

# **MH engineering Co.**

16075 Vineyard Blvd. Morgan Hill, CA 95037 (408) 779-7381 (408) 226-5712 Fax

> Preliminary Storm Water Control Plan Hicks Land, LLC (APN 575-11-009 & 024) 216131 Hicks Road, Los Gatos MHE 217091 June 21, 2022

#### Attachments:

- Clean Water Questionnaire
- Drainage Map (11x17)
- Storm water management plan (24x36)
- Hydrology (Peak Management)





### MH engineering Co.

#### **Project Description:**

This 23.95-acre parcel is located along the southwest side of Hicks Road. Guadalupe reservoir is located downhill on the opposite side of this County maintained road. A small parcel of land (0.21ac) was recently purchased from Santa Clara Valley Water to create a legal access into the property. Site lays in the northeasterly direction with average slopes in the range of 35%. Hicks Road intercepts majority of the existing runoff from this parcel with culvert conveyance into natural swales into the reservoir.

Applicant has proposed to demolish two existing structures and build a primary residence with a detached ADU. Existing oil & screen driveway and turn-around fronting existing house will be re-graded to meet CalFire & Central Fire standards.

#### Low Impact Site Design Measures, Source Control Measures & Stormwater Treatment Measures: Project lies within San Francisco Bay Regional Water Quality Control Board, Region2

Project has provided Stormwater Management Plan Checklist and applicable calculations per the Stormwater Management Guidance Manual for Low Impact Development and Post-Construction Requirements. Project shall meet the applicable requirements of the Stormwater Management Guidance Manual for Low Impact Development and Post-Construction Requirements:

#### Site Design Measures

Following design measures are incorporated into the site layout:

- site layout is conformed along natural landforms
- grading has been optimized
- hardscape is minimized within the scope of project
- structures have been clustered

#### Source Control Measures:

- roof runoff will discharge on splash-blocks and directed onto vegetated areas
- runoff from walkways and other private hardscape is generally directed onto vegetated areas

• portion of the driveway below fire truck turn-around will convey runoff into vegetated swale lined with loose rocks for filtration of storm water

#### Stormwater Treatment Measures:

Roof and yard drainage from proposed buildings will sheet flow through landscaped areas to provide storm water pre-treatment before conveying into proposed SCMs (Rain-tanks). Pre-treated water in both rain-tanks will be retained at 100% with zero release for all storm events.



#### Hydrology (Peak Management):

This parcel is tributary to Guadalupe Reservoir. All collection from the developed portion of the site shall route through proposed SCMs (rain-tank and vegetated detention basin) for water quality treatment and peak management to Pre-Project levels.

Routing model has been setup using SCS method. Hydrographs for post-project events are routed through the BMP storage. Model includes a storage structure and outflow structure. Outflow structure is setup with fixed diameter orifice at fixed elevation. Storage volumes are entered into the file at incremental elevations. A stage/storage rating curve for each BMP is presented at the beginning of each routing.

During the routing, a post project hydrograph of certain rainfall event flows through the SCM, stores the difference of post versus pre project volume and releases at controlled discharge. Orifice size is fine-tuned to keep the release peak discharges at or below pre-project levels.

A typical routing hydrograph graph indicates time increment along the x-axis and inflow runoff along the y-axis. Blue hatched area on the graph between two superimposed curves shows the total volume stored during the full routing. Peak of the outflow hydrograph in red color indicates peak discharge from the BMP with time to peak hour information.

#### SCM Layout Details and System Functionality:

Both SCMs are proposed on private property to be owned and maintained by homeowners.

SCM#1 underground Rain-Tank 'A' consists of 72 double stacked units (1.34x2.25'wide x1.44' high). This treatment/peak management SCM is located under the turn-around area fronting garages. Total storage volume proposed= 751cf

SCM#2 underground Rain-Tank 'B' is identical to Tank 'A' consisting of 72 double stacked units (1.34x2.25'wide x1.44' high). This treatment/peak management SCM is located under the natural terrain west of the main house. A portion of the treated roof and yard capture will be conveyed into this tank. Total storage volume proposed= 751cf

#### DMA (A2-Post) Routing into Tank 'A':

Half of the main house roof drainage and ADU will be conveyed into SCM#1 (Rain-Tank 'A'). Fire truck turnaround and upper portion of driveway will percolate through the permeable pavers, but any excess water will be captured with the proposed catch basin along the driveway at the easterly curb return and piped back into the tank. Per hydrology study routing results, there will full retention of all storm events with zero release out of the SCM. Proposed 8" Ø drainpipe is for an emergency overflow into the 12" culvert. Full storage volume in this tank will infiltrate into the native soils under 48 hours using the lowest percolation rate of 3.24in/hr provided by the soils engineer, which is further reduced by factor of safety 2.



#### Routing Summary DMA (A2-Post):

Event/Post Project Q /Routed Release/ SCM Storage Elev. 2yr/0.21/0.00/708.74 10yr/0.34/0.00/709.94 25yr/0.41/0.00/710.66 100yr/0.47/0.00/711.12

#### DMA (A3-Post) Routing into Tank 'B':

The westerly half of main house roof drainage and minor surrounding hardscape will be conveyed into SCM#2. Per hydrology study routing results, there will be full retention of all storm events with zero release. Full storage volume in this tank will infiltrate into the native soils under 48 hours using the lowest percolation rate of 3.24in/hr provided by the soils engineer, which is further reduced by factor of safety 2.

#### **Routing Summary DMA (A3-Post):**

Event/Post Project Q /Routed Release/ SCM Storage Elev. 2yr/0.18/0.00/703.08 10yr/0.29/0.00/704.19 25yr/0.36/0.00/704.84 100yr/0.41/0.00/705.31

#### **Overall Site Mitigation Summary:**

*Project will fully mitigate the development by reducing post-project flows to at or below pre-project levels per summary below:* 

| Pre-Project Peak Qs |                  | Post Project Peak Qs   |                             |
|---------------------|------------------|------------------------|-----------------------------|
| Event               | A1 Pre-Project Q | A1a + A1b + A2routed F | Release + A3 routed Release |
| 2yr                 | 3.38             | 2.96 + 0.32 + 0 + 0    | = 3.28 ok                   |
| 10yr                | 5.47             | 4.85 + 0.52 + 0 + 0    | = 5.37 ok                   |
| 25yr                | 6.66             | 5.97 + 0.64 + 0 + 0    | = 6.61 ok                   |
| 100yr               | 7.61             | 6.71 + 0.72 + 0 +0     | = 7.43 ok                   |

#### **COUNTY OF SANTA CLARA** Department of Planning and Development

County Government Center, East Wing 70 West Hedding Street, 7<sup>th</sup> Floor San Jose, California 95110



### **CLEAN WATER QUESTIONNAIRE**

#### Which Projects Must Comply with Stormwater Requirements? (READ THIS FIRST)

All projects that create and/or replace 10,000 sq. ft. or more of impervious surface on the project site must fill out this worksheet and submit it with the development project application.

All restaurants, auto service facilities, retail gasoline outlets, and uncovered parking lot projects (stand-alone or part of another development project, including the top uncovered portion of parking structures) that create and/or replace 5,000 sq. ft. or more of impervious surface on the project site must also fill out this worksheet.

Interior remodeling projects, routine maintenance or repair projects such as re-roofing and re-paving, and single family homes that are not part of a larger plan of development are **NOT** required to complete this worksheet.

#### What is an Impervious Surface?

An impervious surface is a surface covering or pavement that prevents the land's natural ability to absorb and infiltrate rainfall/stormwater. Impervious surfaces include, but are not limited to rooftops, walkways, paved patios, driveways, parking lots, storage areas, impervious concrete and asphalt, and any other continuous watertight pavement or covering. Pervious pavement, underlain with pervious soil or pervious storage material (e.g., drain rock), that infiltrates rainfall at a rate equal to or greater than surrounding unpaved areas OR that stores and infiltrates the water quality design volume specified in Provision C.3.d of the Municipal Regional Stormwater Permit (MRP), is not considered an impervious surface.

#### **For More Information**

For more information regarding selection of Best Management Practices for stormwater pollution prevention or stormwater treatment contact the Development Services Office: (408) 299-5770

| 1. Project Information   |
|--|
| Project Name: Hicks, LLC APN # 575-11-009 2024   |
| Project Address: 21631 Hicks Road  |
| Cross Streets:   |
| Applicant/Developer Name: Hicks LLC Divyogi Patel  |
| Project Phase(s): of Engineer: Harry Single, MH Engineering Co.  |
| Project Type (Check all that apply): New Development Redevelopment                                       |
|  |
| Residential Commercial Industrial Mixed Use Institutional  |
| Restaurant Uncovered Parking Retail Gas Outlet Auto Service (SIC code)                                   |
| Other (5013-5014, 5541, 7532-7534, 7536-7539)  |
| Project Description: Denislish two old homes (one legal one illegal)<br>and build 7,8205 f house and ADY |

Project Watershed/Receiving Water (creek, river or bay): Choose from list

#### 2. Project Size

| a. Total Site Area: 1416  | acres  | <b>b.</b> Total Site Ar<br>(including clearing            | ea Disturbed:<br>g, grading, or excavat                   | 1.01<br>ing)  | acres  |
|---|--|---|---|---|--|
| Site Totals   | Total Existing<br>(Pre-project)<br>Area (ft <sup>2</sup> ) | Existing Area<br>Retained <sup>1</sup> (ft <sup>2</sup> ) | Existing Area<br>Replaced <sup>2</sup> (ft <sup>2</sup> ) | New Area<br>Created <sup>2</sup> (ft <sup>2</sup> ) | Total Post-<br>Project Area (ft <sup>2</sup> ) |
| c. Total Impervious Area (IA)   | 11,492   | ¢.  | 1,492   | 20,950  | 20950  |
| d. Total new and replaced impervious area   |  | 1   |   |   | 20 950 (0.48)                                  |
| e. Total Pervious Area (PA) <sup>3</sup>  | 23.83 au   |   |   |   | 13,6800  |
| f. Total Area (IA+PA)   | 24.16ac.   |   |   |   | 47.00 00                                       |
| g. Percent Replacement of IA in Redevelopment Projects: (Existing IA Replaced ÷ Existing Total IA) x 100% 100 % |  |   |   |   |  |

#### 3. State Construction General Permit Applicability:

a. Is #2.b. equal to 1 acre or more?

- Yes, applicant must obtain coverage under the State Construction General Permit (see https://www.waterboards.ca.gov/water\_issues/programs/stormwater/construction.html)
- No, applicant does not need coverage under the State Construction General Permit.

### 4. MRP Provision C.3 Applicability:

a. Is #2.d. equal to 10,000 sq. ft. or more, or 5,000 sq. ft. or more for restaurants, auto service facilities, retail gas outlets, and stand-alone uncovered parking?

Yes, C.3. source control, site design and treatment requirements apply

- No, C.3. source control and site design requirements may apply check with local agency
- b. For redevelopment projects, is #2.g. equal to 50% or more?

Yes, C.3. requirements (site design and source control, as appropriate, and stormwater treatment) apply to the entire site

No, C.3. requirements only apply to the impervious area created and/or replaced

- c. Does the project create and/or replace 5,000 sf or more of impervious surface parking?
  - Yes, C.3. requirements may apply to the entire site check with local agency

### 5. Hydromodification Management (HM) Applicability:

- a. Does the project create and/or replace one acre or more of impervious surface AND is the total post-project impervious area greater than the pre-project (existing) impervious area? ☐ Yes (continue)
  - $\square$  No exempt from HM, go to page 3
- b. Is the project located in an area of HM applicability (green area) on the HM Applicability Map? (<u>www.scvurppp.org/hmp-map</u>)
  - Yes, the project must implement HM requirements
  - □ No, the project is exempt from HM requirements

<sup>1 &</sup>quot;Retained" means to leave existing IA in place. An IA that goes through maintenance (e.g., pavement resurfacing/slurry seal/grind), but no change in grade is considered "retained".

<sup>&</sup>lt;sup>2</sup> The "new" and "replaced" IA are based on the total area of the site and not specific locations on site. For example, impervious parking created over a pervious area is not "new" IA, if an equal amount of pervious area replaces IA somewhere else on the site. Constructed IA on a site that does not exceed the total pre-project IA will be considered "replaced" IA. A site will have "new"

IA only if the total post-project IA exceeds the total pre-project IA (total post-project IA - total pre-project IA = New IA).

### 6. Selection of Specific Stormwater Control Measures:

| 1  |   |   |  |  |  |  |
|--|---|---|--|--|--|--|
| Site Design Measures   | Source Control Measures                               | Treatment Measures                                      |  |  |  |  |
| Minimize land disturbed<br>(e.g., protect trees and soil)  | Wash area/racks, drain to sanitary sewer <sup>5</sup> | None (all impervious surface drains to self-            |  |  |  |  |
| Minimize impervious  | Covered dumpster area,                                | retaining areas)  |  |  |  |  |
| surfaces (e.g., reduction in   | drain to sanitary sewer <sup>6</sup>                  | LID Treatment   |  |  |  |  |
| surface)   | Sanitary sewer connection                             | Bioretention area                                       |  |  |  |  |
| Minimum-impact street or   | or accessible cleanout for                            | Flow-through planter                                    |  |  |  |  |
| parking lot design (e.g.,<br>parking on top of or under  | pool/spa/fountain <sup>6</sup>                        | Tree Well Filter or Trench<br>with bioretention soils   |  |  |  |  |
| buildings)   | Beneficial landscaping                                | Rainwater harvest/use (e.g.,                            |  |  |  |  |
| Cluster structures/ pavement   | pesticides and fertilizers;<br>promotes treatment)    | cistern or rain barrel for<br>designated use, sized for |  |  |  |  |
| Disconnected downspouts  | Outdoor material storage                              | Lafitration transh                                      |  |  |  |  |
| sidewalks, patios to   | protection  |   |  |  |  |  |
| landscaped areas)  | Covers, drains for loading                            | Subsurface Infiltration                                 |  |  |  |  |
| Pervious pavement  | docks, maintenance bays,                              | System (e.g. vault or large                             |  |  |  |  |
| Green roof   | fueling areas   | diameter conduit over drain                             |  |  |  |  |
| Other self-treating <sup>4</sup> area  | Maintenance (pavement                                 | Other marker  |  |  |  |  |
| (e.g., landscaped areas)   | cleaning, good  | - Other Rain-Tanks                                      |  |  |  |  |
| Self-retaining <sup>4</sup> area   | housekeeping)   | Non-LID Treatment Methods                               |  |  |  |  |
| Interceptor trees <sup>3</sup>   | Storm drain labeling                                  | Proprietary high flow rate                              |  |  |  |  |
| Rainwater harvesting and   | Other   | tree box filter <sup>7</sup>                            |  |  |  |  |
| use (e.g., rain barrel, cistern for designated use) <sup>5</sup>   |   | Proprietary high flow media                             |  |  |  |  |
|  |   | proprietary media) <sup>7</sup>                         |  |  |  |  |
| 23,83 ac. or sq. ft. (circle   |   | Vegetated filter strip <sup>8</sup>                     |  |  |  |  |
| one)   |   | Extended detention basin <sup>8</sup>                   |  |  |  |  |
| Protected riparian and   | 1   | Vegetated swale <sup>8</sup>                            |  |  |  |  |
| wetland areas/buffers  | J   | Other   |  |  |  |  |
| (Setback from top of bank:<br>ft.)   |   |   |  |  |  |  |
|  |   |   |  |  |  |  |
|  |   |   |  |  |  |  |
| Flow Duration Controls for Hydromodification Management (HM)   |   |   |  |  |  |  |
|  |   |   |  |  |  |  |
| Detention basin vault  | Sound tank or Bioretention with out                   | tlet Other  |  |  |  |  |
| · · · · · · · · · · · · · · · · · · ·  |   |   |  |  |  |  |
| <sup>4</sup> See SCVURPPP C3 Handbook for def  | initions. https://scvurppp.org/2016/06/20/c-3-stc     | rmwater-handbook-june-2016/                             |  |  |  |  |
| <sup>o</sup> Optional site design measure; does not<br><sup>6</sup> Subject to sanitary sewer authority requ | have to be sized to comply with Provision C           | 2.3.d treatment requirements.                           |  |  |  |  |
| <sup>7</sup> These treatment measures are only allowed if the project qualifies as a "Special Project".      |   |   |  |  |  |  |

<sup>8</sup> These treatment measures are only allowed as part of a multi-step treatment process (i.e., for pretreatment).

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7. Stormwater Treatment Measure (STM) Sizing for Projects with Treatment Requirements

| Stormwater Treatment Measure (STM) | Hydraulic Sizing Criteria Used* |
|------------------------------------|---------------------------------|
|                                    | Choose from list                |

#### \*Key: 1a: Volume – WEF Method

1b: Volume - CASQA BMP Handbook Method

2a: Flow – Factored Flood Flow Method

2b: Flow - CASQA BMP Handbook Method

2c: Flow - Uniform Intensity Method

3: Combination Flow and Volume Design Basis

8. Alternative Certification: Was the treatment system sizing and design reviewed by a qualified thirdparty professional that is not a member of the project team or agency staff?

Yes No Name of Third-party Reviewer\_\_\_\_

#### 9. Operation & Maintenance Information

A. Property Owner's Name

B. Responsible Party for Stormwater Treatment/Hydromodification Control O&M:

- a. Name:
- b. Address:
- c. Phone/E-mail:

#### DEPT. OF PLANNING AND DEVELOPMENT, LDE STAFF USE ONLY

| Questionnaire reviewed:   |  |
|---|--|
| LDE:  |  |
| Date  |  |
| Project Watershed   |  |
| Monterey Bay  |  |
| Project Category (check one):<br>Regulated project<br>Regulated + HMP project<br>Exempt |  |
| O&M Responsibility Mechanism  |  |
| O&M Agreement   |  |
| Other mechanism that assigns responsibility (describe below):                           |  |
| Send copy of final form to Clean Water Program - CleanwaterSCC@cep.sccgov.org           |  |

## ASSOCIATED TERRA CONSULTANTS, Inc.

ENGINEERING GEOLOGY GEOTECHNICAL HYDROGEOLOGY SEPTIC TESTING & DESIGN

June 17, 2022 File No: 260411 L2

Hicks LLC 3 Oakhurst Place Santa Rosa, CA 95409

Subject: VERIFICATION OF INFILTRATOR TESTING RESULTS APN: 575-11-009; 21631 Hicks Road Santa Clara County, California

Dear Hicks LLC:

This letter verifies infiltrator testing results conducted at the subject property, on November 19, 2021, for proposed infiltrator trenches. These tests were set up and conducted in accordance with the County standards, including the pre-soak procedure. *Associated Terra Consultants, Inc.* personnel personally observed the site and conducted all the testing.

We tested two percolation holes at the requested depth of six feet deep. The average yielded result is 18.5 MPI (mins/inch).

We are pleased to have been of service to you at this time. Please do not hesitate to call us if you have any questions regarding this project, or if we can be of any other service. Thank you.

Very truly yours, ASSOCIATED TERRA CONSULTANTS, Inc.



1725 Dell Avenue, Campbell, California 95008 (408) 866-1067 Email: Office@aterracon.com





| Project Impervious Area Table |                            |  |  |  |
|-------------------------------|----------------------------|--|--|--|
| me (APN)                      | Hicks Land LLC (575-11-024 |  |  |  |
| n Submittal Date              | 12/1/2021                  |  |  |  |
| cation                        | 216131 Hicks Road          |  |  |  |
| ase                           | N/A                        |  |  |  |
| be and Description            | Residential                |  |  |  |
| ct Area                       | 24.16ac                    |  |  |  |
| Impervious Area               |                            |  |  |  |
| 1, walks & deck 1591,         | 20,950sf                   |  |  |  |
| ,948)                         |                            |  |  |  |
| Project Impervious Area       | 11,492sf                   |  |  |  |
| aced Impervious Area          | 11,492sf                   |  |  |  |
| Project Impervious Area       | 20,950sf                   |  |  |  |
| ious Area                     | 20,950sf                   |  |  |  |
|                               |                            |  |  |  |



# **MH engineering Co.**

16075 Vineyard Blvd. Morgan Hill, CA 95037 (408) 779-7381 (408) 226-5712 Fax

> Hicks, LLC Hydrology Study (Peak Management)

Accas, time of concentration & hundit loeff 6. The = 0.0078 5 = Ageks Rd. ditch Pre, A=6.26ac+ 10+ 0.0078/1923 0.385 natural(0:30 +0.15+0.15+0.15)=0.75; Cpre.(composit) = <u>6(0.75)+0.26/0.90</u>) A 1 a-lost h= 5. 22 ac.; C= 0.75, Tc 2/3min = 0.76 t Headwall@ Turn troound > Alb-lost A= 0.5 dae; Te=13min to roadaide ditch C= 0.11(0.90)+0.43(0.75) Hoks Load C= 0.11(0.90)+0.43(0.75) 0.54 =0.78 > A2-post to Rain-tank A': Area = 0.3[ac; Tc = 13min C = (0.08) 0.75 + 0.23(0.90) = 0.880.31 A3- Post to Rain-tank TS; Area = 0. 30ac, Te = 15min C = 0.16(0.75) + 0.14(0.90) = 0.82

Eco Rain-tank Volume Calc, Volume = (9x8)2x4.41 = 635 ft<sup>3</sup> Tauk A' Volume to/ voids Rock volume @ Drain (22.25+ 10.72) 4.39 ×14 Totalvol. =757 & finished brade R1M714 TA 8 porter INV.711.30 3.5 1001/10 Tep711.30 TU T X Rain-Tan 142 units \*2 1' 4:39 : Bottom 70691 10V.707.66 6°0°r V 12 thry , Unit size Capacity ON. 50/0 = 6465 171.34 120670 > 2:25-22.25+12.72 7 - B volume Tank Wrated Rim 708,80 NG708.20 38 00000 2.89 +4 - uni INV. 702.06 2 bottom 701.3 - 751 ft ? calc. Tark A above Nolume = 75 9 (see

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 1

A1-Pre

| Hydrograph type | = Mod. Rational | Peak discharge     | = 3.378 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 2 yrs         | Time to peak       | = 15 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 6,080 cuft |
| Drainage area   | = 6.260 ac      | Runoff coeff.      | = 0.76*      |
| Intensity       | = 0.710 in/hr   | Tc by User         | = 15.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 1

A1-Pre

| Hydrograph type | = Mod. Rational | Peak discharge     | = 5.471 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 10 yrs        | Time to peak       | = 15 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 9,848 cuft |
| Drainage area   | = 6.260 ac      | Runoff coeff.      | = 0.76*      |
| Intensity       | = 1.150 in/hr   | Tc by User         | = 15.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 1

A1-Pre

| Hydrograph type | = Mod. Rational | Peak discharge     | = 6.661 cfs   |
|-----------------|-----------------|--------------------|---------------|
| Storm frequency | = 25 yrs        | Time to peak       | = 15 min      |
| Time interval   | = 1 min         | Hyd. volume        | = 11,989 cuft |
| Drainage area   | = 6.260 ac      | Runoff coeff.      | = 0.76*       |
| Intensity       | = 1.400 in/hr   | Tc by User         | = 15.00 min   |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc    |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a          |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 1

A1-Pre

| Hydrograph type | = Mod. Rational | Peak discharge     | = 7.612 cfs   |
|-----------------|-----------------|--------------------|---------------|
| Storm frequency | = 100 yrs       | Time to peak       | = 15 min      |
| Time interval   | = 1 min         | Hyd. volume        | = 13,702 cuft |
| Drainage area   | = 6.260 ac      | Runoff coeff.      | = 0.76*       |
| Intensity       | = 1.600 in/hr   | Tc by User         | = 15.00 min   |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc    |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a          |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 12 / 7 / 2021

### Hyd. No. 6

| Hydrograph type | = Mod. Rational | Peak discharge     | = 2.964 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 2 yrs         | Time to peak       | = 13 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 4,624 cuft |
| Drainage area   | = 5.220 ac      | Runoff coeff.      | = 0.75       |
| Intensity       | = 0.757 in/hr   | Tc by User         | = 13.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 12 / 7 / 2021

### Hyd. No. 6

| Hydrograph type | = Mod. Rational | Peak discharge     | = 4.852 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 10 yrs        | Time to peak       | = 13 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 7,569 cuft |
| Drainage area   | = 5.220 ac      | Runoff coeff.      | = 0.75       |
| Intensity       | = 1.239 in/hr   | Tc by User         | = 13.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 12 / 7 / 2021

### Hyd. No. 6

| Hydrograph type | = Mod. Rational | Peak discharge     | = 5.974 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 25 yrs        | Time to peak       | = 13 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 9,319 cuft |
| Drainage area   | = 5.220 ac      | Runoff coeff.      | = 0.75       |
| Intensity       | = 1.526 in/hr   | Tc by User         | = 13.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 12 / 7 / 2021

### Hyd. No. 6

| Hydrograph type | = Mod. Rational | Peak discharge     | = 6.705 cfs   |
|-----------------|-----------------|--------------------|---------------|
| Storm frequency | = 100 yrs       | Time to peak       | = 13 min      |
| Time interval   | = 1 min         | Hyd. volume        | = 10,460 cuft |
| Drainage area   | = 5.220 ac      | Runoff coeff.      | = 0.75        |
| Intensity       | = 1.713 in/hr   | Tc by User         | = 13.00 min   |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc    |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a          |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 7

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.319 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 2 yrs         | Time to peak       | = 13 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 497 cuft  |
| Drainage area   | = 0.540 ac      | Runoff coeff.      | = 0.78      |
| Intensity       | = 0.757 in/hr   | Tc by User         | = 13.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 7

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.522 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 10 yrs        | Time to peak       | = 13 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 814 cuft  |
| Drainage area   | = 0.540 ac      | Runoff coeff.      | = 0.78      |
| Intensity       | = 1.239 in/hr   | Tc by User         | = 13.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 7

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.643 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 25 yrs        | Time to peak       | = 13 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 1,003 cuft |
| Drainage area   | = 0.540 ac      | Runoff coeff.      | = 0.78       |
| Intensity       | = 1.526 in/hr   | Tc by User         | = 13.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 7

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.721 cfs  |
|-----------------|-----------------|--------------------|--------------|
| Storm frequency | = 100 yrs       | Time to peak       | = 13 min     |
| Time interval   | = 1 min         | Hyd. volume        | = 1,125 cuft |
| Drainage area   | = 0.540 ac      | Runoff coeff.      | = 0.78       |
| Intensity       | = 1.713 in/hr   | Tc by User         | = 13.00 min  |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc   |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a         |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 2

A2-Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.207 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 2 yrs         | Time to peak       | = 13 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 322 cuft  |
| Drainage area   | = 0.310 ac      | Runoff coeff.      | = 0.88*     |
| Intensity       | = 0.757 in/hr   | Tc by User         | = 13.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 2

A2-Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.338 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 10 yrs        | Time to peak       | = 13 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 527 cuft  |
| Drainage area   | = 0.310 ac      | Runoff coeff.      | = 0.88*     |
| Intensity       | = 1.239 in/hr   | Tc by User         | = 13.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 2

A2-Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.416 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 25 yrs        | Time to peak       | = 13 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 649 cuft  |
| Drainage area   | = 0.310 ac      | Runoff coeff.      | = 0.88*     |
| Intensity       | = 1.526 in/hr   | Tc by User         | = 13.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 2

A2-Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.467 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 100 yrs       | Time to peak       | = 13 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 729 cuft  |
| Drainage area   | = 0.310 ac      | Runoff coeff.      | = 0.88*     |
| Intensity       | = 1.713 in/hr   | Tc by User         | = 13.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 3

Eco Rain-Tank 'A'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 2 yrs               | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 2 - A2-Post Routing | Max. Elevation | = 708.74 ft |
| Reservoir name  | = Eco Rain-Tank       | Max. Storage   | = 322 cuft  |



## **Pond Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

#### Pond No. 1 - Eco Rain-Tank

#### **Pond Data**

Pond storage is based on user-defined values.

#### Stage / Storage Table

| Stage (ft) Elevation (ft) |        | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |  |
|---------------------------|--------|---------------------|----------------------|----------------------|--|
| 0.00                      | 706.91 | n/a                 | 0                    | 0                    |  |
| 1.00                      | 707.86 | n/a                 | 171                  | 171                  |  |
| 2.00                      | 708.86 | n/a                 | 171                  | 342                  |  |
| 3.00                      | 709.86 | n/a                 | 171                  | 513                  |  |
| 4.00                      | 710.86 | n/a                 | 171                  | 684                  |  |
| 4.39                      | 711.30 | n/a                 | 67                   | 751                  |  |

#### **Culvert / Orifice Structures**

|                 | [A]      | [B]  | [C]  | [PrfRsr] |                | [A]         | [B]       | [C]  | [D]  |
|-----------------|----------|------|------|----------|----------------|-------------|-----------|------|------|
| Rise (in)       | = 4.00   | 0.00 | 0.00 | 0.00     | Crest Len (ft) | = 0.00      | 0.00      | 0.00 | 0.00 |
| Span (in)       | = 4.00   | 0.00 | 0.00 | 0.00     | Crest El. (ft) | = 0.00      | 0.00      | 0.00 | 0.00 |
| No. Barrels     | = 1      | 0    | 0    | 0        | Weir Coeff.    | = 2.60      | 3.33      | 3.33 | 3.33 |
| Invert El. (ft) | = 711.30 | 0.00 | 0.00 | 0.00     | Weir Type      | = Broad     | Broad     |      |      |
| Length (ft)     | = 13.00  | 0.00 | 0.00 | 0.00     | Multi-Stage    | = No        | No        | No   | No   |
| Slope (%)       | = 4.61   | 0.00 | 0.00 | n/a      | •              |             |           |      |      |
| N-Value         | = .013   | .013 | .013 | n/a      |                |             |           |      |      |
| Orifice Coeff.  | = 0.60   | 0.60 | 0.60 | 0.60     | Exfil.(in/hr)  | = 0.000 (by | Wet area) |      |      |
| Multi-Stage     | = n/a    | No   | No   | No       | TW Elev. (ft)  | = 0.00      | ,         |      |      |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

**Weir Structures** 



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 3

Eco Rain-Tank 'A'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 10 yrs              | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 2 - A2-Post Routing | Max. Elevation | = 709.94 ft |
| Reservoir name  | = Eco Rain-Tank       | Max. Storage   | = 527 cuft  |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 3

Eco Rain-Tank 'A'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 25 yrs              | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 2 - A2-Post Routing | Max. Elevation | = 710.66 ft |
| Reservoir name  | = Eco Rain-Tank       | Max. Storage   | = 649 cuft  |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 3

Eco Rain-Tank 'A'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 100 yrs             | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 2 - A2-Post Routing | Max. Elevation | = 711.12 ft |
| Reservoir name  | = Eco Rain-Tank       | Max. Storage   | = 729 cuft  |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

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### Hyd. No. 4

A3 Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.180 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 2 yrs         | Time to peak       | = 14 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 303 cuft  |
| Drainage area   | = 0.300 ac      | Runoff coeff.      | = 0.82*     |
| Intensity       | = 0.732 in/hr   | Tc by User         | = 14.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 4

A3 Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.293 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 10 yrs        | Time to peak       | = 14 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 493 cuft  |
| Drainage area   | = 0.300 ac      | Runoff coeff.      | = 0.82*     |
| Intensity       | = 1.192 in/hr   | Tc by User         | = 14.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

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### Hyd. No. 4

A3 Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.359 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 25 yrs        | Time to peak       | = 14 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 603 cuft  |
| Drainage area   | = 0.300 ac      | Runoff coeff.      | = 0.82*     |
| Intensity       | = 1.460 in/hr   | Tc by User         | = 14.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

### Hyd. No. 4

A3 Post Routing

| Hydrograph type | = Mod. Rational | Peak discharge     | = 0.407 cfs |
|-----------------|-----------------|--------------------|-------------|
| Storm frequency | = 100 yrs       | Time to peak       | = 14 min    |
| Time interval   | = 1 min         | Hyd. volume        | = 683 cuft  |
| Drainage area   | = 0.300 ac      | Runoff coeff.      | = 0.82*     |
| Intensity       | = 1.653 in/hr   | Tc by User         | = 14.00 min |
| IDF Curve       | = SCC-25in.IDF  | Storm duration     | = 2.0 x Tc  |
| Target Q        | =n/a            | Est. Req'd Storage | =n/a        |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

### Hyd. No. 5

Eco Rain-tank 'B'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 2 yrs               | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 4 - A3 Post Routing | Max. Elevation | = 703.08 ft |
| Reservoir name  | = Eco Rain-tank 'B'   | Max. Storage   | = 303 cuft  |

Storage Indication method used.



Tuesday, 06 / 21 / 2022

## **Pond Report**

Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Tuesday, 06 / 21 / 2022

#### Pond No. 2 - Eco Rain-tank 'B'

#### **Pond Data**

Pond storage is based on user-defined values.

#### Stage / Storage Table

| Stage (ft) Elevation (ft) |        | Contour area (sqft) | Incr. Storage (cuft) | Total storage (cuft) |  |
|---------------------------|--------|---------------------|----------------------|----------------------|--|
| 0.00                      | 701.31 | n/a                 | 0                    | 0                    |  |
| 1.00                      | 702.31 | n/a                 | 171                  | 171                  |  |
| 2.00                      | 703.31 | n/a                 | 171                  | 342                  |  |
| 3.00                      | 704.31 | n/a                 | 171                  | 513                  |  |
| 4.00                      | 705.31 | n/a                 | 171                  | 684                  |  |
| 4.39                      | 705.70 | n/a                 | 71                   | 755                  |  |

#### **Culvert / Orifice Structures**

|                 | [A]    | [B]  | [C]  | [PrfRsr] |                | [A]         | [B]       | [C]  | [D]  |
|-----------------|--------|------|------|----------|----------------|-------------|-----------|------|------|
| Rise (in)       | = 0.00 | 0.00 | 0.00 | 0.00     | Crest Len (ft) | = 0.00      | 0.00      | 0.00 | 0.00 |
| Span (in)       | = 0.00 | 0.00 | 0.00 | 0.00     | Crest El. (ft) | = 708.20    | 0.00      | 0.00 | 0.00 |
| No. Barrels     | = 1    | 0    | 0    | 0        | Weir Coeff.    | = 2.60      | 3.33      | 3.33 | 3.33 |
| Invert El. (ft) | = 0.00 | 0.00 | 0.00 | 0.00     | Weir Type      | = Broad     |           |      |      |
| Length (ft)     | = 0.00 | 0.00 | 0.00 | 0.00     | Multi-Stage    | = No        | No        | No   | No   |
| Slope (%)       | = 0.00 | 0.00 | 0.00 | n/a      | -              |             |           |      |      |
| N-Value         | = .013 | .013 | .013 | n/a      |                |             |           |      |      |
| Orifice Coeff.  | = 0.60 | 0.60 | 0.60 | 0.60     | Exfil.(in/hr)  | = 0.000 (by | Wet area) | )    |      |
| Multi-Stage     | = n/a  | No   | No   | No       | TW Elev. (ft)  | = 0.00      |           |      |      |

Note: Culvert/Orifice outflows are analyzed under inlet (ic) and outlet (oc) control. Weir risers checked for orifice conditions (ic) and submergence (s).

**Weir Structures** 



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

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### Hyd. No. 5

Eco Rain-tank 'B'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 10 yrs              | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 4 - A3 Post Routing | Max. Elevation | = 704.19 ft |
| Reservoir name  | = Eco Rain-tank 'B'   | Max. Storage   | = 493 cuft  |



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#### Tuesday, 06 / 21 / 2022

### Hyd. No. 5

Eco Rain-tank 'B'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 25 yrs              | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 4 - A3 Post Routing | Max. Elevation | = 704.84 ft |
| Reservoir name  | = Eco Rain-tank 'B'   | Max. Storage   | = 603 cuft  |



Hydraflow Hydrographs Extension for Autodesk® Civil 3D® by Autodesk, Inc. v2020.4

Hyd. No. 5

Eco Rain-tank 'B'

| Hydrograph type | = Reservoir           | Peak discharge | = 0.000 cfs |
|-----------------|-----------------------|----------------|-------------|
| Storm frequency | = 100 yrs             | Time to peak   | = n/a       |
| Time interval   | = 1 min               | Hyd. volume    | = 0 cuft    |
| Inflow hyd. No. | = 4 - A3 Post Routing | Max. Elevation | = 705.31 ft |
| Reservoir name  | = Eco Rain-tank 'B'   | Max. Storage   | = 683 cuft  |

Storage Indication method used.



Tuesday, 06 / 21 / 2022