

COUNTY OF SANTA CLARA

General Construction Specifications

GENERAL CONDITIONS

- 1. ALL CONSTRUCTION WORK SHALL BE PERFORMED IN ACCORDANCE WITH THE SOILS AND/OR GEOTECHNICAL STUDY PREPARED BY G2EARTH AND DATED FEBRUARY 22, 2021. THIS REPORT IS SUPPLEMENTED BY: 1) THESE PLANS AND SPECIFICATIONS, 2) THE COUNTY OF SANTA CLARA STANDARD DETAILS...

GRADING

- 1. EXCAVATED MATERIAL SHALL BE PLACED IN THE FILL AREAS DESIGNATED OR SHALL BE HAULED AWAY FROM THE SITE TO A COUNTY APPROVED DISPOSAL SITE. WHERE FILL MATERIAL IS TO BE PLACED ON NATURAL GROUND, IT SHALL BE STRIPPED OF ALL VEGETATION TO ACHIEVE A PROPER BOND WITH THE FILL MATERIAL...

Table with columns: LOCATION, STORAGE BUILDING, DWELLING UNIT, HARDSCAPE, LANDSCAPE, DRIVEWAY/TURNOUTS, UTILITY, IMPROVEMENTS, TOTAL. Rows show quantities for CUT (C.Y.) and FILL (C.Y.) and VERT. DEPTH.

NOTE: FILL VOLUMES INCLUDE 10% SHRINKAGE.

- EXCESSIVE DIRT/MATERIAL SHALL BE REMOVED AND HAULED TO A COUNTY APPROVED DISPOSAL SITE. 7. NOTIFY SOILS ENGINEER TWO (2) DAYS PRIOR TO COMMENCEMENT OF ANY GRADING WORK TO COORDINATE THE WORK IN THE FIELD...

CONSTRUCTION STAKING

- 1. THE DEVELOPER'S ENGINEER IS RESPONSIBLE FOR THE INITIAL PLACEMENT AND REPLACEMENT OF CONSTRUCTION GRADE STAKES. THE STAKES ARE TO BE ADEQUATELY IDENTIFIED, LOCATED, STABILIZED, ETC. FOR THE CONVENIENCE OF CONTRACTORS...

CONSTRUCTION INSPECTION

- 1. CONTRACTOR SHALL NOTIFY PERMIT INSPECTION UNIT, SANTA CLARA COUNTY PRIOR TO COMMENCING WORK AND FOR FINAL INSPECTION OF WORK AND SITE. THE COUNTY REQUIRES A MINIMUM OF 24 HOURS ADVANCE NOTICE FOR GENERAL INSPECTION...

SITE PREPARATION (CLEARING AND GRUBBING)

- 1. EXISTING TREES AUTHORIZED FOR REMOVAL, ROOTS, AND FOREIGN MATERIAL IN AREAS TO BE IMPROVED WILL BE REMOVED TO AN AUTHORIZED DISPOSAL SITE AS FOLLOWS: A) TO A MINIMUM DEPTH OF TWO FEET BELOW THE FINISHED GRADE OF PROPOSED ROADWAYS...

UTILITY LOCATION, TRENCHING & BACKFILL

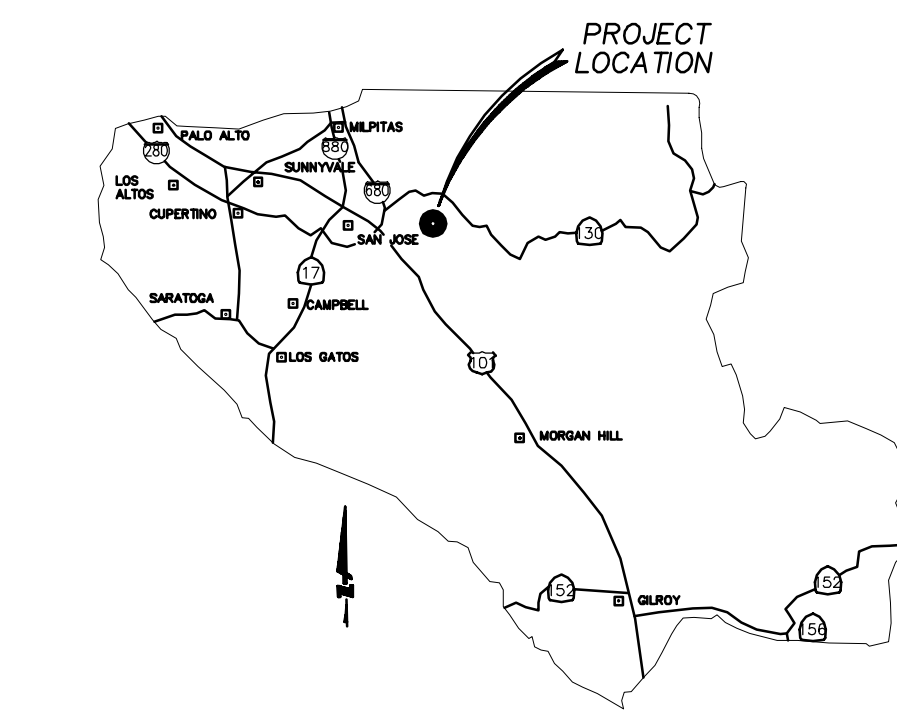
- 1. CONTRACTOR SHALL NOTIFY USA (UNDERGROUND SERVICE ALERT) AT 1-800-277-2600 A MINIMUM OF 24 HOURS BEFORE BEGINNING UNDERGROUND WORK FOR VERIFICATION OF THE LOCATION OF UNDERGROUND UTILITIES...

RETAINING WALLS

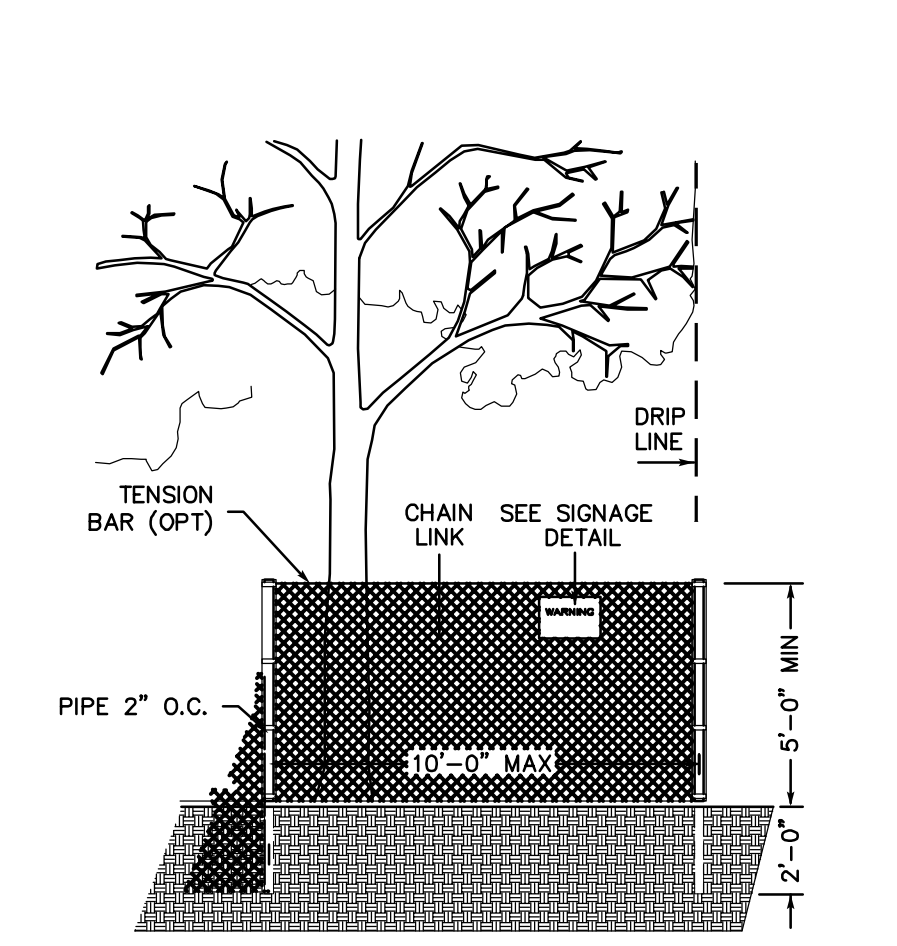
- 1. REINFORCED CONCRETE AND CONCRETE MASONRY UNIT RETAINING WALLS SHALL HAVE FOUNDATION AND REINFORCEMENT INSPECTED BY THE COUNTY ENGINEERING INSPECTOR AND ENGINEER OF RECORD PRIOR TO POURING THE FOUNDATION AND FORMING THE WALL...

AIR QUALITY, LANDSCAPING AND EROSION CONTROL

- 1. WATER ALL ACTIVE CONSTRUCTION AREAS AT LEAST TWICE DAILY. 2. COVER ALL TRUCKS HAULING SOIL, SAND, AND OTHER LOOSE MATERIALS OR REQUIRE ALL TRUCKS TO MAINTAIN AT LEAST TWO FEET OF FREEBOARD...



COUNTY LOCATION MAP



EXISTING TREE PROTECTION DETAILS

- 1. FOR ALL TREES TO BE RETAINED WITH A CANOPY IN THE DEVELOPMENT AREA OR INTERFACES WITH THE LIMITS OF GRADING FOR ALL PROPOSED DEVELOPMENT ON SITE, THE TREES SHALL BE PROTECTED BY THE PLACEMENT OF RIGID TREE PROTECTIVE FENCING...

STORM DRAINAGE AND STORMWATER MANAGEMENT

- 1. DEVELOPER IS RESPONSIBLE FOR ALL NECESSARY DRAINAGE FACILITIES WHETHER SHOWN ON THE PLANS OR NOT. THE DEVELOPER OR HIS SUCCESSOR PROPERTY OWNERS ARE RESPONSIBLE FOR THE ADEQUACY AND CONTINUED MAINTENANCE OF THESE FACILITIES...

AS-BUILT PLANS STATEMENT

THIS IS A TRUE COPY OF THE AS-BUILT PLANS. THERE () WERE () () WERE NOT) MINOR FIELD CHANGES - MARKED WITH THE SYMBOL (?). THERE () WERE () () WERE NOT) PLAN REVISIONS INDICATING SIGNIFICANT CHANGES REVIEWED BY THE COUNTY ENGINEER AND MARKED WITH THE SYMBOL (?).

STREET LIGHTING

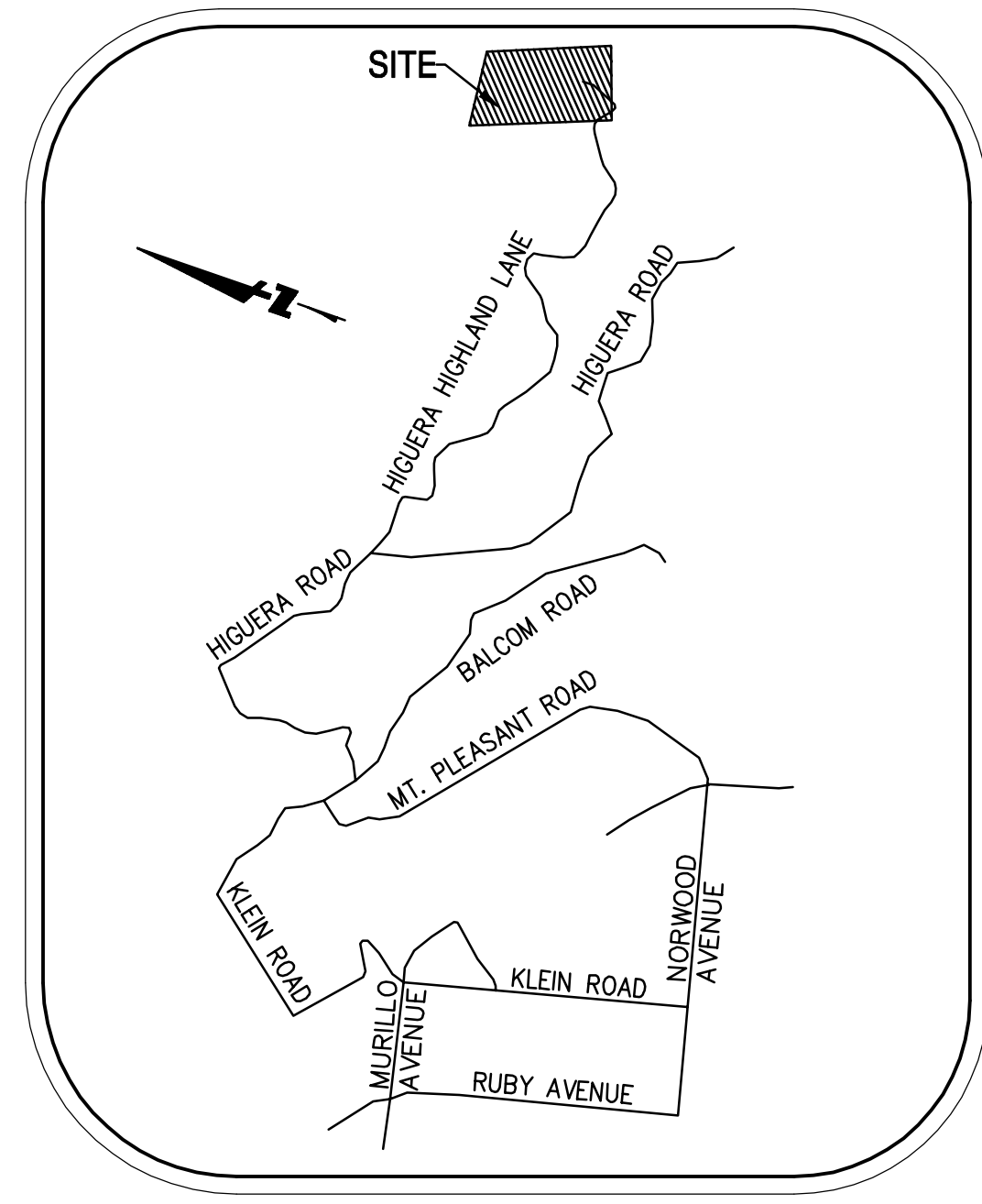
- 1. PACIFIC GAS & ELECTRIC ELECTROLUER SERVICE FEE SHALL BE PAID BY THE DEVELOPER AND/OR HIS AUTHORIZED REPRESENTATIVE.

SANITARY SEWER

- 1. THE SANITARY SEWER AND WATER UTILITIES SHOWN ON THESE PLANS ARE NOT PART OF THIS GRADING PERMIT AND ARE SHOWN FOR REFERENCE ONLY.

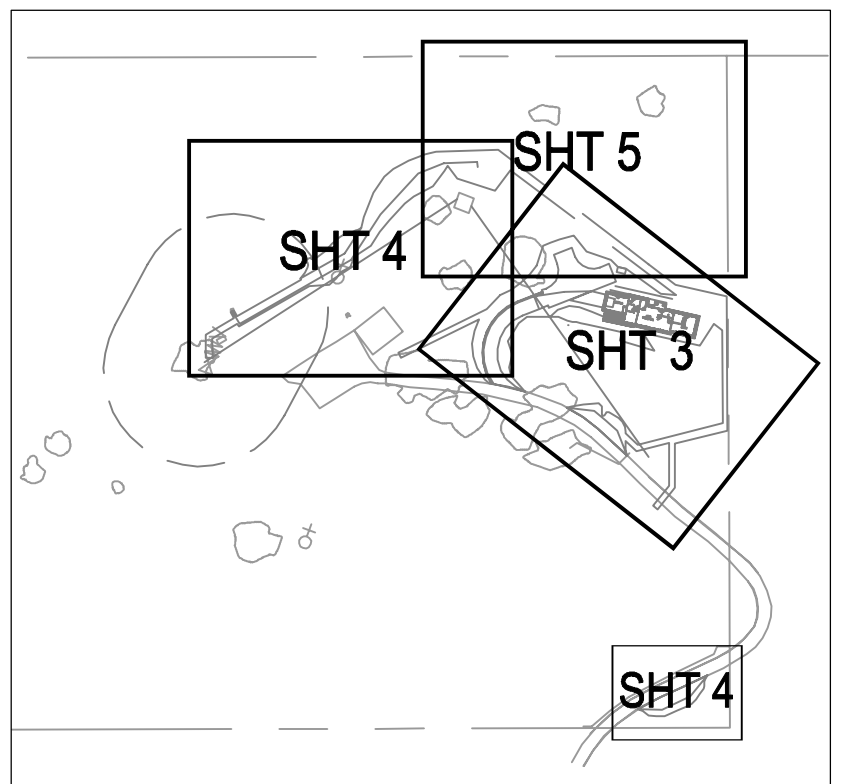
PORTLAND CEMENT CONCRETE

- 1. CONCRETE USED FOR STRUCTURAL PURPOSES SHALL BE CLASS "A" (6 SACK PER CUBIC YARD) AS SPECIFIED IN THE STATE STANDARD SPECIFICATIONS. CONCRETE PLACED MUST DEVELOP A MINIMUM STRENGTH FACTOR OF 2800 PSI IN A SEVEN-DAY PERIOD...



VICINITY MAP

CHANG RESIDENCE 4015 HIGUERA HIGHLAND LN PRIMARY RESIDENCE AND JR ADU BUILDING



SCOPE OF WORK

- 1. THE CONTRACTOR IS RESPONSIBLE FOR THE INSTALLATION OF THE WORK PROPOSED ON THE EROSION CONTROL PLAN. THE ENGINEER OF RECORD IS RESPONSIBLE FOR THE DESIGN OF THE EROSION CONTROL PLANS AND ANY MODIFICATIONS OF THE EROSION CONTROL PLANS TO PREVENT ILLICIT DISCHARGES FROM THE SITE DURING CONSTRUCTION...

SURVEY MONUMENT PRESERVATION

- 1. THE LANDOWNER / CONTRACTOR MUST PROTECT AND ENSURE THE PERPETUATION OF SURVEY MONUMENTS AFFECTED BY CONSTRUCTION ACTIVITIES. 2. PRIOR TO THE START OF CONSTRUCTION, THE CONTRACTOR SHALL LOCATE, STAKE, AND FLAG OR OTHERWISE IDENTIFY WITH PAINT OR OTHER MARKINGS ALL PERMANENT SURVEY MONUMENTS OF RECORD AND ANY UNRECORDED MONUMENTS THAT ARE DISCOVERED...

SHEET INDEX

Table with columns: Sheet No., Description (COUNTY NOTE SHEET, OVERALL SITE PLAN, GRADING AND DRAINAGE PLAN, DRIVEWAY PROFILE, SECTION & DETAILS, BMP-1, BMP-2, Higuera Highland Ln Driveway Widths), Engineer's Name (BKF ENGINEERS), Address (1730 NORTH FIRST STREET, #600 SAN JOSE, CA 95112), Phone No. (408-467-9100), Fax No. (408-467-9199), Date (OCTOBER 9, 2023), Revision 1 Date, APN (654-15-023), Co. File (PLN22-113), Sheet 1 of 8.

COUNTY OF SANTA CLARA DEPT. OF ROADS AND AIRPORTS ISSUED BY: _____ DATE: _____ ENCROACHMENT PERMIT NO. _____

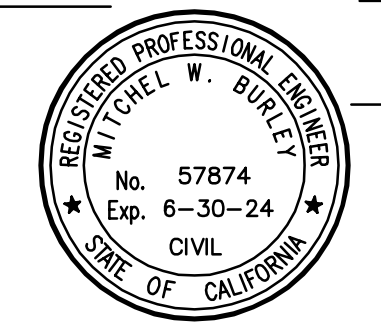
COUNTY OF SANTA CLARA LAND DEVELOPMENT ENGINEERING & SURVEYING GRADING / DRAINAGE PERMIT NO. _____ ISSUED BY: _____ DATE: _____

NO WORK SHALL BE DONE IN THE COUNTY'S RIGHT-OF-WAY WITHOUT AN ENCROACHMENT PERMIT, INCLUDING THE STAGING OF CONSTRUCTION MATERIAL AND THE PLACEMENT OF PORTABLE TOILETS.

ENGINEER'S STATEMENT

I HEREBY STATE THAT THESE PLANS ARE IN COMPLIANCE WITH ADOPTED COUNTY STANDARDS, THE APPROVED TENTATIVE MAP (OR PLAN) AND CONDITIONS OF APPROVAL PERTAINING THERETO FILE NO. XXXXX.

DATE 10-09-2023 SIGNATURE [Signature] 57874 R.C.E. NO. 6-30-2024 EXPIRATION DATE



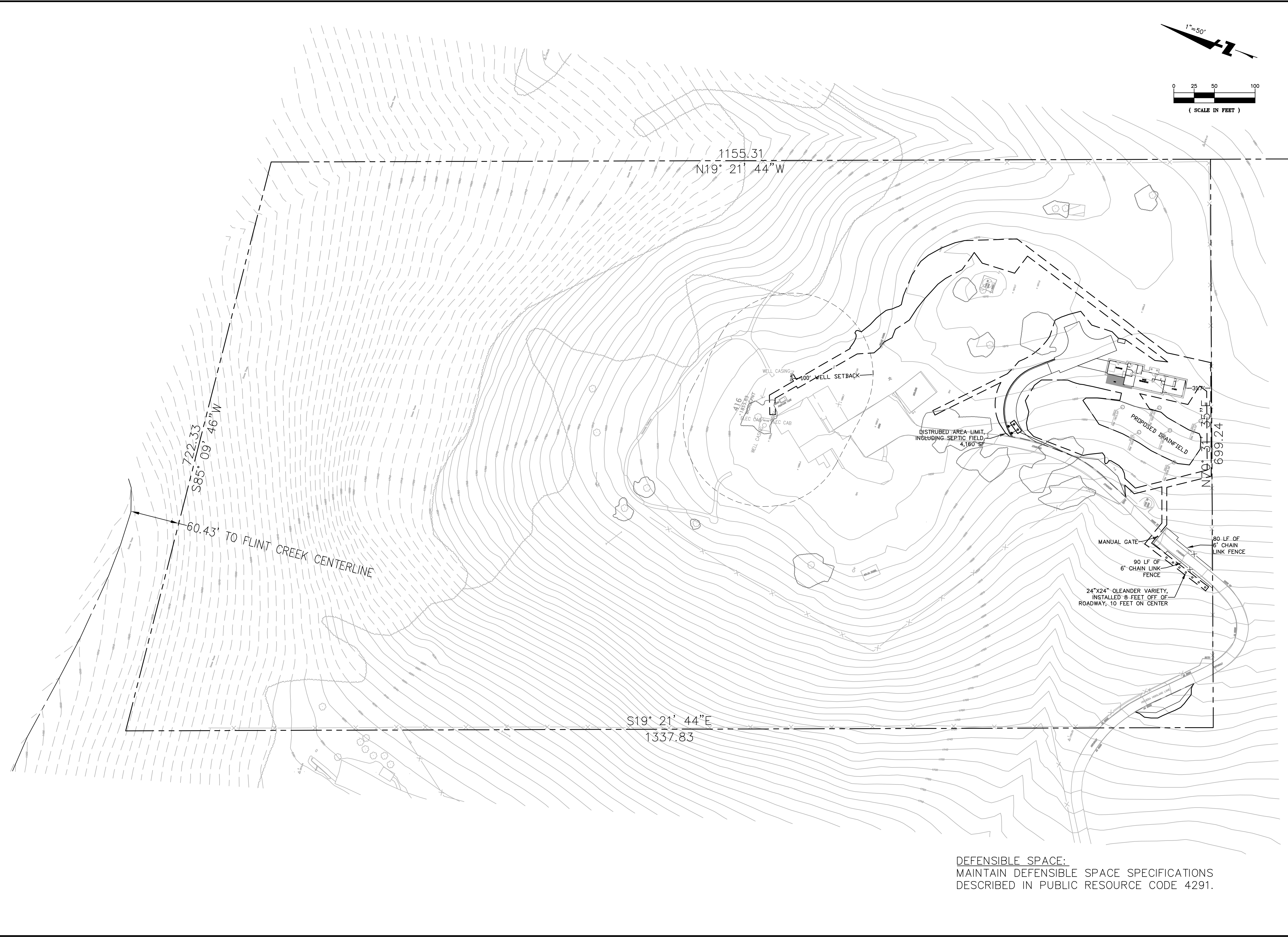
COUNTY ENGINEER'S NOTE

ISSUANCE OF A PERMIT AUTHORIZING CONSTRUCTION DOES NOT RELEASE THE DEVELOPER, PERMITTEE OF ENGINEER FROM RESPONSIBILITY FOR THE CORRECTION OF ERRORS OR OMISSIONS CONTAINED IN THE PLANS. IF, DURING THE COURSE OF CONSTRUCTION, THE PUBLIC INTEREST REQUIRES A MODIFICATION (OR DEPARTURE FROM) THE SPECIFICATIONS OF THE PLANS, THE COUNTY SHALL HAVE THE AUTHORITY TO REQUIRE THE SUSPENSION OF WORK, AND THE NECESSARY MODIFICATION OR DEPARTURE AND TO SPECIFY THE MANNER IN WHICH THE SAME IS TO BE MADE.

DATE _____ SIGNATURE _____ R.C.E. NO. _____ EXPIRATION DATE _____

APPLICANT: CHANG ROAD: HIGUERA HIGHLAND LANE COUNTY FILE NO.: PLN22-113

DRAWING NAME: K:\2021\210001_SJ_4015_Higuera_Highland_Lane\ENG\BASE_FILES\CONSULTANT_BASE\HH_Civil_Plans.dwg
 PLOT DATE: 10-09-23 PLOTTED BY: BUR



BKF
 1730 N. FIRST STREET
 SUITE 600
 SAN JOSE, CA 95112
 (408) 477-0100
 www.bkf.com

CALIFORNIA

**CHANG RESIDENCE
 OVERALL SITE PLAN
 4015 HIGUERA HIGHLAND LANE**
 SANTA CLARA COUNTY

SAN JOSE

Date	No.	Revisions
10-09-23		
Scale 1" = 50'		
Design MWB		
Drawn		
Approved		
Job No 20210001		

Drawing Number:

DEFENSIBLE SPACE:
 MAINTAIN DEFENSIBLE SPACE SPECIFICATIONS
 DESCRIBED IN PUBLIC RESOURCE CODE 4291.

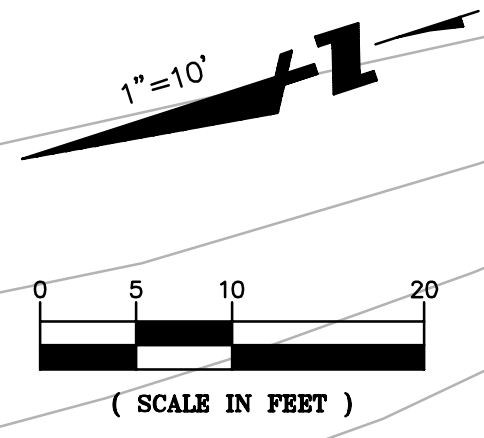
DRAWING NAME: K:\2021\210001_SJ_4015_Higuera_Highland_Lane\EN\BASE_FILES\CONSULTANT_BASE\HH_Civil_Plans.dwg
PLOT DATE: 10-09-23
PLOTTED BY: BUR

MATCHLINE SEE SHEET 3

MATCHLINE SEE SHEET 3

LEGEND

- EDGE OF DRIVEWAY
- CONTOURS
- 2% DRAINAGE ARROW
- x.327.3 SPOT ELEVATION



AREA OF DISTURBANCE = 41,600 SF
 EXISTING IMPERVIOUS AREA = 0 SF
 PROPOSED IMPERVIOUS AREA = 4,250 SF

3,144 SF STRUCTURE, DECK AND TANKS
 1,106 SF DRIVEWAY

EROSION CONTROL LEGEND

- FIBER ROLL PER DETAIL SE-5, SHEET 6.
- STABILIZED CONSTRUCTION ENTRANCE/EXIT PER DETAIL TC-1, SHEET 5.
- ALL DISTURBED AREA SHALL BE TREATED WITH HYDRO-SEEDING.

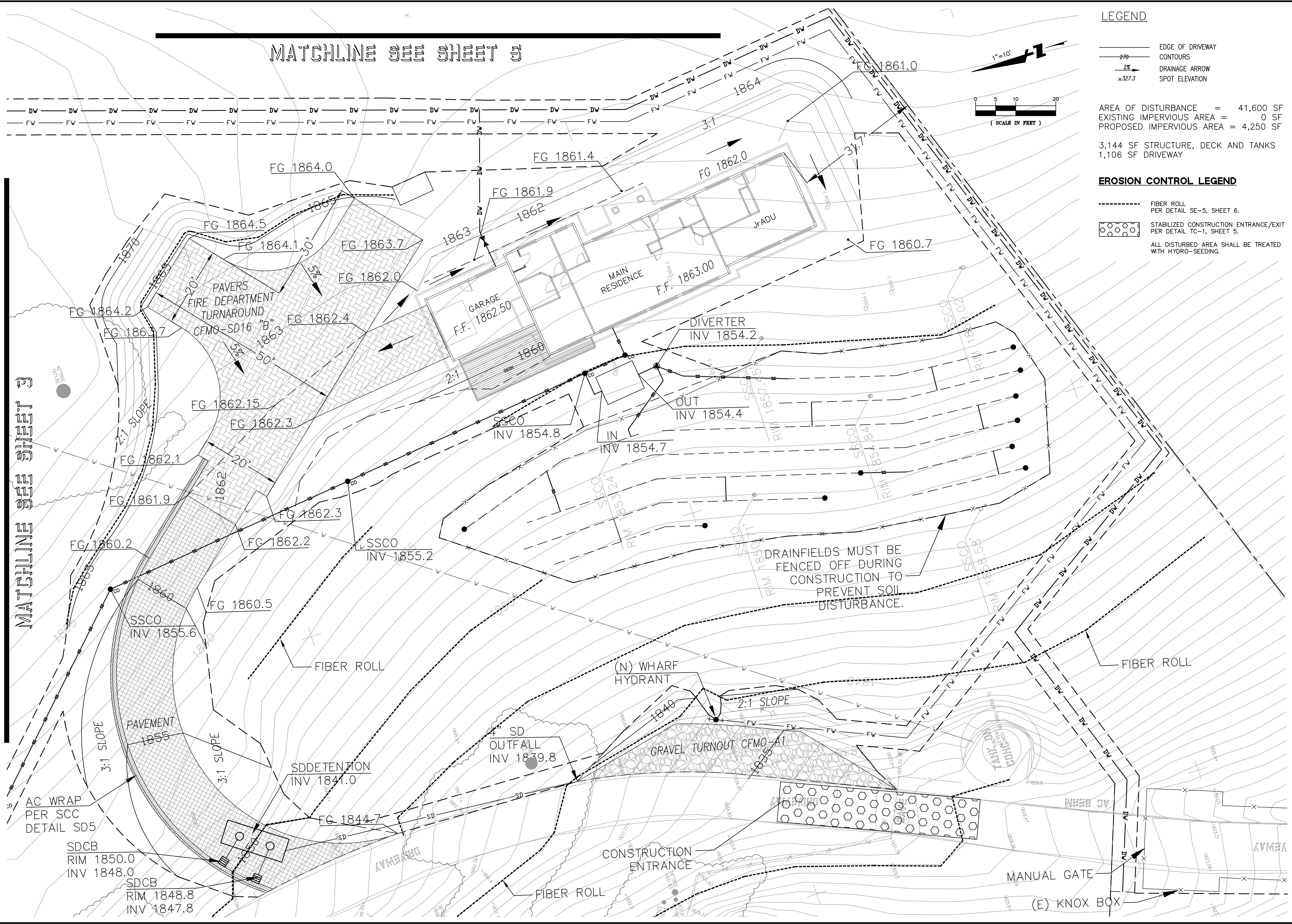
1730 N. FIRST STREET
 SUITE 600
 SAN JOSE, CA 95112
 (408) 477-0700
 www.bkf.com



CALIFORNIA
 SANTA CLARA COUNTY

**CHANG RESIDENCE
 GRADING AND DRAINAGE PLAN
 4015 HIGUERA HIGHLAND LANE**

SAN JOSE

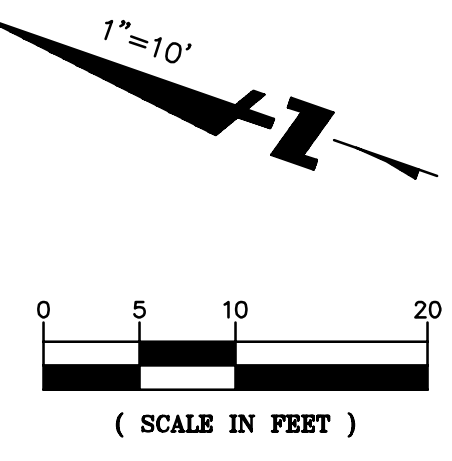
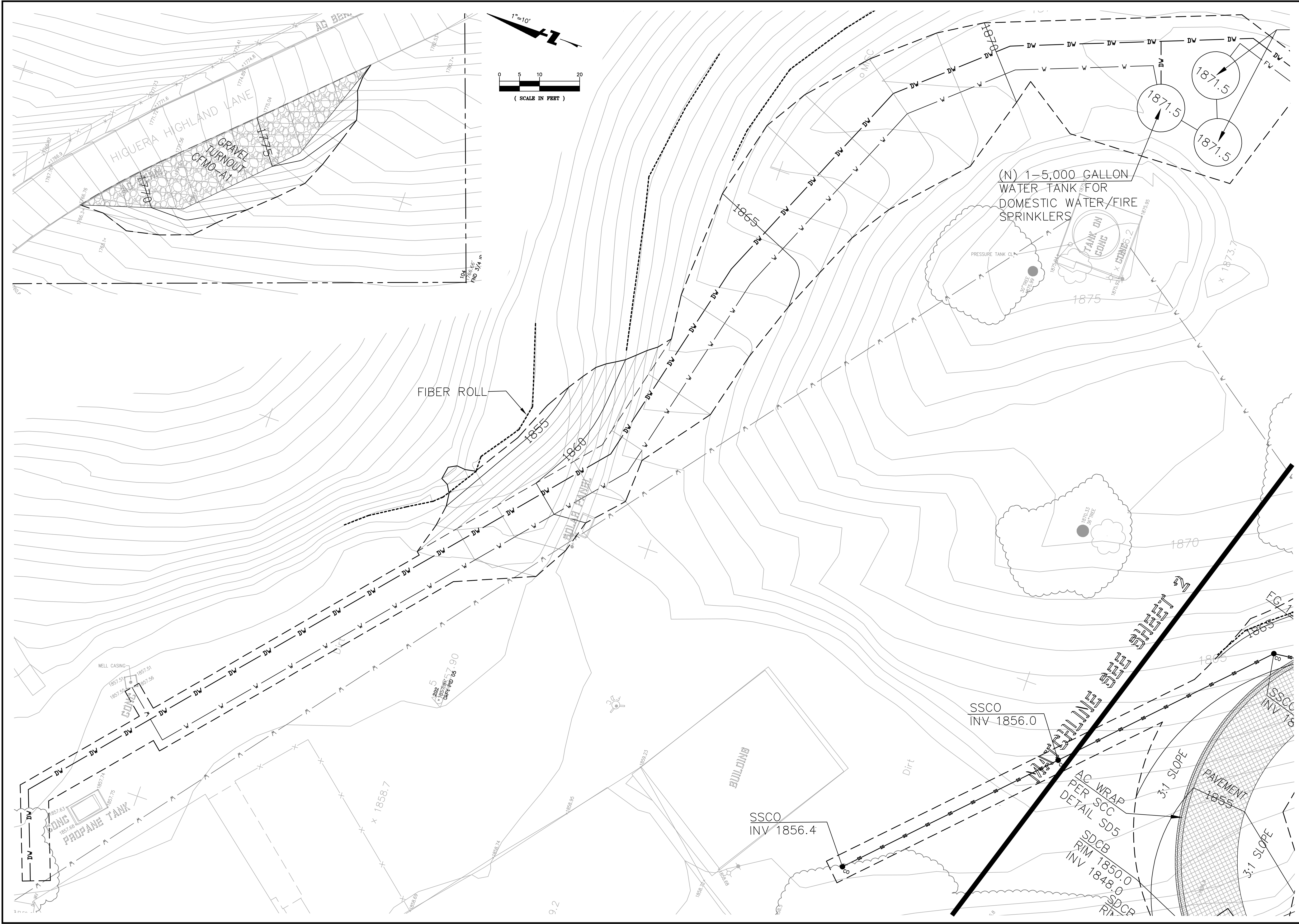


DRAINFIELDS MUST BE FENCED OFF DURING CONSTRUCTION TO PREVENT SOIL DISTURBANCE.

Revisions

Date: 10-09-23
 Scale: 1" = 10'
 Design: MWB
 Drawn:
 Approved:
 Job No: 20210001

DRAWING NAME: K:\2021\210001_SJ_4015_Higuera_Highland_Lane\ENG\BASE_FILES\CONSULTANT_BASE\HH_Civil_Plans.dwg
PLOT DATE: 10-09-23 PLOTTED BY: BUR



1730 N. FIRST STREET
SUITE 600
SAN JOSE, CA 95112
(408) 777-0100
www.bkf.com



CALIFORNIA

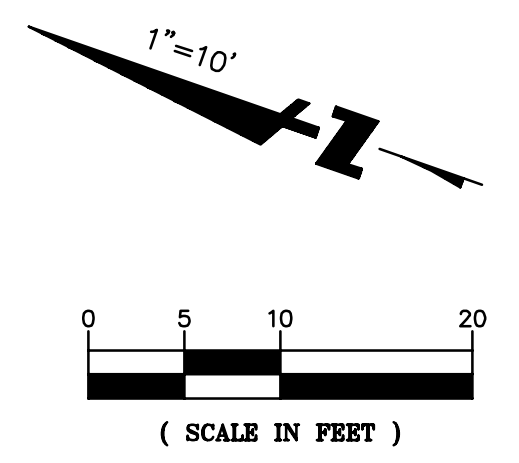
**CHANG RESIDENCE
GRADING AND DRAINAGE PLAN**
4015 HIGUERA HIGHLAND LANE

SANTA CLARA COUNTY
SAN JOSE

Revisions	
No.	Date

Date: 10-09-23
Scale: 1" = 10'
Design: MWB
Drawn: [blank]
Approved: [blank]
Job No: 20210001

DRAWING NAME: K:\2021\210001_SJ_4015_Higuera_Highland_Lane\ENR\BASE_FILES\CONSULTANT_BASE\HH_Civil_Plans.dwg
 PLOT DATE: 10-09-23 PLOTTED BY: BUR



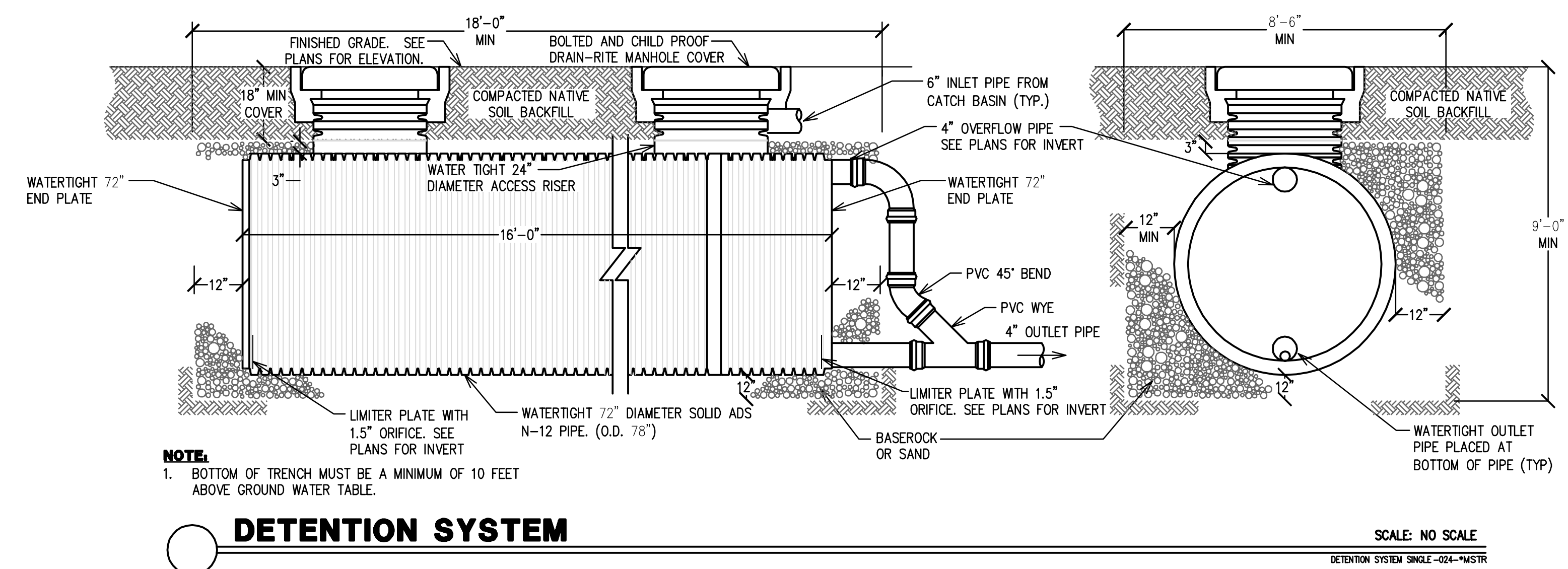
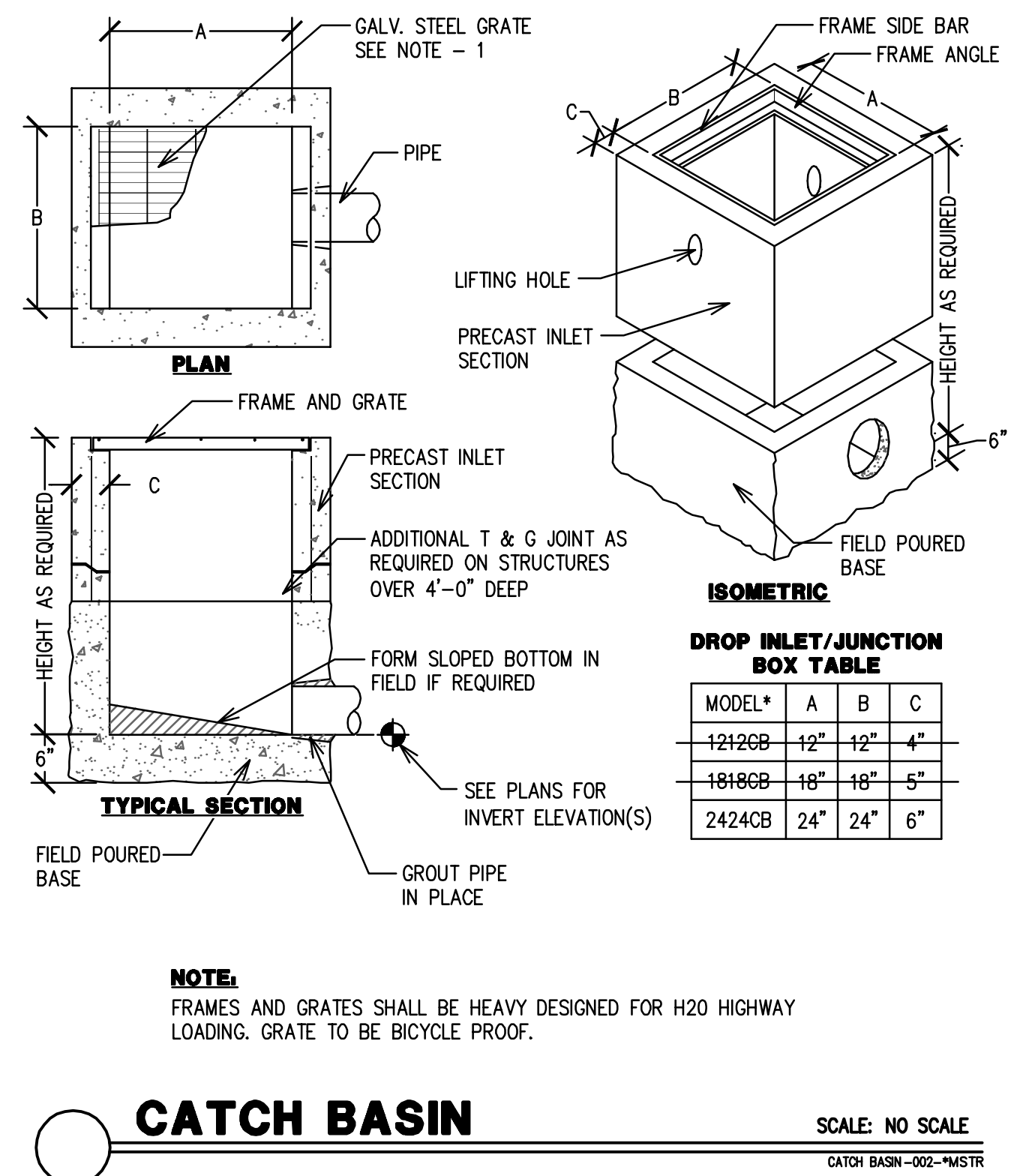
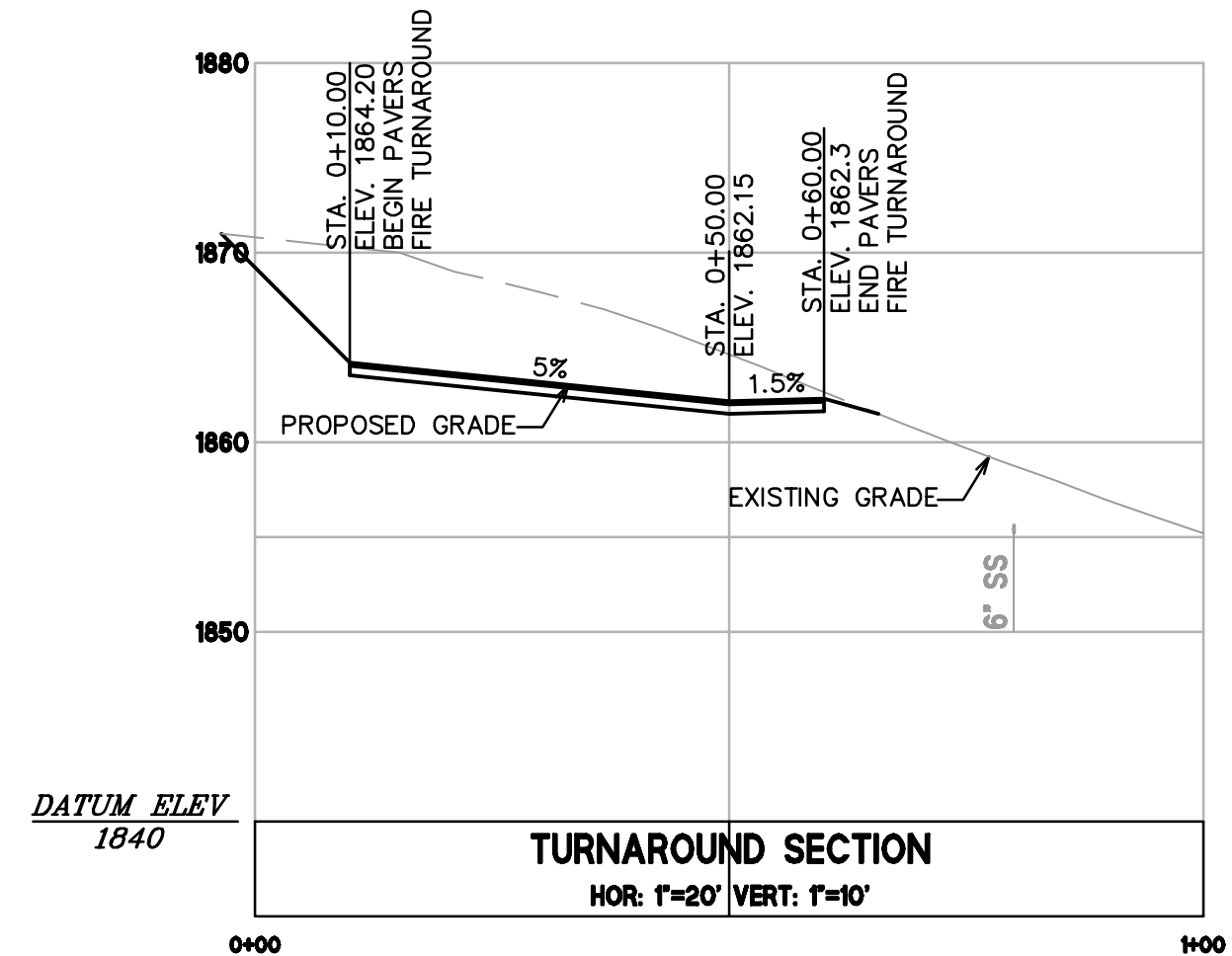
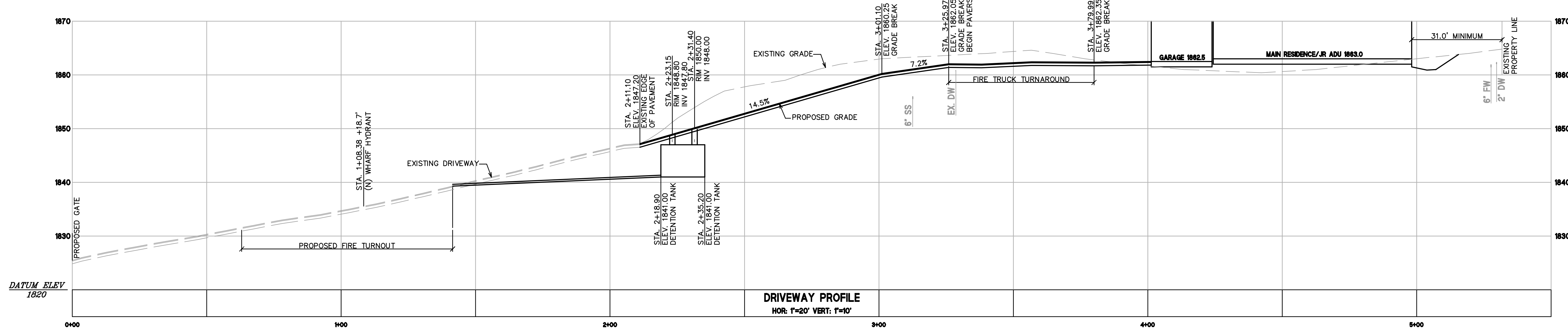
BKF
 1730 N. FIRST STREET
 SUITE 600
 SAN JOSE, CA 95112
 (408) 471-0100
 www.bkf.com

CHANG RESIDENCE
GRADING AND DRAINAGE PLAN
 4015 HIGUERA HIGHLAND LANE
 SANTA CLARA COUNTY
 CALIFORNIA

Revisions	
No.	Date
	10-09-23

Date: 10-09-23
 Scale: 1" = 10'
 Design: MWB
 Drawn: [blank]
 Approved: [blank]
 Job No: 20210001

DRAWING NAME: K:\2021\210001_SJ_4015_Higuera_Highland_Lane\EN\BASE_FILES\CONSULTANT BASE\HH Civil Plans.dwg
 PLOT DATE: 10-09-23 PLOTTED BY: BUR



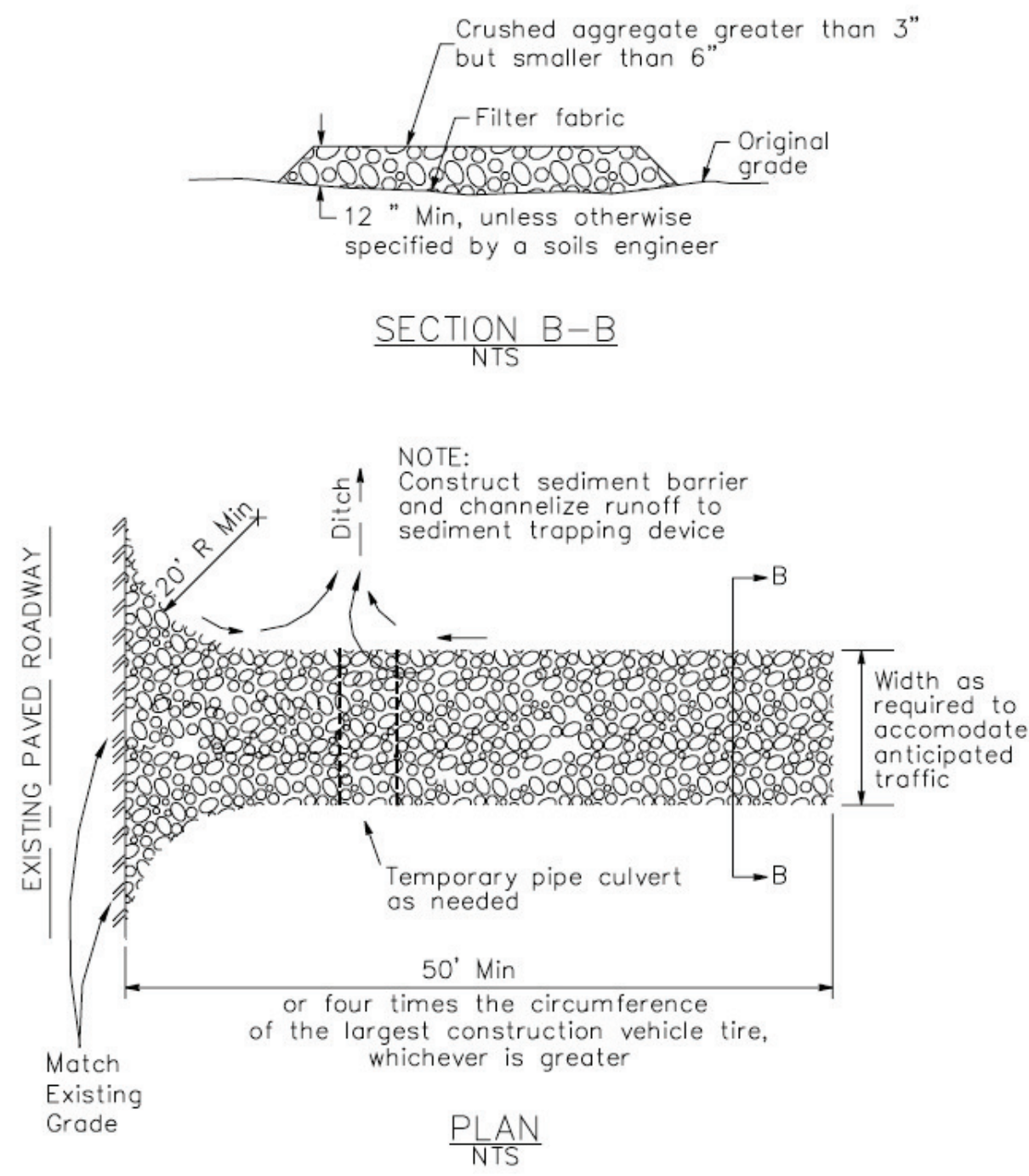
CHANG RESIDENCE
DRIVEWAY PROFILE, SECTION AND DETAILS
 4015 HIGUERA HIGHLAND LANE
 SANTA CLARA COUNTY
 SAN JOSE
 CALIFORNIA
 1730 N. FIRST STREET
 SUITE 600
 SAN JOSE, CA 95112
 (408) 777-0100
 www.bkf.com

No.	Date	By	Check	Scale
10-09-23	AS SHOWN	MWB	MWB	AS SHOWN
		Design		
		Drawn		
		Approved		
		Job No	20210001	

Drawing Number: **6** OF **8**

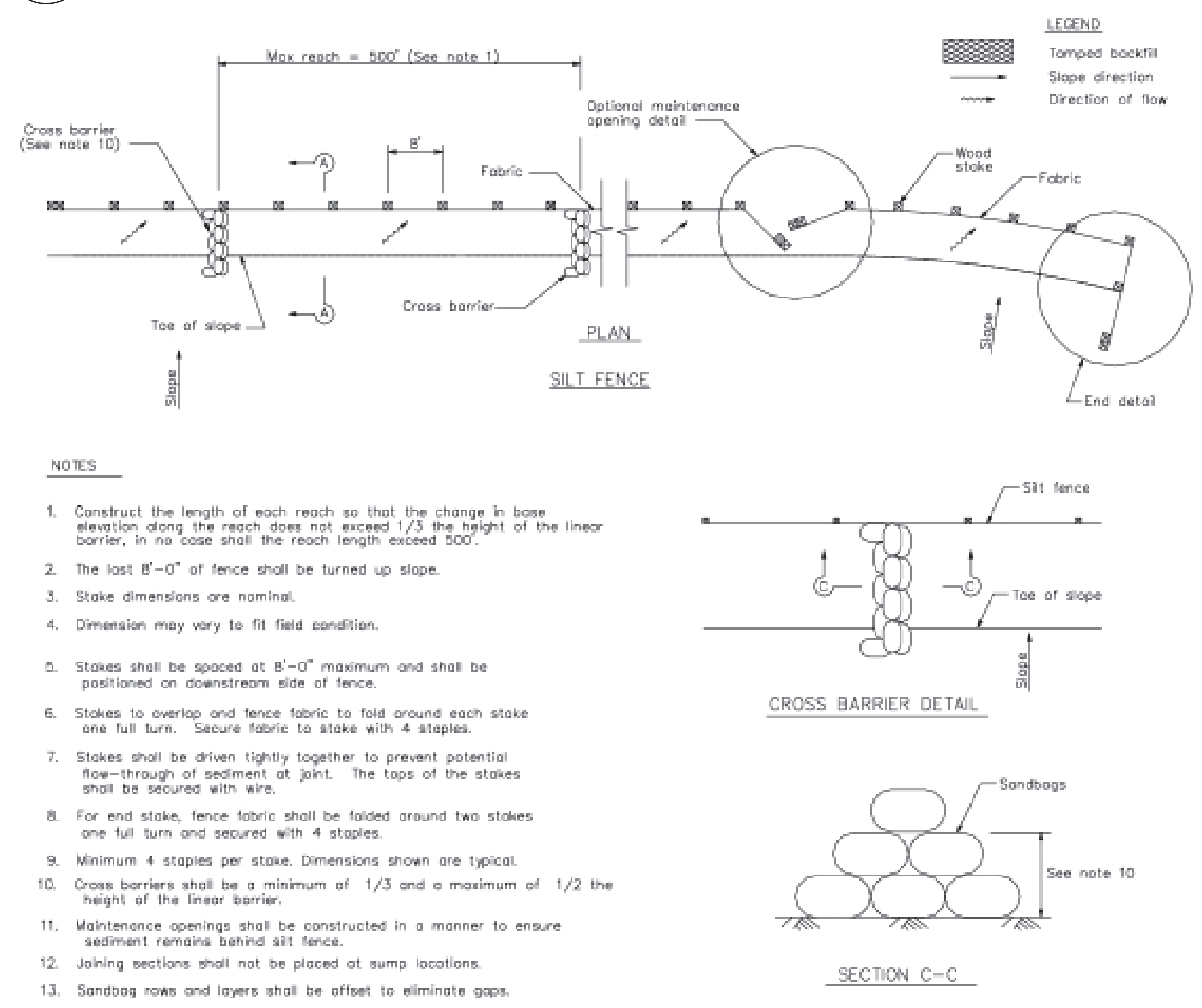
3 Stabilized Construction Entrance/Exit

CASQA Detail TC-1



1 Silt Fence

CASQA Detail SE-1



- NOTES**
- Construct the length of each reach so that the change in base elevation along the reach does not exceed 1/3 the height of the linear barrier, in no case shall the reach length exceed 500'.
 - The last 8'-0" of fence shall be turned up slope.
 - Stake dimensions are nominal.
 - Dimension may vary to fit field condition.
 - Stakes shall be spaced at 8'-0" maximum and shall be positioned on downstream side of fence.
 - Stakes to overlap and fence fabric to fold around each stake one full turn. Secure fabric to stake with 4 staples.
 - Stakes shall be driven tightly together to prevent potential flow-through of sediment at joint. The tops of the stakes shall be secured with wire.
 - For end stake, fence fabric shall be folded around two stakes one full turn and secured with 4 staples.
 - Minimum 4 staples per stake. Dimensions shown are typical.
 - Cross barriers shall be a minimum of 1/3 and a maximum of 1/2 the height of the linear barrier.
 - Maintenance openings shall be constructed in a manner to ensure sediment remains behind silt fence.
 - Joining sections shall not be placed at sump locations.
 - Sandbag rows and layers shall be offset to eliminate gaps.

STANDARD BEST MANAGEMENT PRACTICE NOTES

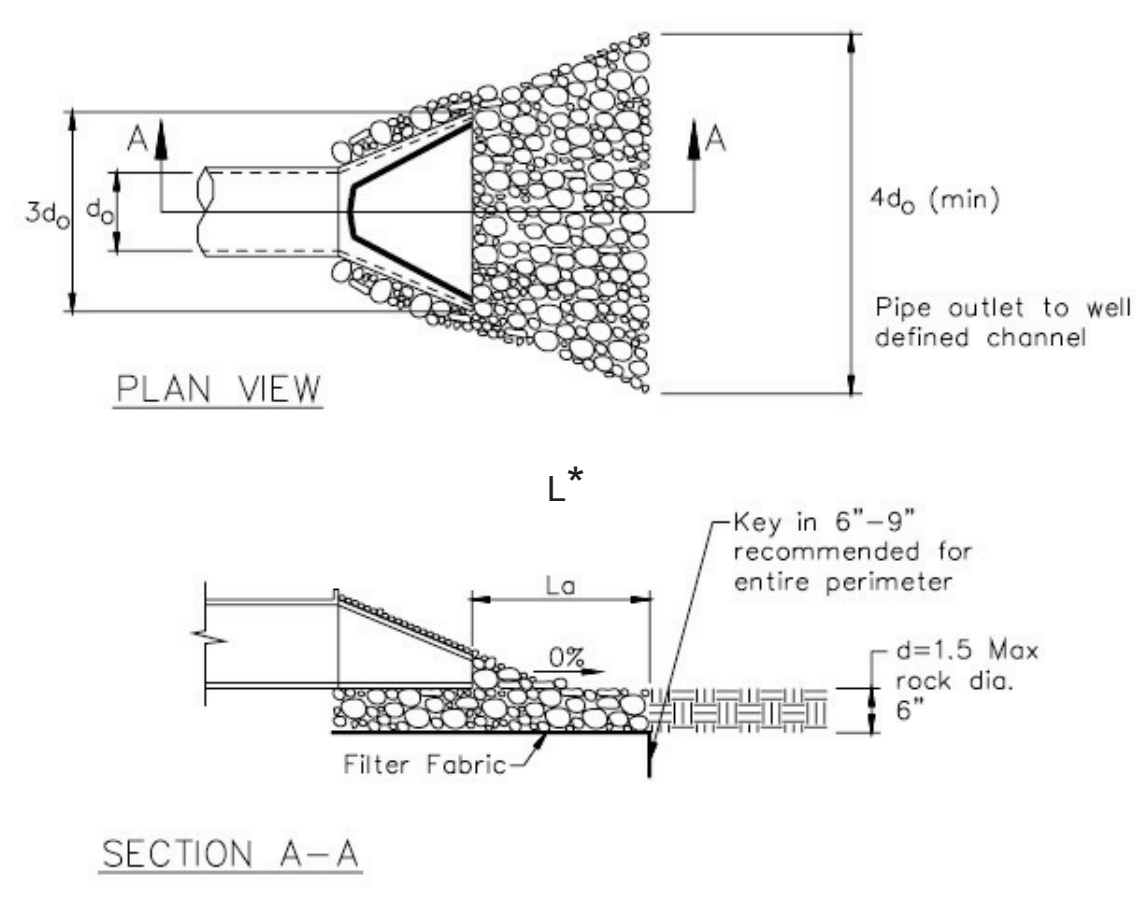
- Solid and Demolition Waste Management:** Provide designated waste collection areas and containers on site away from streets, gutters, storm drains, and waterways, and arrange for regular disposal. Waste containers must be watertight and covered at all times except when waste is deposited. Refer to Erosion & Sediment Control Field Manual, 4th Edition (page C3) or latest.
- Hazardous Waste Management:** Provide proper handling and disposal of hazardous wastes by a licensed hazardous waste material hauler. Hazardous wastes shall be stored and properly labeled in sealed containers constructed of suitable materials. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages C-5 to C-6) or latest.
- Spill Prevention and Control:** Provide proper storage areas for liquid and solid materials, including chemicals and hazardous substances, away from streets, gutters, storm drains, and waterways. Spill control materials must be kept on site where readily accessible. Spills must be cleaned up immediately and contaminated soil disposed properly. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages C-7 to C-8, C-13 to C-14) or latest.
- Vehicle and Construction Equipment Service and Storage:** An area shall be designated for the maintenance, where on-site maintenance is required, and storage of equipment that is protected from stormwater run-on and runoff. Measures shall be provided to capture any waste oils, lubricants, or other potential pollutants and these wastes shall be properly disposed of off site. Fueling and major maintenance/repair, and washing shall be conducted off-site whenever feasible. Refer to Erosion & Sediment Control Field Manual, 4th Edition (page C9) or latest.
- Material Delivery, Handling and Storage:** In general, materials should not be stockpiled on site. Where temporary stockpiles are necessary and approved by the County, they shall be covered with secured plastic sheeting or tarp and located in designated areas near construction entrances and away from drainage paths and waterways. Barriers shall be provided around storage areas where materials are potentially in contact with runoff. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages C-11 to C-12) or latest.
- Handling and Disposal of Concrete and Cement:** When concrete trucks and equipment are washed on-site, concrete wastewater shall be contained in designated containers or in a temporary lined and watertight pit where wasted concrete can harden for later removal. If possible have concrete contractor remove concrete wash water from site. In no case shall fresh concrete be washed into the road right-of-way. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages C-15 to C-16) or latest.
- Pavement Construction Management:** Prevent or reduce the discharge of pollutants from paving operations, using measures to prevent run-on and runoff pollution and properly disposing of wastes. Avoid paving in the wet season and reschedule paving when rain is in the forecast. Residue from saw-cutting shall be vacuumed for proper disposal. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages C-17 to C-18) or latest.
- Contaminated Soil and Water Management:** Inspections to identify contaminated soils should occur prior to construction and at regular intervals during construction. Remediating contaminated soil should occur promptly after identification and be specific to the contaminant identified, which may include hazardous waste removal. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages C-19 to C-20) or latest.
- Sanitary/Septic Water Management:** Temporary sanitary facilities should be located away from drainage paths, waterways, and traffic areas. Only licensed sanitary and septic waste haulers should be used. Secondary containment should be provided for all sanitary facilities. Refer to Erosion & Sediment Control Field Manual, 4th Edition (page C-21) or latest.
- Inspection & Maintenance:** Areas of material and equipment storage sites and temporary sanitary facilities must be inspected weekly. Problem areas shall be identified and appropriate additional and/or alternative control measures implemented immediately, within 24 hours of the problem being identified.

STANDARD EROSION CONTROL NOTES

- Sediment Control Management:**
 - Tracking Prevention & Clean Up:** Activities shall be organized and measures taken as needed to prevent or minimize tracking of soil onto the public street system. A gravel or proprietary device construction entrance/exit is required for all sites. Clean up of tracked material shall be provided by means of a street sweeper prior to an approaching rain event, or at least once at the end of each workday that material is tracked, or, more frequently as determined by the County Inspector. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages B-31 to B-33) or latest.
 - Storm Drain Inlet and Catch Basin Inlet Protection:** All inlets within the vicinity of the project and within the project limits shall be protected with gravel bags placed around inlets or other inlet protection. At locations where exposed soils are present, staked fiber rolls or staked silt fences can be used. Inlet filters are not allowed due to clogging and subsequent flooding. Refer to Erosion & Sediment Control Field Manual, 4th Edition (pages B-49 to B-51) or latest.
 - Storm Water Runoff:** No storm water runoff shall be allowed to drain in to the existing and/or proposed underground storm drain system or other above ground watercourses until appropriate erosion control measures are fully installed.
 - Dust Control:** The contractor shall provide dust control in graded areas as required by providing wet suppression or chemical stabilization of exposed soils, providing for rapid clean up of sediments deposited on paved roads, furnishing construction road entrances and vehicle wash down areas, and limiting the amount of areas disturbed by clearing and earth moving operations by scheduling these activities in phases.
 - Stockpiling:** Excavated soils shall not be placed in streets or on paved areas. Borrow and temporary stockpiles shall be protected with appropriate erosion control measures (tarps, straw bales, silt fences, etc.) to ensure silt does not leave the site or enter the storm drain system or neighboring watercourse.
- Erosion Control:** During the rainy season, all disturbed areas must include an effective combination of erosion and sediment control. It is required that temporary erosion control measures are applied to all disturbed soil areas prior to a rain event. During the non-rainy season, erosion control measures must be applied sufficient to control wind erosion at the site.
- Inspection & Maintenance:** Disturbed areas of the Project's site, locations where vehicles enter or exit the site, and all erosion and sediment controls that are identified as part of the Erosion Control Plans must be inspected by the Contractor before, during, and after storm events, and at least weekly during seasonal wet periods. Problem areas shall be identified and appropriate additional and/or alternative control measures implemented immediately, within 24 hours of the problem being identified.
- Project Completion:** Prior to project completion and signoff by the County Inspector, all disturbed areas shall be reseeded, planted, or landscaped to minimize the potential for erosion on the subject site.
- It shall be the Owner's/Contractor's responsibility to maintain control of the entire construction operation and to keep the entire site in compliance with the erosion control plan.
- Erosion and sediment control best management practices shall be operable year round or until vegetation is fully established on landscaped surfaces.

4 Velocity Dissipation Devices

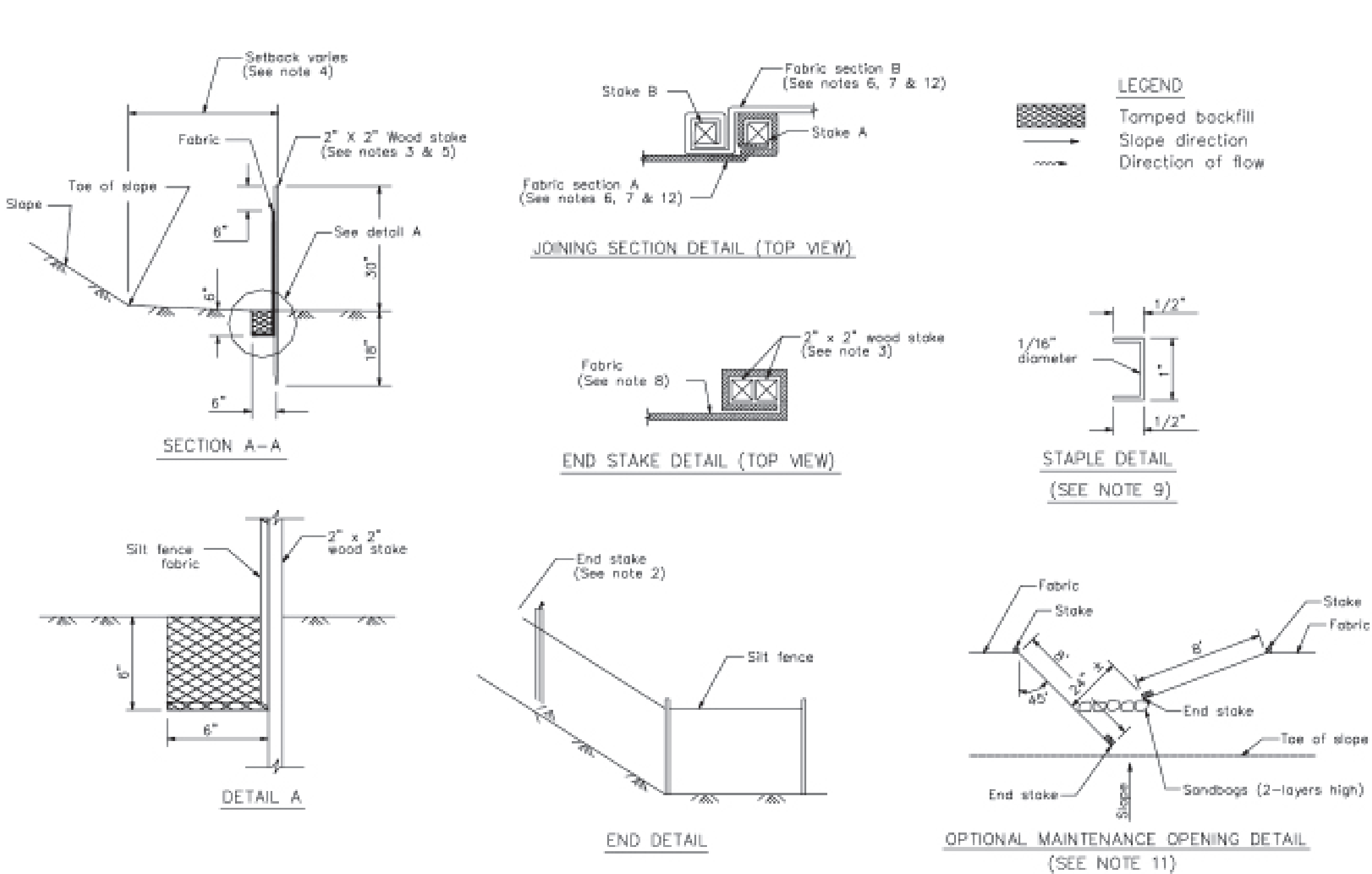
CASQA Detail EC-10



* Length per ABAG Design Standards

2 Silt Fence

CASQA Detail SE-1

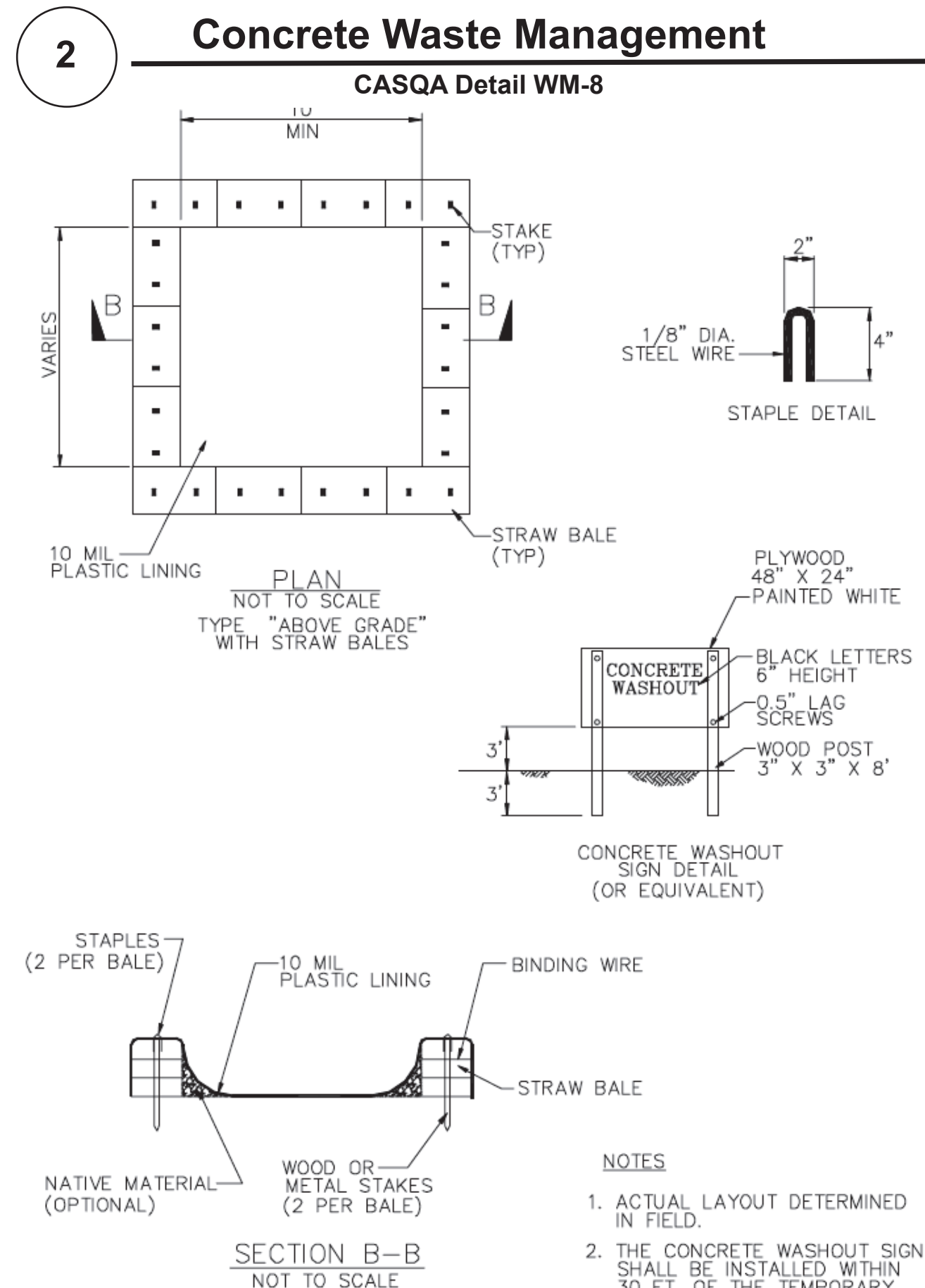
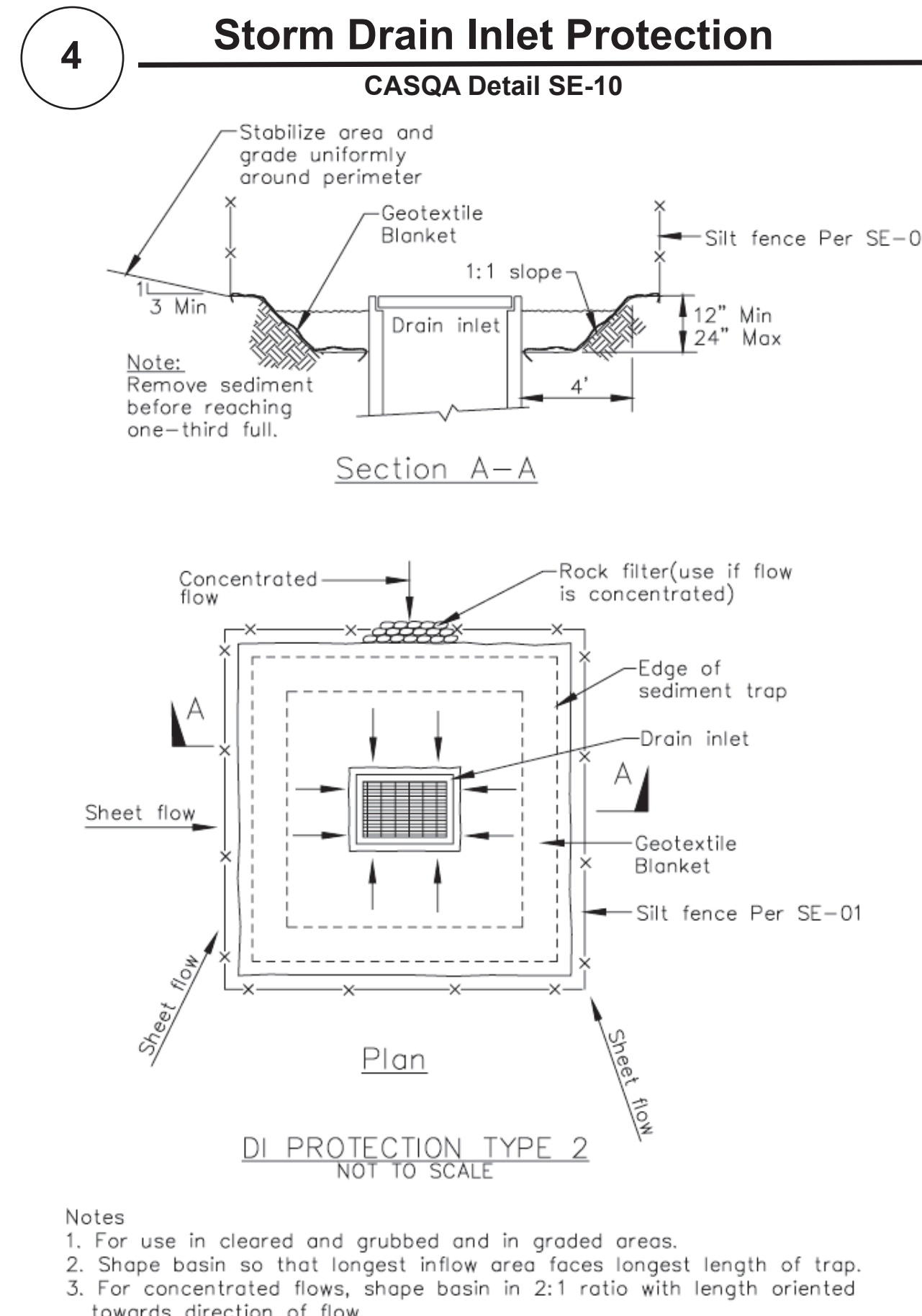
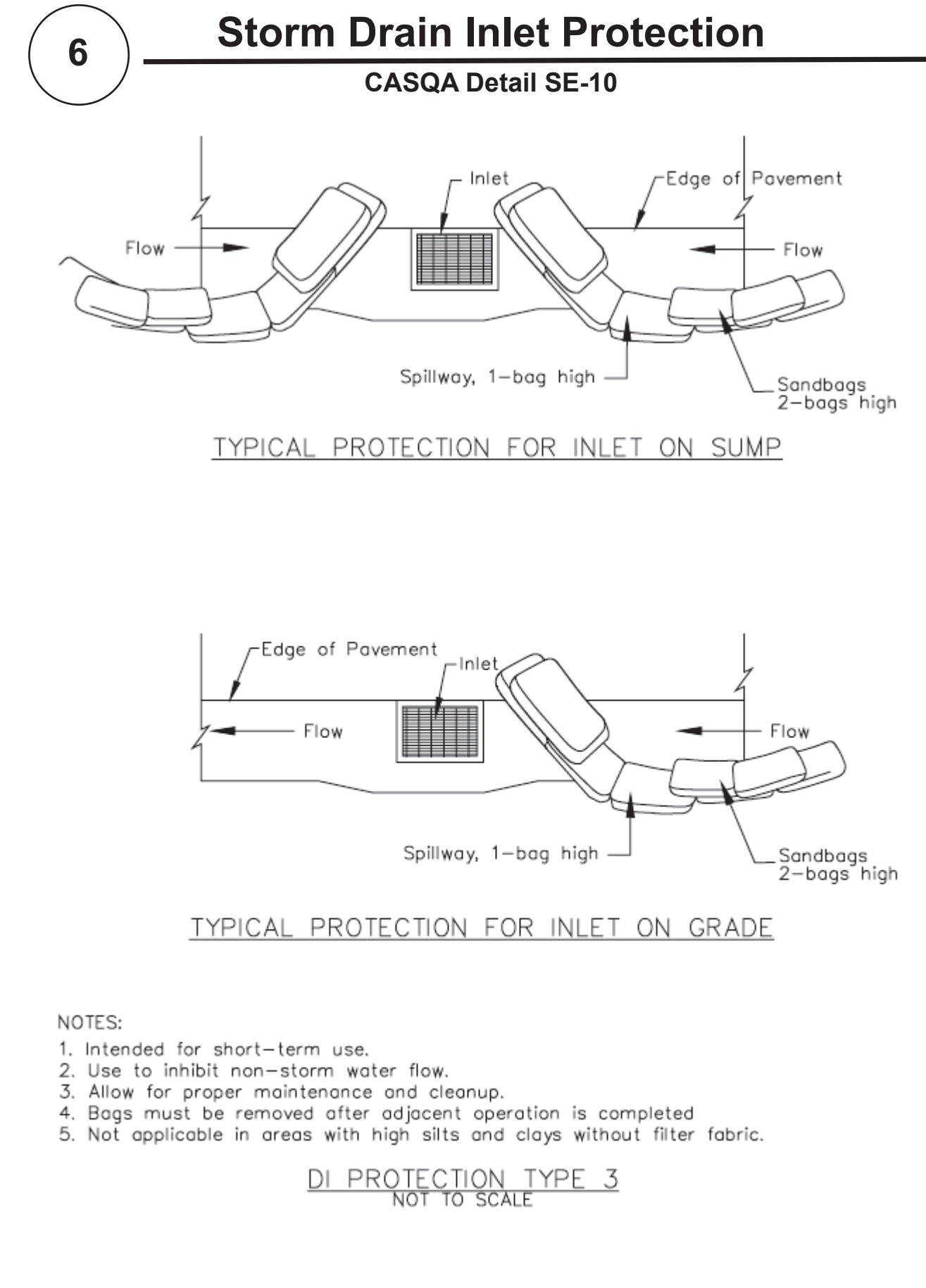
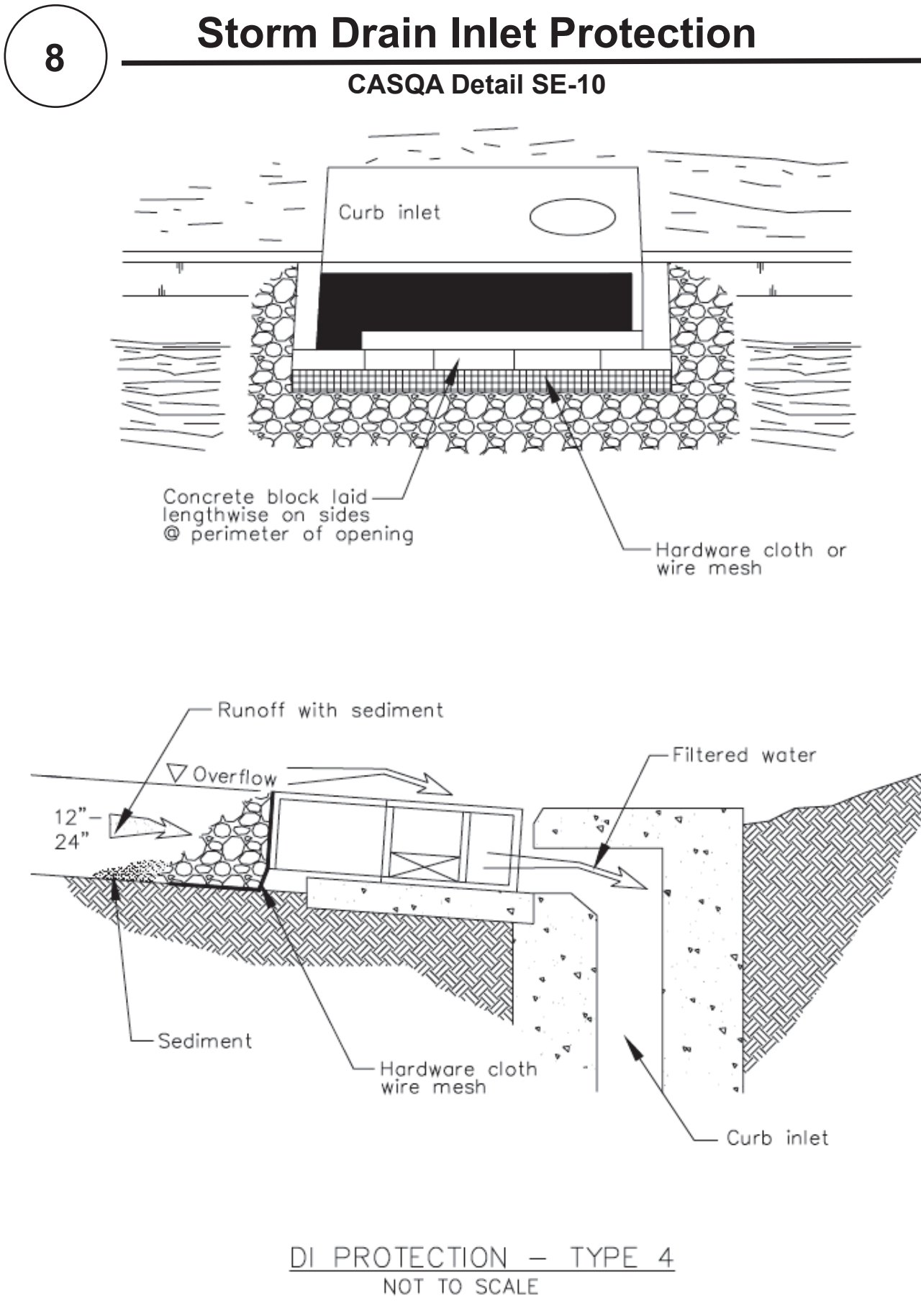
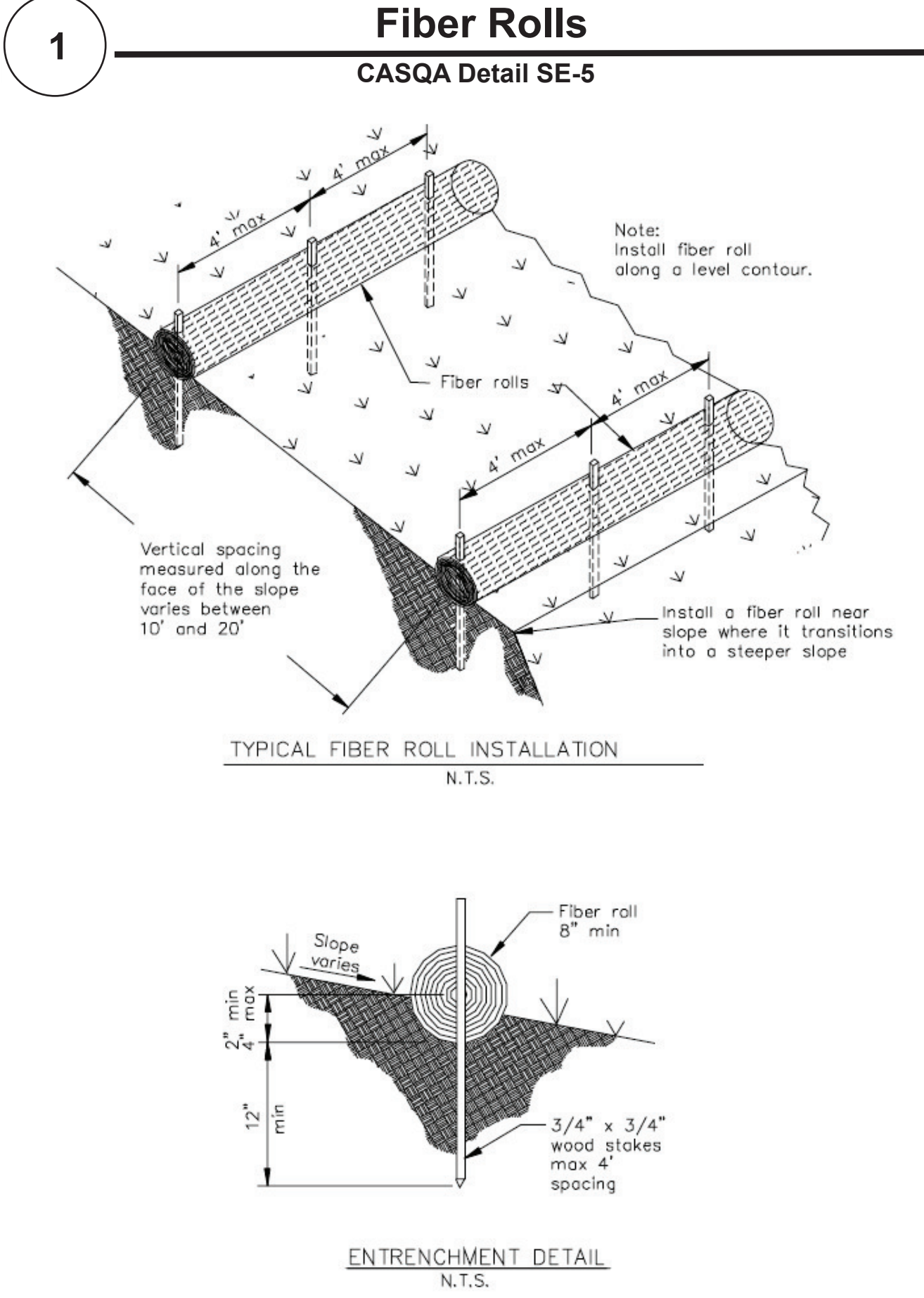
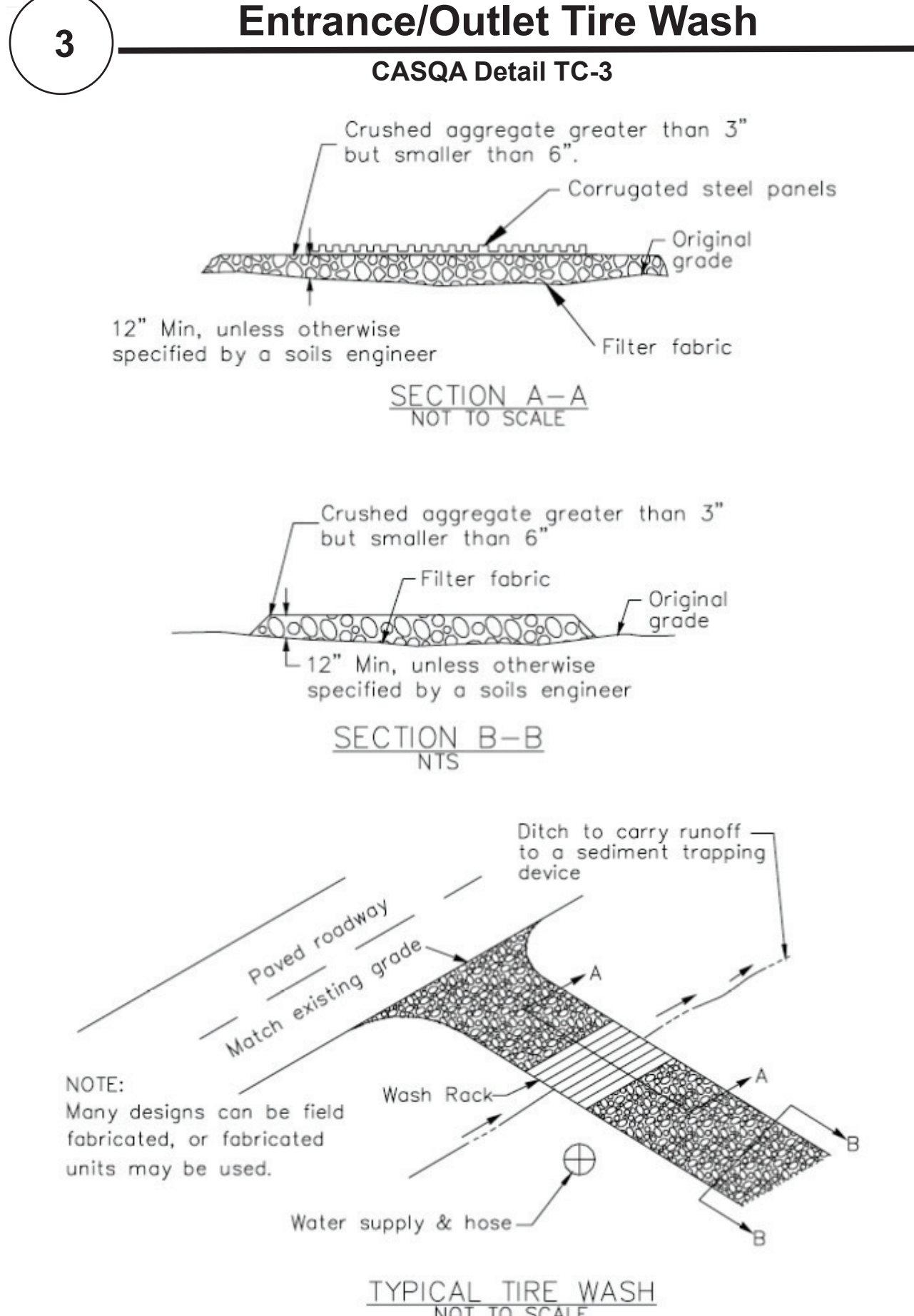
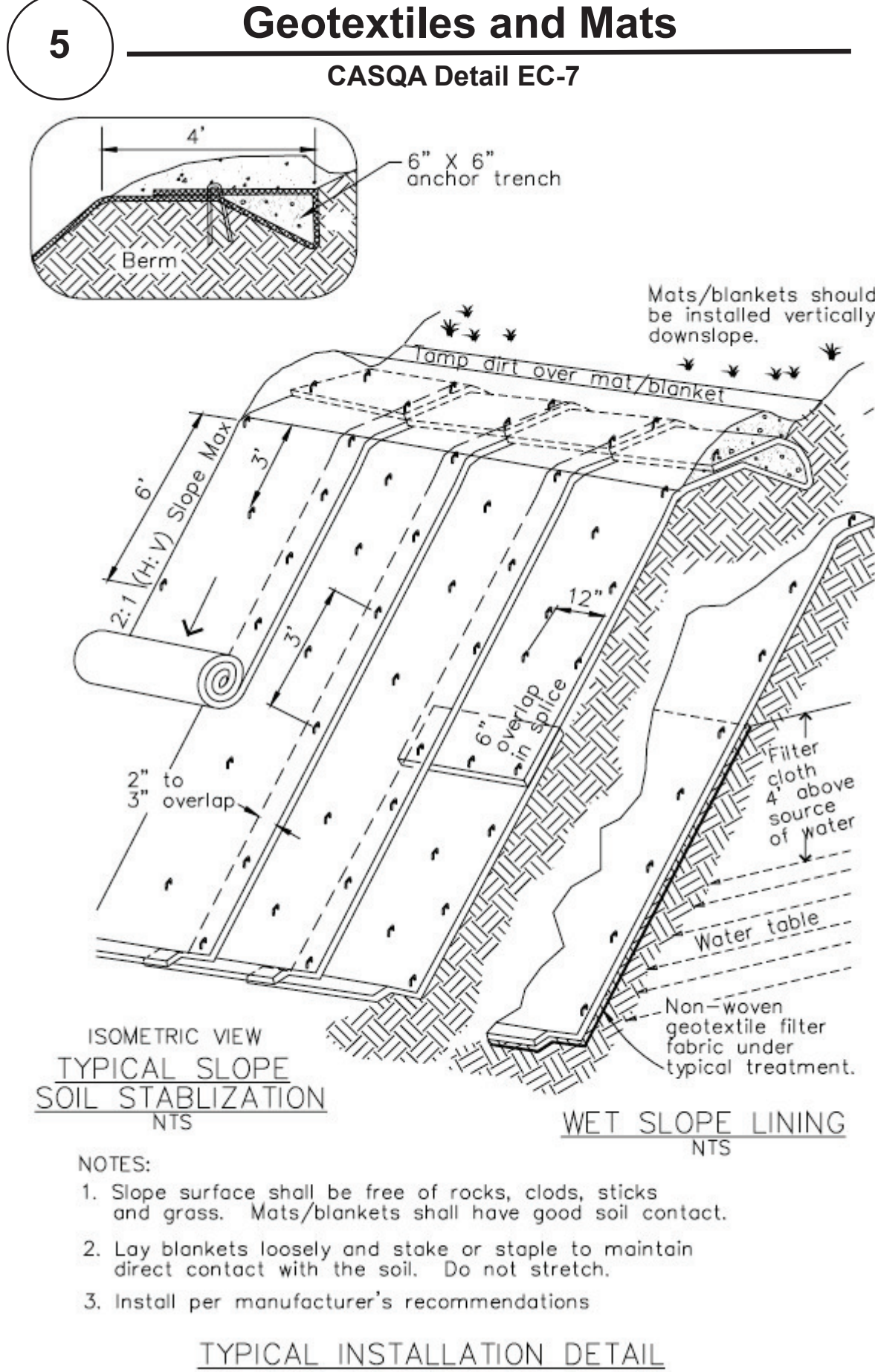
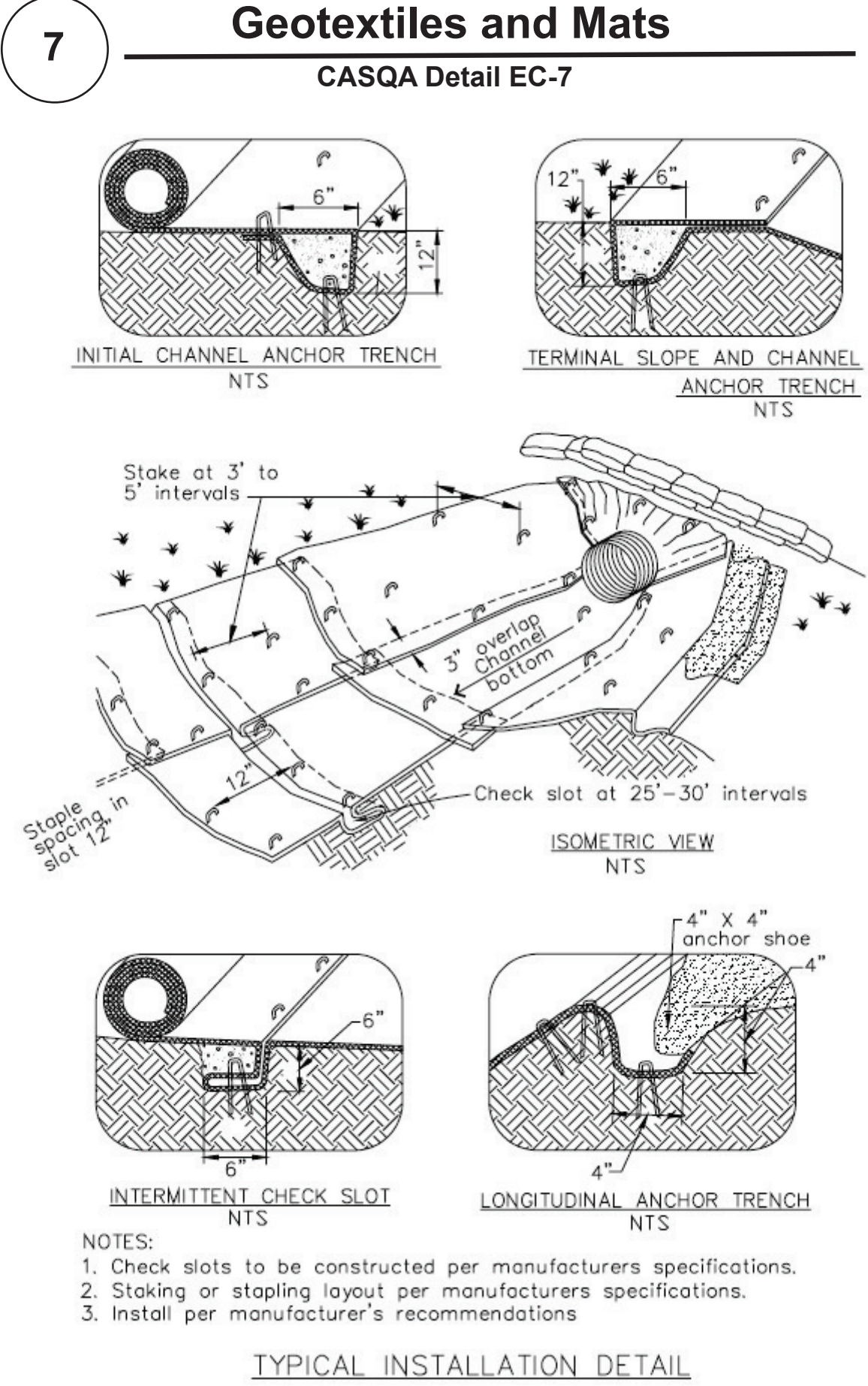


Project Information

CHANG RESIDENCE - 4015 HIGUERA HIGHLAND LANE
SHEET 7 OF 8

Source for Graphics: California Stormwater BMP Handbook, California Stormwater Quality Association, January 2003. Available from www.cabmphandbooks.com.



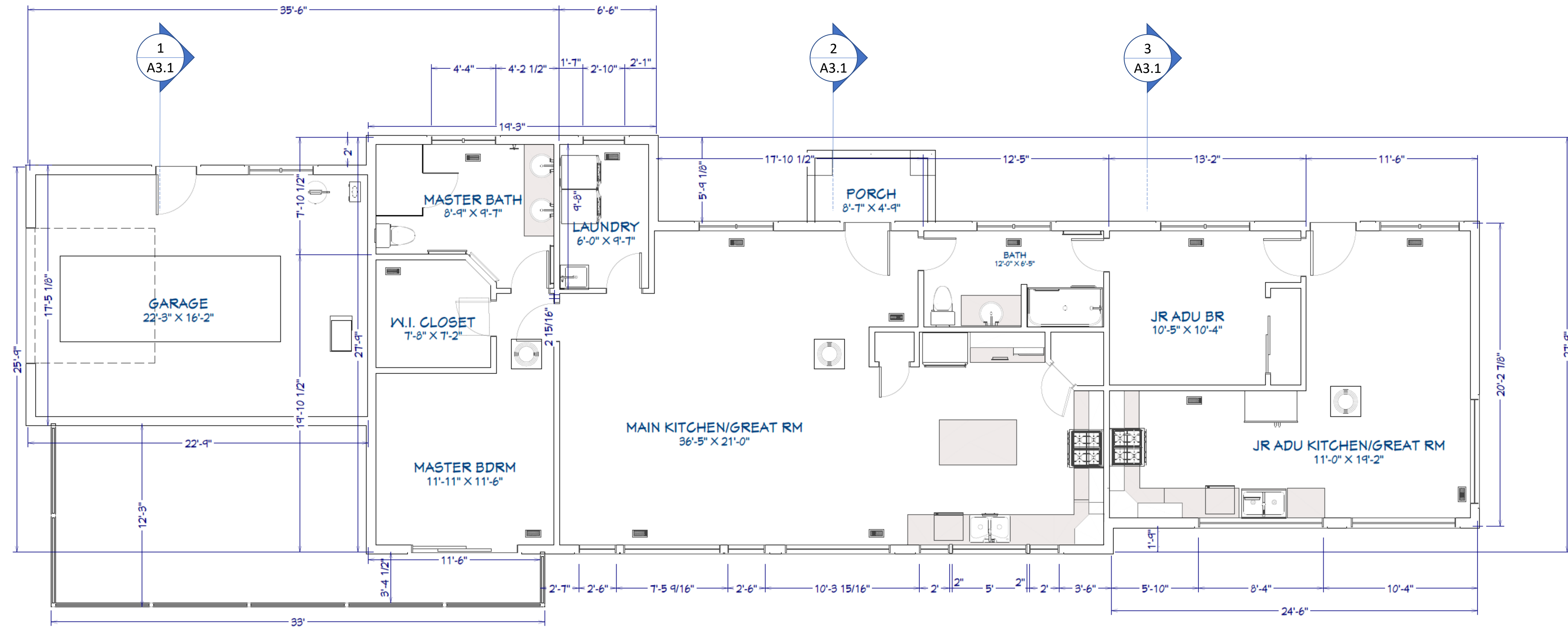


Source for Graphics: California Stormwater BMP Handbook, California Stormwater Quality Association, January 2003. Available from www.cabmphandbooks.com.

Project Information
 CHANG RESIDENCE - 4015 HIGUERA HIGHLAND LANE
 SHEET 8 OF 8

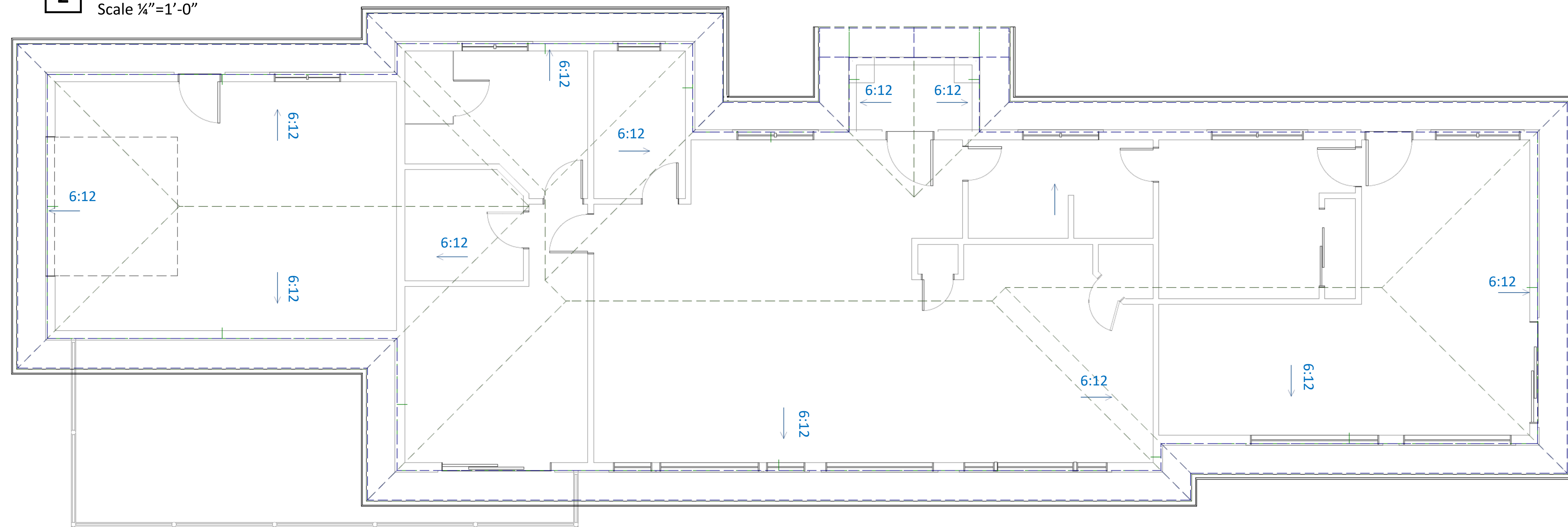


1 Floor Plan
Scale 1/4"=1'-0"



Project
 Chang Wang
 Residence/Jr ADU
 4015 Higuera Highland Ln
 San Jose, CA 95148
 APN: 654-015-023
 Phone: 650-380-2528
 Email: homeofcw@gmail.com

2 Roof Plan
Scale 1/4"=1'-0"

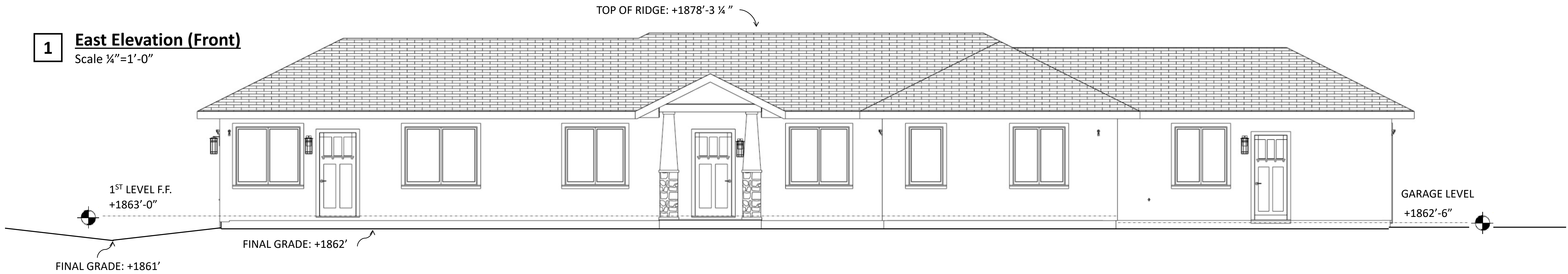


Scale
 1/4"=1'-0"

A1.1

1 East Elevation (Front)

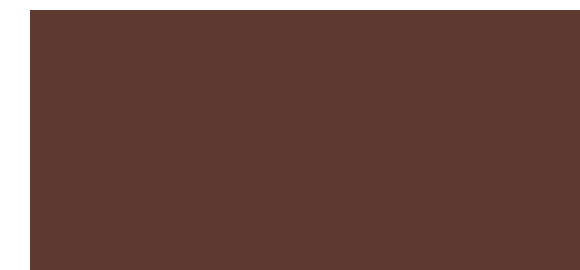
Scale 1/4"=1'-0"



Roof: Owens Corning TruDefinition Duration, Terra Cotta, Item #376825, Model # TK99



Door: Sherwin Williams, 6055 Fiery Brown, LRV 5



Window: Milgard Vinyl, Tan, LRV 25



Trim: Sherwin Williams, 6024 Dressy Rose, LRV 37



Exterior Wall: LaHabra Stucco, 63151 Hanover Base 100, LRV 33



Architecture Accent (Stone Veneer): MSI Natural Earth, Textured Quartz Wall Tile, LRV 30

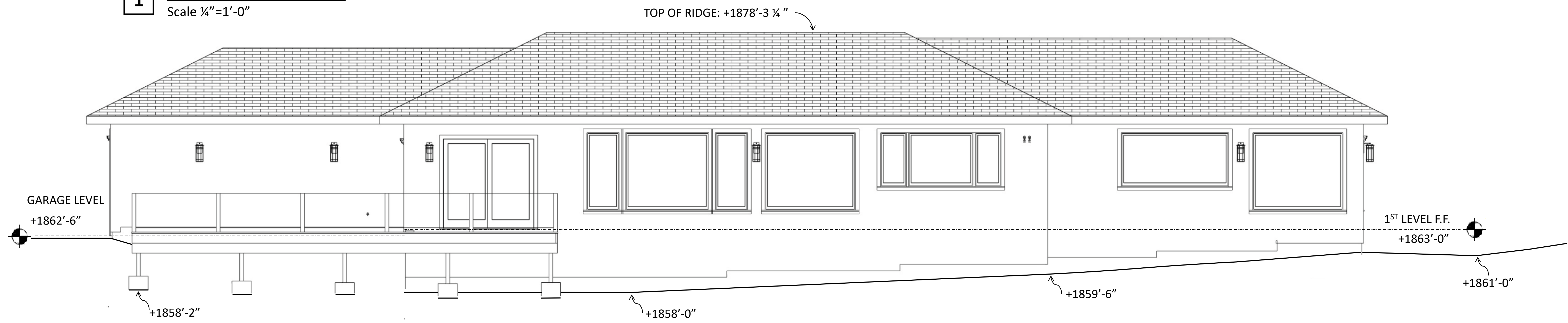


Project
 Chang Wang
 Residence/Jr ADU
 4015 Higuera Highland Ln
 San Jose, CA 95148
 APN: 654-015-023
 Phone: 650-380-2528
 Email: homeofcw@gmail.com

Scale
 1/4"=1'-0"

A2.1

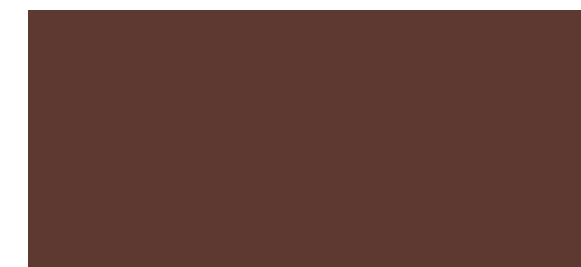
1 West Elevation (Back)
Scale 1/4"=1'-0"



Roof: Owens Corning TruDefinition Duration, Terra Cotta, Item #376825, Model # TK99



Door: Sherwin Williams, 6055 Fiery Brown, LRV 5



Window: Milgard Vinyl, Tan, LRV 25



Trim: Sherwin Williams, 6024 Dressy Rose, LRV 37



Exterior Wall: LaHabra Stucco, 63151 Hanover Base 100, LRV 33



Project

Chang Wang
Residence/Jr ADU

4015 Higuera Highland Ln
San Jose, CA 95148

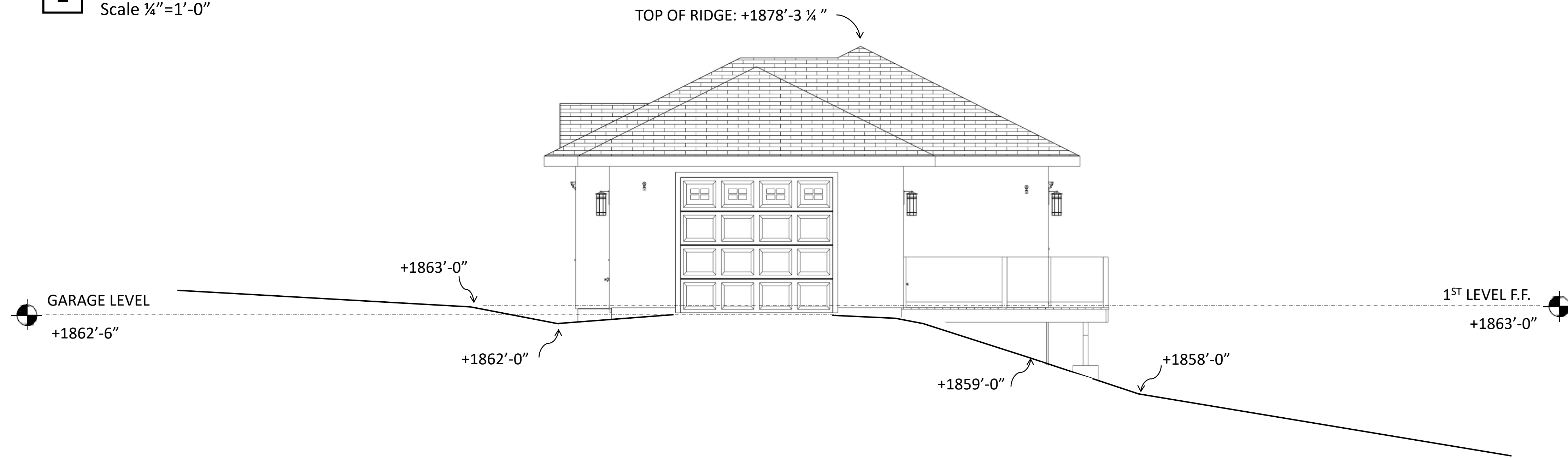
APN: 654-015-023

Phone: 650-380-2528
Email: homeofcw@gmail.com

Scale
1/4"=1'-0"

A2.2

1 North Elevation
Scale 1/4"=1'-0"



Project
Chang Wang
Residence/Jr ADU
4015 Higuera Highland Ln
San Jose, CA 95148
APN: 654-015-023
Phone: 650-380-2528
Email: homeofcw@gmail.com

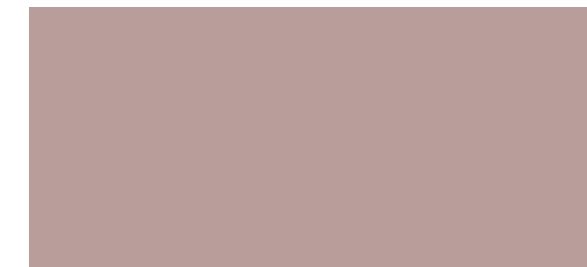
Roof: Owens Corning TruDefinition Duration, Terra Cotta, Item #376825, Model # TK99



Garage Door: Sherwin Williams, 6024 Dressy Rose, LRV 37



Trim: Sherwin Williams, 6024 Dressy Rose, LRV 37

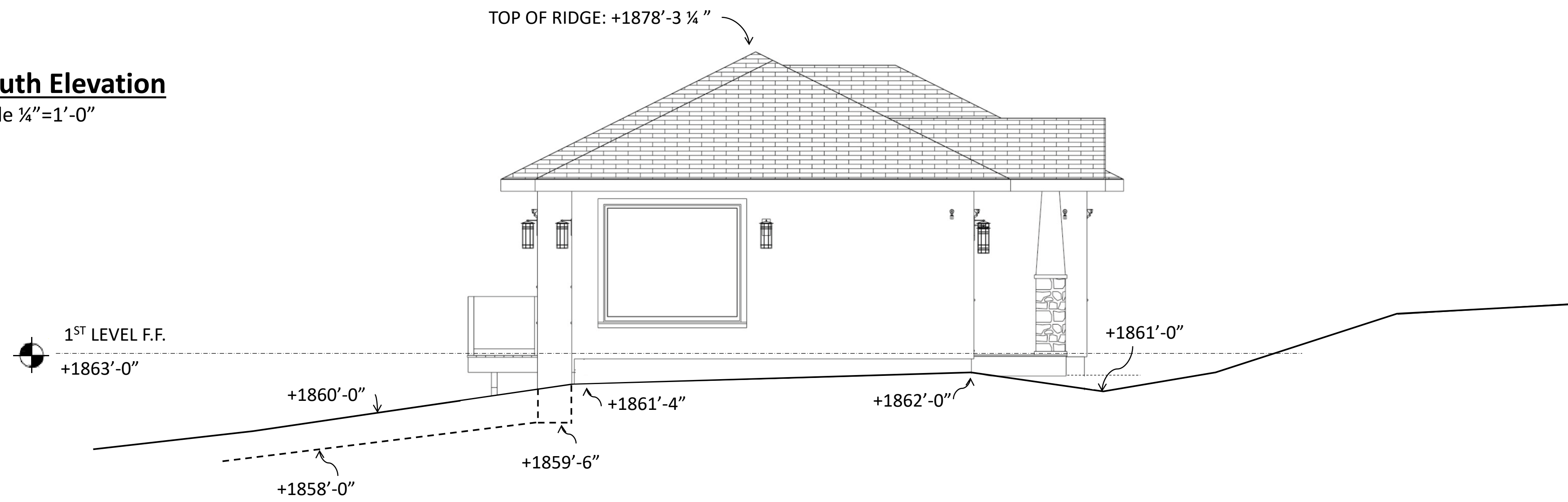


Exterior Wall: LaHabra Stucco, 63151 Hanover Base 100, LRV 33



Scale
1/4"=1'-0"
A2.3

1 South Elevation
Scale 1/4"=1'-0"



Project
 Chang Wang
 Residence/Jr ADU
 4015 Higuera Highland Ln
 San Jose, CA 95148
 APN: 654-015-023
 Phone: 650-380-2528
 Email: homeofcw@gmail.com

Roof: Owens Corning TruDefinition Duration, Terra Cotta, Item #376825, Model # TK99



Window: Milgard Vinyl, Tan, LRV 25



Trim: Sherwin Williams, 6024 Dressy Rose, LRV 37

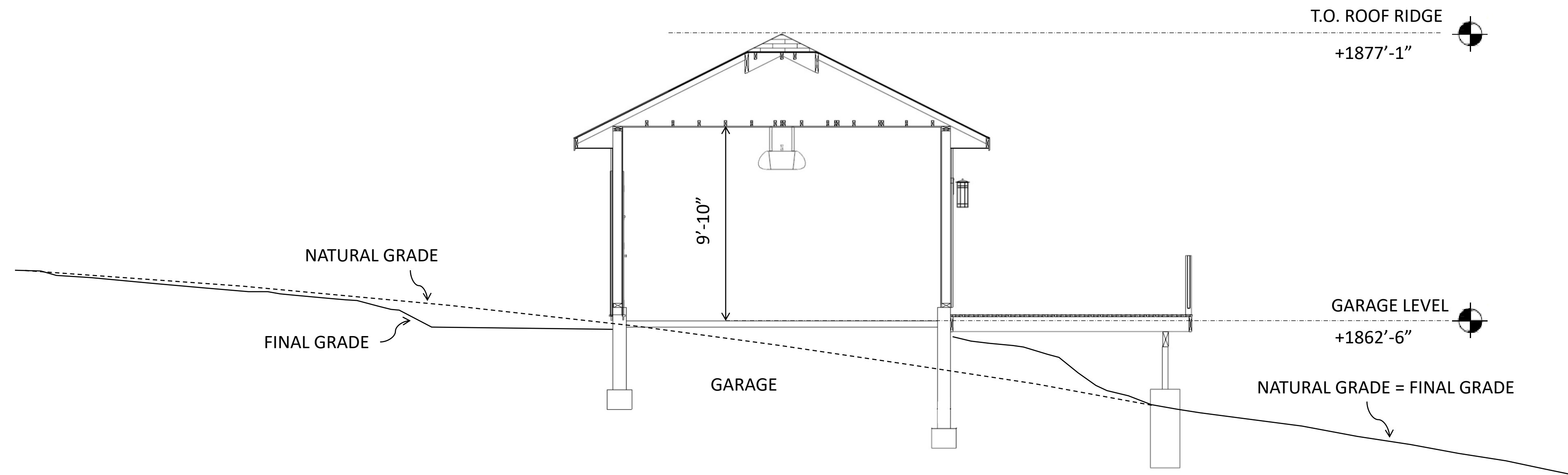


Exterior Wall: LaHabra Stucco, 63151 Hanover Base 100, LRV 33

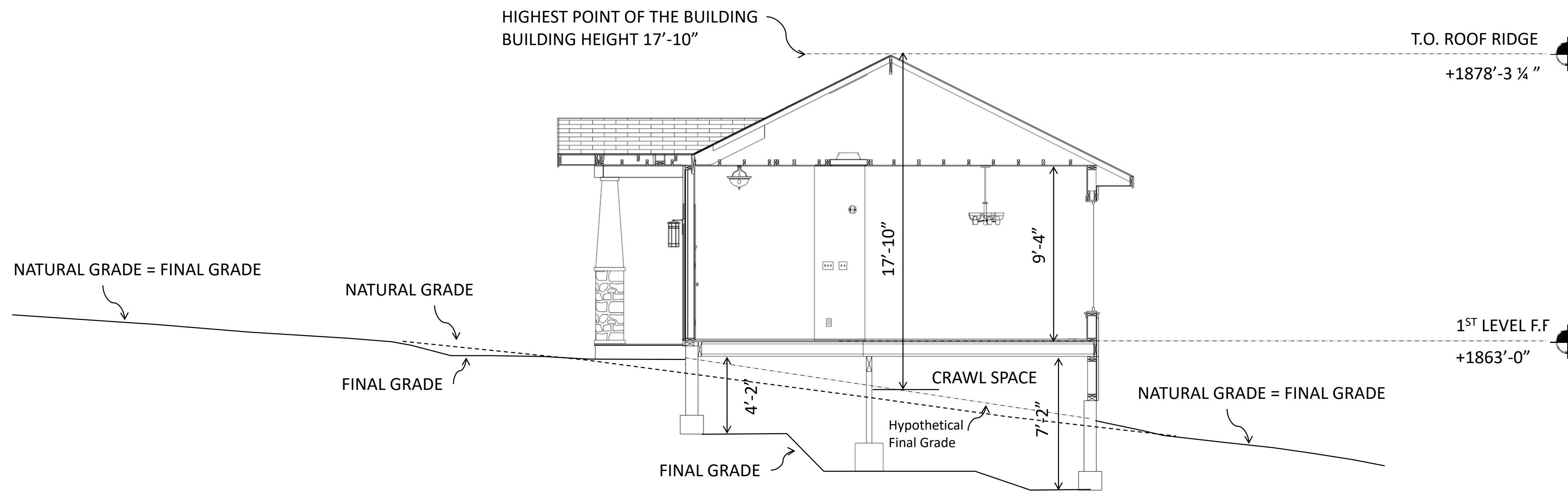


Scale
 1/4"=1'-0"
 A2.4

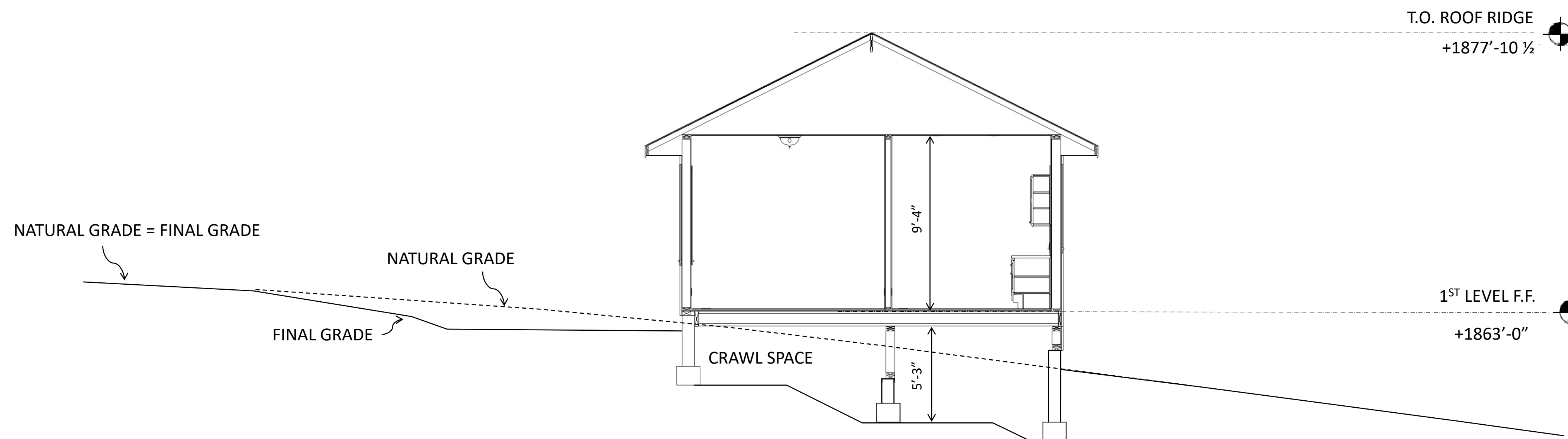
1 SECTION
Scale 1/4"=1'-0"



2 SECTION
Scale 1/4"=1'-0"



3 SECTION
Scale 1/4"=1'-0"



Project
Chang Wang
Residence/Jr ADU

4015 Higuera Highland Ln
San Jose, CA 95148

APN: 654-015-023

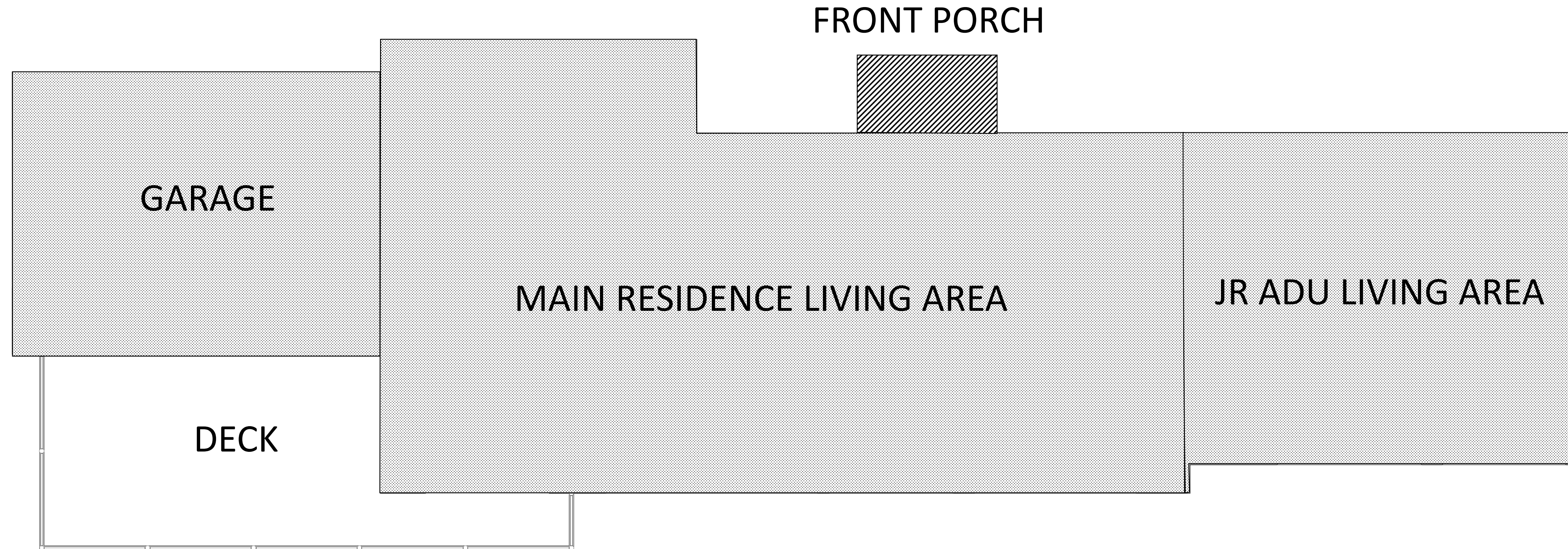
Phone: 650-380-2528
Email: homeofcw@gmail.com

Scale
1/4"=1'-0"

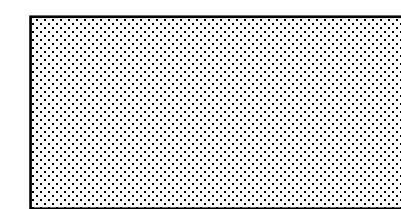
A3.1

Gross Floor Area

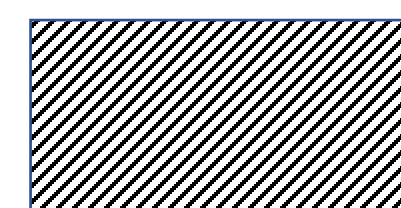
Scale 1/4"=1'-0"



Project
 Chang Wang
 Residence/Jr ADU
 4015 Higuera Highland Ln
 San Jose, CA 95148
 APN: 654-015-023
 Phone: 650-380-2528
 Email: homeofcw@gmail.com
 May 3rd, 2023



= INDICATES LOCATIONS INCLUDED IN THESE CALCULATIONS.



= INDICATES EXTERIOR COVERED LOCATIONS REQUIRED TO BE INCLUDED IN THESE CALCULATIONS.

1 st LEVEL	HABITABLE	NON-HABITABLE	CPVERED EXTERIOR	UN-COVERED EXTERIOR
MAIN RESIDENCE LIVING AREA GARAGE FRONT PORCH DECK	1200 SF	400 SF	41 SF	300 SF
ATTACHED JR ADU LIVING AREA	500 SF			

TOTALS 1700 SF + 400 SF + 41 SF = 2141 SF

Scale
 1/4"=1'-0"

A4.1

Scale 0.8"=50ft



- A: Driveway 12', Shoulder 3'
- B: Driveway 12', Gate Column-Column 17'-7", Gate Hinge-Hinge 16'-11" (details next page)
- C: Fence-Fence 19'-4"
- D: Driveway 14'-6", Shoulder 3'
- E: Driveway 13', Shoulder 3'
- F: Driveway 12', Shoulder >3'
- G: Driveway 18'-6", Shoulder >3'
- H: Driveway 15', Shoulder >4'
- I: Driveway 18", Shoulder 3'
- J: 12' Driveway to Property Line: 175'
- K: Property line to the 1st Turnout: 40'
- L: Gate to Property Line 80'



Scale 0.8"=50ft



B: Driveway at Gate 12'



B: Gate Column-Column 17'-7"



B: Gate Hinge-Column: 4"
Gate Hinge-Hinge: 16'-11"



C: Fence-Fence 19'-4"



Detention Facility Calculation

Area 250.00 x 154.00 38500 sf **0.88** acres
t_c 0.0078(L²/S)^{0.385} + 10 = **11.6** minutes
 L 250 ft
 S 0.06 ft/ft

Runoff Coefficient C = 0.35

x_{T,D} Depth 10-year storm at **20** **0.3491** inches
 $x_{T,D} = A_{T,D} + (B_{T,D}MAP)$ (Figure A-2)

T(min)	A _{T,D}	B _{T,B}
10	0.258682	0.003569
11.6	0.270371	0.003938
15	0.294808	0.004710

i_{T,D} Intensity $i_{T,D} = x_{T,D}/D$ **1.8031** in/hr
D = x_{T,D}/60 min/hr

DRAINAGE SUMMARY:

THE NEW IMPERVIOUS AREA (BUILDING DOWN SPOUTS) WILL BE DIRECTED BY OVERLAND FLOW TO THE RETENTION BASIN/AREA. THE REQUIRED VOLUME OF STORAGE OF 424 CUBIC- FEET WILL BE CONTAINED IN THE 14,000 CUBIC-FOOT RETENTION BASIN/AREA. ONCE THE RETENTION BASIN/AREA IS FULL IT WILL SHEET FLOW TO THE EAST AS IT DOES IN THE CURRENT CONDITION. 3,200 GALLONS

T	A _{T,D}	B _{T,B}	10-Yr Depth	Volume In	Volume Out	Storage
5-min	0.201876	0.002063	0.2473	461	106	355
10-min	0.258682	0.003569	0.3372	629	212	416
15-min	0.294808	0.004710	0.3984	743	319	424
30-min	0.367861	0.007879	0.5412	1009	637	372
1-hr	0.427723	0.014802	0.7534	1405	1274	130
2-hr	0.522608	0.027457	1.1267	2101	2548	-448
3-hr	0.591660	0.038944	1.4484	2700	3822	-1122
6-hr	0.625054	0.070715	2.1808	4066	7645	-3579
12-hr	0.641638	0.111660	3.0982	5776	15289	-9513

10 year - Peak Runoff Rate: CIA = 0.5578 cfs

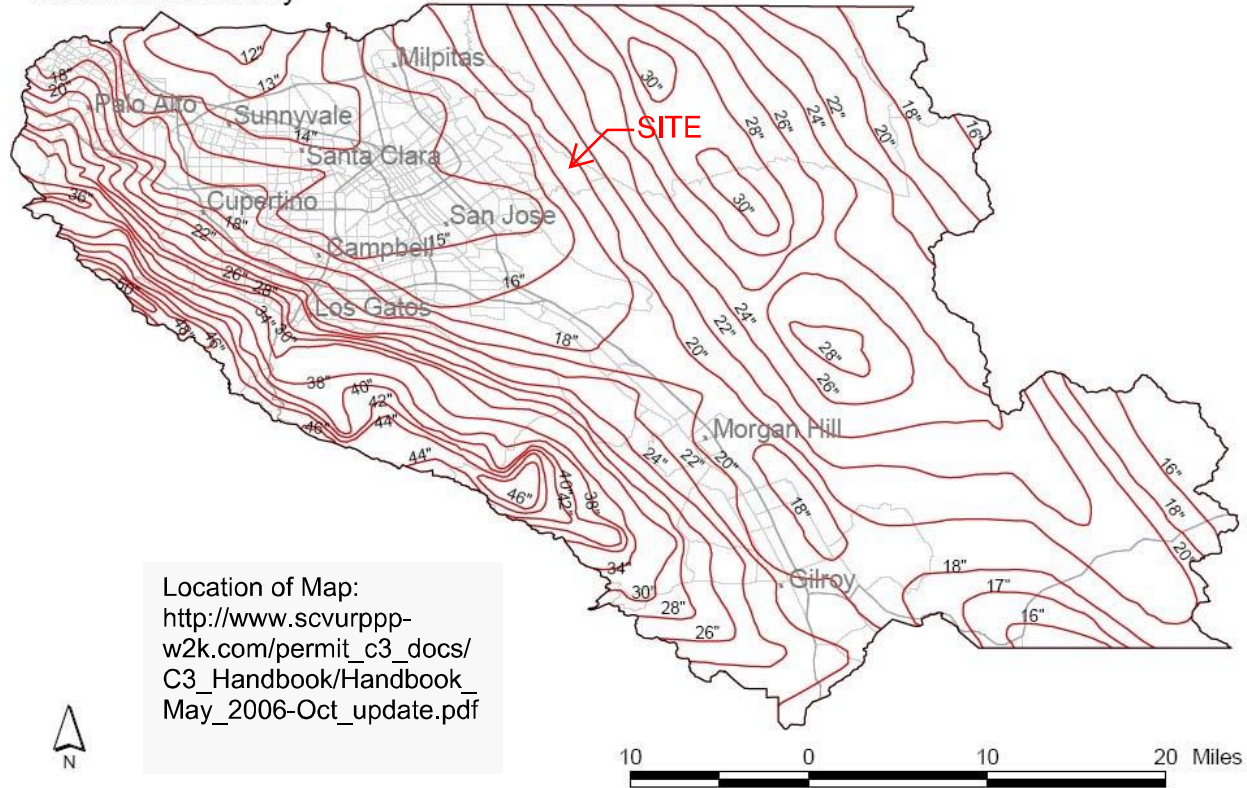
Future Condions **6245** sf

Proposed C % Area
 Roof/Driveway 16.22% 0.85
 Landscape 83.78% 0.35 **0.4311 0.58**





Figure A-2
Mean Annual Precipitation Map
Santa Clara County



SOURCE: Santa Clara Valley Water District, Mean Annual Precipitation Map, San Francisco & Monterey Bay Region, 1998

Figure A-2: Mean Annual Precipitation, Santa Clara County

TRAFFIC LOADING CALCULATION EXAMPLE

The following calculations demonstrate that Belgard permeable interlocking concrete pavement systems satisfy the requirements of meeting or exceeding a 75,000 lb theoretical design load as compared to the required compressive strength of pavers per the requirements in *ASTM C936 Standard Specification for Solid Concrete Interlocking Paving Units (average compressive strength of 8,000 psi)*.

(see attached document for additional information on GVWR, Tire Contact Area, Axle Load Distribution, and Contact Area Pressure Calculations)

Step #1) Determine the maximum wheel load:

GVWR = 76,800 lbs

GAWR (front axle) = 22,800 lbs

GAWR (rear axle) = 54,000 lbs

Front Axle Wheel Loads

$W_L = 22,800 \text{ lb} / 2 \text{ tires per axle} = 11,400 \text{ lbs per tire}$

Rear Axle Wheel Loads

$W_L = 54,000 \text{ lb} / 2 \text{ axles} / 4 \text{ tires per axle} = 6,750 \text{ lbs per tire}$

Step #2) Increase the load by 30% to account for dynamic forces associated with moving vehicles:

Front Axle: $W_{L-Dynamic} = 11,400 \text{ lbs} \times 1.30 = 14,820 \text{ lbs per tire}$

Rear Axle: $W_{L-Dynamic} = 6,750 \text{ lbs} \times 1.30 = 8,775 \text{ lbs per tire}$

Step #3) Determine the tire contact area:

The Contact Area was measured for a Pierce Fire Truck (GVWR = 76,800 lbs) and was determined to be 104 in² per front tire, and 90 in² per rear tire. (See measurements in the attached document)

Step #4) Determine the stress exerted per tire in the dynamic load:

$$\sigma_{\text{tire}} = \frac{W_{L-dynamic}}{A_{\text{contact}}} = \frac{14,820 \text{ lbs}}{104 \text{ in}^2} = 142.5 \frac{\text{lbs}}{\text{in}^2}$$

Step # 5) Compare Belgard PICP to GVWR = 76,800 lbs:

Belgard permeable interlocking concrete pavers are manufactured to ASTM C936 standards requiring an average compressive strength of 8,000 psi. As illustrated above, the maximum theoretical tire pressure exerted on the pavement surface is 142.50 psi so stresses are effectively transferred to the base and subgrade using Belgard permeable pavers. This significant factor of safety makes Belgard permeable pavers viable solution for a flexible pavement system subjected to Fire Truck Loading.



**Design Considerations for
Concrete Paver Surfaced Access Lanes
Subjected to Fire Truck Loading
January 25, 2018**

A common question posed by design professionals is the ability of interlocking concrete pavers (ICPs) and permeable interlocking concrete pavers (PICPs) to withstand fire truck loading. This is due to the relatively large axle weights they exert along with the fact that fire trucks are critical service vehicles that must be able to access sites in emergency situations. As with other heavily loaded vehicles, like trash trucks and some delivery vehicles, fire trucks typically exhibit relatively large axle loads which apply critically high service loads for which pavements must be designed.

In terms of structural design for entry, access lane and roadway applications, pavements must be designed to resist rutting, bearing capacity of the supporting pavement system and resistance to repeated axle load applications. Pavement design procedures typically utilize information which describe the strength of the subgrade soils, axle loadings and frequency, and strength of the various layered pavement components. The actual design procedures for flexible and rigid pavements are well documented in Civil Engineering texts with ICPs & PICPs well recognized to behave and follow the design procedures set forth for “flexible” pavement design. References for ICP and PICP pavement design are provided in the Appendix to this report (ASCE 2016, Caltrans 2016, ICPI 2011, UC Davis 2010).

While not a comprehensive primer on pavement design, the focus of this report is to demonstrate that ICP and PICPs are not adversely affected by heavily loaded vehicles and are suitable for use in vehicular areas exposed to fire truck loadings. The primary discussion herein will focus on fire truck loadings on ICP/PICP systems as they relate to:

- Design ESALs applied to the pavement system.
- Fire truck wheel and axle loads relative to the strength of the paver
- Point loads that may occur when the stabilizer outriggers are in place.

Because this document focuses on fire truck loading, data on a typical heavily loaded fire truck was obtained for a “ladder truck” used by the City of Scottsdale, AZ. The vehicle chosen was the heaviest vehicle in the Fire Departments fleet and is considered to be on the upper end of the fire vehicle loading spectrum. A few images of the vehicle and its characteristics are provided below:



Pierce
 Manufactured by: Pierce Manufacturing, Inc.
 - Custom Designed and Manufactured Exclusively For -
CITY OF SCOTTSDALE

Yr of Mfg	Aug - 2008	Job No.	20203	WO No.	8648384
GVWR	34,836 KG (76,800 LB)	Tire-Limited Max Speed	65 mph	Chassis	Velocity
GAWR		TIRES	RIMS	COLD TIRE INFLATION	
Front	10,342 KG (22,800 LB)	425/65R22.5 (L)	22.50x12.25	827 kPa (120 PSI)	SINGLE
Rear	24,494 KG (54,000 LB)	12R22.5 (H)	22.50x8.25	827 kPa (120 PSI)	DUAL

VEHICLE CONFORMS TO ALL APPLICABLE U.S. FEDERAL MOTOR VEHICLE SAFETY STANDARDS IN EFFECT ON THE DATE OF MANUFACTURE SHOWN ABOVE.



City of Scottsdale AZ Ladder Engine

Pierce Manufacturing ID Decal; GVWR - 76,800 lbs
 GAWR – Front=22,800 lbs = 11,400 lbs/tire:
 GAWR - Rear=54,000 lbs = 27,000 lbs/axle; 6750 lbs/tire
 Cold Tire Inflation Pressure – 120 psi (single and dual)
 Max Load per Single Tire – 11,400 lbs



Approximate Front Tire Contact Area = 13" x 8" = 104 in²
 Max Load / Tire at 120 psi cold = 11,400 lbs = 95 in² minimum contact area



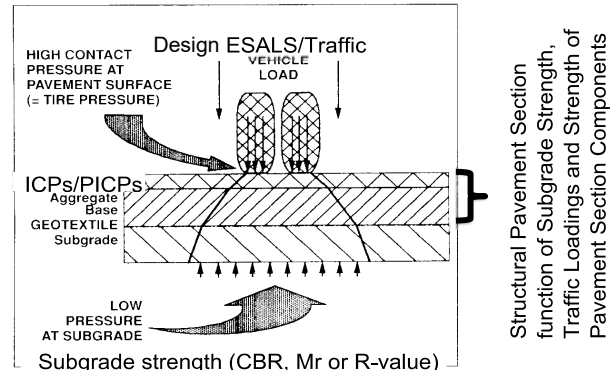
Tire Contact Area = 10" x 9" = 90 sq. in./tire
 GAWR Rear = 54,000 lb/2=27,000 lbs=Single Axle Load
 Load per Dual set of Tires = 27,000 lbs/4 tires = 6750 lbs
 Contact pressure is approximately = 75 psi

DESIGN ESALS

Design references have been developed by several credible organizations including ASCE, ICPI and Caltrans as shown below. In almost all cases, the design guidelines for the structural aspects of the pavements are based on the *1993 AASHTO Guide for Design of Pavement Structures* (AASHTO 1993). As with the design references for ICP and PICPs, the 1993 AASHTO document

calculates the thickness of a roadway cross section required to withstand the applied loads for the given lifespan based on the native soils strength and traffic loading. The supporting soil strength is typically described by a CBR value (California Bearing Ratio), M_r (Resilient Modulus), R-value or some other geotechnical measurement describing the strength of the supporting soil.

The traffic loading is typically described by TI (Traffic Index), ESALs (Equivalent Single Axle Loads) or other measurement to express the traffic type and equivalent damage (VLF, Vehicle Load Factor) created by each type of vehicle as compared to the passage of a “standard” 18,000-pound axle load (one 18,000 lb ESAL provides a unit value of 1.0). For perspective on ESALs, passenger cars have a Vehicle Load Factor (VLF) of 0.0004 whereas a fully loaded fire truck as shown above would have a VLF of about 7.0. Hence, it would take about 17,500 autos to effect the same level of deterioration on the pavement as 1 pass of a fire truck. It should be noted that not all fire trucks exhibit this same degradational effect on pavements as most are lighter and exhibit lower axle loads than the Ladder Truck which has a GVWR (Gross Vehicle Weight Rating) of 76,800 lbs.

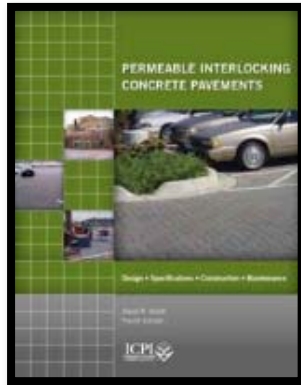


Although it is evident by the VLFs shown above that fire trucks can exert high ESALs on the pavement surface, it is important to note that roads are designed around hundreds of thousands, if not millions of ESALs, so the impact of the occasional fire truck is actually marginal. Notable in the design procedure is that the axle/tire loads applied to the completed pavement system is transferred through the pavement to the subgrade via a series of structural layers which distribute the vehicle loads to a relatively large area of the subgrade. The distribution of the loads through the pavement system enables relatively weak subgrades to support very high concentrated axle/wheel loads much like a snowshoe for a trapper or wide tracks of low ground pressure vehicles to traverse low strength materials which would otherwise not support the weight of applied loads. Along those same lines, pavement design isn't so much about how much a vehicle weighs but rather the load transfer of axle loads through the pavement system and how many passes can be achieved prior to development of unacceptable rutting or excessive pavement deterioration.

To further expand on this subject, pavement sections for standard asphaltic concrete (AC) and aggregate base systems and interlocking concrete pavement (ICP) systems are essentially identical in thickness with the wearing course being the primary difference in the systems. In essence, an 80mm (3-1/8") thick paver laid on 1" of bedding sand provides a layer coefficient of 1.82 which is the same as 4-1/8" of asphaltic concrete having a structural coefficient of 0.44/inch ($0.44 \times 4-1/8 = 1.82$). The aggregate base and subbase section used to distribute the wearing course loads provide the same support to either an AC or ICP system. The above analogy can be verified by comparing section thicknesses for designs done in accordance with AASHTO (AASHTO 1993) and ICPI (ICPI 2011) or ASCE (ASCE 2016) methods.

Permeable Interlocking Concrete Pavement systems (PICPs) employ essentially the same wearing surface (typically an 80mm paver underlain by 2" of No. 8 stone) and open graded load transfer/water storage/conveyance layer aggregate (No. 57 and No. 2 stone). Because of the introduction of water and the somewhat less dense structure of the base layers, a somewhat lower

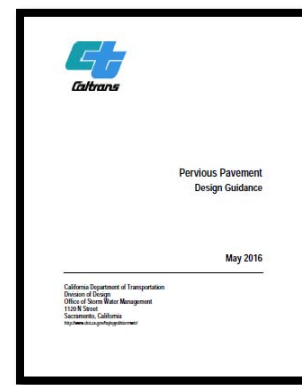
structural coefficient is employed for the components that make up the PICP system than what is used for conventional ICP systems but the design concept is identical.



ICPI – Permeable Interlocking
Concrete Pavements



ASCE 58-16 – Structural Design
of Interlocking Concrete Pavement
for Municipal Streets
and Roadways



Caltrans – Porous Pavement
Design Guidance

TIRE CONTACT PRESSURES

In terms of being able to withstand the surface pressure exerted by fire truck tires, the gross axle weight rating (GAWR) on a two tire (steering) axle and tandem axle (rear axles) for the 76,800 lb fire truck referenced above is 22,800 lbs and 54,000 lbs, respectively. Each tire is rated at a max load rating of 11,400 lbs at a cold inflation tire pressure of 120 psi. Checking the expected contact area between the pavement and tire yields $11,400 \text{ lb} / 120 \text{ psi} = 95 \text{ in}^2$ minimum contact area for the front tires. Physical measurements of the actual tire/pavement contact area are shown in the images above and results in an area of about $13'' \times 8'' = 104 \text{ in}^2$ which is within tolerance of the measurement methods employed. Likewise, the measured contact area of the rear tires was determined to be about $10'' \times 9'' = 90 \text{ in}^2$ for each tire or about $90 \text{ in}^2 / \text{tire} \times 8 \text{ tires} = 720 \text{ in}^2$ to support 54,000 lb rear axle load resulting in a tire bearing pressure on the pavement of 75 psi. The 75 psi pressure as measured in the fire station bay suggests that the truck was not fully loaded and hence not exerting the full 120 psi pressure of the tire rating. Based on the above, it can be concluded that the 120 psi contact pressure of the tires is reasonable for both front and back tires.

Any concrete paver offered under the Belgard line is made in accordance with ASTM C936, which requires an average compressive strength of 8,000 psi with no individual unit being less than 7,200 psi. So, simply put, the pavers are on average $7200 / 120 = 60$ times stronger in compression than required to withstand the surface pressure that would be exerted under the extreme loading conditions imposed by fire truck traffic.

POINT LOADS

When the stabilizer outriggers are in place, a point load of as much as 45,000 pounds can be applied to the pavement surface. Although significant, when distributed over an “unfactored” stabilizer plate surface area of 0.97 square feet (area of 10x14 inches), this equates to a surface pressure of 322 psi, which again is well within the compressive strength capabilities of Belgard pavers.

PAVER DAMAGE

As a final thought, should one or more pavers become damaged, individual units can be removed and replaced without compromising the structural integrity of the system (instruction manual available upon request).

EXAMPLE PROJECTS

In order to demonstrate the acceptance of Belgard pavers in the local market, a sample list of **permeable interlocking concrete paver (PICP)** installations is provided. The list includes project from CA which are in fire access lanes and subject to heavy vehicle loadings. Similar lists are available for ICPs upon request.

Permeable Paver Installations Subject to Fire Truck Loading

- Shearwater Creek Townhomes 72,000 SF
 - Pujol St, Temecula / Entrances and interior streets – Fire Truck rated
- Villa Catalina Condos 4,100 SF
 - 616 Catalina Ave, Redondo Beach – Entrance and shared motor courtyard – Fire truck rated
- Villa Catalina Condos 5,600 SF
 - 618 Catalina Ave, Redondo Beach – entrance and shared motor courtyard – Fire truck rated
- Hilton Garden Inn 5,700 SF
 - 4216 El Camino Real, Palo Alto – Entrance Fire truck rated
- Buffalo Wild Wings 7,900 SF
 - 845 East Ave, Chico – parking Fire truck rated
- Tahizzle Communities 7,600 SF
 - Truckee, Tahoe
- San Jose Downtown Health Center 12,000 SF
 - 777 E Santa Clara St, San Jose – Drive Entrance/Parking areas/Travel Lanes
- Wisdom Way 8,900 SF
 - 1898 Wisdom Way, Modesto – Parking Lot with Heavy Vehicle Access
- Avanti 64,000 SF
 - 23600 Park Sorrento Calabasas – Entrances and interior streets – Fire Truck rated
- Wren Avenue & Byer St. Pedestrian Crosswalk 9,000 SF
 - Wren Ave. & Byer St, Gilroy – Street crosswalks – Fire truck rated
- Vista Rio Apartments 46,000 SF
 - 3901 Briggs St, Jurupa Valley – Entrances and interior streets – Fire truck rated
- Siena Apartments 69,000 SF
 - 7801 Juniper Ave, Fontana – Entrances and interior streets – Fire truck rated.
- 4100 Del Rey Apartments 7,600 SF
 - 4100 Del Rey Ave, Marina Del Rey – Parking areas – Fire truck rated.
- Fountain Valley Town Center 6,500 SF
 - Fountain Valley – Parking/Fire Truck Rated
- Aldi Store #19 6,200 SF
 - 112 Lakewood Blvd, Downey – Parking Lot/Drive Entrances
- Chick Fil A Restaurant 3,200 SF

- 3771 E Thousand oaks Blvd, Thousand Oaks – Parking areas – Fire truck rated
- ColRich Camel Row 24,000 SF
 - Camel Country Rd & Camel Mtn. Rd, San Diego – Drive Entrances/Community Roadways – Fire Truck Rated
- San Leandro Tech Campus 10,000 SF
 - 1333 Martinez, San Leandro – Entrance to University Parking Lot/Parking Lot/Heavy Traffic/Entrance to Parking Garage
- Gateway Shopping Center / Sprouts Market 20,000 SF
 - 1300 Pinole Valley Rd, Pinole – Shopping Center Entrance/Parking/Drive Aisles
- Bardis Homes – 60,000 ft2
 - Sacramento, CA – Entrances, Streets all pavements through-out new residential development – Fire Truck Rated.
- Northwest Land Park – 90,000 ft2
 - Sacramento, CA – Community Entrances, streets and drive lanes of new housing development – Fire Truck Rated.
- Calvin Christian Church – 6,000 ft2
 - Escondido – Fire Lane access to rear of building.

REFERENCES

AASHTO 1993. Guide for Design of Pavement Structures, American Association of State Highway and Transportation Officials, Washington, DC.

ASCE 2016. Structural Design of Interlocking Concrete Pavement for Municipal Streets and Roadways, ASCE/T&DI/ICPI Standard 58-16, American Society of Civil Engineers, Reston , VA.

Caltrans 2016. Pervious Pavement Design Guidance, California Department of Transportation, Division of Design, Office of Storm Water Management, Sacramento, CA.

ICPI 2011. Permeable Interlocking Concrete Pavements, Fourth Edition, interlocking Concrete Pavement Institute, Herndon, VA.

UC Davis 2010. Research Report – Laboratory Testing and Modeling for Structural Performance of Fully Permeable Pavements Under Heavy Traffic: Final Report, U.C. Davis, Department of Civil and Environmental Engineering (CTSW-RT-10-249.04), Davis, CA.