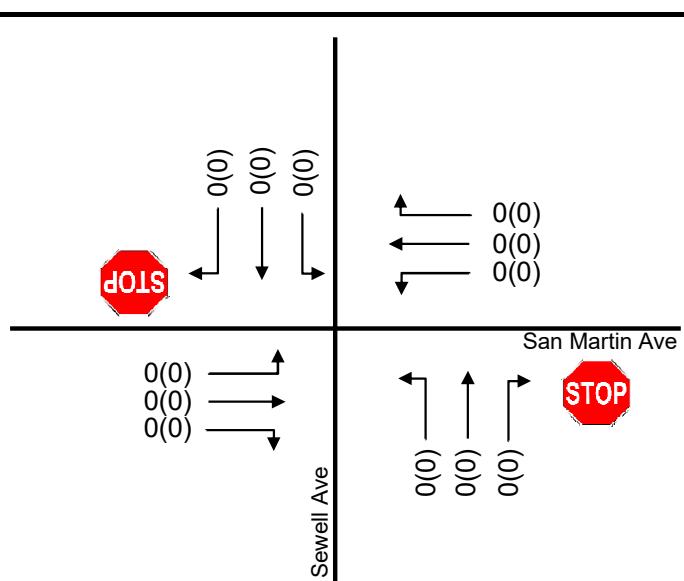




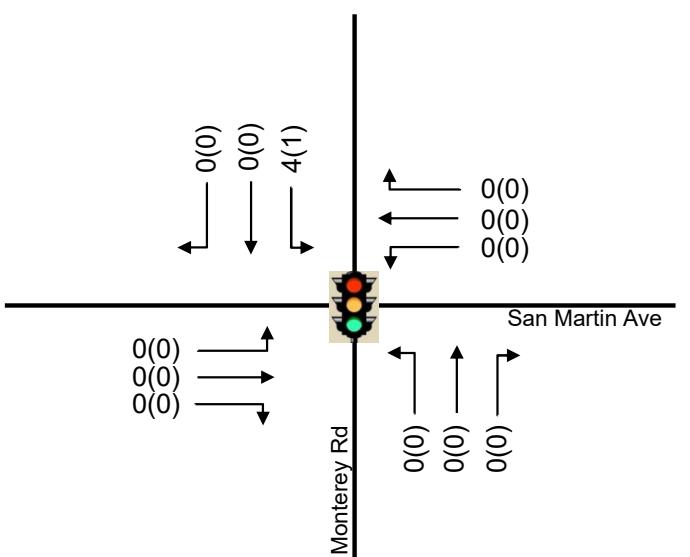
1. Monterey Road / Burbank Avenue

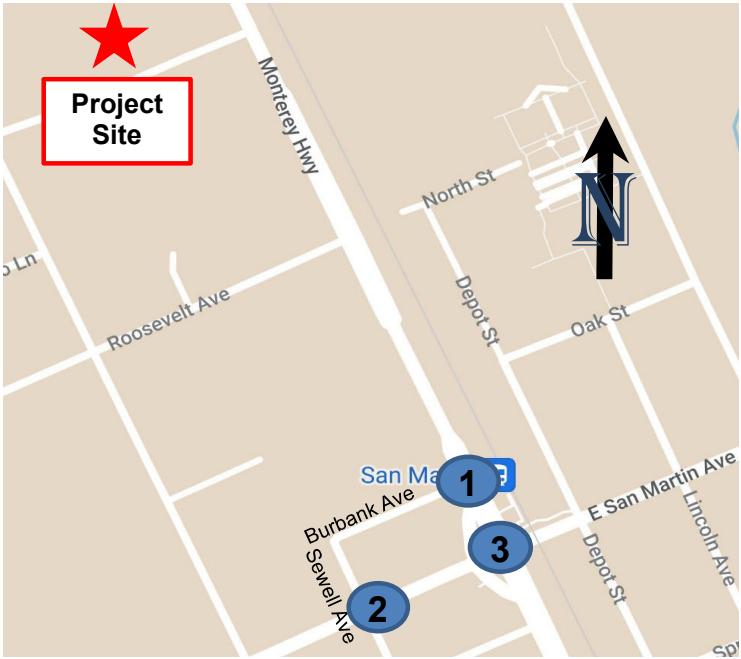


2. Sewell Ave / San Martin Ave

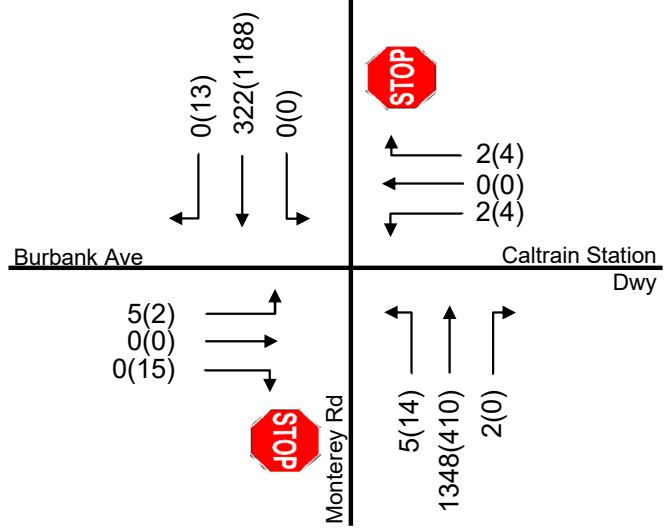


3. Monterey Road / San Martin Avenue



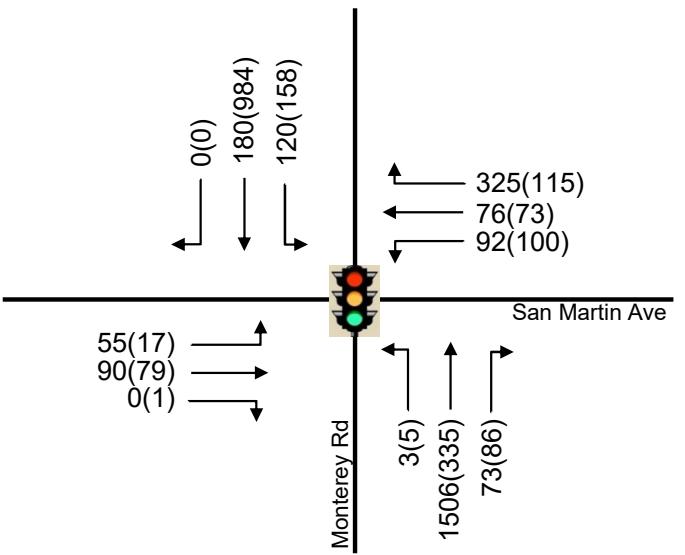
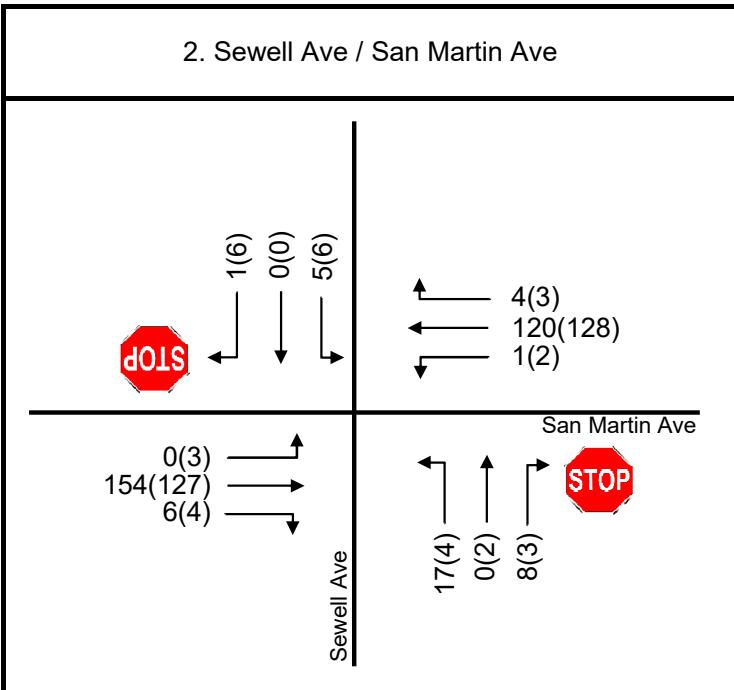


### 1. Monterey Road / Burbank Avenue



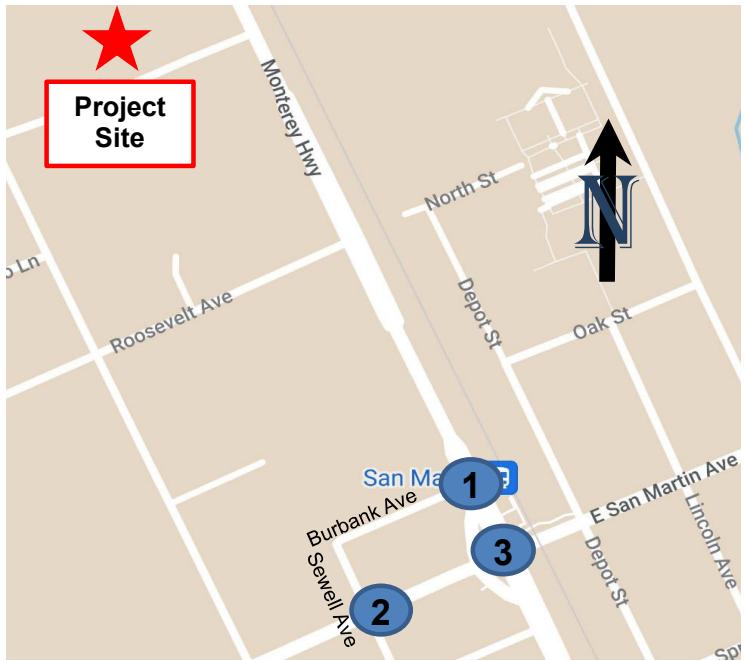
### 2. Sewell Ave / San Martin Ave

### 3. Monterey Road / San Martin Avenue

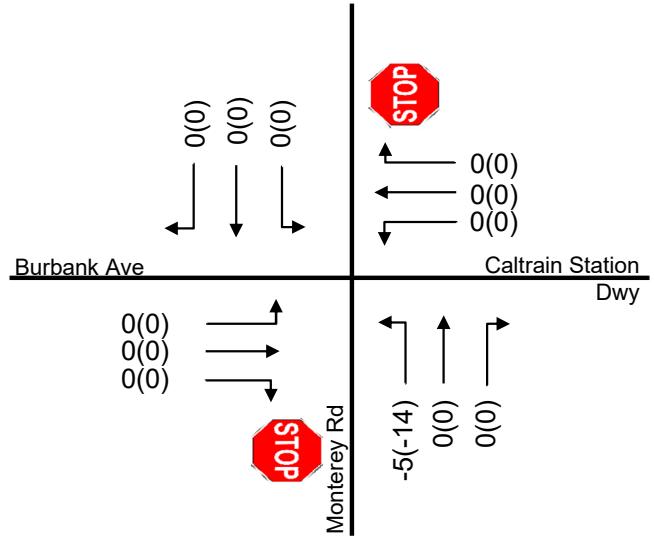


### Exhibit 4

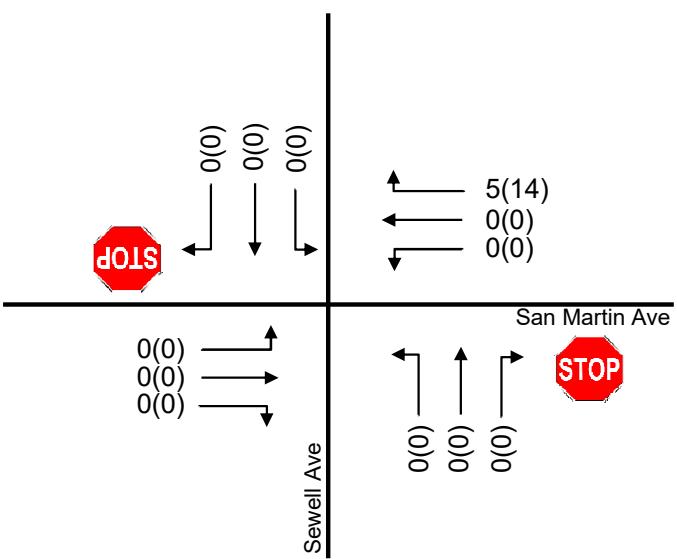
**Existing Plus Project Conditions  
(Without Reassignment)  
AM & PM Peak Hour Volumes**



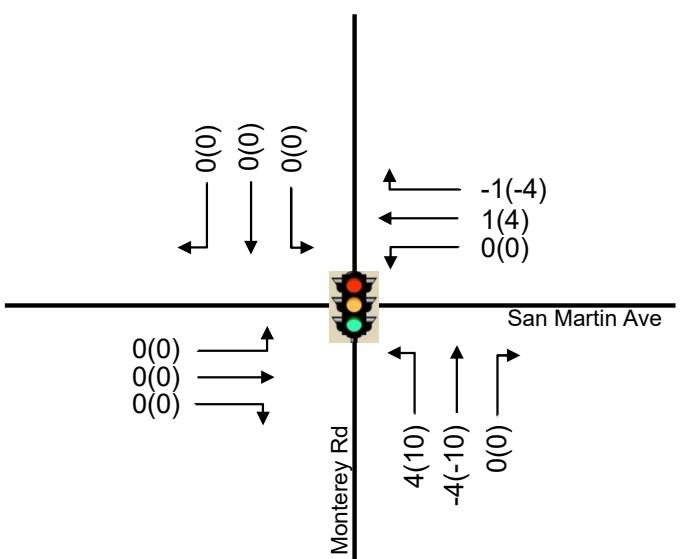
### 1. Monterey Road / Burbank Avenue

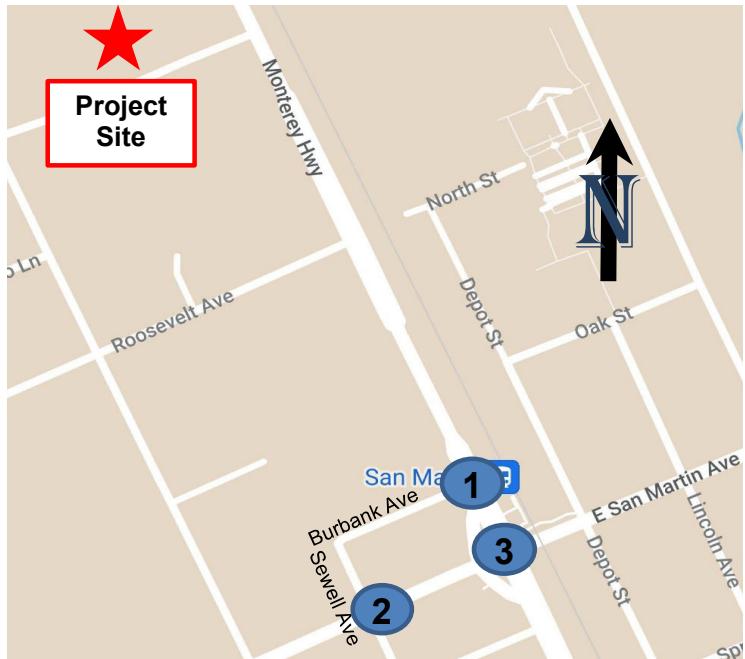


### 2. Sewell Ave / San Martin Ave

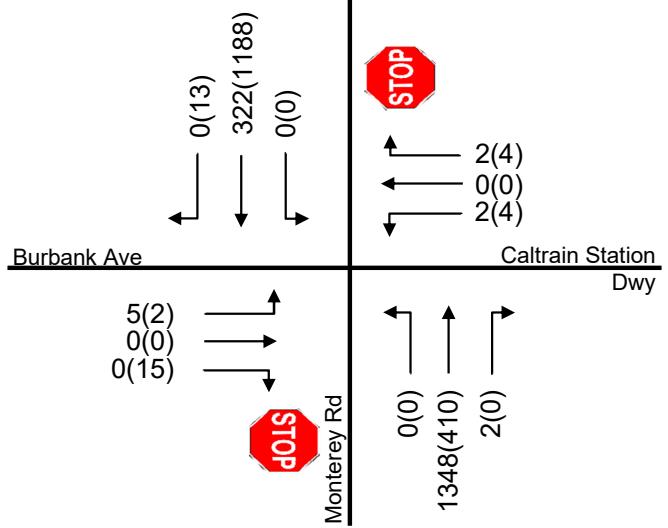


### 3. Monterey Road / San Martin Avenue



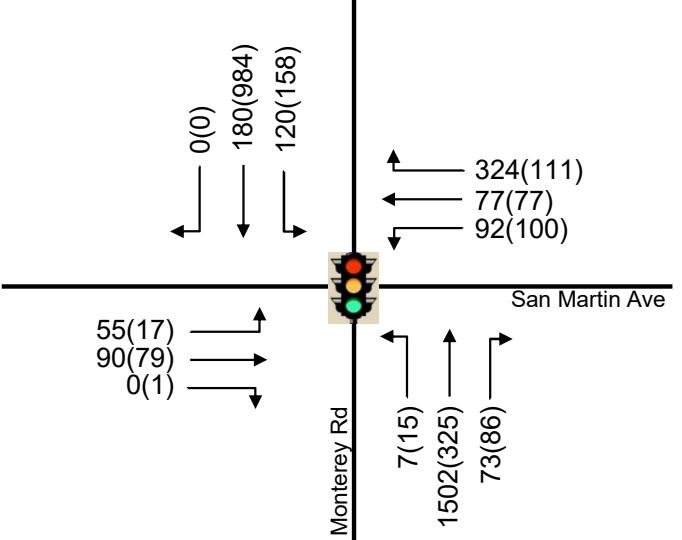
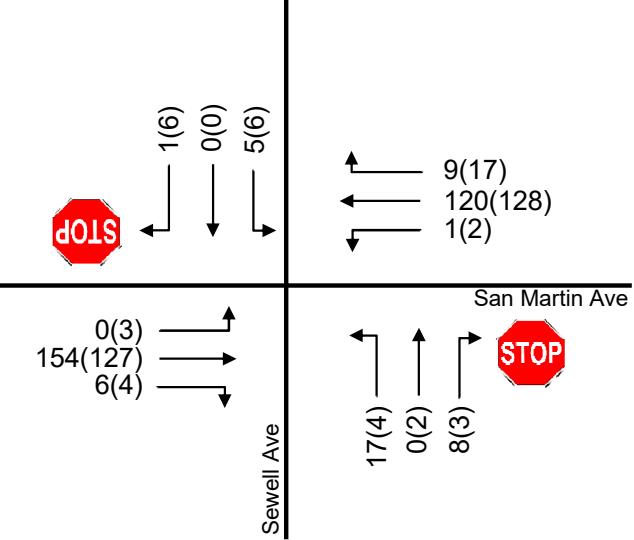


### 1. Monterey Road / Burbank Avenue



### 2. Sewell Ave / San Martin Ave

### 3. Monterey Road / San Martin Avenue



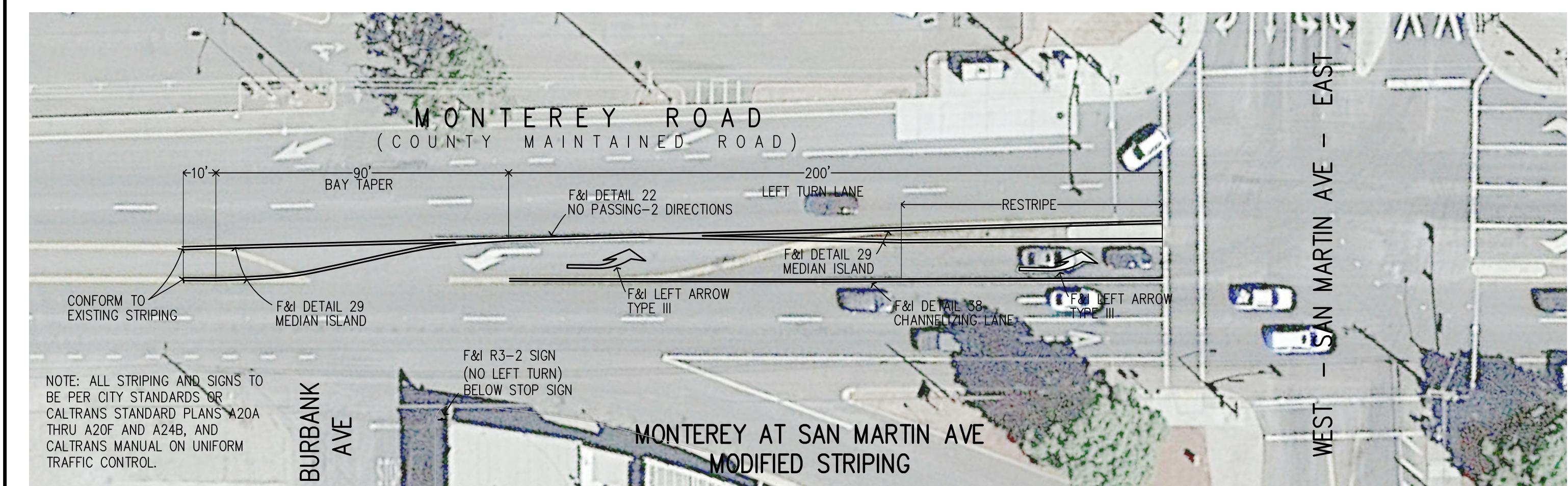
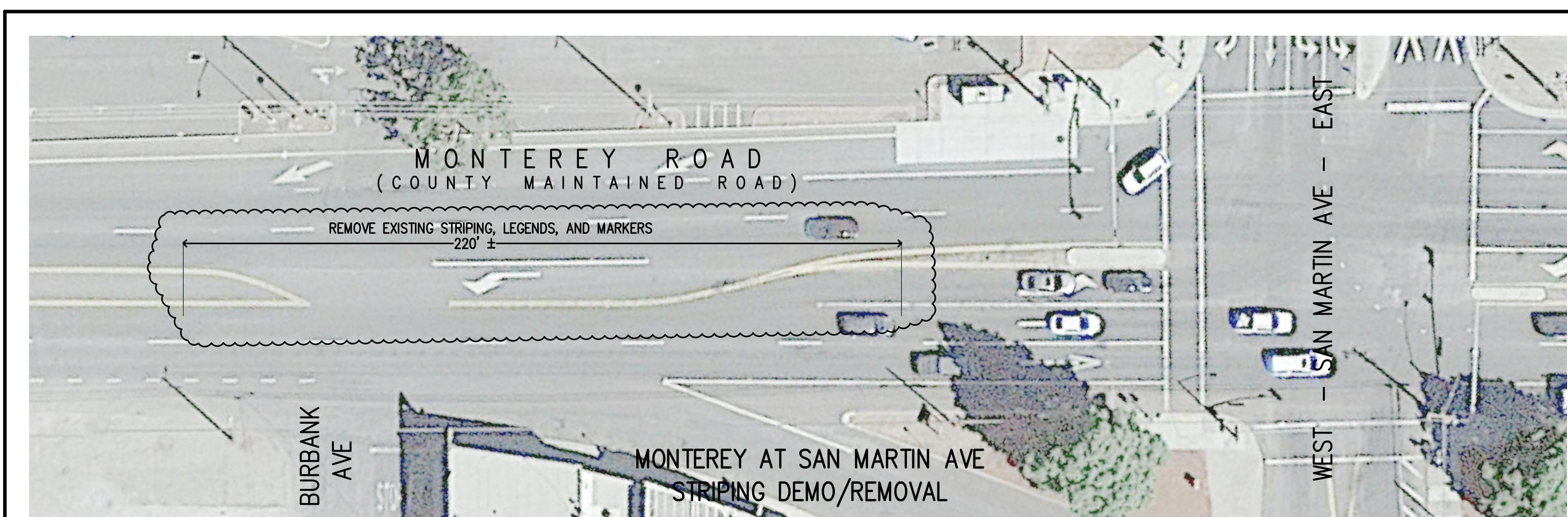
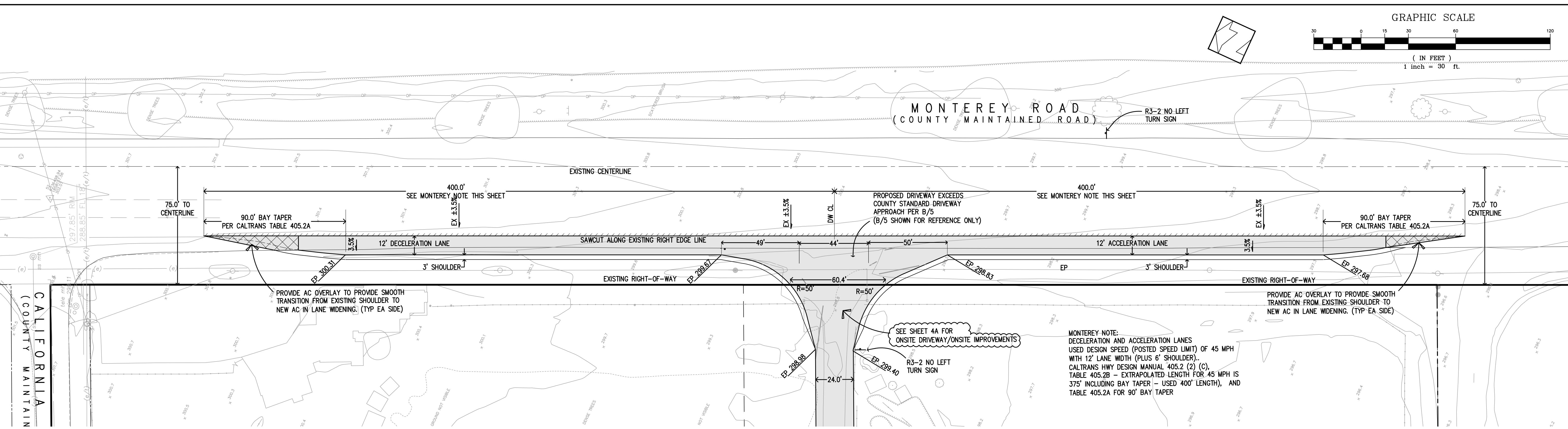
N-S Street	E-W Street	Existing Lane Configuration	Existing Intersection Control	LOS Standard	Peak Hour	Existing Plus Project Conditions			
						Without Closure		With Closure	
						Delay	LOS	Delay	LOS
1 Monterey Road	Burbank Avenue	NB 1-L, 2-T, 1-R SB 1-T, 1-T/R EB 1-L/T/R WB 1-L/T/R	Two-Way Stop	D/D	AM	26.8/38.1	D/E	26.3/37.5	D/E
					PM	18.3/18.7	C/C	18.8/18.0	C/C
2 Sewell Avenue	San Martin Avenue	NB 1-L/T/R SB 1-L/T/R EB 1-L/T/R WB 1-L/T/R	Two-Way Stop	D/D	AM	11.0/10.9	B/B	11.0/11.0	B/B
					PM	10.2/9.8	B/A	10.1/9.8	B/A
3 Monterey Road	San Martin Avenue	NB 1-L, 2-T, 1-R SB 1-L, 1-T, 1-T/R EB 1-L/T/R WB 2-L, 1-T, 1-R	Signal	D	AM	63.1	E	62.6	E
					PM	22.1	C+	22.1	C+

Notes:

1. L, T, R = Left, Through, Right.
2. NB, SB, EB, WB = Left, Through, Right, Northbound, Southbound, Eastbound, Westbound.
3. \* = Delay exceeds 3000 seconds
4. Overall and side-street level of service standards for Santa Clara County are LOS D.
5. N/A = Not Applicable. This intersection or control type does not exist under this scenario.
6. For signalized and all-way stop intersection analysis, delay is average overall delay in seconds per vehicle (sec/veh).  
For one- and two-way stop intersections, delays are side-street approach operations, also in seconds per vehicle (sec/veh).
7. Analysis performed using 2000 Highway Capacity Manual methodologies.
8. Level of service calculations can be found in **Appendix B**.
9. LOS highlighted in red indicates intersection operating below level of service standard.
10. Delays highlighted in bold indicate intersection impacts.

## Appendix A

Design of  
Left Turn Lane Improvements  
on Monterey Road between  
Burbank and San Martin Avenues



MODIFY STRIPING AT MONTEREY AND SAN MARTIN AVE

REVISIONS:		
DATE	DESCRIPTION	BY:

DATE: \_\_\_\_\_  
HORIZ. SC.  
VERT. SC.  
DESIGNED  
CHECKED  
DRAWN B

date: \_\_\_\_\_  
Hanna - Brunetti

Amanda Joy Musy-Verdel  
R.C.E. # 69278

ENGINEER ★

## REFERENCE

**M**

UNINCORPORATED  
NOVEMBER 2022

# PRELIMINARY PLANS NOT FOR CONSTRUCTION

**APPROVED FOR ISSUANCE  
REFER TO ENCROACHMENT AND/OR  
CONSTRUCTION PERMIT AND PLAN  
COVER SHEET FOR SPECIAL  
CONDITIONS AND PERMIT NUMBERS**

# Monterey Road Widening/Driveway

NWC #5 Partnership - 13755 Monterey Road - apn 779-09-061

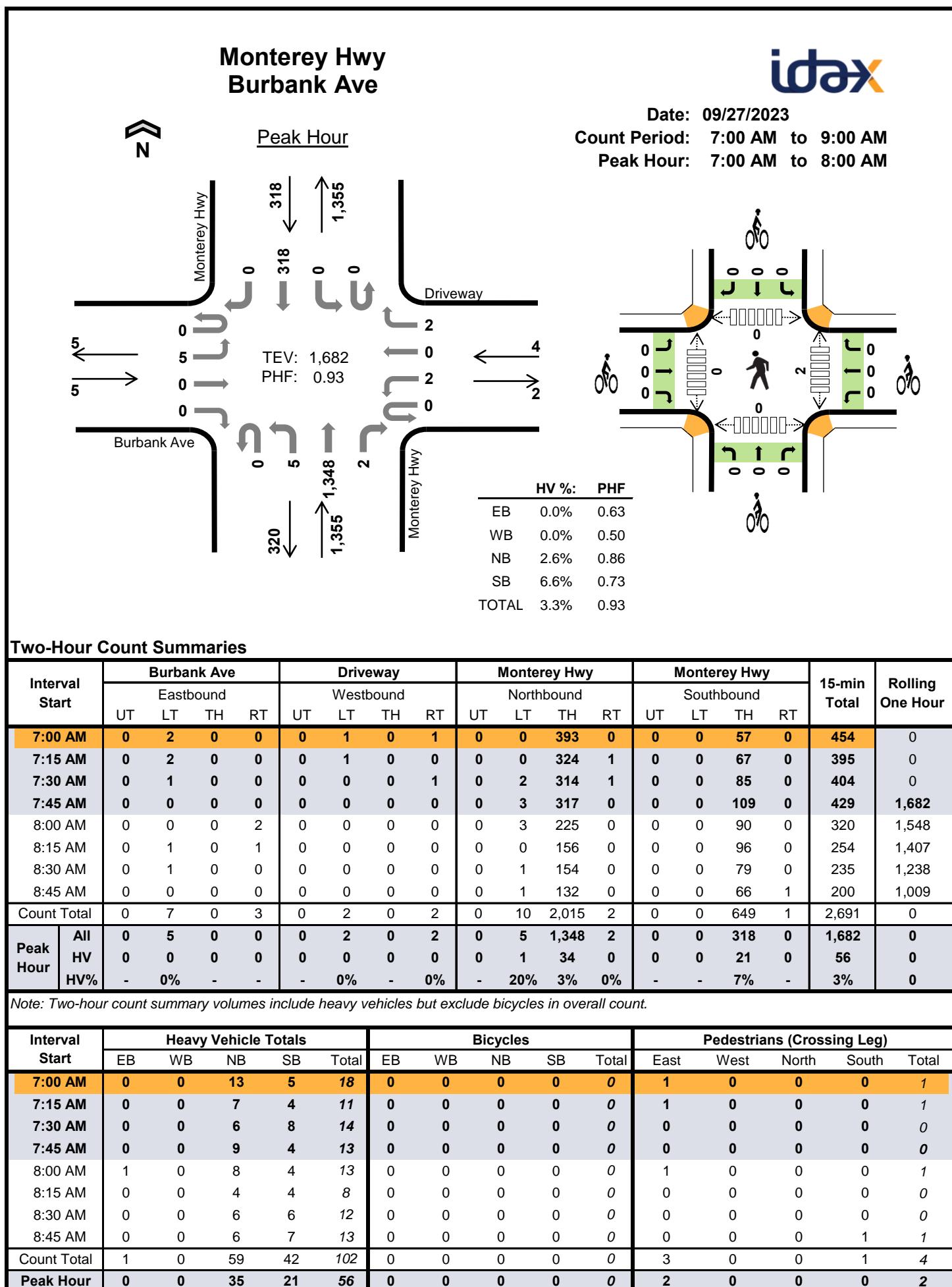
S A N T A   C L A R A   C O U N T Y  
C A L I F O R N I A

SHEET  
**4B**  
OF 8  
JOB NO.  
**1907**

## Appendix B

Traffic Volume

Data Collection



Two-Hour Count Summaries - Heavy Vehicles																		
Interval Start	Burbank Ave				Driveway				Monterey Hwy				Monterey Hwy				15-min Total	Rolling One Hour
	Eastbound		Westbound		Northbound		Southbound											
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT		
7:00 AM	0	0	0	0	0	0	0	0	0	0	13	0	0	0	5	0	18	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	7	0	0	0	4	0	11	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	8	0	14	0
7:45 AM	0	0	0	0	0	0	0	0	0	1	8	0	0	0	4	0	13	56
8:00 AM	0	0	0	1	0	0	0	0	0	0	8	0	0	0	4	0	13	51
8:15 AM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	4	0	8	48
8:30 AM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	6	0	12	46
8:45 AM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	6	1	13	46
Count Total	0	0	0	1	0	0	0	0	0	1	58	0	0	0	41	1	102	0
Peak Hour	0	0	0	0	0	0	0	0	0	1	34	0	0	0	21	0	56	0
Two-Hour Count Summaries - Bikes																		
Interval Start	Burbank Ave				Driveway				Monterey Hwy				Monterey Hwy				15-min Total	Rolling One Hour
	Eastbound		Westbound		Northbound		Southbound											
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT			
7:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
7:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
7:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
7:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:00 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:15 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:30 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
8:45 AM	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Count Total	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0		0	0
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																		

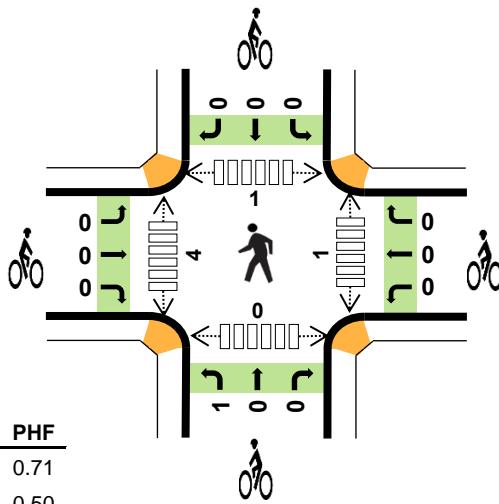
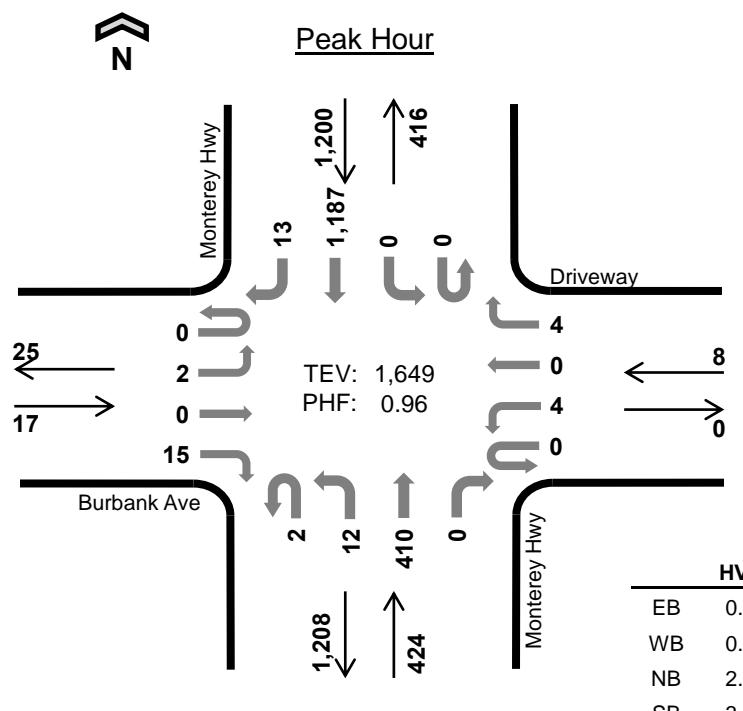
# Monterey Hwy Burbank Ave



Date: 09/27/2023

Count Period: 4:00 PM to 6:00 PM

Peak Hour: 4:30 PM to 5:30 PM



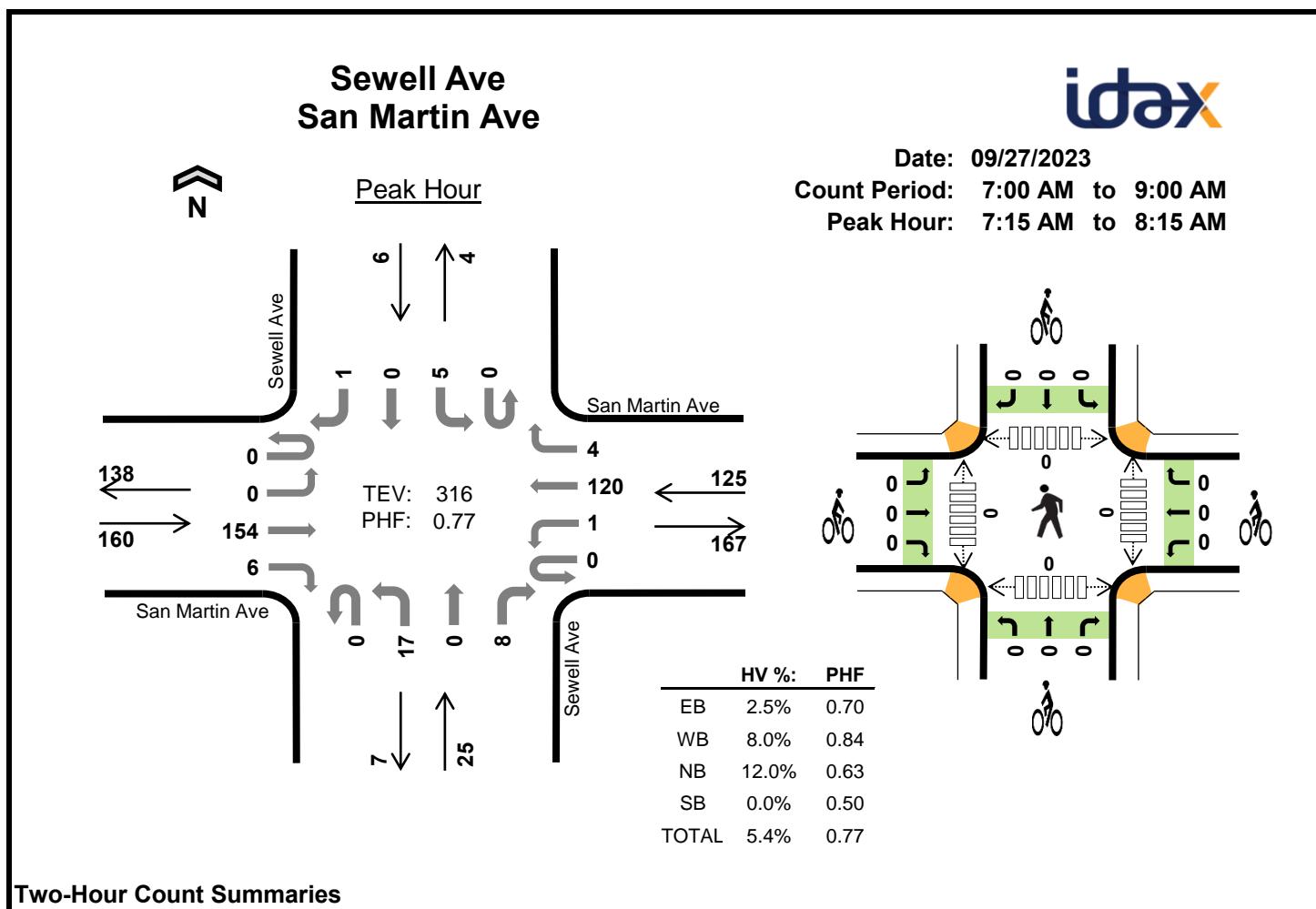
## Two-Hour Count Summaries

Interval Start	Burbank Ave				Driveway				Monterey Hwy				Monterey Hwy				15-min Total	Rolling One Hour		
	Eastbound		Westbound		Northbound		Southbound		UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT
4:00 PM	0	1	0	2	0	0	0	0	0	2	99	0	0	0	219	2	325	0	0	0
4:15 PM	0	1	0	4	0	2	0	0	1	1	93	0	0	0	264	4	370	0	0	0
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>4</b>	<b>102</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>289</b>	<b>4</b>	<b>403</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>109</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>307</b>	<b>2</b>	<b>428</b>	<b>1,526</b>	<b>1,526</b>	<b>1,526</b>
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>5</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>1</b>	<b>96</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>284</b>	<b>5</b>	<b>395</b>	<b>1,596</b>	<b>1,596</b>	<b>1,596</b>
<b>5:15 PM</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>1</b>	<b>1</b>	<b>3</b>	<b>103</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>307</b>	<b>2</b>	<b>423</b>	<b>1,649</b>	<b>1,649</b>	<b>1,649</b>
5:30 PM	0	1	0	0	0	0	0	0	0	2	87	1	1	0	299	2	393	1,639	1,639	1,639
5:45 PM	0	1	0	3	0	2	0	0	0	3	77	1	0	0	250	1	338	1,549	1,549	1,549
Count Total	0	6	0	24	0	8	0	4	3	20	766	2	1	0	2,219	22	3,075	0	0	0
Peak Hour	All	0	2	0	15	0	4	0	2	12	410	0	0	0	1,187	13	1,649	0	0	0
	HV	0	0	0	0	0	0	0	0	0	12	0	0	0	36	0	48	0	0	0
	HV%	-	0%	-	0%	-	0%	-	0%	0%	0%	3%	-	-	3%	0%	3%	0	0	0

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	0	0	4	3	7	0	0	0	0	0	0	4	0	0	4
4:15 PM	0	0	4	8	12	0	0	0	0	0	0	2	0	0	2
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>10</b>	<b>12</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>6</b>	<b>12</b>	<b>18</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>2</b>
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>4</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>0</b>	<b>0</b>	<b>4</b>
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>10</b>	<b>12</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
5:30 PM	0	0	4	5	9	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	6	6	12	0	0	0	0	0	0	0	0	0	0
Count Total	0	0	30	58	88	0	0	1	0	1	1	10	1	0	12
Peak Hour	0	0	12	36	48	0	0	1	0	1	1	4	1	0	6

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	Burbank Ave				Driveway				Monterey Hwy				Monterey Hwy				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
4:00 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	3	0	7	0		
4:15 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	8	0	12	0		
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>10</b>	<b>0</b>	<b>12</b>	<b>0</b>			
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>12</b>	<b>0</b>	<b>18</b>	<b>49</b>			
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>4</b>	<b>0</b>	<b>6</b>	<b>48</b>			
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>10</b>	<b>0</b>	<b>12</b>	<b>48</b>			
5:30 PM	0	0	0	0	0	0	0	0	0	0	4	0	0	0	5	0	9	45		
5:45 PM	0	0	0	0	0	0	0	0	0	0	6	0	0	0	6	0	12	39		
Count Total	0	0	0	0	0	0	0	0	0	0	30	0	0	0	58	0	88	0		
<b>Peak Hour</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>12</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>36</b>	<b>0</b>	<b>48</b>	<b>0</b>			
Two-Hour Count Summaries - Bikes																				
Interval Start	Burbank Ave				Driveway				Monterey Hwy				Monterey Hwy				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
4:00 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
4:15 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	0		
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>		
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>1</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>1</b>	<b>1</b>		
5:30 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	1		
5:45 PM	0	0	0		0	0	0		0	0	0		0	0	0		0	1		
Count Total	0	0	0		0	0	0		1	0	0		0	0	0		1	0		
<b>Peak Hour</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>1</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>1</b>	<b>0</b>		
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																				

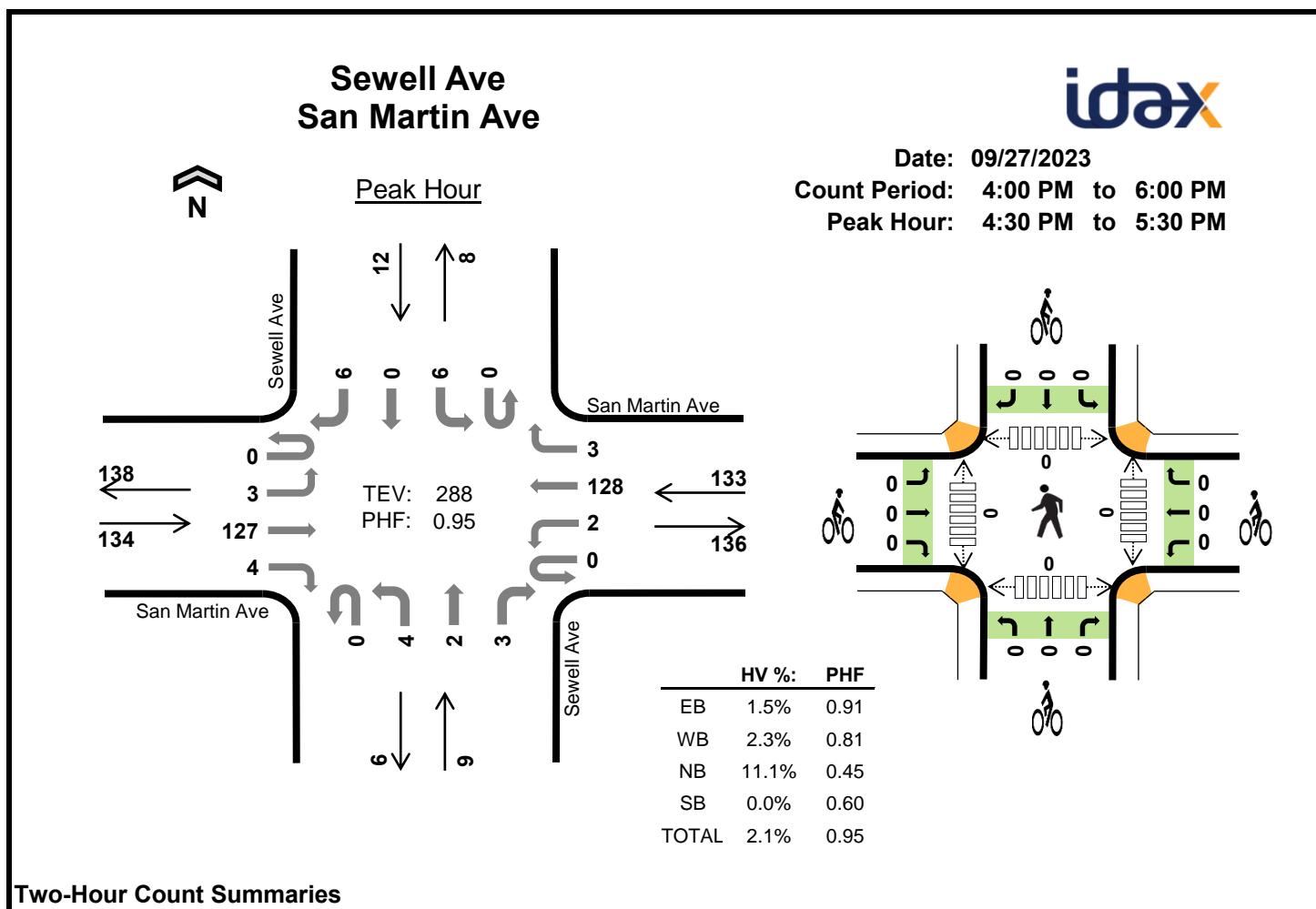
**Two-Hour Count Summaries**

Interval Start	San Martin Ave				San Martin Ave				Sewell Ave				Sewell Ave				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
7:00 AM	0	0	13	0	0	0	18	0	0	4	1	2	0	2	0	1	41	0		
7:15 AM	0	0	21	3	0	1	26	1	0	9	0	1	0	0	0	0	62	0		
7:30 AM	0	0	46	1	0	0	23	1	0	3	0	2	0	2	0	1	79	0		
7:45 AM	0	0	57	0	0	0	36	0	0	2	0	5	0	2	0	0	102	284		
8:00 AM	0	0	30	2	0	0	35	2	0	3	0	0	0	1	0	0	73	316		
8:15 AM	0	1	21	2	0	0	27	1	0	3	0	0	0	0	0	0	55	309		
8:30 AM	0	0	23	1	0	0	11	0	0	0	0	0	0	0	0	0	35	265		
8:45 AM	0	0	17	1	0	0	22	0	0	0	0	1	0	2	0	0	43	206		
Count Total	0	1	228	10	0	1	198	5	0	24	1	11	0	9	0	2	490	0		
Peak Hour	All	0	0	154	6	0	1	120	4	0	17	0	8	0	5	0	1	316	0	
	HV	0	0	4	0	0	0	10	0	0	2	0	1	0	0	0	0	17	0	
	HV%	-	-	3%	0%	-	0%	8%	0%	-	12%	-	13%	-	0%	-	0%	5%	0	

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
7:00 AM	0	0	1	0	1	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	4	1	0	5	0	0	0	0	0	0	0	0	0	0
7:30 AM	2	3	0	0	5	0	0	0	0	0	0	0	0	0	0
7:45 AM	1	1	1	0	3	0	0	0	0	0	0	0	0	0	0
8:00 AM	1	2	1	0	4	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	3	0	0	3	1	0	0	0	1	0	0	0	0	0
8:30 AM	0	1	0	0	1	0	0	0	0	0	0	0	0	0	0
8:45 AM	1	2	0	1	4	0	0	0	0	0	1	0	0	1	2
Count Total	5	16	4	1	26	1	0	0	0	1	1	0	0	1	2
Peak Hour	4	10	3	0	17	0	0	0	0	0	0	0	0	0	0

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	San Martin Ave				San Martin Ave				Sewell Ave				Sewell Ave				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
7:00 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1	0		
7:15 AM	0	0	0	0	0	0	4	0	0	1	0	0	0	0	0	0	5	0		
7:30 AM	0	0	2	0	0	0	3	0	0	0	0	0	0	0	0	0	5	0		
7:45 AM	0	0	1	0	0	0	1	0	0	0	0	1	0	0	0	0	3	14		
8:00 AM	0	0	1	0	0	0	2	0	0	1	0	0	0	0	0	0	4	17		
8:15 AM	0	0	0	0	0	0	3	0	0	0	0	0	0	0	0	0	3	15		
8:30 AM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	11		
8:45 AM	0	0	1	0	0	0	2	0	0	0	0	0	0	1	0	0	4	12		
Count Total	0	0	5	0	0	0	16	0	0	3	0	1	0	1	0	0	26	0		
Peak Hour	0	0	4	0	0	0	10	0	0	2	0	1	0	0	0	0	17	0		
Two-Hour Count Summaries - Bikes																				
Interval Start	San Martin Ave				San Martin Ave				Sewell Ave				Sewell Ave				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
7:00 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
7:15 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
7:30 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
7:45 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
8:00 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
8:15 AM	0	1	0		0	0	0		0	0	0		0	0	0	1	1	1		
8:30 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	1		
8:45 AM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	1		
Count Total	0	1	0		0	0	0		0	0	0		0	0	0	1	0	0		
Peak Hour	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
Note: U-Turn volumes for bikes are included in Left-Turn, if any.																				

**Two-Hour Count Summaries**

Interval Start	San Martin Ave				San Martin Ave				Sewell Ave				Sewell Ave				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
4:00 PM	0	1	25	1	0	0	34	3	0	0	1	2	0	0	1	2	70	0		
4:15 PM	0	0	30	3	0	1	29	1	0	1	2	1	0	1	0	0	69	0		
<b>4:30 PM</b>	<b>0</b>	<b>3</b>	<b>32</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>37</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>76</b>	<b>0</b>		
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>32</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>27</b>	<b>2</b>	<b>0</b>	<b>3</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>1</b>	<b>72</b>	<b>287</b>		
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>35</b>	<b>2</b>	<b>0</b>	<b>1</b>	<b>23</b>	<b>1</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>3</b>	<b>70</b>	<b>287</b>		
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>28</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>41</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>70</b>	<b>288</b>		
5:30 PM	0	1	22	1	0	1	29	2	0	1	0	0	0	0	1	1	59	271		
5:45 PM	0	1	25	0	0	0	37	0	0	2	0	1	0	1	1	3	71	270		
Count Total	0	6	229	9	0	4	257	9	0	8	5	7	0	8	3	12	557	0		
Peak Hour	All	0	3	127	4	0	2	128	3	0	4	2	3	0	6	0	6	288	0	
	HV	0	0	2	0	0	0	3	0	0	1	0	0	0	0	0	0	6	0	
	HV%	-	0%	2%	0%	-	0%	2%	0%	-	25%	0%	0%	-	0%	-	0%	2%	0	

Note: Two-hour count summary volumes include heavy vehicles but exclude bicycles in overall count.

Interval Start	Heavy Vehicle Totals					Bicycles					Pedestrians (Crossing Leg)				
	EB	WB	NB	SB	Total	EB	WB	NB	SB	Total	East	West	North	South	Total
4:00 PM	2	2	0	0	4	0	0	0	0	0	0	0	0	1	1
4:15 PM	1	0	0	0	1	0	1	0	0	1	0	0	0	0	0
<b>4:30 PM</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>4:45 PM</b>	<b>1</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
<b>5:15 PM</b>	<b>1</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>
5:30 PM	0	1	0	0	1	0	0	0	0	0	0	0	0	1	1
5:45 PM	3	1	0	0	4	0	0	0	0	0	0	0	0	0	0
Count Total	8	7	1	0	16	0	1	0	0	1	0	0	0	2	2
Peak Hour	<b>2</b>	<b>3</b>	<b>1</b>	<b>0</b>	<b>6</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>

Two-Hour Count Summaries - Heavy Vehicles																				
Interval Start	San Martin Ave				San Martin Ave				Sewell Ave				Sewell Ave				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT	UT	LT	TH	RT				
4:00 PM	0	0	2	0	0	0	2	0	0	0	0	0	0	0	0	0	4	0		
4:15 PM	0	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0		
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>		
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>9</b>		
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>5</b>		
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>6</b>		
5:30 PM	0	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1	6		
5:45 PM	0	0	3	0	0	0	1	0	0	0	0	0	0	0	0	0	4	7		
Count Total	0	0	8	0	0	0	7	0	0	1	0	0	0	0	0	0	16	0		
<b>Peak Hour</b>	<b>0</b>	<b>0</b>	<b>2</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>3</b>	<b>0</b>	<b>0</b>	<b>1</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>6</b>	<b>0</b>		
Two-Hour Count Summaries - Bikes																				
Interval Start	San Martin Ave				San Martin Ave				Sewell Ave				Sewell Ave				15-min Total	Rolling One Hour		
	Eastbound				Westbound				Northbound				Southbound							
	LT	TH	RT		LT	TH	RT		LT	TH	RT		LT	TH	RT					
4:00 PM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
4:15 PM	0	0	0		0	1	0		0	0	0		0	0	0	0	1	0		
<b>4:30 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>		
<b>4:45 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>		
<b>5:00 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>1</b>		
<b>5:15 PM</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>		
5:30 PM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
5:45 PM	0	0	0		0	0	0		0	0	0		0	0	0	0	0	0		
Count Total	0	0	0		0	1	0		0	0	0		0	0	0	0	1	0		
<b>Peak Hour</b>	<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>		<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>	<b>0</b>		

Note: U-Turn volumes for bikes are included in Left-Turn, if any.

## Appendix C

Intersection

Level of Service

Calculations

Existing Plus Project

Conditions

Without Closure

and

With Closure

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1001 Monterey Rd / Burbank Ave

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: E[ 38.1]

Street Name:	Monterey Rd				Burbank Ave												
Approach:	North Bound		South Bound		East Bound		West Bound										
Movement:	L	-	T	-	R	L	-	T	-	R	L	-	T	-	R		
Control:	Uncontrolled				Uncontrolled				Stop Sign				Stop Sign				
Rights:	Include				Include				Include				Include				
Lanes:	1	0	2	0	1	0	0	0	1	0	0	0	0	0	1	0	0

## Volume Module:AM Peak Hour

Base Vol:	5	1348	2	0	322	0	5	0	0	2	0	2
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	5	1348	2	0	322	0	5	0	0	2	0	2
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93	0.93
PHF Volume:	5	1449	2	0	346	0	5	0	0	2	0	2
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	5	1449	2	0	346	0	5	0	0	2	0	2

## Critical Gap Module:

Critical Gp:	4.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	7.5	xxxx	xxxxx	7.5	6.5	6.9
FollowUpTim:	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	xxxx	xxxxx	3.5	4.0	3.3

## Capacity Module:

Cnflict Vol:	346	xxxx	xxxxx	xxxx	xxxx	xxxxx	1082	xxxx	xxxxx	1633	1806	725
Potent Cap.:	1202	xxxx	xxxxx	xxxx	xxxx	xxxxx	172	xxxx	xxxxx	67	78	368
Move Cap.:	1202	xxxx	xxxxx	xxxx	xxxx	xxxxx	170	xxxx	xxxxx	67	78	368
Volume/Cap:	0.00	xxxx	xxxx	xxxx	xxxx	xxxxx	0.03	xxxx	xxxxx	0.03	0.00	0.01

## Level Of Service Module:

2Way95thQ:	0.0	xxxx	xxxxx	xxxx	xxxx	xxxxx	0.1	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	8.0	xxxx	xxxxx	xxxxx	xxxx	xxxxx	26.8	xxxx	xxxxx	xxxxx	xxxx	xxxxx
LOS by Move:	A	*	*	*	*	*	D	*	*	*	*	*
Movement:	LT	-	LTR	-	RT		LT	-	LTR	-	RT	
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	113	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxx	0.1	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxx	38.1	xxxxx
Shared LOS:	*	*	*	*	*	*	*	*	*	*	E	*
ApproachDel:	xxxxxx		xxxxxx				26.8				38.1	
ApproachLOS:	*		*				D				E	

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1002 Sewell Ave / San Martin Ave

Average Delay (sec/veh): 1.1 Worst Case Level Of Service: B[ 11.0]

Street Name: Sewell Ave San Martin Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

-----|-----|-----|-----|-----|-----|-----|-----|

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1 0 0 0 1! 0 0

-----|-----|-----|-----|-----|-----|-----|-----|

Volume Module:AM Peak Hour

Base Vol: 17 0 8 5 0 1 0 154 6 1 120 4

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 17 0 8 5 0 1 0 154 6 1 120 4

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77

PHF Volume: 22 0 10 6 0 1 0 200 8 1 156 5

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 22 0 10 6 0 1 0 200 8 1 156 5

-----|-----|-----|-----|-----|-----|-----|-----|

Critical Gap Module:

Critical Gp: 7.2 6.6 6.3 7.1 6.5 6.2 xxxxx xxxx xxxx 4.2 xxxx xxxx

FollowUpTim: 3.6 4.1 3.4 3.5 4.0 3.3 xxxxx xxxx xxxx 2.3 xxxx xxxx

-----|-----|-----|-----|-----|-----|-----|-----|

Capacity Module:

Cnflict Vol: 366 368 204 370 369 158 xxxx xxxx xxxx 208 xxxx xxxx

Potent Cap.: 572 546 812 586 560 887 xxxx xxxx xxxx 1328 xxxx xxxx

Move Cap.: 571 545 812 579 560 887 xxxx xxxx xxxx 1328 xxxx xxxx

Volume/Cap: 0.04 0.00 0.01 0.01 0.00 0.00 xxxx xxxx xxxx 0.00 xxxx xxxx

-----|-----|-----|-----|-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx 7.7 xxxx xxxx

LOS by Move: \* \* \* \* \* \* \* \* \* \* A \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx 631 xxxx xxxx 614 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

SharedQueue:xxxxx 0.2 xxxx xxxx 0.0 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx 11.0 xxxx xxxx 10.9 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* B \* \* B \* \* \* \* \* \* \* \*

ApproachDel: 11.0 10.9 xxxxxx

ApproachLOS: B B \*

\*\*\*\*\*

Note: Queue reported is the number of cars per lane.

\*\*\*\*\*

## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #1003 Monterey Rd / San Martin Ave

Cycle (sec): 120 Critical Vol./Cap.(X): 1.004  
 Loss Time (sec): 16 Average Delay (sec/veh): 63.1  
 Optimal Cycle: 180 Level Of Service: E

Street Name: Monterey Rd San Martin Ave  
 Approach: North Bound South Bound East Bound West Bound  
 Movement: L - T - R L - T - R L - T - R L - T - R  
 Control: Protected Protected Split Phase Split Phase  
 Rights: Include Include Include Include  
 Min. Green: 7 10 10 7 10 10 10 10 10 10 10 10  
 Y+R: 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0 4.0  
 Lanes: 1 0 2 0 1 1 0 1 1 0 0 0 2 0 1 0 1

Volume Module:AM Peak Hour  
 Base Vol: 3 1506 73 120 180 0 55 90 0 92 76 325  
 Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 Initial Bse: 3 1506 73 120 180 0 55 90 0 92 76 325  
 User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 PHF Adj: 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84 0.84  
 PHF Volume: 4 1793 87 143 214 0 65 107 0 110 90 387  
 Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0  
 Reduced Vol: 4 1793 87 143 214 0 65 107 0 110 90 387  
 PCE Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 MLF Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 FinalVolume: 4 1793 87 143 214 0 65 107 0 110 90 387

Saturation Flow Module:  
 Sat/Lane: 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900 1900  
 Adjustment: 0.92 1.00 0.92 0.92 0.97 0.92 0.95 0.95 0.92 0.83 1.00 0.92  
 Lanes: 1.00 2.00 1.00 1.00 2.00 0.00 0.38 0.62 0.00 2.00 1.00 1.00  
 Final Sat.: 1750 3800 1750 1750 3700 0 683 1117 0 3150 1900 1750

Capacity Analysis Module:  
 Vol/Sat: 0.00 0.47 0.05 0.08 0.06 0.00 0.10 0.10 0.00 0.03 0.05 0.22  
 Crit Moves: \*\*\*\* \*\*\*\* \*\*\*\*  
 Green Time: 7.0 56.4 56.4 9.8 38.9 0.0 11.5 11.5 0.0 26.4 26.4 26.4  
 Volume/Cap: 0.04 1.00 0.11 1.00 0.18 0.00 1.00 1.00 0.00 0.16 0.22 1.00  
 Uniform Del: 53.3 31.8 17.8 55.1 29.1 0.0 54.3 54.3 0.0 37.8 38.3 46.8  
 IncremntDel: 0.1 22.3 0.1 76.6 0.1 0.0 69.8 69.8 0.0 0.1 0.3 46.9  
 InitQueueDel: 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0 0.0  
 Delay Adj: 1.00 1.00 1.00 1.00 1.00 0.00 1.00 1.00 0.00 1.00 1.00 1.00  
 Delay/Veh: 53.5 54.2 17.8 131.7 29.2 0.0 124.1 124 0.0 37.9 38.6 93.7  
 User DelAdj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00  
 AdjDel/Veh: 53.5 54.2 17.8 131.7 29.2 0.0 124.1 124 0.0 37.9 38.6 93.7  
 LOS by Move: D- D- B F C A F F A D+ D+ F  
 HCM2kAvgQ: 0 41 2 7 3 0 11 11 0 2 3 21

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1001 Monterey Rd / Burbank Ave

Average Delay (sec/veh): 0.4 Worst Case Level Of Service: C[ 18.7]

Street Name:	Monterey Rd	Burbank Ave		
Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	1 0 2 0 1	0 0 1 1 0	0 0 1! 0 0	0 0 1! 0 0

## Volume Module:PM Peak Hour

Base Vol:	14	410	0	0	1188	13	2	0	15	4	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	14	410	0	0	1188	13	2	0	15	4	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
PHF Volume:	15	427	0	0	1238	14	2	0	16	4	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	15	427	0	0	1238	14	2	0	16	4	0	4

## Critical Gap Module:

Critical Gp:	4.1	xxxx	xxxxx	xxxxx	xxxx	xxxxx	7.5	6.5	6.9	7.5	6.5	6.9
FollowUpTim:	2.2	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	3.5	4.0	3.3

## Capacity Module:

Cnflict Vol:	1251	xxxx	xxxxx	xxxx	xxxx	xxxxx	1487	1701	626	1075	1707	214
Potent Cap.:	563	xxxx	xxxxx	xxxx	xxxx	xxxxx	86	91	427	174	90	791
Move Cap.:	563	xxxx	xxxxx	xxxx	xxxx	xxxxx	84	89	427	164	88	791
Volume/Cap:	0.03	xxxx	xxxx	xxxx	xxxx	xxxx	0.02	0.00	0.04	0.03	0.00	0.01

## Level Of Service Module:

2Way95thQ:	0.1	xxxx	xxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	11.6	xxxx	xxxxx	xxxxx	xxxx	xxxxx	xxxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
LOS by Move:	B	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT											
Shared Cap.:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxx	289	xxxxx	xxxx	272	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	0.2	xxxxx	xxxxx	0.1	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx	xxxxx	18.3	xxxxx	xxxxx	18.7	xxxxx
Shared LOS:	*	*	*	*	*	*	*	C	*	*	C	*
ApproachDel:	xxxxxx		xxxxxx					18.3			18.7	
ApproachLOS:	*		*					C			C	

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1002 Sewell Ave / San Martin Ave

Average Delay (sec/veh): 0.9 Worst Case Level Of Service: B[ 10.2]

Street Name: Sewell Ave San Martin Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

Volume Module:PM Peak Hour

Base Vol: 4 2 3 6 0 6 3 127 4 2 128 3

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 4 2 3 6 0 6 3 127 4 2 128 3

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95

PHF Volume: 4 2 3 6 0 6 3 134 4 2 135 3

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 4 2 3 6 0 6 3 134 4 2 135 3

Critical Gap Module:

Critical Gp: 7.2 6.6 6.3 7.1 6.5 6.2 4.1 xxxx xxxx 4.1 xxxx xxxx

FollowUpTim: 3.6 4.1 3.4 3.5 4.0 3.3 2.2 xxxx xxxx 2.2 xxxx xxxx

Capacity Module:

Cnflict Vol: 286 284 136 285 285 136 138 xxxx xxxx 138 xxxx xxxx

Potent Cap.: 649 610 889 667 624 912 1446 xxxx xxxx 1446 xxxx xxxx

Move Cap.: 643 608 889 661 622 912 1446 xxxx xxxx 1446 xxxx xxxx

Volume/Cap: 0.01 0.00 0.00 0.01 0.00 0.01 0.00 xxxx xxxx 0.00 xxxx xxxx

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx 0.0 xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx 7.5 xxxx xxxx 7.5 xxxx xxxx

LOS by Move: \* \* \* \* \* \* A \* \* \* A \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx 698 xxxx xxxx 767 xxxx xxxx xxxx xxxx xxxx xxxx

SharedQueue:xxxxx 0.0 xxxx xxxx 0.1 xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx 10.2 xxxx xxxx 9.8 xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* B \* \* A \* \* \* \* \* \* \* \*

ApproachDel: 10.2 9.8 XXXXXX XXXXXX

ApproachLOS: B A \* \* \*

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Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #1003 Monterey Rd / San Martin Ave

Cycle (sec):	70	Critical Vol./Cap.(X):	0.495
Loss Time (sec):	16	Average Delay (sec/veh):	22.1
Optimal Cycle:	53	Level Of Service:	C+

Street Name:	Monterey Rd	San Martin Ave
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Approach:	North Bound	South Bound	East Bound	West Bound
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Movement:	L - T - R	L - T - R	L - T - R	L - T - R
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Control:	Protected	Protected	Split Phase	Split Phase
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Rights:	Include	Include	Include	Include
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Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10
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Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0
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Lanes:	1 0 2 0 1	1 0 1 1 0	0 0 1! 0 0	2 0 1 0 1
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## Volume Module:PM Peak Hour

Base Vol:	5 335 86	158 984 0	17 79 1	100 73 115
-----------	----------	-----------	---------	------------

Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
-------------	----------------	----------------	----------------	----------------

Initial Bse:	5 335 86	158 984 0	17 79 1	100 73 115
--------------	----------	-----------	---------	------------

User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
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PHF Adj:	0.95 0.95 0.95	0.95 0.95 0.95	0.95 0.95 0.95	0.95 0.95 0.95
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PHF Volume:	5 353 91	166 1036 0	18 83 1	105 77 121
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Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
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Reduced Vol:	5 353 91	166 1036 0	18 83 1	105 77 121
--------------	----------	------------	---------	------------

PCE Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
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MLF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
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FinalVolume:	5 353 91	166 1036 0	18 83 1	105 77 121
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Saturation Flow Module:				
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Sat/Lane:	1900 1900 1900	1900 1900 1900	1900 1900 1900	1900 1900 1900
-----------	----------------	----------------	----------------	----------------

Adjustment:	0.92 1.00 0.92	0.92 0.97 0.92	0.92 0.92 0.92	0.83 1.00 0.92
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Lanes:	1.00 2.00 1.00	1.00 2.00 0.00	0.18 0.81 0.01	2.00 1.00 1.00
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Final Sat.:	1750 3800 1750	1750 3700 0	307 1425 18	3150 1900 1750
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## Capacity Analysis Module:

Vol/Sat:	0.00 0.09 0.05	0.10 0.28 0.00	0.06 0.06 0.06	0.06 0.03 0.04	0.07
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Crit Moves:	****	****	****	****	****
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Green Time:	7.0 20.0 20.0	14.0 27.0 0.0	10.0 10.0 10.0	10.0 10.0 10.0	10.0
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Volume/Cap:	0.03 0.32 0.18	0.48 0.73 0.00	0.41 0.41 0.41	0.23 0.28 0.48	
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Uniform Del:	28.4 19.7 18.8	24.8 18.3 0.0	27.3 27.3 27.3	26.6 26.8 27.6	
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IncremntDel:	0.1 0.2 0.2	1.0 1.9 0.0	1.1 1.1 1.1	0.3 0.6 1.5	
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InitQueueDel:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0
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Delay Adj:	1.00 1.00 1.00	1.00 1.00 0.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00
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Delay/Veh:	28.5 19.9 19.0	25.8 20.2 0.0	28.4 28.4 28.4	26.9 27.4 29.1	
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User DelAdj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00
--------------	----------------	----------------	----------------	----------------	------

AdjDel/Veh:	28.5 19.9 19.0	25.8 20.2 0.0	28.4 28.4 28.4	26.9 27.4 29.1	
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LOS by Move:	C B- B-	C C+ A	C C C	C C C	C
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HCM2kAvgQ:	0 3 2	3 10 0	3 3 3	1 2 3	
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Note: Queue reported is the number of cars per lane.					
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## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1001 Monterey Rd / Burbank Ave

Average Delay (sec/veh): 0.2 Worst Case Level Of Service: E[ 37.5]

Street Name:	Monterey Rd	Burbank Ave		
Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign
Rights:	Include	Include	Include	Include
Lanes:	0 0 2 0 1	0 0 1 1 0	1 0 0 0 0	0 0 1! 0 0

## Volume Module:AM Peak Hour

Base Vol:	0 1348	2 0	322 0	5 0	0 0	2 0	0 2
Growth Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	0 1348	2 0	322 0	5 0	0 0	2 0	0 2
User Adj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	0.93 0.93	0.93 0.93	0.93 0.93	0.93 0.93	0.93 0.93	0.93 0.93	0.93 0.93
PHF Volume:	0 1449	2 0	346 0	5 0	0 0	2 0	0 2
Reduct Vol:	0 0	0 0	0 0	0 0	0 0	0 0	0 0
FinalVolume:	0 1449	2 0	346 0	5 0	0 0	2 0	0 2

## Critical Gap Module:

Critical Gp:	xxxxx xxxx xxxx xxxx xxxx xxxx	7.5 xxxx xxxx	7.5 6.5 6.9
FollowUpTim:	xxxxx xxxx xxxx xxxx xxxx xxxx	3.5 xxxx xxxx	3.5 4.0 3.3

## Capacity Module:

Cnflict Vol:	xxxx xxxx xxxx xxxx xxxx	1071 xxxx xxxx	1623 1796 725
Potent Cap.:	xxxx xxxx xxxx xxxx	175 xxxx xxxx	68 79 368
Move Cap.:	xxxx xxxx xxxx xxxx xxxx	174 xxxx xxxx	68 79 368
Volume/Cap:	xxxx xxxx xxxx xxxx xxxx	0.03 xxxx xxxx	0.03 0.00 0.01

## Level Of Service Module:

2Way95thQ:	xxxx xxxx xxxx xxxx xxxx	0.1 xxxx xxxx	xxxx xxxx xxxx xxxx
Control Del:	xxxxx xxxx xxxx xxxx xxxx	26.3 xxxx xxxx	xxxx xxxx xxxx xxxx
LOS by Move:	* * * * *	*	D * * * * *
Movement:	LT - LTR - RT	LT - LTR - RT	LT - LTR - RT
Shared Cap.:	xxxx xxxx xxxx xxxx xxxx	xxxx xxxx xxxx	xxxx 115 xxxx
SharedQueue:	xxxxx xxxx xxxx xxxx xxxx	xxxxx xxxx xxxx	xxxxx 0.1 xxxx
Shrd ConDel:	xxxxx xxxx xxxx xxxx xxxx	xxxxx xxxx xxxx	xxxxx 37.5 xxxx
Shared LOS:	* * * * *	*	* * * * E *
ApproachDel:	xxxxxx	xxxxxx	26.3 37.5
ApproachLOS:	*	*	D E

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1002 Sewell Ave / San Martin Ave

Average Delay (sec/veh): 1.1 Worst Case Level Of Service: B[ 11.0]

Street Name: Sewell Ave San Martin Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

-----|-----|-----|-----|-----|-----|-----|-----|

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1 0 0 0 1! 0 0

-----|-----|-----|-----|-----|-----|-----|-----|

Volume Module:AM Peak Hour

Base Vol: 17 0 8 5 0 1 0 154 6 1 120 9

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 17 0 8 5 0 1 0 154 6 1 120 9

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77 0.77

PHF Volume: 22 0 10 6 0 1 0 200 8 1 156 12

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 22 0 10 6 0 1 0 200 8 1 156 12

-----|-----|-----|-----|-----|-----|-----|-----|

Critical Gap Module:

Critical Gp: 7.2 6.6 6.3 7.1 6.5 6.2 xxxxx xxxx xxxx 4.2 xxxx xxxx

FollowUpTim: 3.6 4.1 3.4 3.5 4.0 3.3 xxxxx xxxx xxxx 2.3 xxxx xxxx

-----|-----|-----|-----|-----|-----|-----|-----|

Capacity Module:

Cnflict Vol: 369 374 204 373 372 162 xxxx xxxx xxxx 208 xxxx xxxx

Potent Cap.: 570 541 812 584 558 883 xxxx xxxx xxxx 1328 xxxx xxxx

Move Cap.: 568 541 812 576 558 883 xxxx xxxx xxxx 1328 xxxx xxxx

Volume/Cap: 0.04 0.00 0.01 0.01 0.00 0.00 xxxx xxxx xxxx 0.00 xxxx xxxx

-----|-----|-----|-----|-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx xxxx xxxx 7.7 xxxx xxxx

LOS by Move: \* \* \* \* \* \* \* \* \* \* A \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx 629 xxxx xxxx 611 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

SharedQueue:xxxxx 0.2 xxxx xxxx 0.0 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx 11.0 xxxx xxxx 11.0 xxxx xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* B \* \* B \* \* \* \* \* \* \* \*

ApproachDel: 11.0 11.0 xxxxxx xxxxxx

ApproachLOS: B B \* \*

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Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #1003 Monterey Rd / San Martin Ave

Cycle (sec):	120	Critical Vol./Cap.(X):	1.002
Loss Time (sec):	16	Average Delay (sec/veh):	62.6
Optimal Cycle:	180	Level Of Service:	E

Street Name:	Monterey Rd	San Martin Ave
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Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R

Control:	Protected	Protected	Split Phase	Split Phase
Rights:	Include	Include	Include	Include
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0
Lanes:	1 0 2 0 1	1 0 1 1 0	0 1 0 0 0	2 0 1 0 1

Volume Module:AM Peak Hour

Base Vol:	7 1502	73	120	180	0	55	90	0	92	77	324
Growth Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Initial Bse:	7 1502	73	120	180	0	55	90	0	92	77	324
User Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
PHF Adj:	0.84 0.84	0.84	0.84 0.84	0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84	0.84 0.84
PHF Volume:	8 1788	87	143	214	0	65	107	0	110	92	386
Reduc Vol:	0 0	0	0 0	0	0 0	0 0	0 0	0 0	0 0	0 0	0 0
Reduced Vol:	8 1788	87	143	214	0	65	107	0	110	92	386
PCE Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
MLF Adj:	1.00 1.00	1.00	1.00 1.00	1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
FinalVolume:	8 1788	87	143	214	0	65	107	0	110	92	386

Saturation Flow Module:

Sat/Lane:	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900	1900 1900
Adjustment:	0.92 1.00	0.92 0.92	0.92 0.97	0.92 0.92	0.95 0.95	0.95 0.95	0.92 0.92	0.83 1.00	1.00 0.92		
Lanes:	1.00 2.00	1.00 1.00	1.00 2.00	0.00 0.00	0.38 0.62	0.62 0.00	0.00 2.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
Final Sat.:	1750 3800	1750 1750	1750 3700	0 0	683 1117	1117 0	0 3150	1900 1900	1900 1750		

Capacity Analysis Module:

Vol/Sat:	0.00 0.47	0.05 0.08	0.06 0.00	0.10 0.10	0.10 0.00	0.03 0.03	0.05 0.05	0.22 0.22
Crit Moves:	****	****	****	****	****	****	****	****
Green Time:	7.0 56.3	56.3 9.8	38.9 0.0	11.5 11.5	0.0 26.4	26.4 26.4	26.4 26.4	26.4 26.4
Volume/Cap:	0.08 1.00	0.11 1.00	0.18 0.00	1.00 1.00	0.00 0.00	0.16 0.16	0.22 0.22	1.00 1.00
Uniform Del:	53.5 31.8	31.8 17.8	55.1 29.1	0.0 54.3	54.3 54.3	0.0 37.8	38.4 38.4	46.8 46.8
IncremntDel:	0.3 21.8	21.8 0.1	75.9 0.1	0.0 69.1	69.1 69.1	0.0 0.1	0.3 0.3	46.4 46.4
InitQueueDel:	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0
Delay Adj:	1.00 1.00	1.00 1.00	1.00 1.00	0.00 1.00	1.00 1.00	0.00 1.00	1.00 1.00	1.00 1.00
Delay/Veh:	53.8 53.6	53.6 17.8	131.0 29.2	0.0 123.4	123 123	0.0 37.9	38.6 38.6	93.2 93.2
User DelAdj:	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00	1.00 1.00
AdjDel/Veh:	53.8 53.6	53.6 17.8	131.0 29.2	0.0 123.4	123 123	0.0 37.9	38.6 38.6	93.2 93.2
LOS by Move:	D- D-	B F	C A	F F	F A	D+ D+	D+ D+	F
HCM2kAvgQ:	0 40	2 7	3 0	11 11	11 0	2 2	3 3	21 21

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1001 Monterey Rd / Burbank Ave

Average Delay (sec/veh): 0.3 Worst Case Level Of Service: C[ 18.0]

Street Name:	Monterey Rd			Burbank Ave		
Approach:	North Bound	South Bound	East Bound	West Bound		
Movement:	L - T - R	L - T - R	L - T - R	L - T - R		
Control:	Uncontrolled	Uncontrolled	Stop Sign	Stop Sign		
Rights:	Include	Include	Include	Include		
Lanes:	0 0 2 0 1	0 0 1 1 0	0 0 1! 0 0	0 0 1! 0 0		

## Volume Module:PM Peak Hour

Base Vol:	0	410	0	0	1188	13	2	0	15	4	0	4
Growth Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Initial Bse:	0	410	0	0	1188	13	2	0	15	4	0	4
User Adj:	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
PHF Adj:	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96	0.96
PHF Volume:	0	427	0	0	1238	14	2	0	16	4	0	4
Reduct Vol:	0	0	0	0	0	0	0	0	0	0	0	0
FinalVolume:	0	427	0	0	1238	14	2	0	16	4	0	4

## Critical Gap Module:

Critical Gp:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	7.6	6.6	7.0	7.6	6.6	7.0
FollowUpTim:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxx	3.5	4.0	3.3	3.5	4.0	3.3

## Capacity Module:

Cnflict Vol:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	1458	1671	626	1046	1678	214
Potent Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	90	94	425	181	93	788
Move Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	89	94	425	175	93	788
Volume/Cap:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	0.02	0.00	0.04	0.02	0.00	0.01

## Level Of Service Module:

2Way95thQ:	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
Control Del:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxxx	xxxxxx	xxxx	xxxxx	xxxx	xxxx	xxxxx
LOS by Move:	*	*	*	*	*	*	*	*	*	*	*	*
Movement:	LT - LTR - RT											
Shared Cap.:	xxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxx	295	xxxxxx	xxxx	286	xxxxx
SharedQueue:	xxxxx	xxxx	xxxxx	xxxxx	xxxx	xxxxxx	xxxxxx	0.2	xxxxxx	xxxxx	0.1	xxxxx
Shrd ConDel:	xxxxx	xxxx	xxxxxx	xxxx	xxxx	xxxxxx	xxxxxx	18.0	xxxxxx	xxxxx	18.0	xxxxx
Shared LOS:	*	*	*	*	*	*	*	C	*	*	C	*
ApproachDel:	xxxxxx		xxxxxx				18.0			18.0		
ApproachLOS:	*		*					C			C	

Note: Queue reported is the number of cars per lane.

## Level Of Service Computation Report

2000 HCM Unsignalized Method (Base Volume Alternative)

Intersection #1002 Sewell Ave / San Martin Ave

Average Delay (sec/veh): 0.8 Worst Case Level Of Service: B[ 10.1]

Street Name: Sewell Ave San Martin Ave

Approach: North Bound South Bound East Bound West Bound

Movement: L - T - R L - T - R L - T - R L - T - R

-----|-----|-----|-----|-----|-----|-----|-----|

Control: Stop Sign Stop Sign Uncontrolled Uncontrolled

Rights: Include Include Include Include

Lanes: 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0 0 0 1! 0 0

-----|-----|-----|-----|-----|-----|-----|-----|

Volume Module: PM Peak Hour

Base Vol: 4 2 3 6 0 6 3 127 4 2 128 17

Growth Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

Initial Bse: 4 2 3 6 0 6 3 127 4 2 128 17

User Adj: 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00 1.00

PHF Adj: 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95 0.95

PHF Volume: 4 2 3 6 0 6 3 134 4 2 135 18

Reduct Vol: 0 0 0 0 0 0 0 0 0 0 0 0

FinalVolume: 4 2 3 6 0 6 3 134 4 2 135 18

-----|-----|-----|-----|-----|-----|-----|-----|

Critical Gap Module:

Critical Gp: 7.1 6.5 6.2 7.1 6.5 6.2 4.1 xxxx xxxx 4.1 xxxx xxxx

FollowUpTim: 3.5 4.0 3.3 3.5 4.0 3.3 2.2 xxxx xxxx 2.2 xxxx xxxx

-----|-----|-----|-----|-----|-----|-----|-----|

Capacity Module:

Cnflict Vol: 293 299 136 293 292 144 153 xxxx xxxx 138 xxxx xxxx

Potent Cap.: 662 615 918 658 617 903 1428 xxxx xxxx 1446 xxxx xxxx

Move Cap.: 655 613 918 653 615 903 1428 xxxx xxxx 1446 xxxx xxxx

Volume/Cap: 0.01 0.00 0.00 0.01 0.00 0.01 0.00 xxxx xxxx 0.00 xxxx xxxx

-----|-----|-----|-----|-----|-----|-----|-----|

Level Of Service Module:

2Way95thQ: xxxx xxxx xxxx xxxx xxxx 0.0 xxxx xxxx 0.0 xxxx xxxx

Control Del:xxxxx xxxx xxxx xxxx xxxx 7.5 xxxx xxxx 7.5 xxxx xxxx

LOS by Move: \* \* \* \* \* \* A \* \* \* A \* \* \*

Movement: LT - LTR - RT LT - LTR - RT LT - LTR - RT LT - LTR - RT

Shared Cap.: xxxx 712 xxxx xxxx 758 xxxx xxxx xxxx xxxx xxxx xxxx

SharedQueue:xxxxx 0.0 xxxx xxxx 0.1 xxxx xxxx xxxx xxxx xxxx xxxx

Shrd ConDel:xxxxx 10.1 xxxx xxxx 9.8 xxxx xxxx xxxx xxxx xxxx xxxx

Shared LOS: \* B \* \* A \* \* \* \* \* \* \* \*

ApproachDel: 10.1 9.8 XXXXXX XXXXXX

ApproachLOS: B A \* \* \*

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Note: Queue reported is the number of cars per lane.

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## Level Of Service Computation Report

2000 HCM Operations Method (Base Volume Alternative)

Intersection #1003 Monterey Rd / San Martin Ave

Cycle (sec):	70	Critical Vol./Cap.(X):	0.506
Loss Time (sec):	16	Average Delay (sec/veh):	22.1
Optimal Cycle:	53	Level Of Service:	C+

Street Name: Monterey Rd San Martin Ave

Approach:	North Bound	South Bound	East Bound	West Bound
Movement:	L - T - R	L - T - R	L - T - R	L - T - R

Control:	Protected	Protected	Split Phase	Split Phase
Rights:	Include	Include	Include	Include
Min. Green:	7 10 10	7 10 10	10 10 10	10 10 10
Y+R:	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0	4.0 4.0 4.0
Lanes:	1 0 2 0 1	1 0 1 1 0	0 0 1! 0 0	2 0 1 0 1

Volume Module: PM Peak Hour

Base Vol:	15 325 86	158 984 0	17 79 1	100 77 111
Growth Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Initial Bse:	15 325 86	158 984 0	17 79 1	100 77 111
User Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
PHF Adj:	0.95 0.95 0.95	0.95 0.95 0.95	0.95 0.95 0.95	0.95 0.95 0.95
PHF Volume:	16 342 91	166 1036 0	18 83 1	105 81 117
Reduct Vol:	0 0 0	0 0 0	0 0 0	0 0 0
Reduced Vol:	16 342 91	166 1036 0	18 83 1	105 81 117
PCE Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
MLF Adj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00
Final Volume:	16 342 91	166 1036 0	18 83 1	105 81 117

Saturation Flow Module:

Sat/Lane:	1900 1900 1900	1900 1900 1900	1900 1900 1900	1900 1900 1900
Adjustment:	0.92 1.00 0.92	0.92 0.97 0.92	0.92 0.92 0.92	0.83 1.00 0.92
Lanes:	1.00 2.00 1.00	1.00 2.00 0.00	0.18 0.81 0.01	2.00 1.00 1.00
Final Sat.:	1750 3800 1750	1750 3700 0	307 1425 18	3150 1900 1750

Capacity Analysis Module:

Vol/Sat:	0.01 0.09 0.05	0.10 0.28 0.00	0.06 0.06 0.06	0.06 0.03 0.04	0.07
Crit Moves:	****	*****	****	****	
Green Time:	7.0 20.0 20.0	14.0 27.0 0.0	10.0 10.0 10.0	10.0 10.0 10.0	10.0
Volume/Cap:	0.09 0.32 0.18	0.48 0.73 0.00	0.41 0.41 0.41	0.23 0.30 0.47	
Uniform Del:	28.6 19.6 18.8	24.8 18.3 0.0	27.3 27.3 27.3	26.6 26.9 27.6	
IncremntDel:	0.2 0.2 0.2	1.0 1.9 0.0	1.1 1.1 1.1	0.3 0.6 1.4	
InitQueueDel:	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0 0.0 0.0	0.0
Delay Adj:	1.00 1.00 1.00	1.00 1.00 0.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00
Delay/Veh:	28.8 19.8 19.0	25.8 20.2 0.0	28.4 28.4 28.4	26.9 27.5 28.9	
User DelAdj:	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00 1.00 1.00	1.00
AdjDel/Veh:	28.8 19.8 19.0	25.8 20.2 0.0	28.4 28.4 28.4	26.9 27.5 28.9	
LOS by Move:	C B- B-	C C+ A	C C C	C C C	C
HCM2kAvgQ:	0 3 2	3 10 0	3 3 3	1 2 3	

Note: Queue reported is the number of cars per lane.