MACK'S YAKS Staging Barn

Alum Rock

10818 Crothers Rd \\
San Jose, CA
95127

Build It Green
Smart Solutions From The Ground Up

ABBREVIATIONS

A	AMPERES	DR	DOOR	INCL.	"INCLUDE, INCLUSIVE"	R	RADIUS
A.B.	ANCHOR BOLT	E.A.	EXPANSION ANCHOR	INSUL.	INSULATION	R.D.L.	ROOF DRAIN LEADER
A.F.F.	ABOVE FINISHED FLOOR	E.F.	EXHAUST FAN	INT.	INTERIOR	R.D.O.	ROOF DRAIN OVERFLOW
A.F.G.	ABOVE FINISHED GRADE	E.J.	EXPANSION JOINT	J-BOX	JUNCTION BOX	R.O.	ROUGH OPENING
A/C	AIR CONDITIONING	E.N.	END NAILING	JCT	JUNCTION	R.O.W. or R/W	RIGHT OF WAY
ABC	AGGREGATE BASE COURSE	E.W.	EACH WAY	JST.	JOIST	REF	REFRIGERATOR
ABS	ACRYLONITRILE-BUTADIENE-STYRENE	EA.	EACH	JT.	JOINT	REF.	REFERENCE
ABV.	ABOVE	EL	ELEVATION	K-D	KNOCK DOWN	REINF.	REINFORCED
ACB	ASBESTOS-CEMENT BOARD	ELECT.	"ELECTRIC, ELECTRICAL"	KD	KILN DRIED	REQ'D.	REQUIRED
ACOU.	ACOUSTIC	ELEV.	ELEVATOR	KO	KNOCK OUT	RET.	RETURN
ACT	ACOUSTICAL CEILING TILE	EMC	ELECTRICAL METALLIC CONDUIT	L.E.D.	LIGHT EMITTING DIODE	REV.	REVISION
ADD.	ADDITION or ADDENDUM	EMT	ELECTRICAL METALLIC TUBING	L.FT.	LINEAR FEET	RM	ROOM
AG	ABOVE GRADE	ENT	ELECTRICAL NON-METALLIC TUBING	LAM	LAMINATE	RMV.	REMOVE
AHU	AIR HANDLER UNIT	EQ.	EQUAL	LAT.	LATERAL	S.C.	SOLID CORE
AL. or ALUM.	ALUMINUM	EQUIP.	EQUIPMENT	LAV	LAVATORY	S.D.	SMOKE DETECTOR
ALT.	ALTERNATE	EST.	ESTIMATE	LD.	LEAD	S.O.V.	SHUT OFF VALVE
ANL	ANNEALED	EVAP.	EVAPORATIVE COOLER	LIN.	LINEAR	S/L	SKYLIGHT
ASPH.	ASPHALT	EWC	ELECTRIC DRINKING COOLER	LINO.	LINOLEUM	S/S	STAINLESS STEEL
AVG	AVERAGE	EXC	EXCAVATE	LT.	LIGHT	SC	SELF CLOSING
AWG	AMERICAN WIRE GAUGE	EXH.	EXHAUST	LTG.	LIGHTING	SCHED.	SCHEDULE
∠	ANGLE	EXIST. or E	EXISTING	LVL	LAMINATED VENEER LUMBER	SECT.	SECTION
B.M.	BENCH MARK	EXT.	EXTERIOR	M.B.	MACHINE BOLT	SES	SERVICE ENTRANCE SECTION
B.N.	BOUNDARY NAILING	F.A.	FIRE ALARM	M.H.	MANHOLE	SH	SHEET
B.O.	BOTTOM OF	F.C.	FAN COIL	M.I.	MALLEABLE IRON	SHT'G.	SHEATHING
B.O.F.	BOTTOM OF FOOTING	F.C.O.	FLOOR CLEAN OUT	M.O.	MASONRY OPENING	SIM.	SIMILAR
B.U.	BUILT UP	F.D.	FLOOR DRAIN	MAR.	MARBLE	SPA.	SPACE
B/C	BACK OF CURB	F.E.	FIRE EXTINGUISHER	MAS.	MASONRY	SPECS	SPECIFICATIONS
BD.	BOARD	F.N.	FIELD NAILING	MAT'L.	MATERIAL	SPKR.	SPEAKER
BLDG	BUILDING	F.O.	FACE OF	MAX.	MAXIMUM	SQ. FT.	SQUARE FEET
BLK.	BLOCK	F.S.	FLOOR SINK	MECH.	MECHANICAL	SQ. IN.	SQUARE INCHES
BLKG.	BLOCKING	F/G	FIBERGLASS	MED.	MEDIUM	STC	SOUND TRANSMISSION CLASS
BM.	BEAM	FAB.	FABRICATE	MFG.	MANUFACTURING	STD.	STANDARD
BR	BRASS	FACP	FIRE ALARM CONTROL PANEL	MFR.	MANUFACTURER	STL.	STEEL
BRG.	BEARING	FDC	FIRE DEPARTMENT CONNECTION	MIN.	MINIMUM	SUSP.	SUSPENDED
BRZ	BRONZE	FDN.	FOUNDATION	MISC.	MISCELLANEOUS	SW	SWITCH
C.A.P.	CONCRETE ASBESTOS PIPE	FHC	FIRE HOSE CABINET	MOD	MODULAR	SYM	SYMMETRICAL
C.D.	CONSTRUCTION DOCUMENTS	FIN.	FINISH	MTL.	METAL	SYS.	SYSTEM
C.I.P.	CAST IN PLACE	FL	FLOOR	MUL	MULLION	T & G	TONGUE AND GROOVE
C.J.	CONTROL JOINT	FLG.	FLOORING	N.I.C.	NOT IN CONTRACT	T.B.	THROUGH BOLT
C.O.	CLEAN OUT	FLUOR.	FLUORESCENT	N.T.S.	NOT TO SCALE	T.M.B.	TELEPHONE MOUNTING BOARD
~~C.T.~~	~~CERAMIC TILE~~	~~FP~~	~~FIRE PROOF~~	NCM	NON-CORROSIVE METAL	T.O.	TOP OF
CAB	CABINET	FTG.	FOOTING	NFC	NOT FOR CONSTRUCTION	T.O.B.	TOP OF BEAM
CAM.	CAMBER	FURN.	FURNISH	NLR.	NAILER	T.O.C.	TOP OF CURB
CCTV	CLOSED CIRCUIT TELEVISION	G.I.	GALVANIZED IRON	NO.	NUMBER	T.O.F.	TOP OF FOOTING
CEM.	CEMENT	GA.	GAUGE	NOM.	NOMINAL	T.O.J.	TOP OF JOIST
CER	CERAMIC	GALV.	GALVANIZED	O.C.	ON CENTER	T.O.M.	TOP OF MASONRY
CFM	CUBIC FEET PER MINUTE	GAR.	GARAGE	O.D.	OUTSIDE DIAMETER	T.O.S.	TOP OF SLAB
CH or ⊏	CHANNEL	GFCI	GROUND FAULT CIRCUIT INTERRUPTER	O.H.	OVER HANG	T.O.W.	TOP OF WALL
CKT. BKR.	CIRCUIT BREAKER	GFI	GROUND FAULT INTERRUPTER	O.I.	ORNAMENTAL IRON	T.S.	TUBE STEEL
CL or ℄	CENTERLINE	GL	GLASS	O.R.	OUTSIDE RADIUS	T.V.	TELEVISION OUTLET
CLG.	CEILING	GLB	GLUE LAMINATED BEAM	OAI	OUTSIDE AIR INTAKE	TEL.	TELEPHONE
CLKG.	CAULKING	GM	GRADE MARK	OH	OVER HEAD	TH.	THRESHOLD
CLO.	CLOSET	GM	GATE VALVE	OPNG.	OPENING	THD.	THREADED
CLR.	CLEAR	GRC	GALVANIZED RIGID TUBING	OPPO.	OPPOSITE	THK.	THICK
CMU	CONCRETE MASONRY UNIT	GYP.	GYPSUM	P.C.	PRECAST CONCRETE	THRU	THROUGH
CNTRD.	CENTERED	GYP. BD.	GYPSUM BOARD	P.L. or ℄	PROPERTY LINE	TLT.	TOILET
COL.	COLUMN	H.B.	HOSE BIBB	P.LAM.	PLASTIC LAMINATE	TRANS.	TRANSFORMER
COMB.	COMBINATION	H.C.	HOLLOW CORE	P.O.C.	POINT OF CONNECTION	TYP.	TYPICAL
CONC.	CONCRETE	H.M.	HOLLOW METAL	PERF.	PERFORATED	UNF.	UNFINISHED
CONST.	CONSTRUCTION	H/C	HANDICAPPED	PERP. or ⊥	PERPENDICULAR	UR	URINAL
CONT.	CONTINUOUS	HDBD.	HARDBOARD	PH or ø	PHASE	V.B.	VAPOR BARRIER
CONTR.	CONTRACTOR	HDW	HARDWARE	PL.	PLASTER	V.I.F.	VERIFY IN FIELD
CU	COPPER	HGT.	HEIGHT	PL. or ℄	PLATE	VA	VOLT AMPERE
d	PENNY	HOR.	HORIZONTAL	PLAS.	PLASTIC	VCT	VINYL COMPOSITION TILE
D.F.	DRINKING FOUNTAIN	HTR	HEATER	PLUMB.	PLUMBING	VERT.	VERTICAL
D.G.	DECOMPOSED GRANITE	HVAC	"HEATING, VENTILATING & AIR CONDITIONING"	PLYWD.	PLYWOOD	W/C	WATER CLOSET
D.S.	DOWN SPOUT	HW	HOT WATER	PORC.	PORCELAIN	WDW	WINDOW
D/W	DISHWASHER	HYD.	HYDRAULIC	PREFAB.	PREFABRICATED	WCT	WAINSCOT
DBL.	DOUBLE	I.C.	INTERCOM OUTLET	PSF	POUNDS PER SQUARE FOOT	WP	WEATHER PROOF
DEMO	DEMOLITION	I.D.	INSIDE DIAMETER	PSI	POUNDS PER SQUARE INCH	WT.	WEIGHT
DIA. or ø	DIAMETER	I.F.	INSIDE FACE	PTN.	PARTITION	W/	WITH
DIAG.	DIAGONAL	ID	IDENTIFICATION	PVC	POLYVINYLCLORIDE	W/O	WITHOUT
DIM.	DIMENSION	IG	ISOLATED GROUND	PWR.	POWER	WD.	WOOD
DL	DEAD LOAD	IMC	INTERMEDIATE METALLIC CONDUIT	Q.T.	QUARRY TILE	W.I.	WROUGHT IRON
DN.	DOWN	IMPG	IMPREGNATED	QTY.	QUANTITY	YD.	YARD

ADDITIONAL DOCUMENTS & REQUIREMENTS

GEOTECHNICAL INVESTIGATION / SOIL REPORT - Attached

SPRINKLER PLAN - N/A? - Differed Submitttal

TITLE 24 CALIFORNIA ENERGY CODE COMPLIANCE - N/A

ARBORIST

DUE TO THE SCOPE OF WORK FOR THIS PROJECT, NO ARBORIST REPORT HAS BEEN PREPARED. IN THE EVENT THAT PROJECT SCOPE CHANGES, CONTACT ARCHITECT & AUTHORITY HAVING JURISDICTION (AHJ) TO DETERMINE IF AN ARBORIST REPORT IS REQUIRED. NO WORK SHALL BE COMMENCED WITHIN THE DRIPLINE OF ANY TREE PROTECTED BY ANY AHJ PRIOR TO RETAINING A LICENSED ARBORIST.

COORDINATION REQUIREMENTS

COORDINATE WITH ARCHITECT

SEE CONSTRUCTION OBSERVATION NOTE ON THIS SHEET. PROVIDE ARCHITECT WITH MINIMUM 48 HOUR NOTICE OF MILESTONE REQUIRING CONSTRUCTION OBSERVATION. COPY ARCHITECT ON ALL CORRESPONDENCE WITH ALL PROJECT CONSULTANTS.

AREA CALCULATIONS

BUILDING AREA:

MAIN LEVEL	3500sq. ft
TOTAL AREA:	3500sq. ft
TOTAL PARCEL AREA:	6.58 Acres

CONSTRUCTION OBSERVATION REQUIRED

GENERAL CONTRACTOR IS REQUIRED TO SCHEDULE & COORDINATE THE FOLLOWING MANDATORY CONSTRUCTION OBSERVATION SITE VISITS WITH ARCHITECT PRESENT. PROVIDE NOTICE TO ARCHITECT AT LEAST 48 HOURS PRIOR TO SUCH VISITS. PRIOR TO BEGINNING WORK, PROVIDE ARCHITECT & OWNER WITH A CRITICAL PATH SCHEDULE SHOWING THE FOLLOWING CONSTRUCTION MILESTONES:

INITIALS	REQD	SITE VISIT MILESTONE
______	☐	PRE CONSTRUCTION SITE MEETING
______	☒	AFTER FINISH REMOVAL, PRIOR TO STRUCTURAL DEMOLITION
______	☒	ROUGH FRAMING
______	☐	WINDOW SELECTION, PRIOR TO ORDERING WINDOWS
______	☐	ROUGH ELECTRICAL, MOUNTED BOXES PRIOR TO PULLING WIRE
______	☐	FRAMING & INSULATION, PRIOR TO COVERING FRAMING W/ FINISHES

ADDITIONALLY, CONTRACTOR SHALL SCHEDULE A MANDATORY WALKTHRU WITH ARCHITECT & OWNER PRESENT AT SUBSTANTIAL COMPLETION.

______ ☒ SUBSTANTIAL COMPLETION PRIOR TO GRANTING OCCUPANCY

ARCHITECT'S INITIALS ARE REQUIRED TO THE LEFT OF EACH SITE VISIT LISTED PRIOR TO PROCEEDING WITH SUBSEQUENT WORK & INDICATE ONLY THAT ARCHITECT WAS PRESENT & PROVIDED WITH THE OPPORTUNITY TO OBSERVE CONSTRUCTION AT THAT PHASE.

PROJECT DATA

PROJECT ADDRESS:	10818 Crothers Rd. San Jose, CA
OWNER/MANAGER:	Mack Larson & Jothi Murali-Larson
APN:	612-44-033
ZONING:	Residential /Agricultural - RR-d1 (100%)
LOT AREA:	Computed Size (GIS): 286,820 sq. ft. / 6.6 acres
BUILDING AREA:	See Area Calculations on this sheet 2
STORIES:	
CONSTRUCTION TYPE:	[1] Typ VA
FIRE SPRINKLERS:	Deferred Submittal
OCCUPANCY:	Group R-4
APPLICABLE CODES:	County of Santa Clara Municipal Code

2019 CA RESIDENTIAL BUILDING CODE
2019 CA Bldg Code, 2013 CA Res Bldg Code, 2013 CA Elec Code
2019 CA Mech Code, 2013 CA Plmbg Code, 2013 CA Energy Code
2019 CA Fire Code, 2013 CalGreen Code, 2013 CA Ref Stds Code
All as amended by The State Of California and Local Jurisdiction(s).

SHEET INDEX

PROJECT TEAM

OWNER/MANAGER
Mack Larson & Jothi Murali-Larson
larsonmack@icloud.com

DRAFTING FIRM
ECOSTRUCTION
PO BOX 62
Geyserville, CA 95441
ecostruction@att.net
831-588-0234

GEOTECHNICAL ENGINEER
BUTANO GEOTECHNICAL
213 Green Vally Rd. Suite E
Freedom, CA 95019
(831)724-2612
www.butanogeotech.com

SURVEY
CARROLL ENGINEERING
1101 S. Winchester Blvd. #H-184
San Jose, CA 95128
(408) 261-9800
philip@carroll-engineering.com

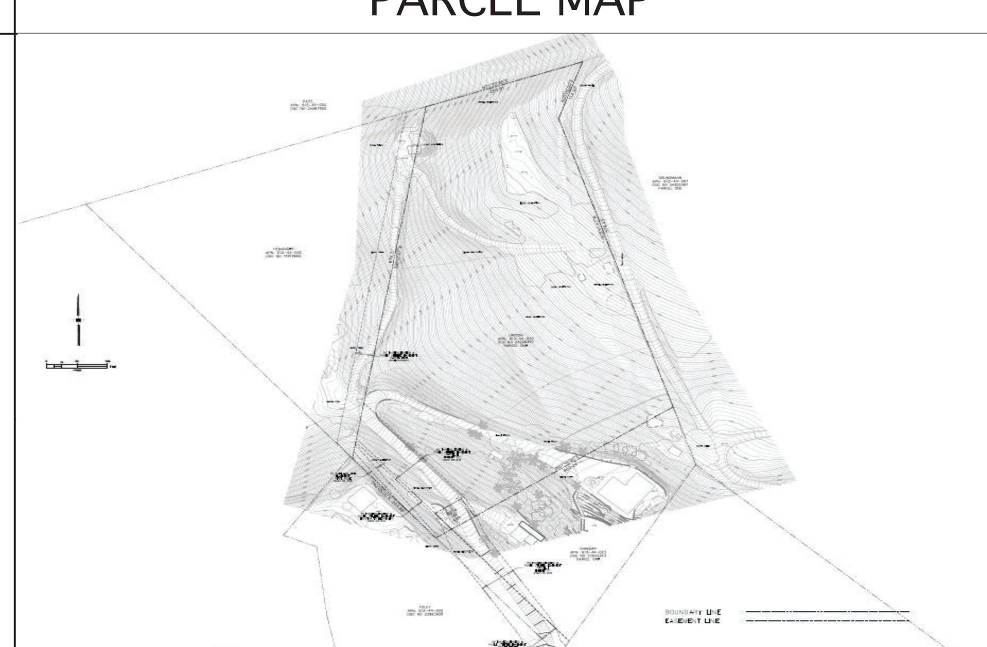
SEPTIC ENGINEER
N/A

STRUCTURAL ENGINEER
THANG LE and ASSOCIATES
319 E. Foothill Blvd.
Arcadia, CA 91006
(626) 538-2702
info@thanglese.com

CIVIL ENGINEER
DVC Group, Inc.
513 Center St.
Healdsburg, CA 95448
(707)775-8986
dan@dvcgroup.net

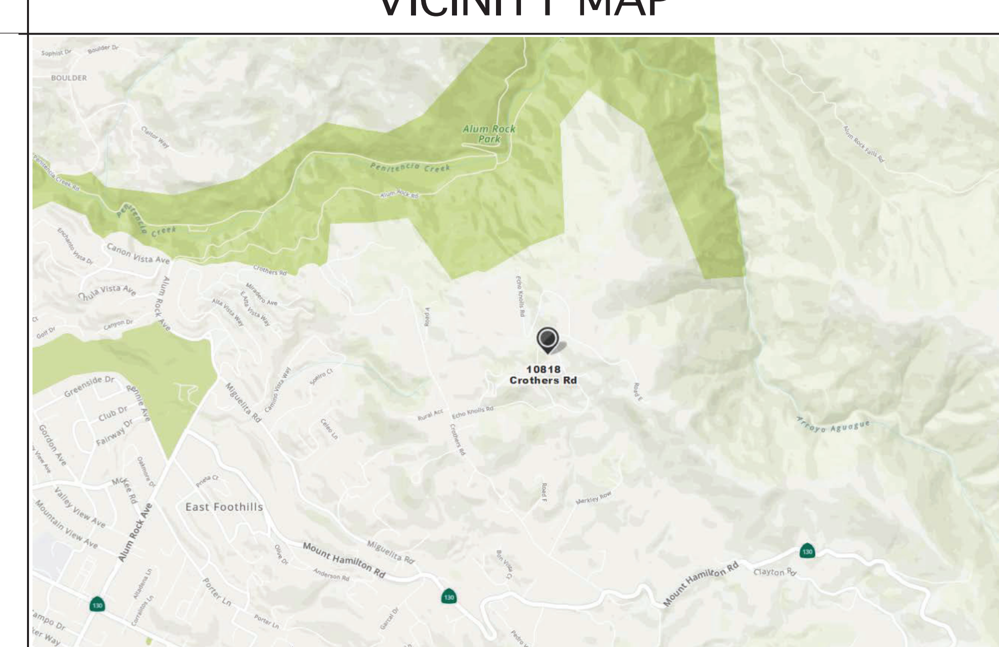
GEOLOGIST
BAYSIDE GEOLOGY
BUTANO GEOTECHNICAL
213 Green Vally Rd. Suite E
Freedom, CA 95019
(831)724-2612
www.butanogeotech.com

PARCEL MAP



Please Note: Orientation of other plans within the set may vary from this Parcel Map.

VICINITY MAP



Please Note: Orientation of other plans within the set may vary from this Vicinity Map.

PROJECT SCOPE

New 3500sq. ft Staging and Feed storage Barn

eco-struction

Print Date: 11/02/2022

Rev	Description	Date:
	Building & Site Approval Application	TBD

Project

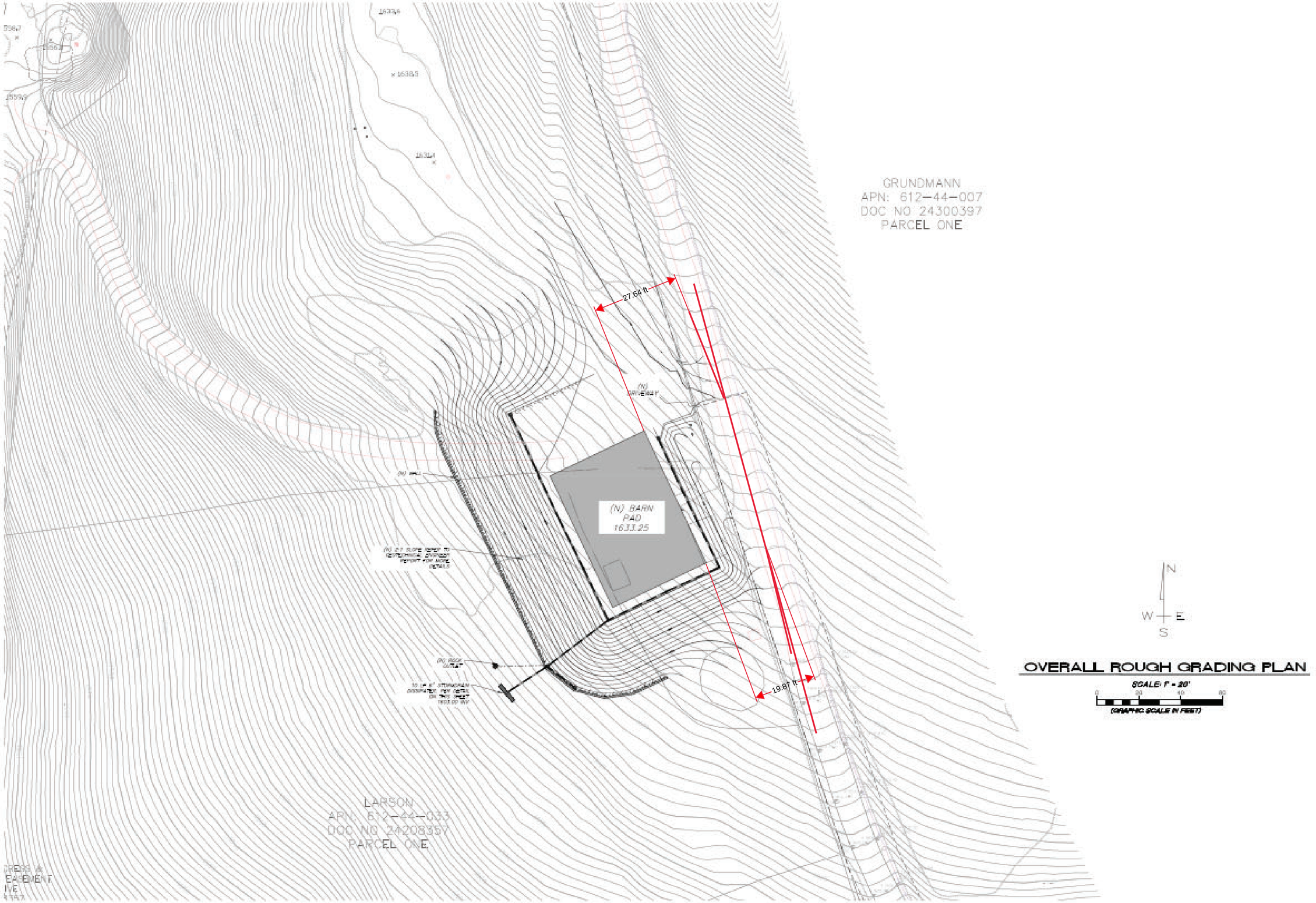
Mack'sYaks
10818 Crothers Rd
San Jose, CA 95127

Date:	Nov. 2022
Project:	#15057
Scale:	As Noted
Drawn by:	Heritage Barn

Sheet Title:

COVER SHEET & PROJECT INFO

A-1



OVERALL ROUGH GRADING PLAN

SCALE: 1" = 20'

0 20 40 80

(GRAPHIC SCALE IN FEET)

REVISED	DATE

DVC GROUP INC.
PLANNING + ENGINEERING + C.M.
676 CENTER STREET
HEALDSBURG, CA. 95448
(707) 395-0888

DANIEL JOHN MORRIS
RCE 90269

DATE

10618 CROTHERS ROAD
OVERALL ROUGH GRADING PLAN

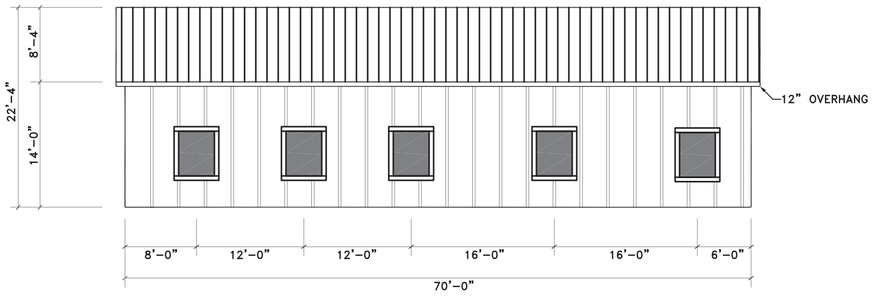
62-44-036
10618 CROTHERS ROAD
SAN JOSE CA.

DECEMBER 07, 2022

JOB NO.
06-21

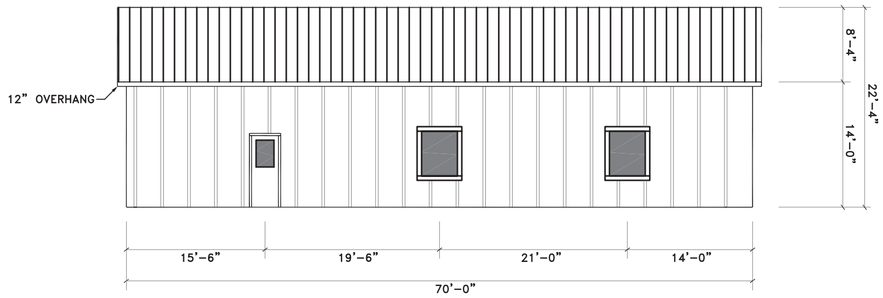
SHEET NO.
C6

OF 7 SHEETS



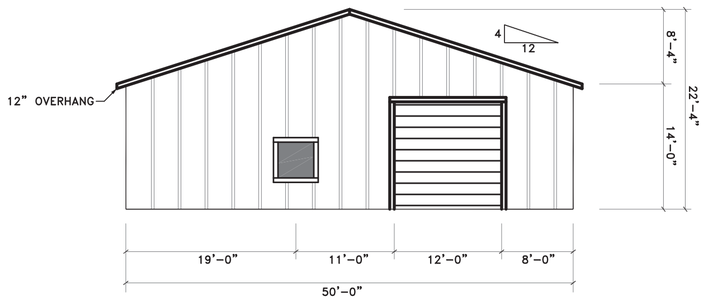
BARN – WEST ELEVATION

SCALE 1/8" = 1'



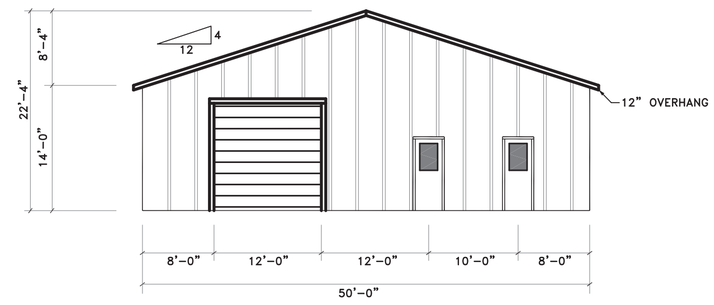
BARN – EAST ELEVATION

SCALE 1/8" = 1'



BARN – SOUTH ELEVATION

SCALE 1/8" = 1'



BARN – NORTH ELEVATION

SCALE 1/8" = 1'

eco-struction

DATE	REVISION	BY

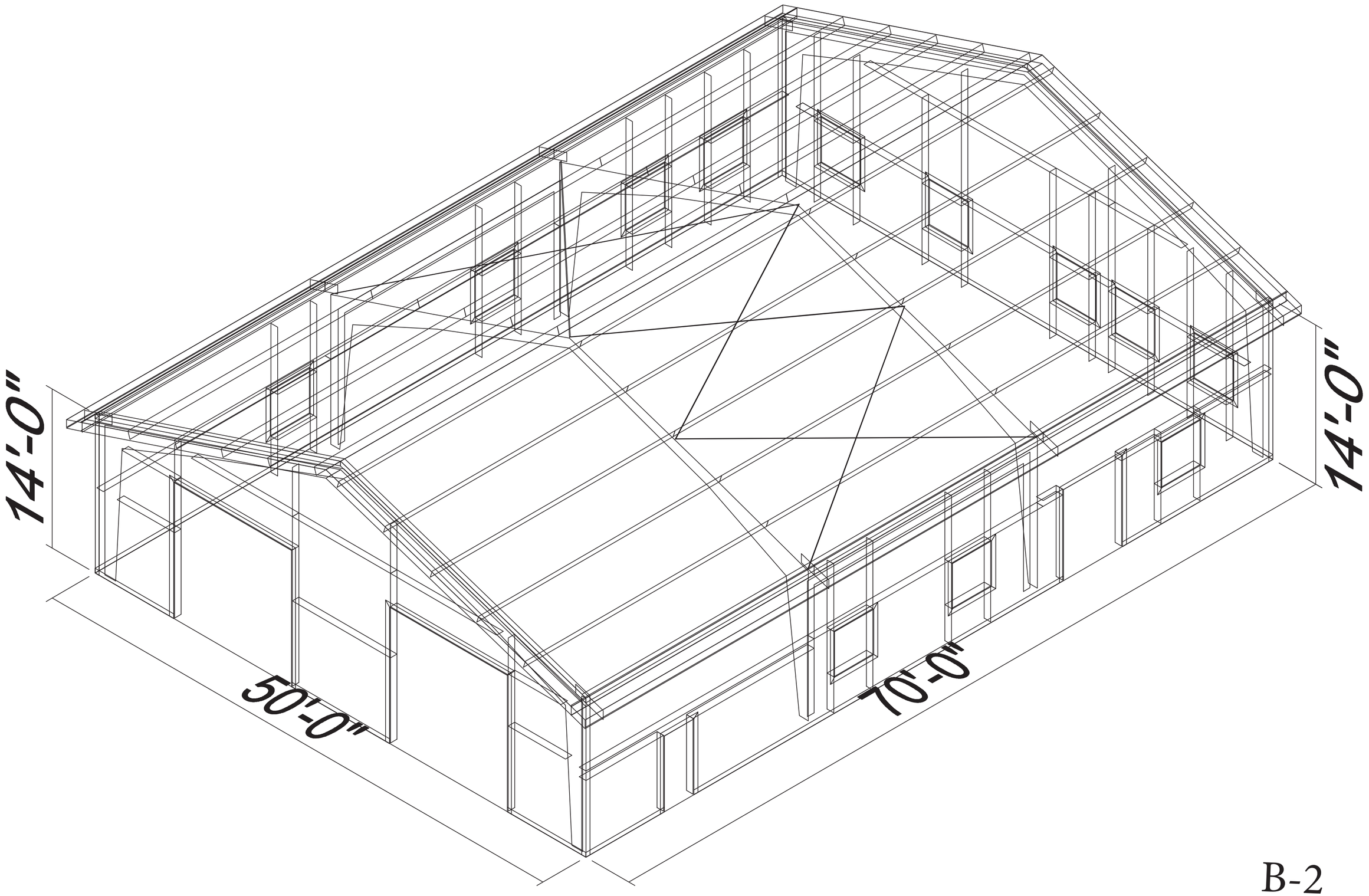
eco-struction

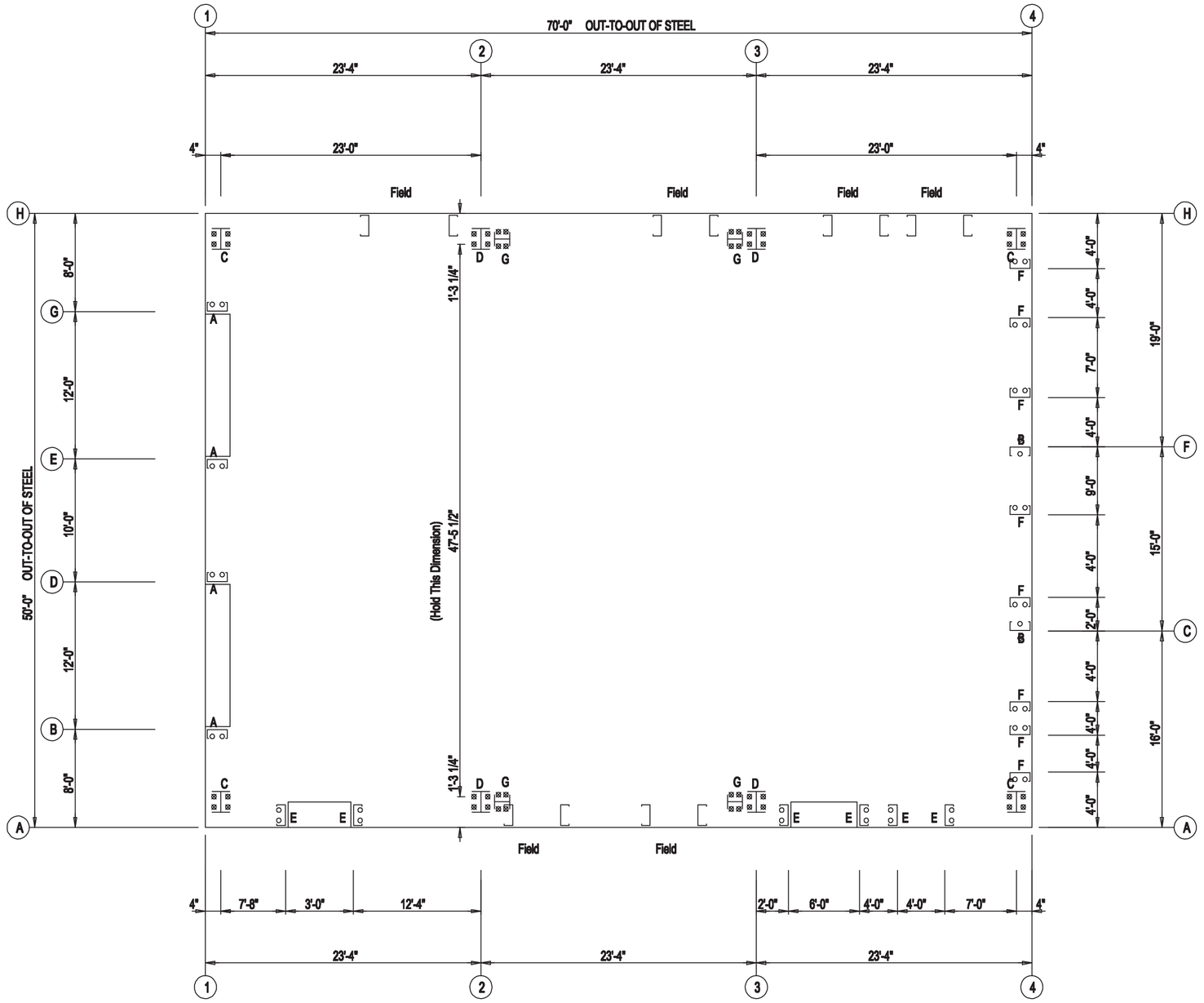
LARSON RESIDENCE
10818 Crothers Road
San Jose, CA 95127

BARN ELEVATIONS

DRAWN: NN
CHECKED: MS
DATE: 1-6-23
SCALE: 1/8"=1'

SHEET
B1


14'-0"
50'-0"
70'-0"
14'-0"



ANCHOR BOLT PLAN

NOTE: ALL BASE PLATES @ 100.0' (U.N.)
ASSUMED FINISH FLOOR @ 100.0' (U.N.)

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:
CUSTOMER: OWNER:
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		F1	0

A-3

GENERAL NOTES

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.
2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTHED OTHERWISE)
 A) A REACTION TABLE IS PROVIDED WITH REACTIONS FOR EACH LOAD GROUP
 B) RIGID FRAMES
  (1) SEE NOTE 3.
 C) ENDWALLS
  (1) SEE NOTE 3.
 D) X-BRACING
  1) X-BRACING REACTIONS ARE INCLUDED IN VALUES SHOWN IN THE REACTION TABLES AS NOTED IN THE BRACING REACTIONS TABLE.
  2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICTION FACTOR, OMEGAo.
  3) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE ENDWALL, INDIVIDUAL TRANSVERSE SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR, OMEGAo.
 E) THE METAL BUILDING MANUFACTURER IS RESPONSIBLE ONLY FOR THE PORTION OF THE ANCHOR ROD DESIGN PERTAINING TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE BEARING AND THE ANCHOR ROD'S SHEAR AND TENSION. THE METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE ANCHOR ROD EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL, AND CONSTRUCTION OF THE FOUNDATION EMBEDMENT. THE END USE CUSTOMER SHALL ASSURE THAT ADEQUATE PROVISIONS ARE MADE TO THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER COMPETENT IN THE DESIGN OF SUCH STRUCTURES.
  I) (REF. APPENDIX A3 OF THE MBMA METAL BUILDING BUILDING SYSTEMS MANUAL)
 F) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.
3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN. THE FOUNDATION ENGINEER SHALL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS FOR PROPER FOUNDATION DESIGN.
 A) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.
 B) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL WITH A LOAD FACTOR OF 1.0, AND DO NOT CONTAIN THE RHO FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIS/HER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

ENDWALL COLUMN:

ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Grout (in)
1	G	2	0.625	3.500	8.000	0.250	0.0
1	E	2	0.625	3.500	8.000	0.250	0.0
1	D	2	0.625	3.500	8.000	0.250	0.0
1	B	2	0.625	3.500	8.000	0.250	0.0
4	C	2	0.625	7.000	8.000	0.250	0.0
4	F	2	0.625	7.000	8.000	0.250	0.0

NOTES FOR REACTIONS

BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (FT)	= 50
LENGTH (FT)	= 70
EAVE HEIGHT (FT)	= 14 / 14
ROOF SLOPE (rise/12)	= 4.0:12 / 4.0:12
DEAD LOAD (psf)	= 2.500
COLLATERAL LOAD (psf)	= 6
ROOF LIVE LOAD (psf)	= 20.00
FRAME LIVE LOAD (psf)	= 20
ROOF SNOW LOAD (psf)	= 0
GROUND SNOW LOAD (psf)	= 0.00
WIND SPEED (MPH)	= 92
WIND CODE	= CBC 19
EXPOSURE	= C
CLOSED/OPEN	= Closed
IMPORTANCE - WIND	= 1.00
IMPORTANCE - SEISMIC	= 1.00
SEISMIC ZONE	= D

REACTION KEY:

WIND Left/Right 1 = (with +GCpi Internal Pressure)
WIND Left/Right 2 = (with -GCpi Internal Pressure)
Wind_Long 1 = Wind Load Case B at Left EW
Wind_Long 2 = Wind Load Case B at Right EW
MIN_SNOW = Minimum Snow (Pm) per code
E#UNB_SL_L = Endwall Unbalanced Snow Left
E#UNB_SL_R = Endwall Unbalanced Snow Right
F#UNB_SL_L = Rigid Frame Unbalanced Snow Left
F#UNB_SL_R = Rigid Frame Unbalanced Snow Right

ANCHOR BOLT SUMMARY

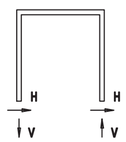
	Qty	Locate	Dia (in)	Type	Proj (in)
○	28	Jamb	5/8"	F1554	2.00
○	12	Endwall	5/8"	F1554	2.00
⊠	32	Frame	3/4"	F1554	2.50
⊠	16	WindCol	3/4"	F1554	2.50

BUILDING BRACING REACTIONS

Wall Loc	Wall Line	Col Line	Reactions in plane of wall ± Reactions(k) Wind Horz	Wind Vert	Seismic Horz	Seismic Vert	Panel_Shear (lb/ft) Wind	Panel_Shear (lb/ft) Seis	Note
L_EW	1								(h)
F_SW	A	2,3							(a)
R_EW	4								(h)
B_SW	H	2,3							(a)

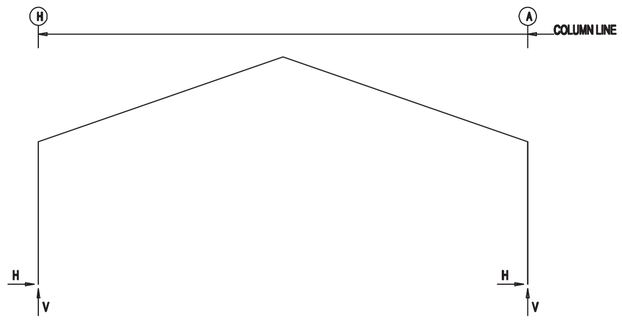
(a)Wind bent in bay
(h)Rigid frame at endwall

WIND BENT REACTIONS



Wall Loc	Wall Line	Col Line	± Reactions Wind(k) Horz	Wind(k) Vert	Seismic(k) Horz	Seismic(k) Vert	Bolt(in) Qty	Bolt(in) Dia	Base_Plate(in) Width	Base_Plate(in) Length	Base_Plate(in) Thick
F_SW	A	2	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375
F_SW	A	3	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375
B_SW	H	3	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375
B_SW	H	2	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375

FRAME LINES: 1 2 3 4



RIGID FRAME:

ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Grout (in)
1	H	4	0.750	6.000	13.50	0.375	0.0
1	A	4	0.750	6.000	13.50	0.375	0.0

RIGID FRAME:

ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Grout (in)
2*	H	4	0.750	6.000	11.50	0.375	0.0
2*	A	4	0.750	6.000	11.50	0.375	0.0

2* Frame lines: 2 3

RIGID FRAME:

ANCHOR BOLTS & BASE PLATES

Frm Line	Col Line	Anc. Bolt Qty	Anc. Bolt Dia	Base Plate (in) Width	Base Plate (in) Length	Base Plate (in) Thick	Grout (in)
4	H	4	0.750	6.000	13.50	0.375	0.0
4	A	4	0.750	6.000	13.50	0.375	0.0

RIGID FRAME:

BASIC COLUMN REACTIONS (k)

Frame Line	Column Line	Dead Horiz	Dead Vert	Collateral Horiz	Collateral Vert	Live Horiz	Live Vert	Wind_Left1 Horiz	Wind_Left1 Vert	Wind_Right1 Horiz	Wind_Right1 Vert	Wind_Left2 Horiz	Wind_Left2 Vert
1	H	0.6	1.5	1.1	2.1	3.3	7.1	-4.0	-6.6	-0.2	-4.5	-3.9	-4.7
1	A	-0.6	1.5	-1.1	2.1	-3.3	7.1	0.2	-4.5	4.0	-6.6	0.1	-2.5
4	H	0.6	1.5	1.1	2.1	3.3	7.1	-4.0	-6.6	-0.2	-4.5	-3.9	-4.7
4	A	-0.6	1.5	-1.1	2.1	-3.3	7.1	0.2	-4.5	4.0	-6.6	0.1	-2.5
2*	H	1.0	2.2	1.9	3.7	6.1	12.3	-6.0	-9.2	0.6	-6.0	-6.1	-5.9
2*	A	-1.0	2.2	-1.9	3.7	-6.1	12.3	-0.6	-6.0	6.0	-9.2	-0.5	-2.6

Frame Line	Column Line	Wind_Right2 Horiz	Wind_Right2 Vert	Wind_Long1 Horiz	Wind_Long1 Vert	Wind_Long2 Horiz	Wind_Long2 Vert	Seismic_Left Horiz	Seismic_Left Vert	Seismic_Right Horiz	Seismic_Right Vert
1	H	-0.1	-2.5	-0.7	-5.2	-1.5	-4.5	-1.7	-0.9	1.7	0.9
1	A	3.9	-4.7	1.5	-4.5	0.7	-5.2	-1.7	0.9	1.7	-0.9
4	H	-0.1	-2.5	-0.7	-5.2	-1.5	-4.5	-1.7	-0.9	1.7	0.9
4	A	3.9	-4.7	1.5	-4.5	0.7	-5.2	-1.7	0.9	1.7	-0.9
2*	H	0.5	-2.6	-0.7	-7.1	-1.7	-6.3	-2.5	-1.3	2.5	1.3
2*	A	6.1	-5.9	1.7	-6.3	0.7	-7.1	-2.5	1.3	2.5	-1.3

2* Frame lines: 2 3

ENDWALL COLUMN:

BASIC COLUMN REACTIONS (k)

Frm Line	Col Line	Dead Vert	Wind Press Horz	Wind Suct Horz
1	G	0.1	-1.2	1.2
1	E	0.1	-1.7	1.8
1	D	0.1	-1.7	1.8
1	B	0.1	-1.2	1.2
4	C	0.1	-2.2	2.3
4	F	0.1	-2.5	2.7

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS

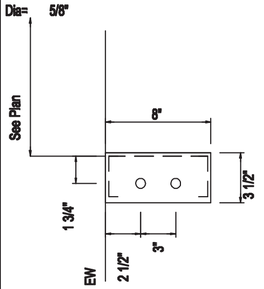
2513 MCCAIN BLVD. STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:
CUSTOMER: OWNER:
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		F2	0

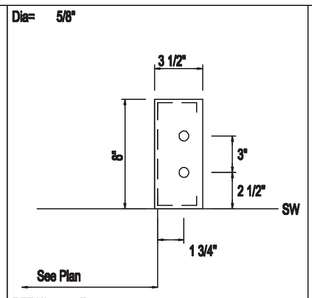
S-1

Dia= 5/8"



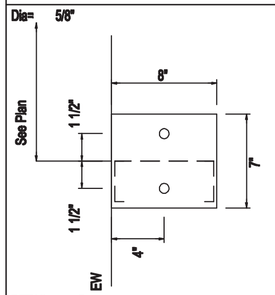
DETAIL A

Dia= 5/8"



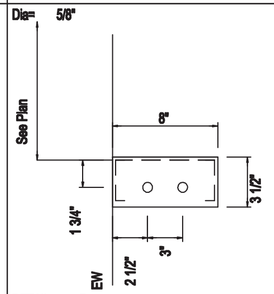
DETAIL E

Dia= 5/8"



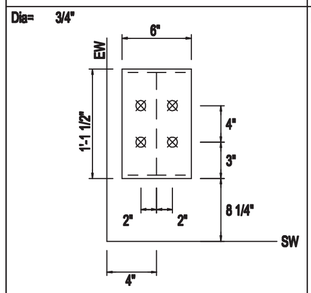
DETAIL B

Dia= 5/8"



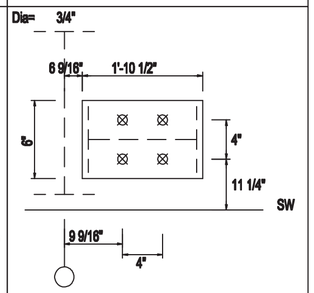
DETAIL F

Dia= 3/4"



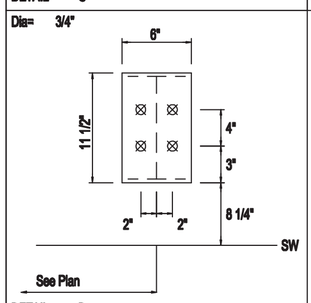
DETAIL C

Dia= 3/4"

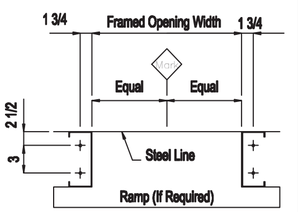


DETAIL G

Dia= 3/4"



DETAIL D



ARDia	Framed Opening AR Layout
5/8"	

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS

2513 MCCAIN BLVD. STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

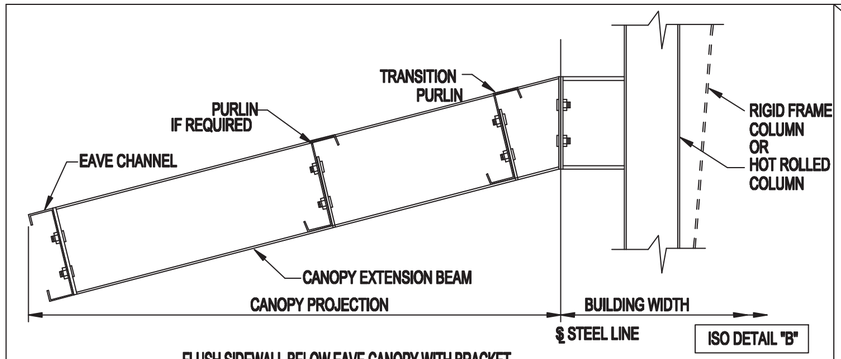
PROJECT:

CUSTOMER: OWNER:

LOCATION:

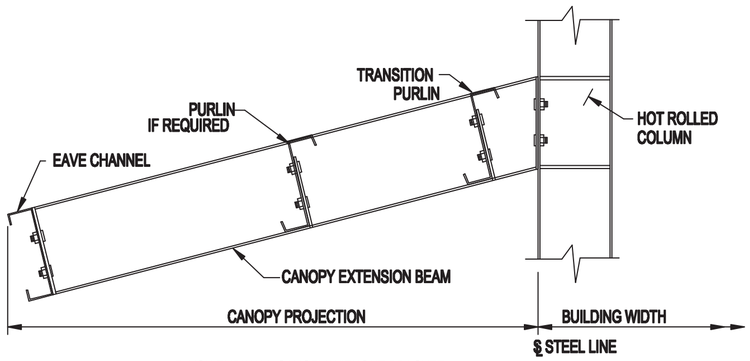
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		F3	0

S-2

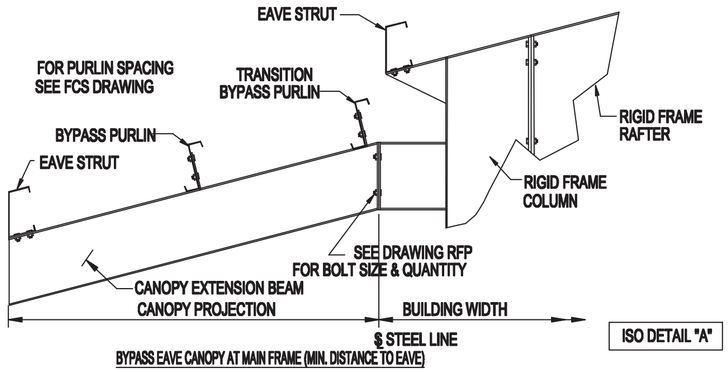


FLUSH SIDEWALL BELOW EAVE CANOPY WITH BRACKET

ISO DETAIL "B"

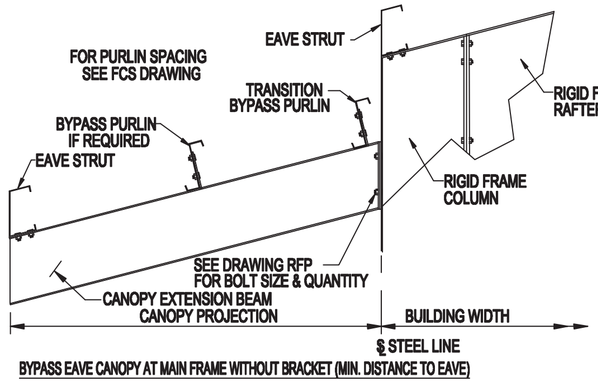


FLUSH ENDWALL CANOPY WITHOUT BRACKET

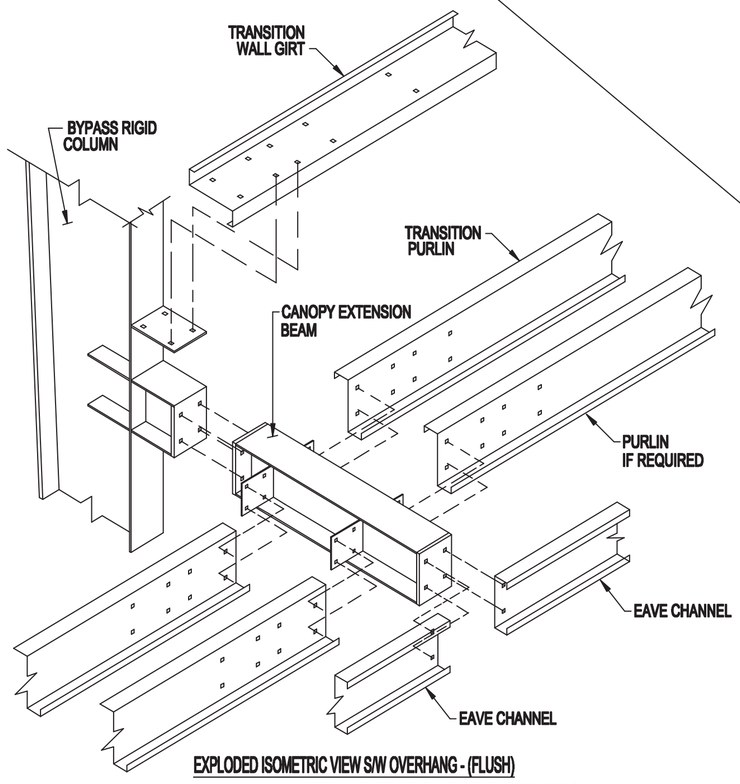


BYPASS EAVE CANOPY AT MAIN FRAME (MIN. DISTANCE TO EAVE)

ISO DETAIL "A"



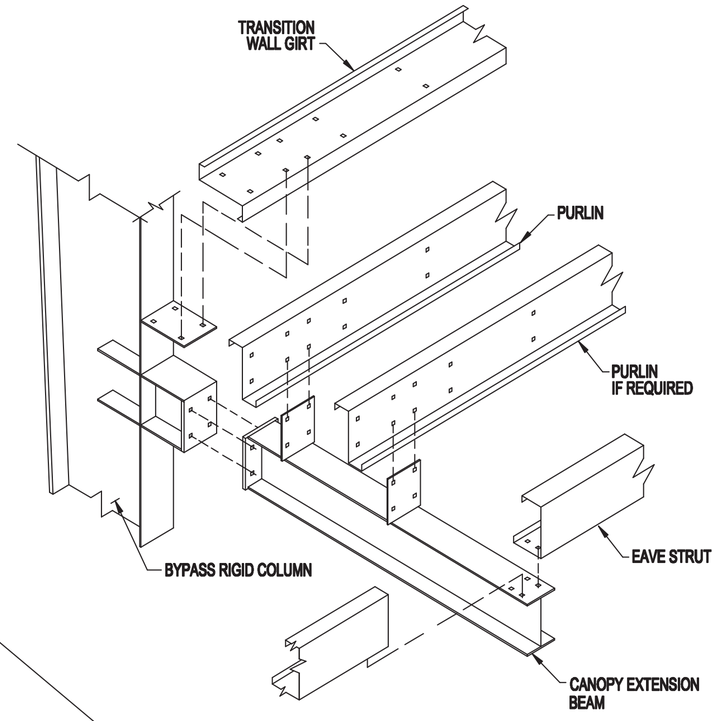
BYPASS EAVE CANOPY AT MAIN FRAME WITHOUT BRACKET (MIN. DISTANCE TO EAVE)



EXPLODED ISOMETRIC VIEW S/W OVERHANG - (FLUSH)

REFER TO SECTION FOR FASTENERS

ISO DETAIL "B"



EXPLODED ISOMETRIC VIEW S/W OVERHANG - (BY-PASS)

REFER TO SECTION FOR FASTENERS

ISO DETAIL "A"

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	9/15/16	FOR ERECTOR INSTALLATION	X	X	X

PROJECT: X

CUSTOMER: X OWNER:

LOCATION: X

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	10/07/2016	N.T.S.	1	A	X		0

S-3

GENERAL NOTES AND DETAILS

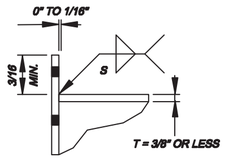
WS-1A Date Issued: July 31, 2020

FLANGE THICK	FILLET S
1/4"	3/16"
5/16"	1/4"
3/8"	5/16"

FILLETS NOT ALLOWED ON CRANE BRACKETS EXCEPT AS NOTED ON WS6. USE DETAILS B2 AND B3 ON CRANE BRACKETS WHERE FILLET WELDS ARE NOT ALLOWED.

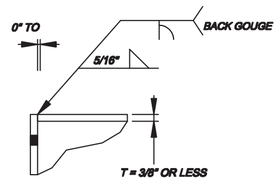
EXTENDED CONNECTION PLATE AND BRACKET TO FLANGE

B1



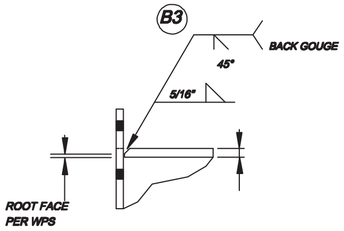
FLUSH OR RECESSED CONNECTION PLATE AND BRACKET TO FLANGE

B2



EXTENDED, FLUSH OR RECESSED CONNECTION PLATE AND BRACKET TO FLANGE

B3



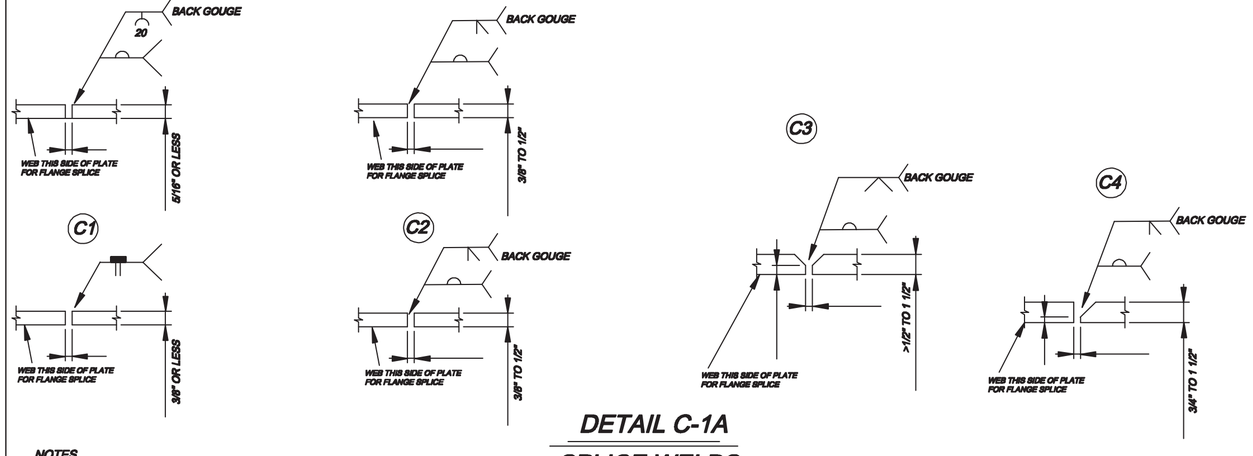
DETAIL B-1A GENERAL USAGE GUIDELINES

CONDITION	LIMIT(S)	DETAIL TO USE
FLANGE TO CONNECTION PLATE	FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8" CONNECTION PLATE IS EXTENDED PAST FLANGE ENOUGH TO ALLOW REQUIRED FILLET SIZE	B1
	FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8" CONNECTION PLATE IS FLUSH TO RECESSED TO FLANGE SURFACE	B2
	FLANGE THICKNESS GREATER THAN 3/8"	B3
CRANE BRACKET TO FLANGE	BRACKET FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8"	B2
	BRACKET FLANGE THICKNESS GREATER THAN 3/8"	B3
NON-CRANE BRACKET TO FLANGE	NON-CRANE BRACKET FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8"	B1
	NON-CRANE BRACKET FLANGE THICKNESS GREATER THAN 3/8"	B3
COLUMN FLANGE TO MOMENT BASE PLATE, STIFFENER TO CONNECTION PLATE OR ANY CONDITION CALLING FOR DETAIL B-1A BUT NOT MENTIONED ABOVE.	WELDED PLATE THICKNESS LESS THAN OR EQUAL TO 3/8" PLATE IS EXTENDED PAST WELDED PLATE ENOUGH TO ALLOW REQUIRED FILLET SIZE	B1
	WELDED PLATE THICKNESS LESS THAN OR EQUAL TO 3/8" PLATE DOES NOT EXTEND PAST WELDED PLATE ENOUGH TO ALLOW REQUIRED FILLET SIZE	B2
	WELDED PLATE THICKNESS GREATER THAN 3/8"	B3

DETAIL B-1A

SEE NOTE C2 AND C3 IN DETAIL C-1A

FLANGE TO CONNECTION PLATE AND BRACKET TO FLANGE



DETAIL C-1A

SPLICE WELDS

NOTES

C2 ANY PREQUALIFIED OR "PROCEDURE QUALIFIED" CJP WELD MAY ALSO BE USED FOR DETAILS B-1A AND C-1A, WITH A WRITTEN CBB APPROVED WPDS.
C3 SEE GENERAL NOTE 7 FOR RUN TAB REQUIREMENTS

1:2 1/2 THICKNESS TRANSITION REQUIRED FOR CANADA PROJECTS

TABLE 1
MIN. FILLET WELD SIZES

THICKER PLATE	THINNER PLATE (USUALLY THE WEB)						
	< 1/4"	1/4"	5/16"	3/8"	1/2"	5/8"	3/4"
UNDER 1/4	3/16"	-	-	-	-	-	-
1/4"	3/16"	3/16"	-	-	-	-	-
5/16 THRU 1/2	3/16"	3/16"	3/16"	3/16"	3/16"	-	-
5/8 THRU 3/4	3/16"	1/4"	1/4"	1/4"	1/4"	1/4"	1/4"
OVER 3/4	3/16"	1/4"	5/16"	5/16"	5/16"	5/16	5/16"

TABLE 2
GENERAL FILLET WELDS

THINNER PLATE THICKNESS	ONE SIDE FILLET SIZE	BOTH SIDES FILLET SIZE
UNDER 1/4	3/16"	3/16"
1/4"	1/4"	3/16"
5/16"	5/16"	1/4"
OVER 5/16	5/16"	5/16"

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

GENERAL NOTES

1. ALL SAW WEB TO FLANGE WELDS TO BE CONTINUOUS ONE SIDE ONLY UNLESS NOTED ON FABRICATION DOCUMENTS.
2. INCREASE FILLET SIZE BY 1/16 FOR EACH 1/16 OF GAP IF GAP AT ROOT IS GREATER THAN 1/16. MAXIMUM FILLET ROOT PERMITTED IS 3/16. SEE DETAIL D-1A ON THIS SHEET.
3. FILLET WELD SIZE IS NO GREATER THAN THE SHORTEST LEG SIZE.
4. FOLLOW APPROPRIATE WELDING PROCEDURE SPECIFICATIONS FOR ALL WELDS,
5. ALL CLIPS SHOULD BE WELDED AT 90-DEG TO THEIR SUPPORTING SURFACE UNLESS OTHERWISE NOTED IN THESE DRAWINGS OR THE PROJECT SHOP DRAWINGS.
6. SC2 AND SC280 FLANGE BRACE CLIPS SHOULD BE ALIGNED WITH THE PURLIN/GIRT CLIP ABOVE. REFER TO DETAIL ON SHEET WS-5. REFER TO DETAIL ON SHEET WS-5.
7. WELD RUN-TABS (RUN-ON AND RUN-OFF) SHALL BE USED ON ALL CJP CONNECTIONS, EXCEPT WHERE JOINT GEOMETRY AND/OR INTERFERENCE PREVENT THE PLACEMENT OF A WELD TAB. THIS SHALL BE DONE BY USE OF WELD TABS ALIGNED IN SUCH A MANNER TO PROVIDE AN EXTENSION OF THE JOINT PREPARATION FOR PURPOSE OF WELD PASS INITIATION AND TERMINATION. STIFFENER WELDS ENDING AT OR THE INSIDE CORNER OF THE STIFFENED MEMBER OR A CLIP NEAR THAT CORNER ARE EXAMPLES OF WELDS THAT CANNOT BE TERMINATED ON A WELD TAB. WELD RUN-TABS SHALL BE REMOVED UPON COMPLETION OF THE JOINT. ENDS OF WELDED BUT JOINTS SHALL BE FINISHED SO AS NOT TO REDUCE THE WIDTH BEYOND THE DETAILED WIDTH OR THE ACTUAL WIDTH FURNISHED, WHICHEVER IS THE GREATER, BY MORE THAN 1/8".

DETAIL A-1A

FILLET INCREASE AT SKEWED WELD JOINTS

THE FOLLOWING TABLES PROVIDE THE REQUIRED MODIFICATION TO THE REQUESTED/REQUIRED FILLET WELD BASED ON THE JOINT GEOMETRY.

NC - INDICATES THAT NO CHANGE TO THE REQUESTED/REQUIRED FILLET WELD IS REQUIRED.
CJP - INDICATES THE REQUESTED/REQUIRED FILLET WELD MUST BE REPLACED BY A CJP WELD.
+1/16 - INDICATES THE REQUESTED/REQUIRED FILLET WELD LEG SIZE MUST BE INCREASE BY 1/16" DUE TO THE SKEWED JOINT.
+1/8 - INDICATES THE REQUESTED/REQUIRED FILLET WELD LEG SIZE MUST BE INCREASE BY 1/8" DUE TO THE SKEWED JOINT.
+3/16 - INDICATES THE REQUESTED/REQUIRED FILLET WELD LEG SIZE MUST BE INCREASE BY 3/16" DUE TO THE SKEWED JOINT.
GENERAL NOTE 2 IS IN ADDITION TO ANY INCREASE SHOWN IN THE TABLES BELOW.
WHEN THE FILLET SIZE MUST BE INCREASED THE FINAL FILLET WELD SIZE SHALL NOT EXCEED 3/8". IF THE FINAL FILLET WELD SIZE EXCEEDS 3/8" THEN USE CJP.

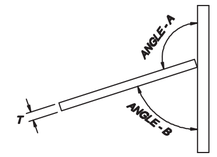
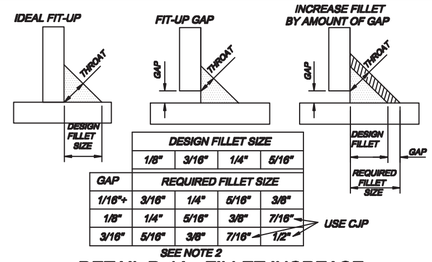


TABLE D-1
REQUIRED MODIFICATION TO FILLET WELD ON ANGLE-B SIDE OF PLATE

	ANGLE-B (DEGREES)			
T (IN.)	60< ANGLE-B ≤ 90	45< ANGLE-B ≤ 60	30< ANGLE-B ≤ 45	_ _
0.1340	NC	+1/16	CJP	
0.1560	NC	+1/16	CJP	
0.1850	NC	+1/16	CJP	
5/16"	NC	+1/16	CJP	
1/2"	NC	+1/16	CJP	
> 5/8"	NC	+1/16	CJP	

TABLE D-2
REQUIRED MODIFICATION TO FILLET WELD ON ANGLE-A SIDE OF PLATE

	ANGLE-A (DEGREES)							
T (IN.)	90 ANGLE-A< 96	96< ANGLE-A< 106	106< ANGLE-A< 119	119< ANGLE-A< 125	125< ANGLE-A< 135	ANGLE-A > 135	_ _	_ _
0.1340	NC	+1/16	+1/8	+3/16	CJP			
0.1560	NC	+1/16	+1/8	+3/16	CJP			
0.1850	NC	+1/16	+3/16	+3/16	CJP			
3/16"	NC	+1/16	+3/16	+3/16	CJP			
1/4"	NC	+1/8	+3/16	+3/16	CJP	CJP		
5/16"	NC	+1/8	+3/16	CJP	CJP	CJP		
3/8"	+1/16	+1/8	+3/16	CJP	CJP	CJP		
1/2"	+1/16	+3/16	+3/16	CJP	CJP	CJP		
> 5/8"	+1/8	+3/16	CJP	CJP	CJP	CJP		



GAP	DESIGN FILLET SIZE 1/8"	3/16"	1/4"	5/16"
	REQUIRED FILLET SIZE			
1/16"+	3/16"	1/4"	5/16"	3/8"
1/8"	1/4"	5/16"	3/8"	7/16"
3/16"	5/16"	3/8"	7/16"	1/2"

USE CJP

SEE NOTE 2

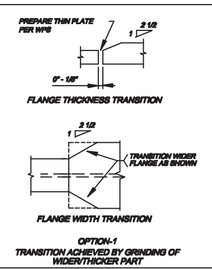
DETAIL D-1A FILLET INCREASE

PROJECT:						
CUSTOMER:				OWNER:		
LOCATION:						
CAD	DATE	SCALE N.T.S.	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER / ISSUE

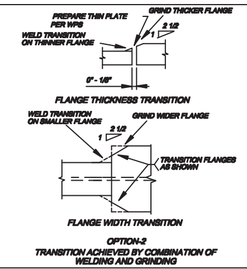
S-4

GENERAL NOTES AND DETAILS

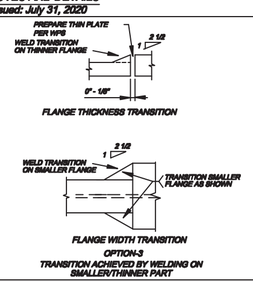
WS-1B Date Issued: July 31, 2020



OPTION-1
TRANSITION ACHIEVED BY GRINDING OF WIDER/THICKER PART



OPTION-2
TRANSITION ACHIEVED BY COMBINATION OF WELDING AND GRINDING



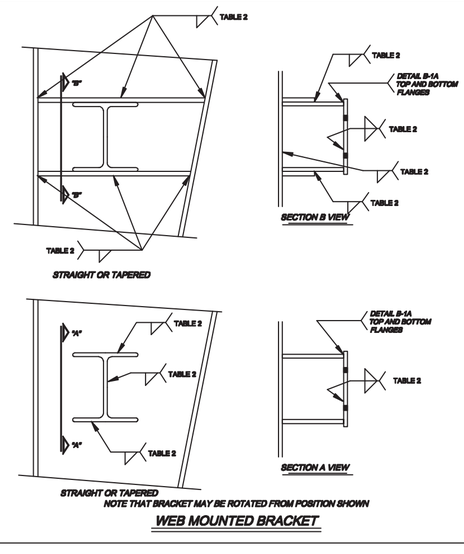
OPTION-3
TRANSITION ACHIEVED BY WELDING ON SMALLER/THINNER PART

FOR CANADA JOBS ONLY

TRANSITION CAN BE ACHIEVED BY WELDING ON THE THINNER/SMALLER PLATE (OPTION-3), BY GRINDING THE THICKNER/WIDER PLATE (OPTION-1) OR BY A COMBINATION OF BOTH (OPTION-2) TO GET THE REQUIRED 1:2 1/2 TRANSITION.
AT KNEE CONNECTION PLATES THIS TRANSITION MUST BE ACHIEVE BY USING OPTION-3, WELDING ON THE THINNER/SMALLER PLATE. DO NOT MODIFY THE KNEE CONNECTION PLATE TO GET THE REQUIRED 1:2 1/2 TRANSITION.
FOR STEPPED THICKNESS, THINNER PLATE END PREPARATION MAY BE USED IF TRANSITION OF THICKER PLATE IS PREPARED PRIOR TO WELDING.

END PLATE WELD GUIDANCE FOR GENERIC BEAMS BASED ON PARENT PIECE MARK

PARENT MARK PREFIX	DESCRIPTION	SHEET	DETAIL	SHEET DETAIL		
BB× (see note 1)	Bracket	NA	WS-2	Below Eave Bracket		
	Crane End Stop	End Plate Weld Requirements must be provided by the design engineer				
BG×	Beam, OR Girt	WS-5 M-5 and L-5	WS-5	Detail D-5		
BM×	Mezzanine Beam	WS-5 M-5 and L-5	WS-5	Detail D-5		
BP×	Purlin, Post or Column Beam	WS-5 M-5 and L-5	WS-5	Detail D-5		
BR×	Roof Beam	WS-5 M-5 and L-5	WS-5	Detail D-5		
BS×	Spandrel Beam	WS-5 M-5 and L-5	WS-5	Detail D-5		
BW×	Wall Beam	WS-5 M-5 and L-5	WS-5	Detail D-5		
BE×	Underhung Crane Bracket Extension	NA	WS-6	Underhung Crane Bracket Extension		



WEB MOUNTED BRACKET

SEAL WELD NOTES:

1. *SEAL WELDS SHALL ONLY BE PROVIDED WHEN SPECIFICALLY REQUESTED ON THE SHOP DRAWINGS.*
2. *SEAL WELDS SHALL MEET ALL THE SAME QUALITY AND WORKMANSHIP REQUIREMENTS AS ANY OTHER WELD ON A MEMBER AND CRITERIA OF SECTION 1.5.8 OF THE CSB WELD MANUAL AND CLAUSE 5.4.10 OF CSA W59-13.*
3. *SEAL WELD SIZE SHALL BE PER TABLE 2 SHOWN ON THE WELD STANDARD DRAWINGS*
4. *FOR MEMBERS REQUIRING SEAL WELDS CSB WILL PROVIDE MEMBERS TO MEET CLASS I OR CLASS II CRITERIA AS DEFINED BY AMERICAN GALVANIZERS ASSOCIATION (AGA) UNLESS OTHERWISE SPECIFICALLY REQUESTED BY THE CUSTOMER. PROPER VENTING FOR OVERLAPPING PARTS SHALL BE PROVIDED IN THE FORM OF UNWELDED PORTIONS OR HOLES PER TABLE4 SHOWN ON THIS SHEET FOR ALL MEMBERS TO BE HOT DIPPED GALVANIZED AFTER FABRICATION.*

Table 4

VENT HOLES FOR OVERLAPPED AREAS FOR STEELS 1/2 in. (12.75 mm) OR LESS IN THICKNESS		
OVERLAPPED AREA in.2(cm2)	VENT HOLES	UNWELDED AREA
UNDER 16 (103)	NONE	NONE
16 (103) TO UNDER 64 (413)	ONE 3/8 in. (1 cm)	1 in. (2.5 cm)
64 (413) TO UNDER 400 (2580)	ONE 1/2 in. (1.28 cm)	2 in. (5.1 cm)
400 (2580) AND GREATER, EACH 400 (2580)	ONE 3/4 in. (1.91 cm)	4 in. (10.2 cm)
VENT HOLES FOR OVERLAPPED AREAS FOR STEELS GREATER THAN 1/2 in. (12.75 mm) IN THICKNESS		
OVERLAPPED AREA in.2(cm2)	VENT HOLES	UNWELDED AREA
UNDER 16 (103)	NONE	NONE
16 (103) TO UNDER 64 (413)	NONE	NONE
64 (413) TO UNDER 400 (2580)	ONE 1/2 in. (1.28 cm)	2 in. (5.1 cm)
400 (2580) AND GREATER, EACH 400 (2580)	ONE 3/4 in. (1.91 cm)	4 in. (10.2 cm)

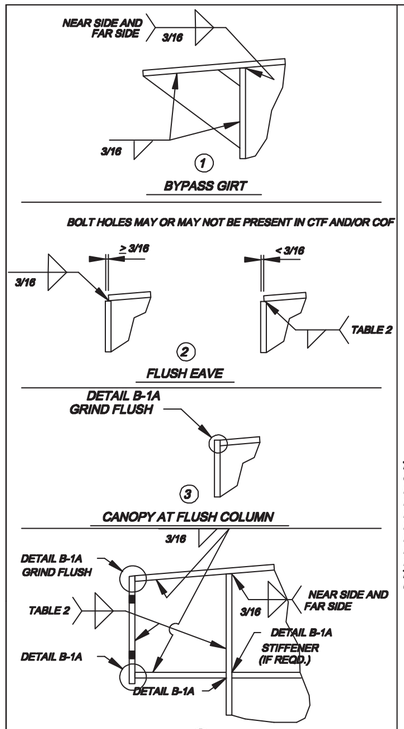
SEAL WELDS

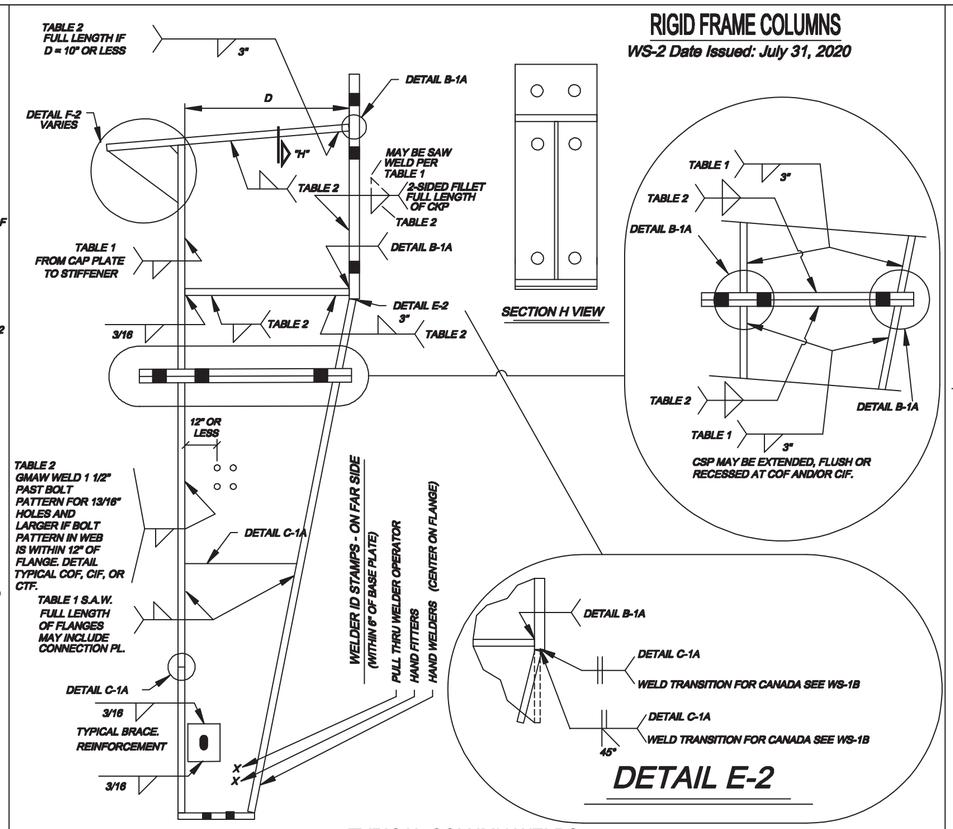
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

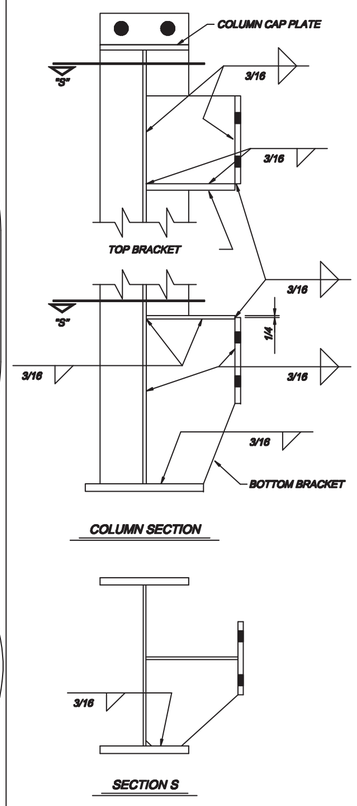
PROJECT:
CUSTOMER: OWNER:
LOCATION:

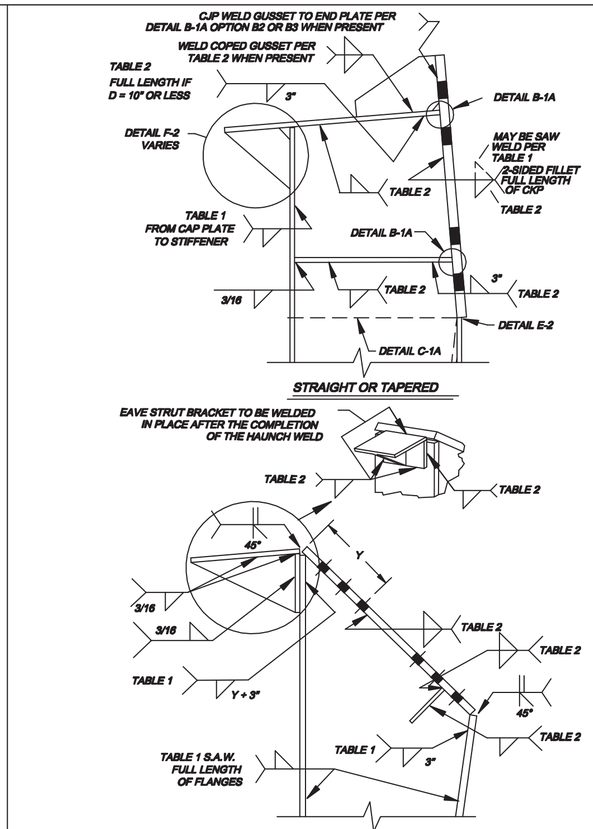
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

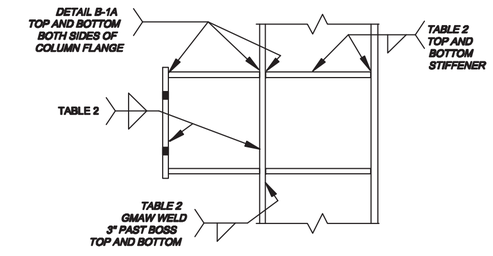
S-5

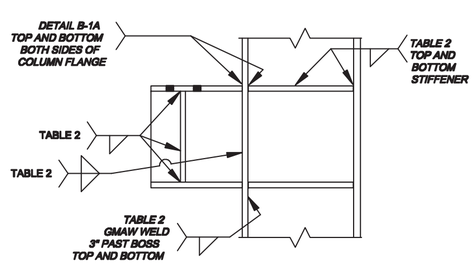


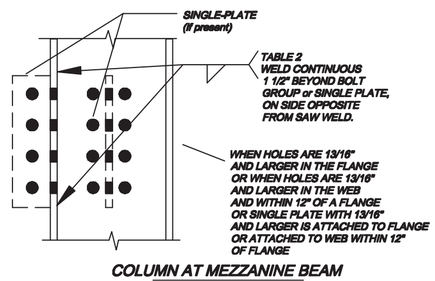




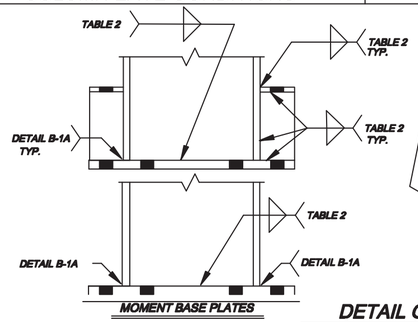




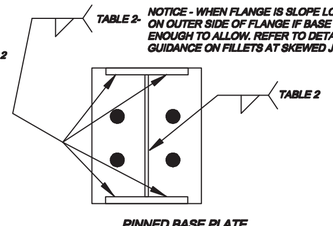




DETAILS RELATED TO MEZZANINE



DETAIL G-2



ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

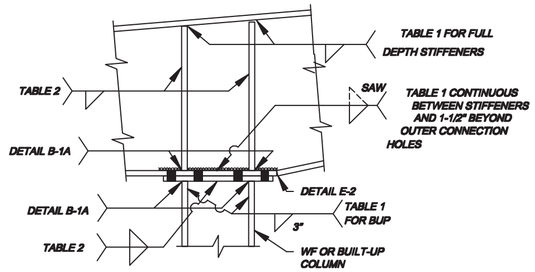
PROJECT:

CUSTOMER: OWNER:

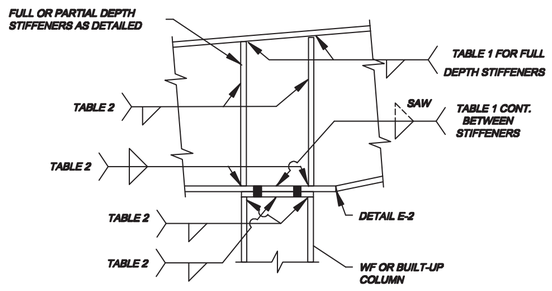
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

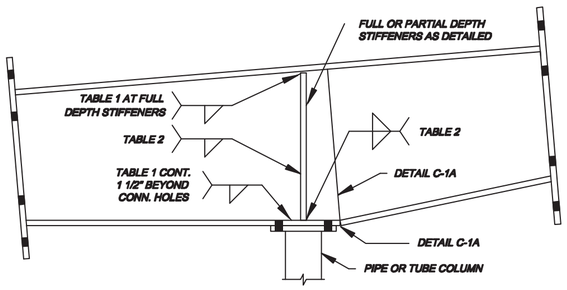
S-6

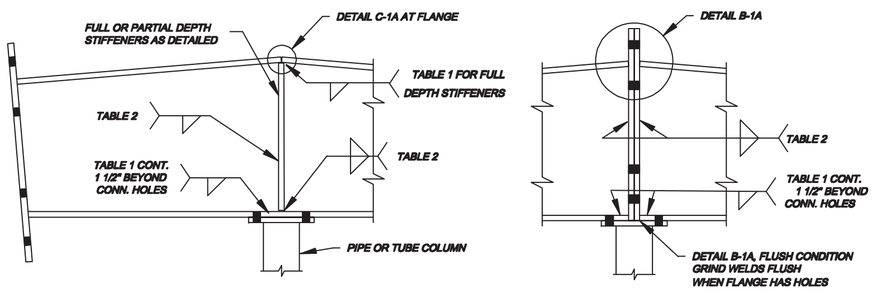


COLUMN MOMENT CONNECTION
(HAS BOLTS OUTSIDE OF FLANGES OR STIFFENER)



COLUMN PINNED CONNECTION
(ALL HOLES INSIDE OF FLANGES OR STIFFENERS)

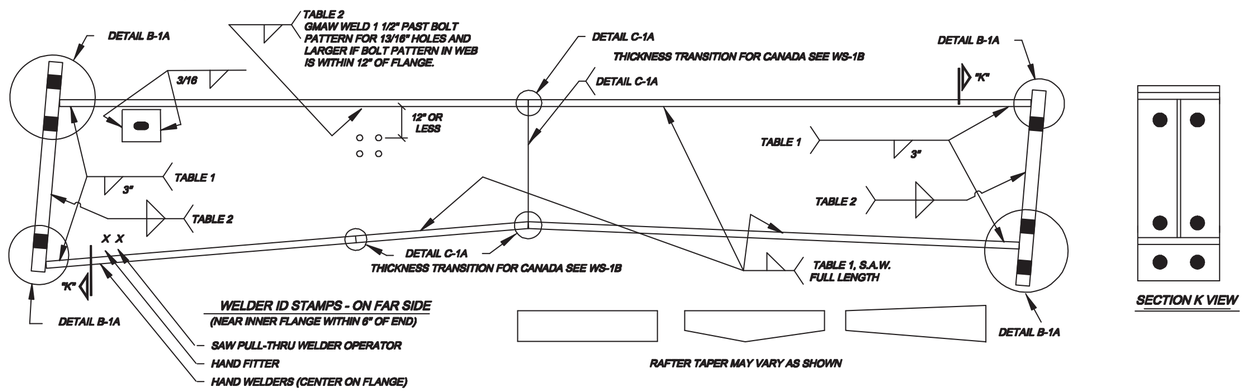




RAFTER INTERIOR SUPPORTS

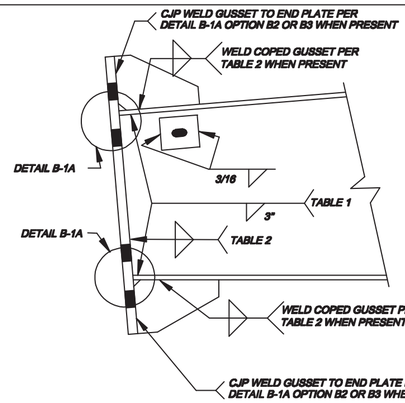
RIGID FRAME RAFTERS

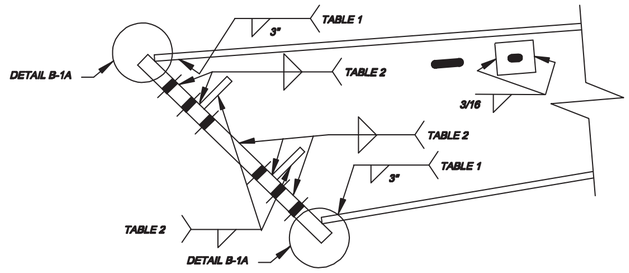
WS-3 Date Issued: July 31, 2020



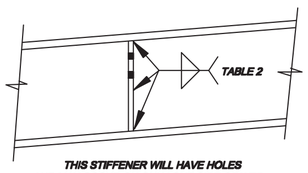
SECTION K VIEW

TYPICAL RAFTER WELDS





RAFTER MAY VARY AS SHOWN



THIS STIFFENER WILL HAVE HOLES
AND MAY OCCUR ON ONLY ONE SIDE OF WEB

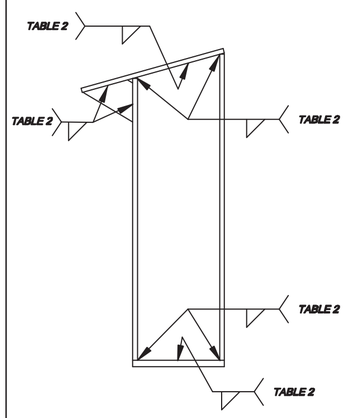
HIP RAFTER STIFFENER
FOR SKEWED RAFTER SUPPORT

		DESCRIPTION	BY	CK'D	DSN								
						PROJECT:							
						CUSTOMER:				OWNER:			
						LOCATION:							
						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
								N.T.S.					

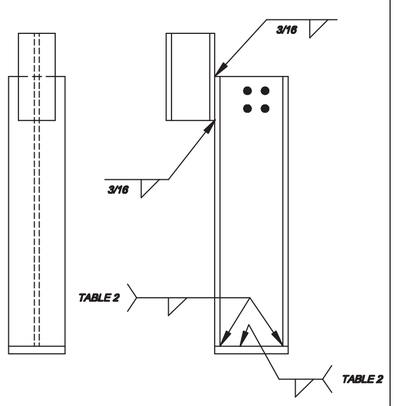
S-7

COLUMNS

WS-4 Date Issued: July 31, 2020

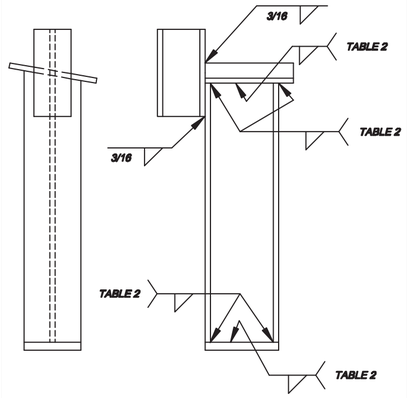


CORNER COLUMN



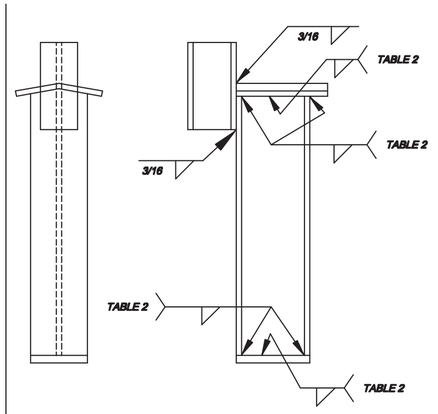
ENDWALL COLUMN

(AT RIGID FRAME)



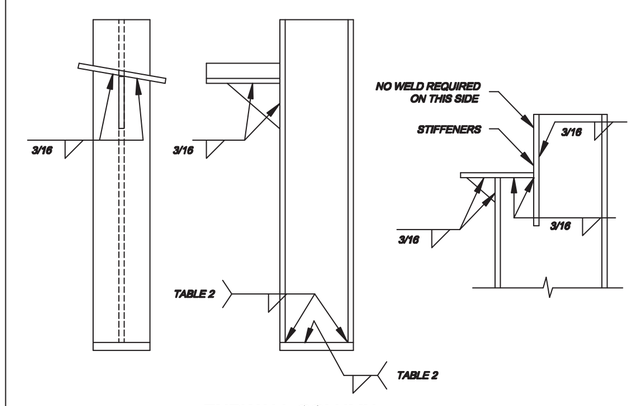
ENDWALL COLUMN

(BYPASS GIRT)



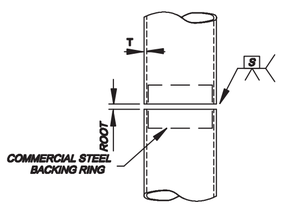
ENDWALL COLUMN

(BYPASS GIRT)



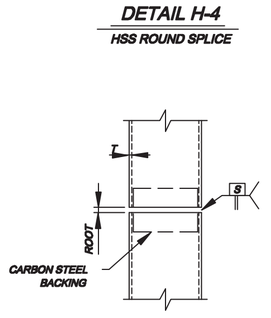
ENDWALL COLUMN

(FLUSH OR INSET GIRT)



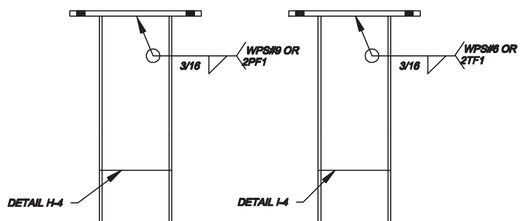
DETAIL H-4

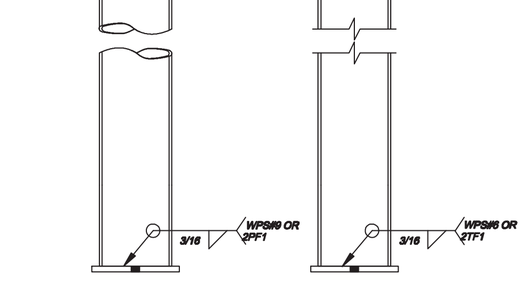
HSS ROUND SPLICE



DETAIL I-4

RECTANGULAR TUBING SPLICE





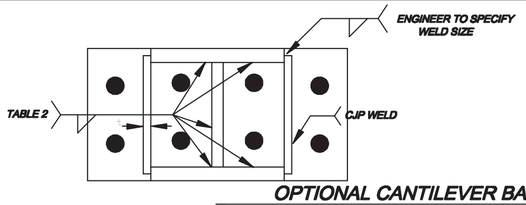
PIPE

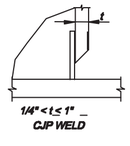
RECT. TUBE

WELDS LARGER THAN THESE STANDARD SIZES MAY BE REQUIRED
REFER TO FABRICATION DOCUMENTS

INTERIOR PIPE OR TUBE COLUMNS

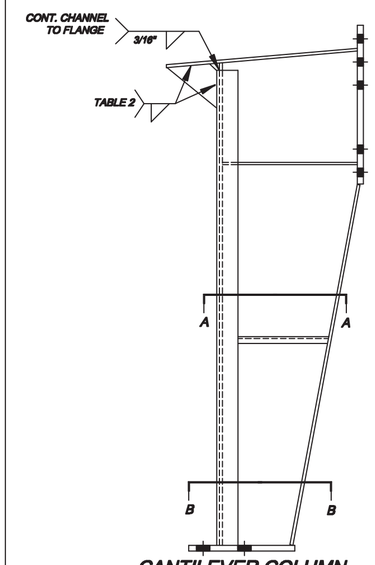
ANY PREQUALIFIED OR "PROCEDURE QUALIFIED" FULL PEN WELD MAY ALSO BE USED WITH COMPANY APPROVED WPS.



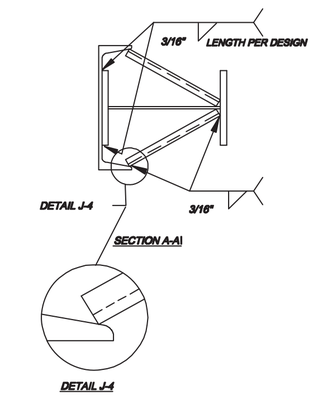


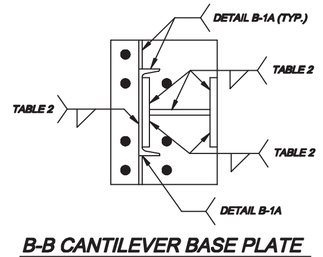
OPTIMAL CANTILEVER BASE PLATE

STRAIGHT COLUMN WITH DEPTH RESTRICTIONS

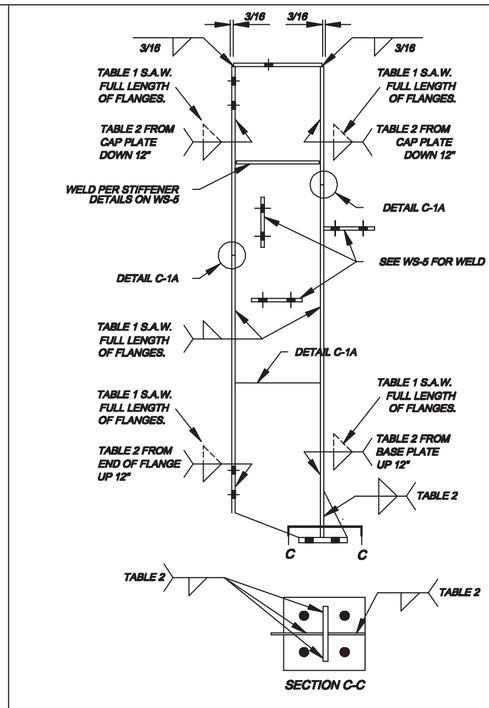


CANTILEVER COLUMN





B-B CANTILEVER BASE PLATE



FIXED BASE CANTILEVER COLUMN

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

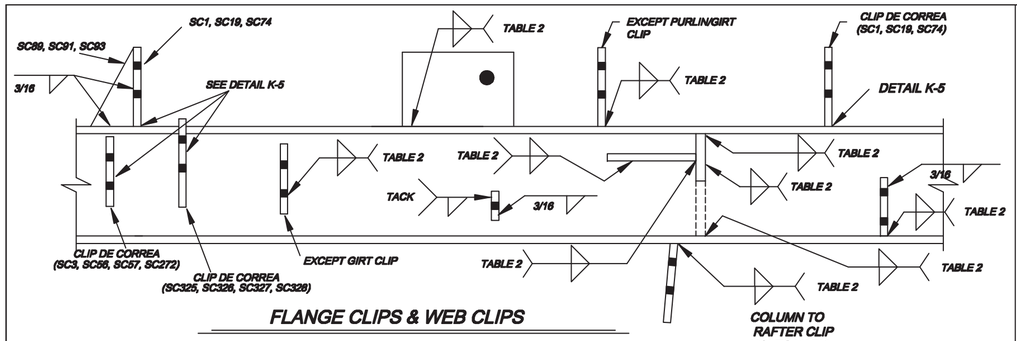
PROJECT:

CUSTOMER: OWNER:

LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

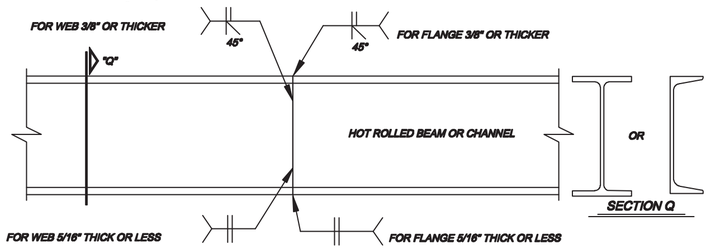
S-8



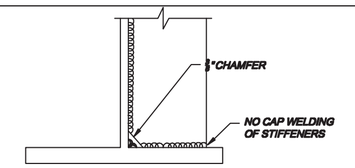
FLANGE CLIPS & WEB CLIPS

BEAM WELDING

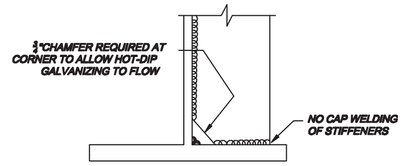
WS-5 Date Issued: July 31, 2020



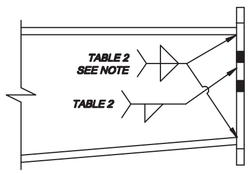
HOT ROLLED BEAM OR CHANNEL SPLICE



STIFFENER WELDING DETAIL FOR MEMBERS TO BE PRIMED or PAINTED

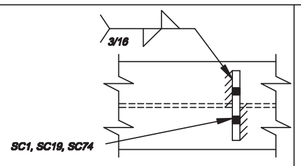


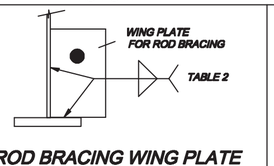
STIFFENER WELDING DETAIL FOR MEMBERS TO BE HOT-DIPPED GALVANIZED



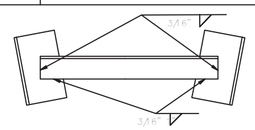
NOTE: IF CONNECTION PLATE IS 3/8 THICK OR MORE THEN WELD PER DETAIL B-1A.

DETAIL N-5
END RAFTER
STRAIGHT OR TAPERED BEAM

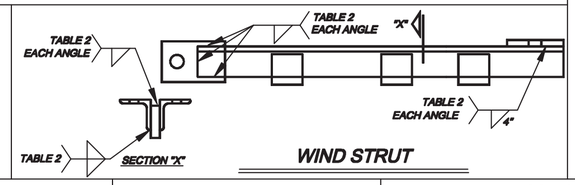




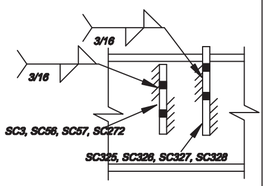
ROD BRACING WING PLATE



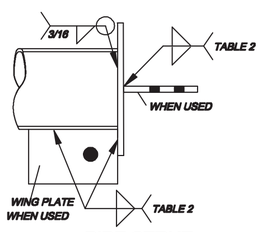
RIDGE-TIE (PC80)



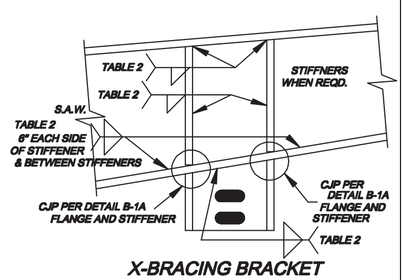
WIND STRUT



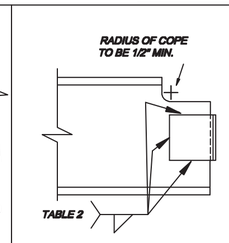
DETAIL K-5
CBB PURLIN/GIRT CLIP



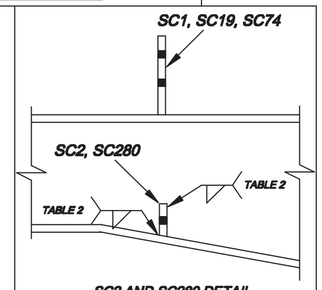
PIPE STRUT



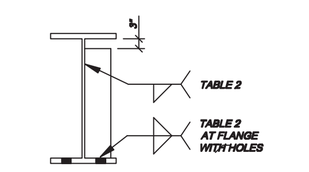
X-BRACING BRACKET



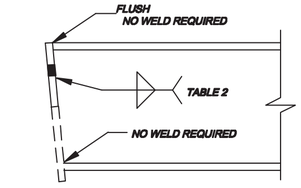
STANDARD SHEAR CONNECTION AND COPES



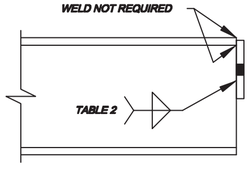
SC2 AND SC280 DETAIL



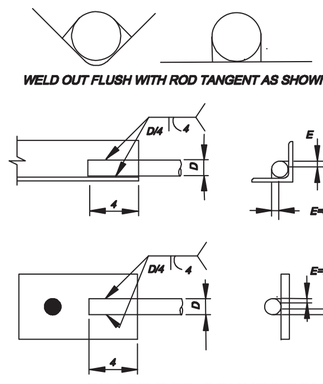
BEARING STIFFENER



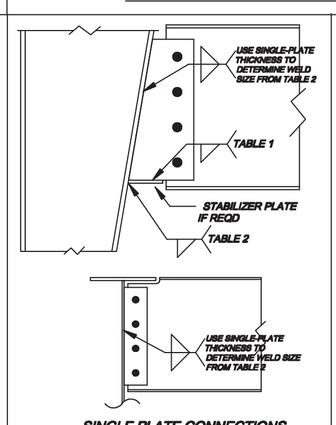
DETAIL M-5
PARTIAL OR FULL DEPTH END PLATE FLUSH AT TOP FLANGE
END RAFTER AT CORNER COLUMN



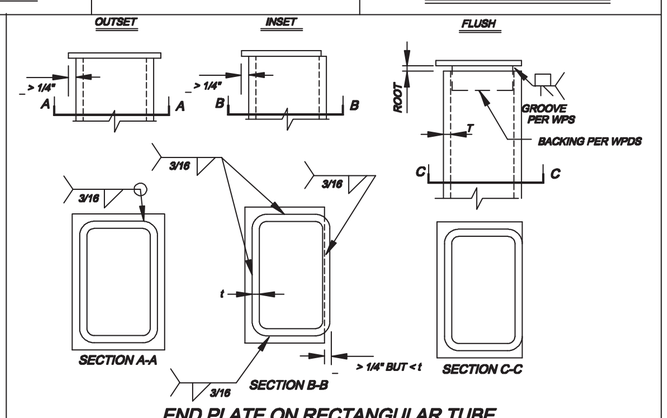
DETAIL L-5
PARTIAL DEPTH END PLATE RECESSED AT FLANGE
LEAN-TO OR END PLATE CONNECTION



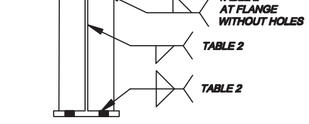
BRACE ROD CONNECTION



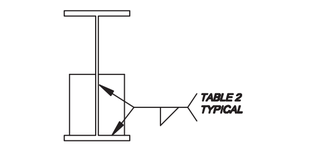
SINGLE PLATE CONNECTIONS



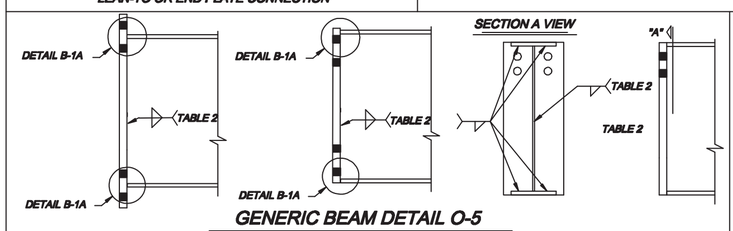
END PLATE ON RECTANGULAR TUBE



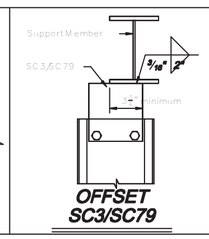
FULL DEPTH STIFFENER



PINCH STIFFENER



GENERIC BEAM DETAIL O-5



OFFSET SC3/SC79

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

PROJECT:

CUSTOMER: OWNER:

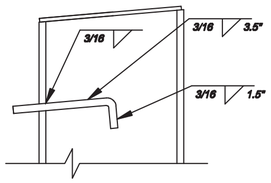
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

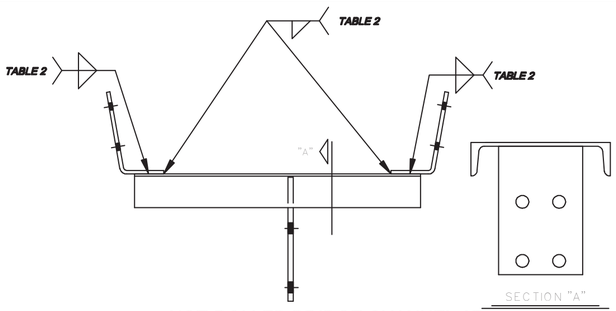
S-9

BEAM WELDING-2

WS-5A Date Issued: July 31, 2020



EAVE STRUT CLIP (P264)



HOT-ROLLED BRIDGE CHANNEL AT PEAK

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

PROJECT:

CUSTOMER: OWNER:

LOCATION:

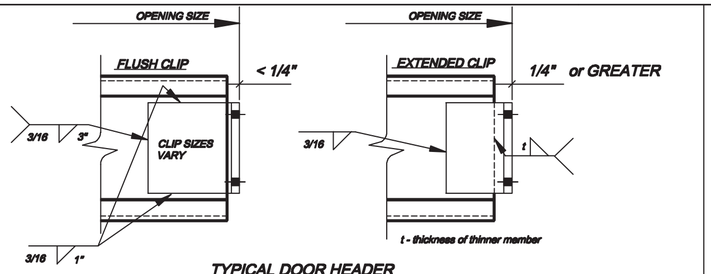
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

S-10

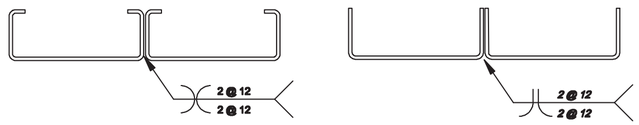
COLD-FORMED MEMBERS

WS-8 Date Issued: July 31, 2020

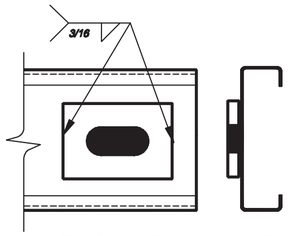
NOTE: REFER TO CBB WELD MANUAL FOR WPS's



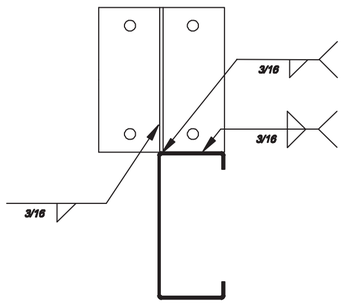
TYPICAL DOOR HEADER



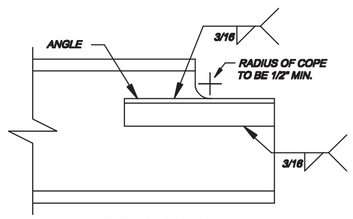
DOUBLE DOOR JAMBS / HEADER



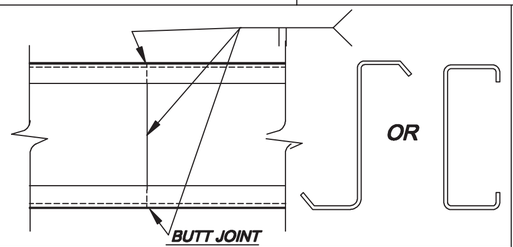
TYPICAL SLOT REINFORCEMENT



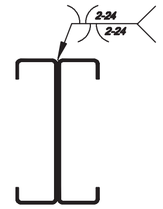
ANTI-ROLL CLIP ON COLD FORM CEE



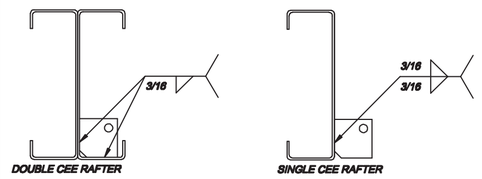
TYPICAL COPING



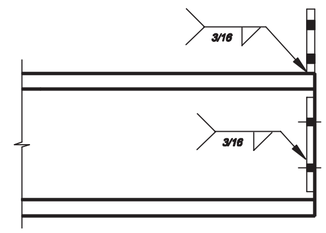
BUTT JOINT



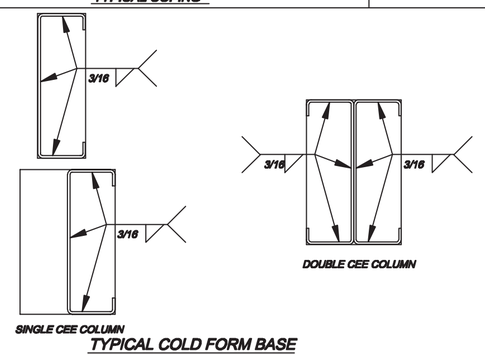
TYPICAL STITCH WELDS



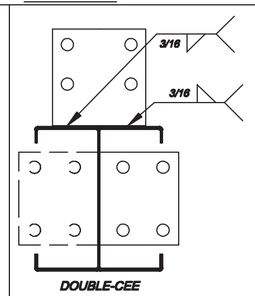
SC2 TO RAFTER

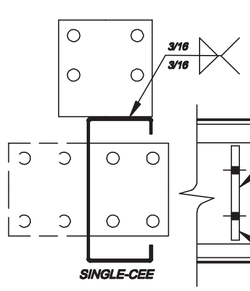


CLIPS AT END OF COLD FORM MEMBER

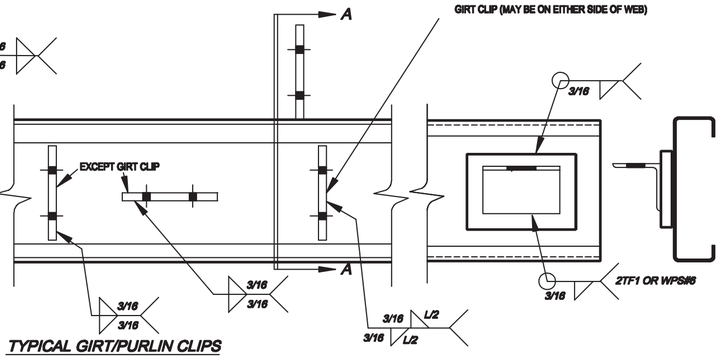


TYPICAL COLD FORM BASE

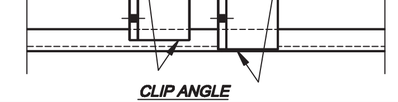


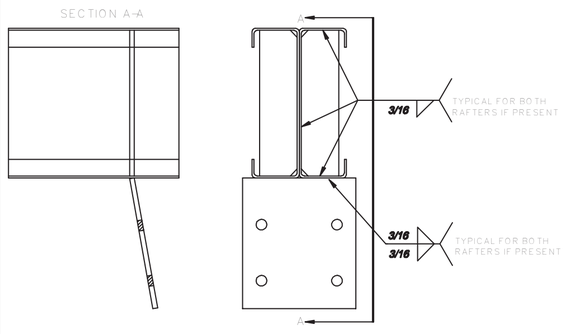


SECTION A-A

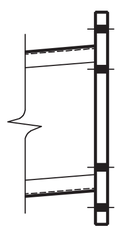


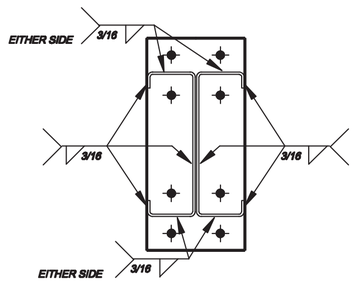
TYPICAL GIRT/PURLIN CLIPS

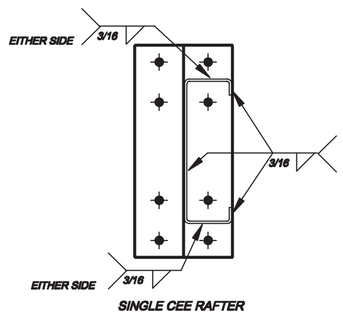




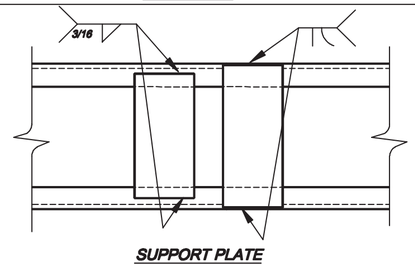
COLD-FORM RAFTER WEB STIFFENER & COLUMN CLIP

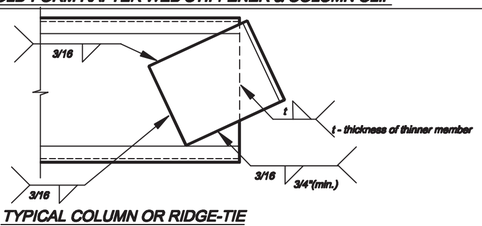




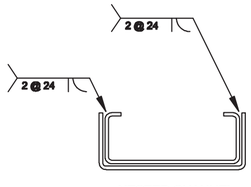


RAFTER CONNECTION





TYPICAL COLUMN OR RIDGE-TIE



NESTED CHANNEL

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

PROJECT:

CUSTOMER: OWNER:

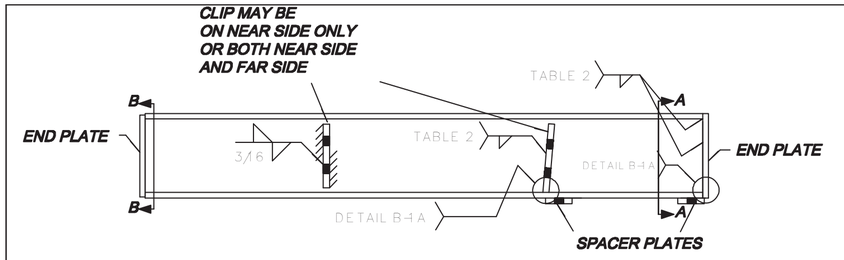
LOCATION:

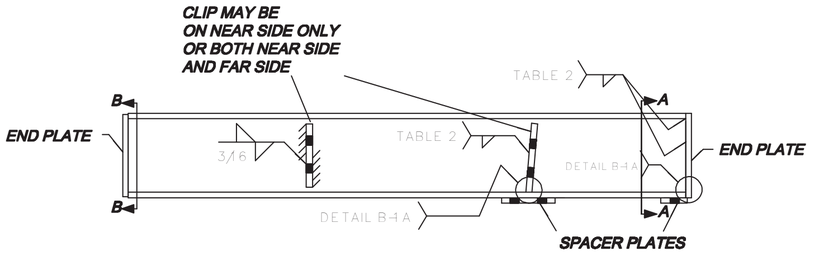
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

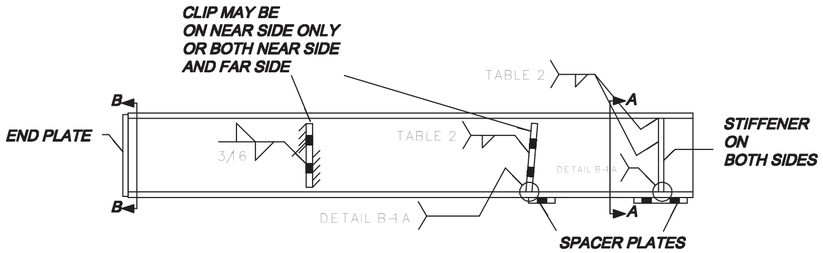
S-11

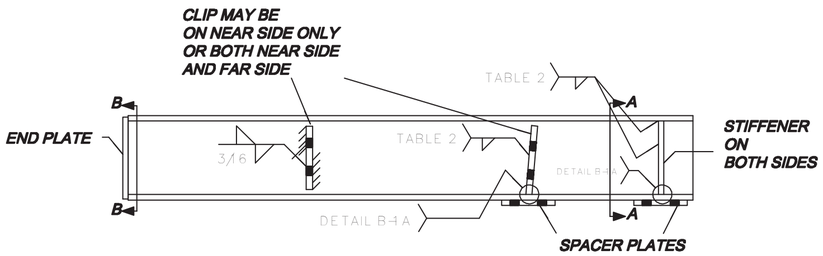
CANOPIES & FASCIAS

WS-7 Date Issued: July 31, 2020



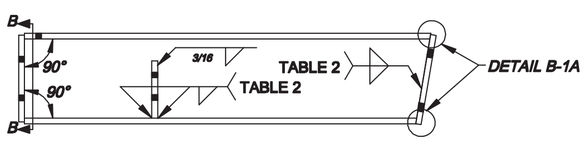




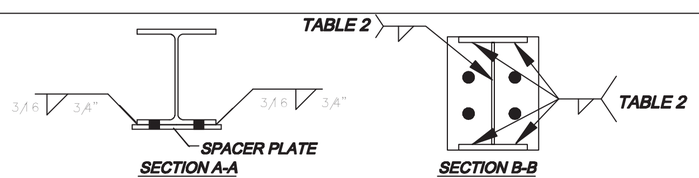


(AT EAVE)

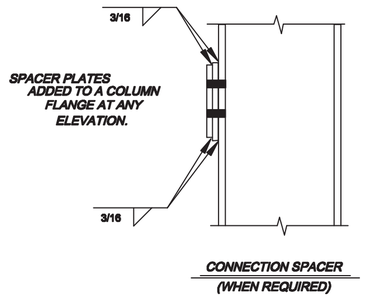
NOTE: DETAIL B-1A WELD IS REQUIRED FOR END PLATE ON BEAM END THAT IS CUT ON A SLOPE.



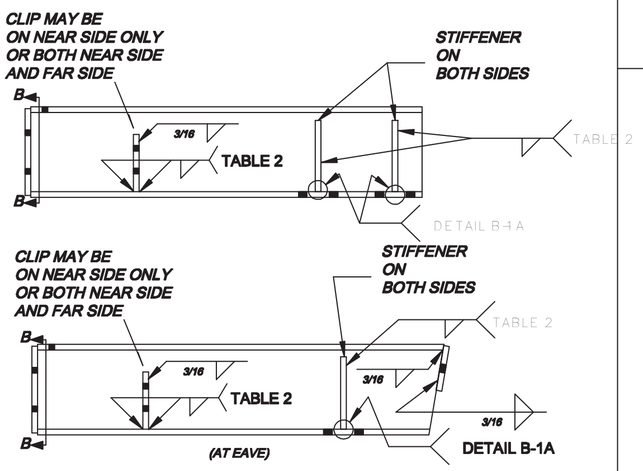
(BELOW EAVE)



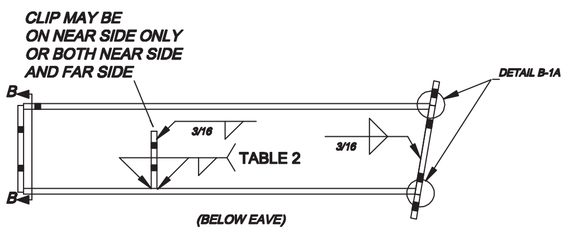
FLUSH CANOPY RAFTERS



CONNECTION SPACER
(WHEN REQUIRED)



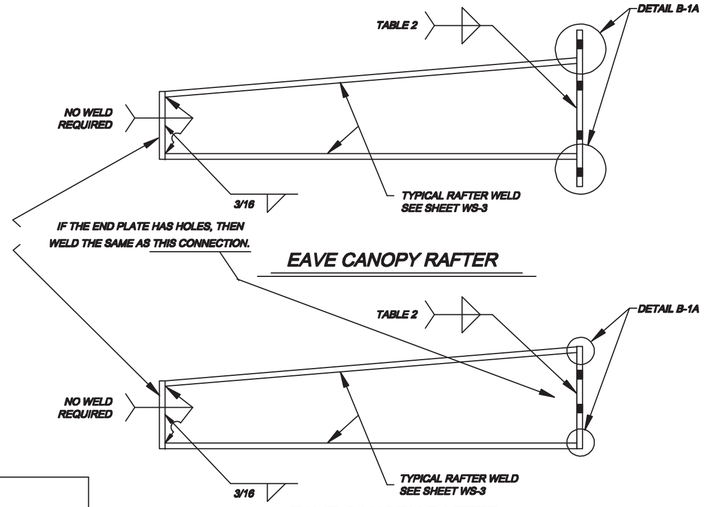
(AT EAVE)



(BELOW EAVE)

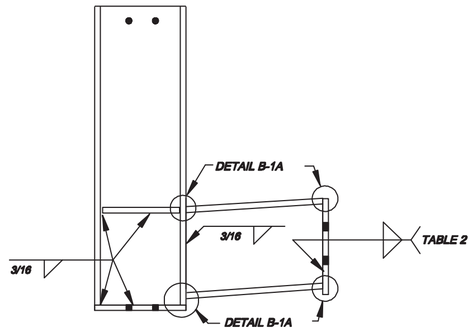
FLUSH CANOPY RAFTERS

(OLD STYLE)



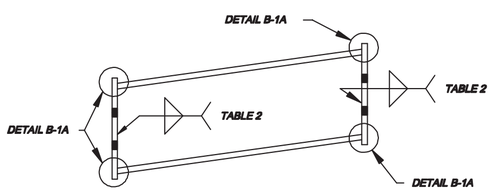
EAVE CANOPY RAFTER

EAVE CANOPY RAFTER



BUILT-UP COLUMN WELDING
SEE SHEET WS-2

FACADE/FASCIA FRAME



FACADE/FASCIA RAFTER

(FOR FACADE/FASCIA PROJECTION GREATER THAN 6'-0)

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

PROJECT:

CUSTOMER: OWNER:

LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

S-12

TABLE OF CONTENTS

WS-1 Date Issued: JULY 31, 2020

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN

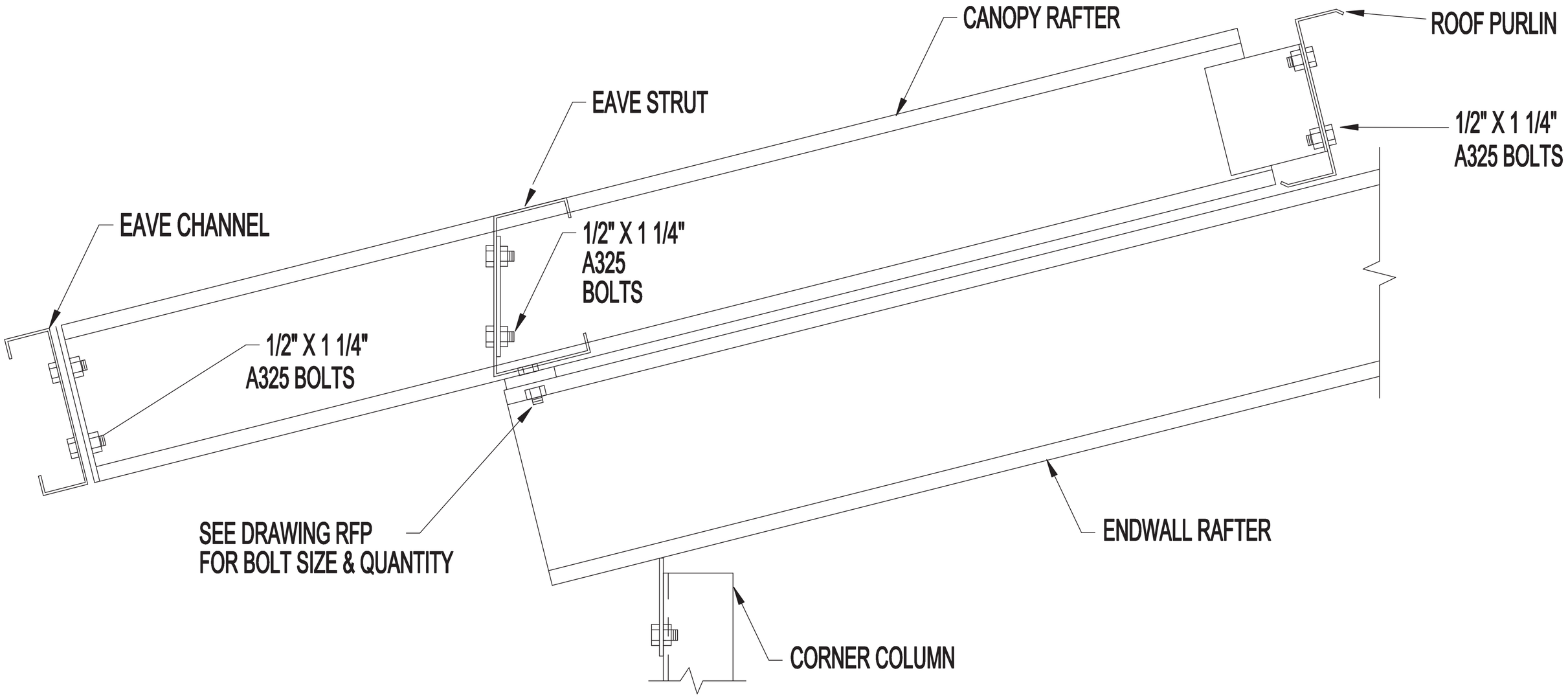
PROJECT:

CUSTOMER: OWNER:

LOCATION:

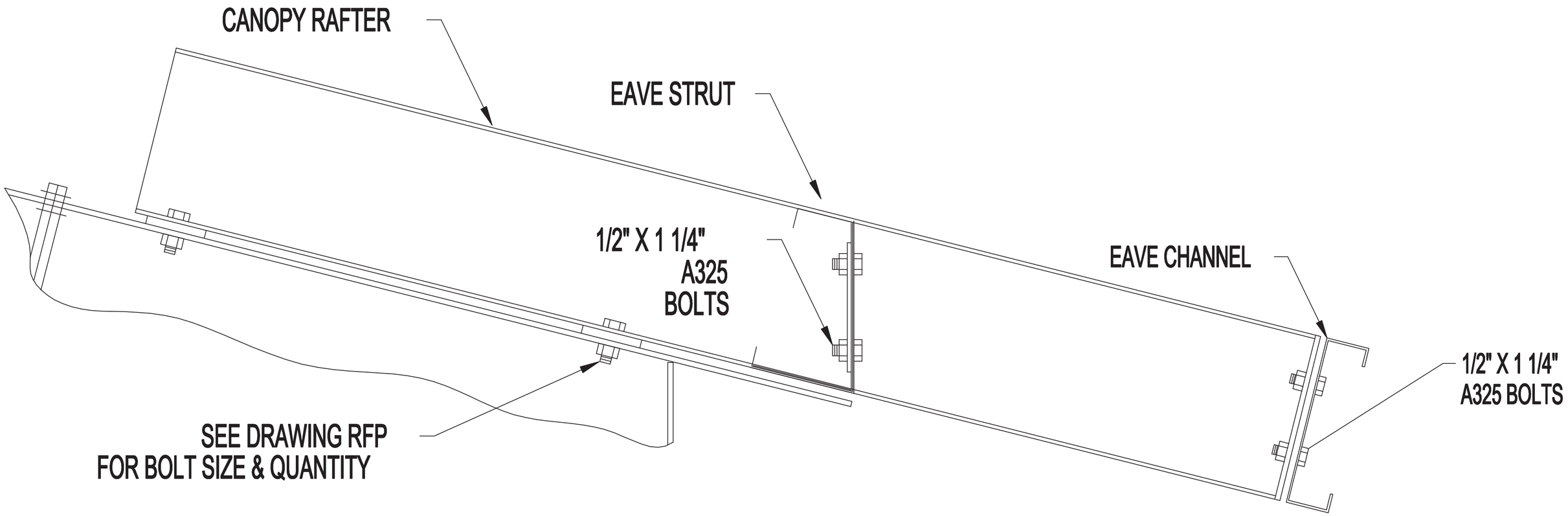
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

S-13



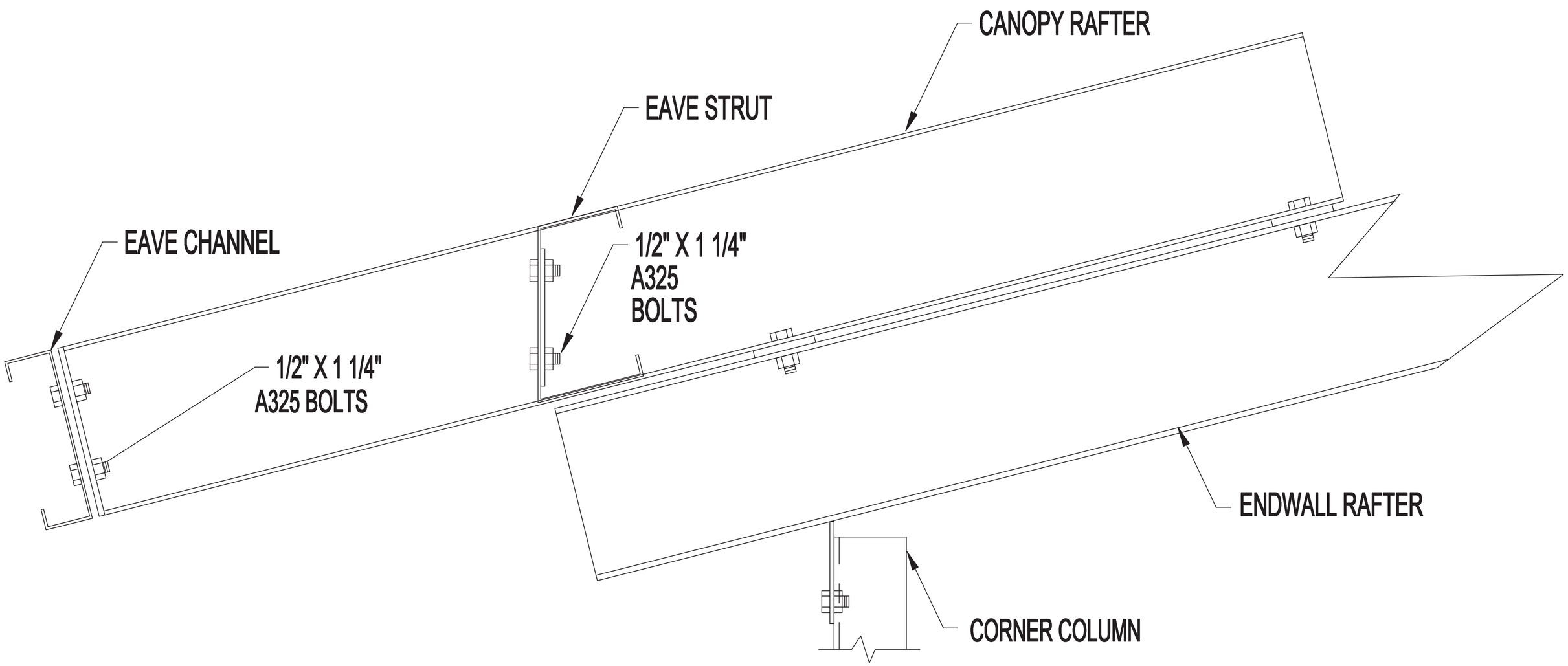
EAVE CANOPY AT "C" ENDWALL RAFTER

X2



EAVE CANOPY AT MAIN FRAME

X3



EAVE CANOPY AT HOT ROLLED ENDWALL RAFTER

X4

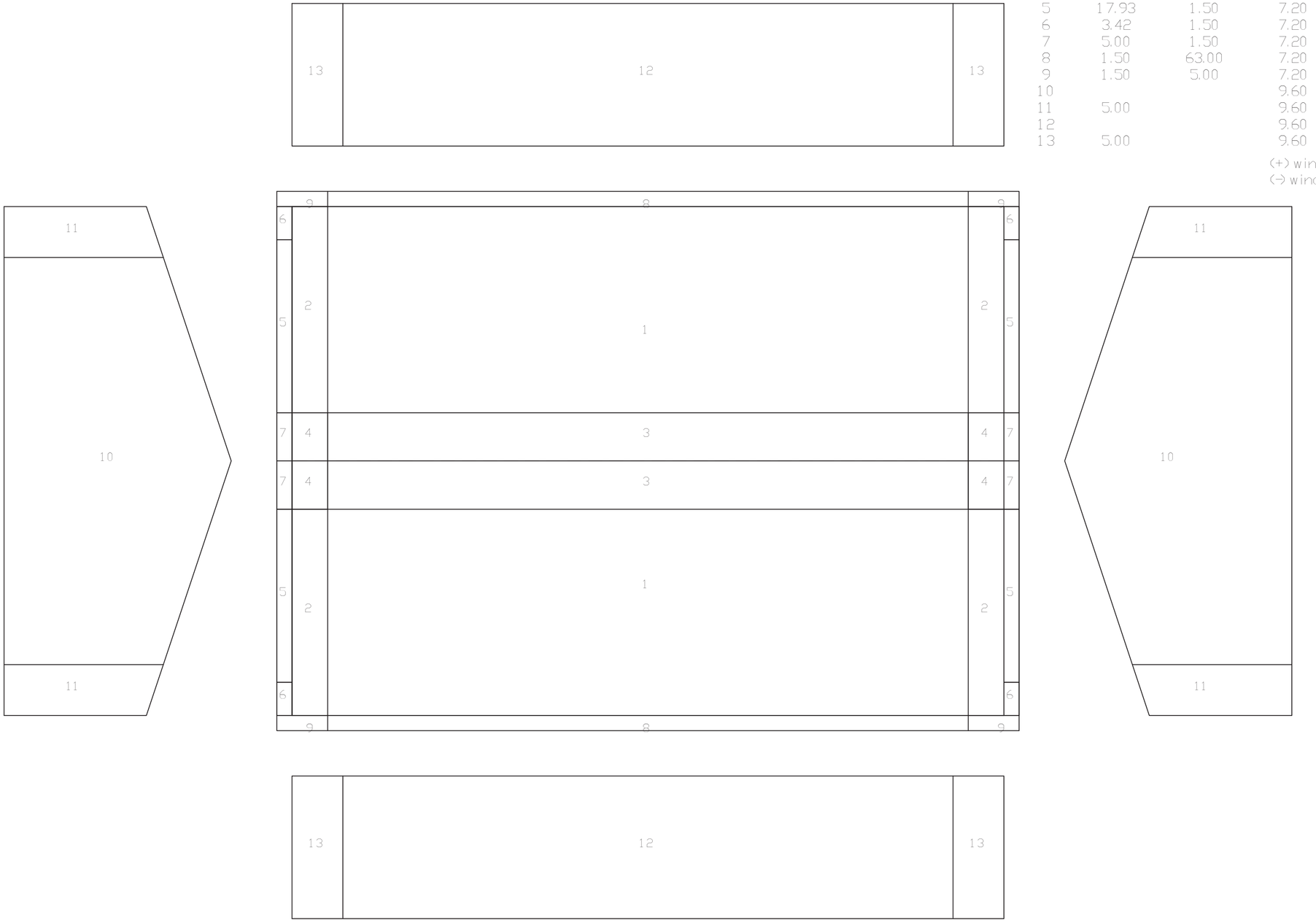
EXTENSION2

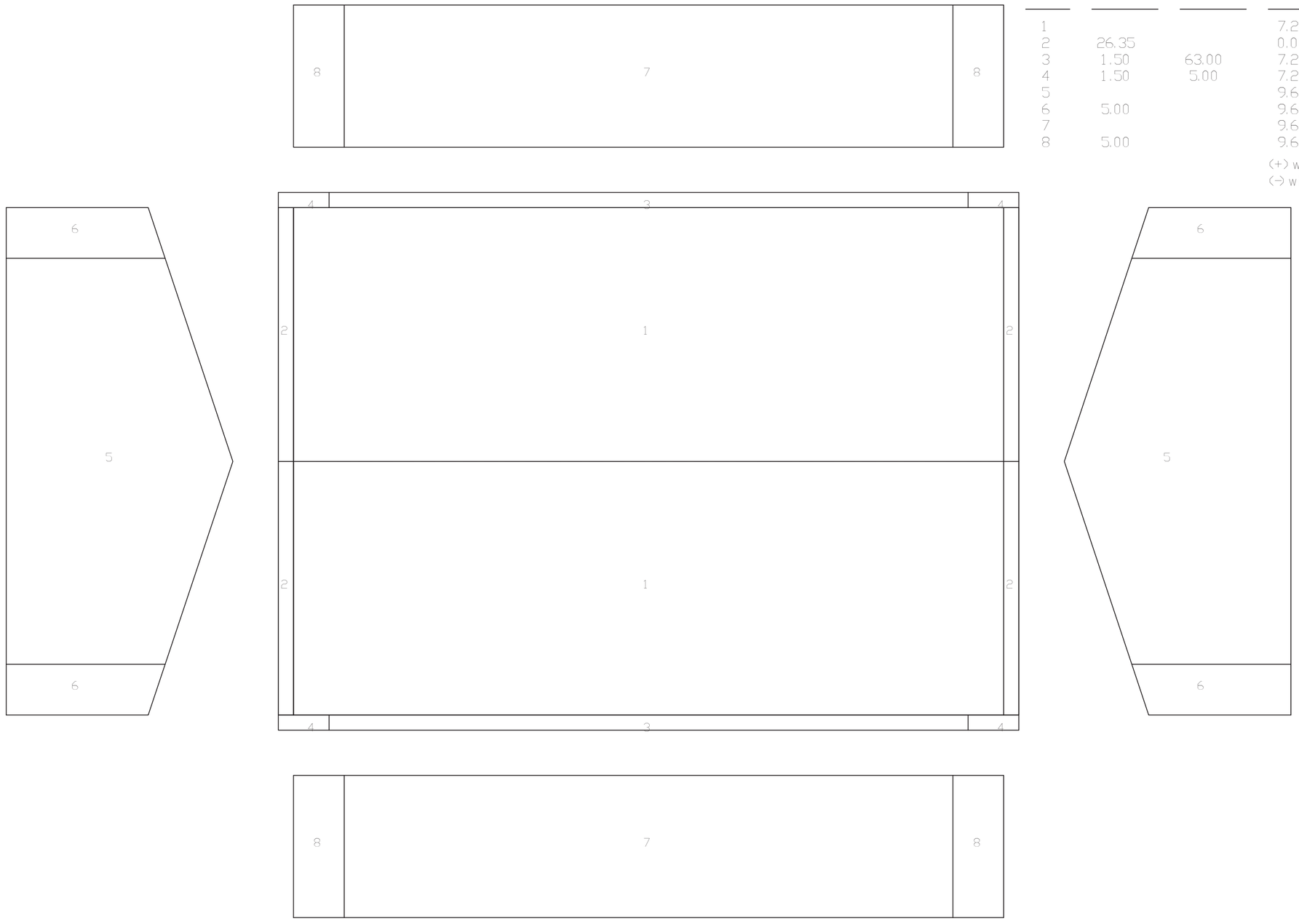
EXTENSION1

Components & Cladding (Factored)

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			7.20	7.20	-9.60	-21.20
2		3.50	7.20	7.20	-15.26	-30.95
3	5.00		7.20	7.20	-15.26	-30.95
4	5.00	3.50	7.20	7.20	-19.30	-36.67
5	17.93	1.50	7.20	7.20	-30.91	-34.13
6	3.42	1.50	7.20	7.20	-34.46	-39.85
7	5.00	1.50	7.20	7.20	-38.69	-45.79
8	1.50	63.00	7.20	7.20	-14.59	-24.31
9	1.50	5.00	7.20	7.20	-19.55	-39.87
10			9.60	11.47	-10.52	-12.45
11	5.00		9.60	11.47	-11.46	-15.31
12			9.60	11.46	-10.50	-12.42
13	5.00		9.60	11.46	-11.45	-15.28

(+) wind towards surface
(-) wind away from surface

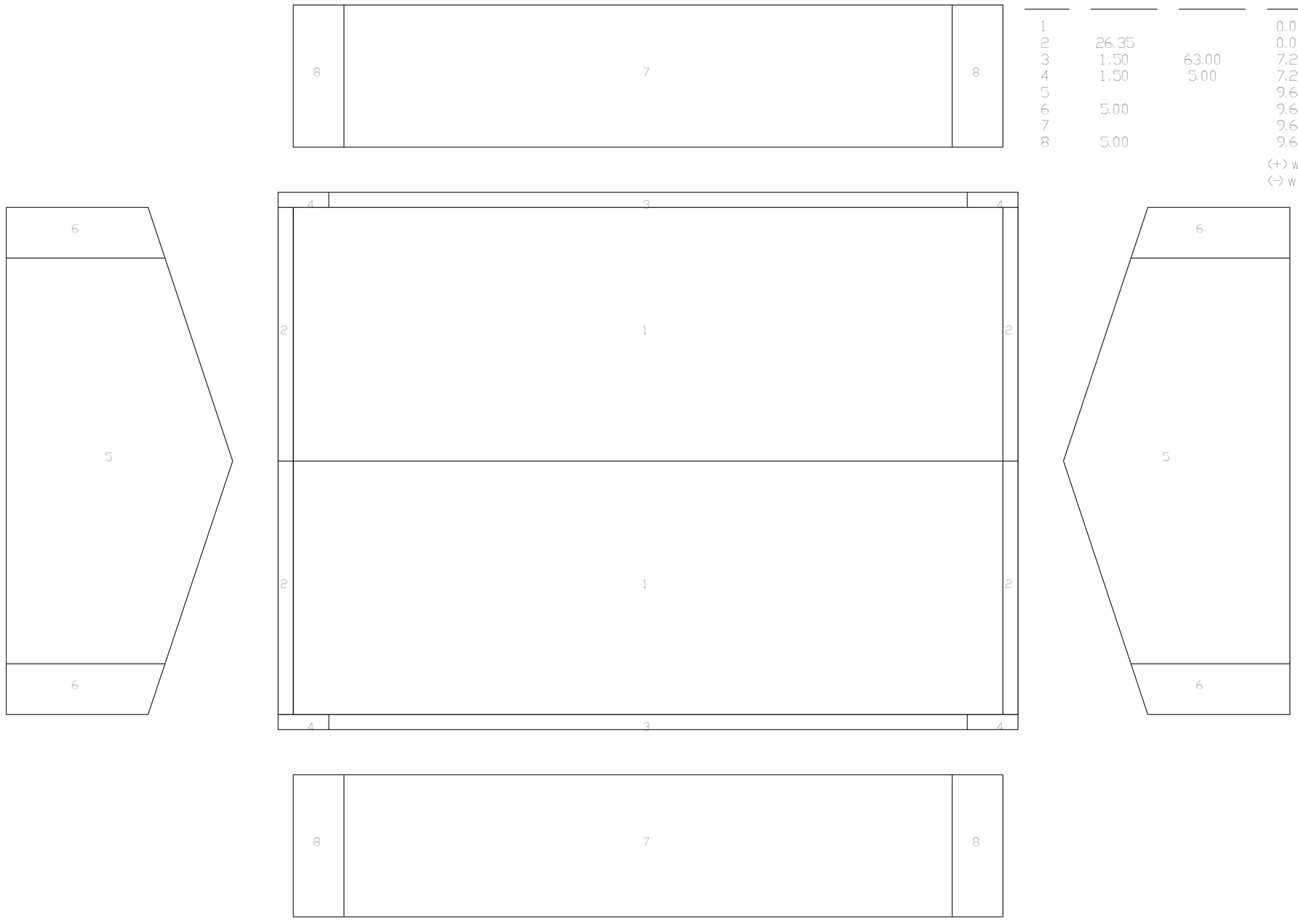




Components & Cladding (Factored)

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			7.20	7.20	-9.60	-21.20
2	26.35		0.00	0.00	0.00	0.00
3	1.50	63.00	7.20	7.20	-14.59	-24.31
4	1.50	5.00	7.20	7.20	-19.55	-39.87
5			9.60	11.47	-10.52	-12.45
6	5.00		9.60	11.47	-11.46	-15.31
7			9.60	11.46	-10.50	-12.42
8	5.00		9.60	11.46	-11.45	-15.28

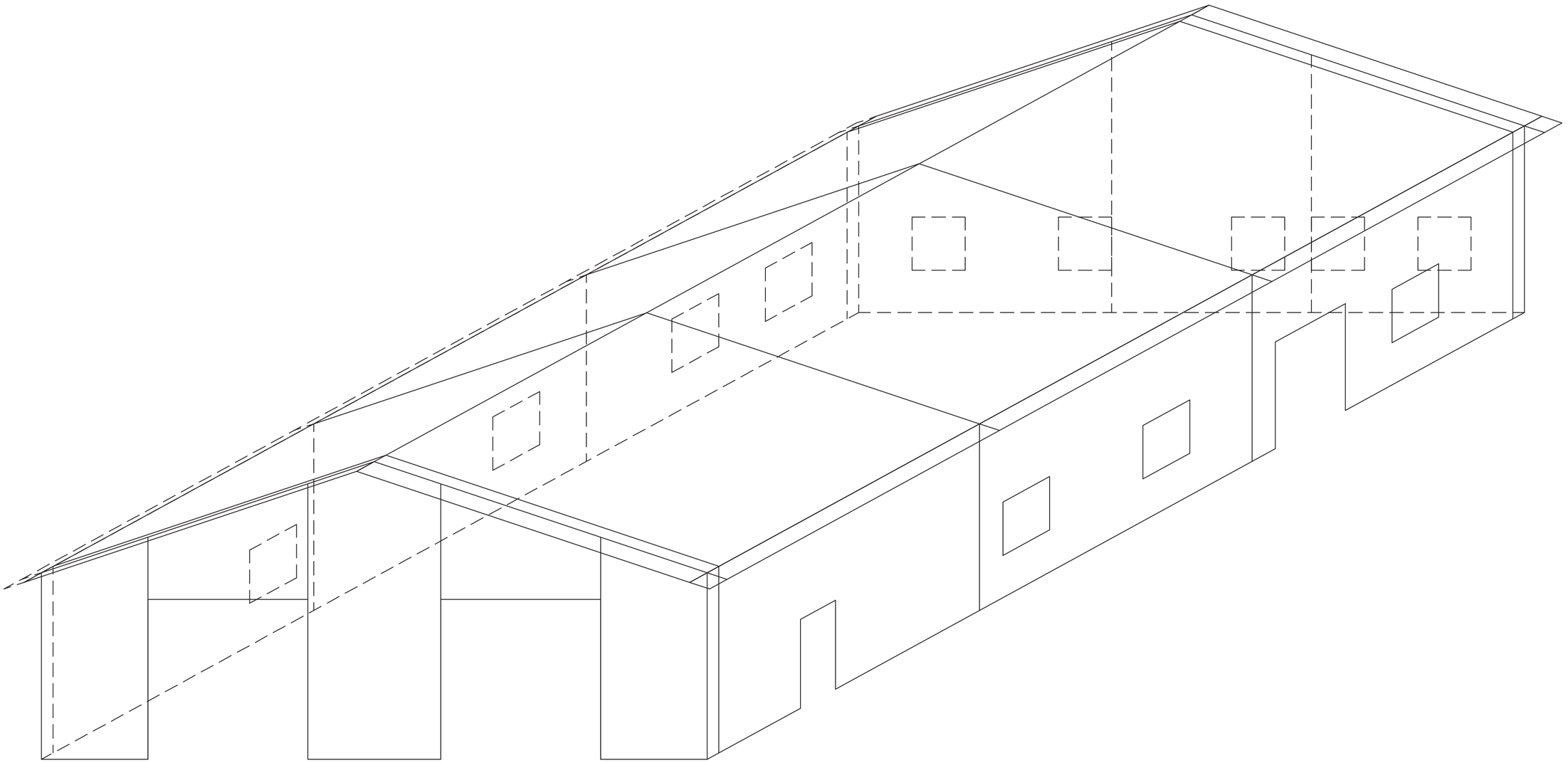
(+) wind towards surface
(-) wind away from surface



Components & Cladding (Factored)

Zone	Width (ft)	Length (ft)	Pressure(psf) Member	Pressure(psf) Panel	Suction(psf) Member	Suction(psf) Panel
1			0.00	0.00	-9.60	0.00
2	26.35		0.00	0.00	0.00	0.00
3	1.50	63.00	7.20	7.20	-14.59	-24.31
4	1.50	5.00	7.20	7.20	-19.55	-39.87
5			9.60	11.47	-10.52	-12.45
6	5.00		9.60	11.47	-11.46	-15.31
7			9.60	11.46	-10.50	-12.42
8	5.00		9.60	11.46	-11.45	-15.28

(+) wind towards surface
(-) wind away from surface



HERITAGE
BUILDING SYSTEMS

ACCREDITED® AC 472

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

BUILDER/CONTRACTOR RESPONSIBILITIES

Drawing Validity –These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

Builder Acceptance of Drawings –Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice APR 10 Section 4.4.1)

Code Official Approval –It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

Builder is responsible for State, Federal and OSHA safety compliance –The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

Building Erection –The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice APR 10 Section 7.10.3)

Discrepancies –Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice APR 10 Section 3.3)

Materials by Others –All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturers assumptions will govern.

Modification of the Metal Building from Plans –The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

Foundation Design –The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA MBSM Chapter 4 Section 3.2.2 and Section A3)

Download panel installation manuals from:
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Descargue los manuales de instalación del panel desde:
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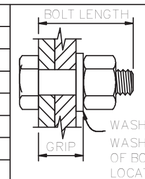
1/2"Ø A325 BOLT GRIP TABLE (UNLESS NOTED)	
GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
Over 9/16" TO 1 1/16"	1 3/4" F.T.
Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
Over 1 13/16" TO 2 1/16"	2 3/4"
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS	
F.T. DENOTES FULLY THREADED	



NOTE:
FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

Rev. 11/15/2021

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacture nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacture's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, The fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-436

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum defection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS...: Building A

Roof Limits	Rafters	Purlins	Panels
Live L/	@F360	@F354	@F357
Snow L/	@F360	@F354	@F357
Wind L/	@F360	@F355	@F358
Total Gravity L/	@F360	@F354	@F357

Frame Limits	Sidesway	Portal Frame Sidesway	
Live H/	@F359		
Snow H/	@F359		
Wind H/	@F359		
Seismic H/	@F363		
Crane H/	@F362		
Total Gravity H/	@F359		
Total Wind H/	@F359	@F361	
Total Seismic H/	@F363	@F364	

Wall Limits	Limit
Total Wind Panels L/	@F356
Total Wind Girts L/	@F353
Total Wind EW Columns L/	@F350

The Service Seismic limit as shown here is at service level loads.

ENGINEERING DESIGN CRITERIA

Building Code..... @F324
Building Risk Category..... @U619

Roof Dead Load
Superimposed..... @F301 psf
Collateral..... @F305 psf (Total)
(0.00 psf Ceiling @F305 psf Other)
Roof Live Load.....@F302 psf @F304 reduction

Snow
Ground Snow Load (Pg)..... @U600 psf
Snow Load Importance Factor (Is) @U614
Snow Exposure Factor (Ce)..... @U615
Thermal Factor (Ct)..... @U612
Flat Roof Snow Load (Pf)..... @F303 psf
Minimum Roof Snow Load (Pm)..... @U648 psf

Wind
Ultimate Wind Speed (Vult)..... @F307 mph
Nominal Wind Speed (Vasd)..... @U636 mph
Serviceability Wind Speed..... @U646 mph
Ground Elevation Factor..... @U654 (@U655SL)
Wind Exposure Category..... @F309
Internal Pressure Coefficient (GCpi) @U623 /@U624
Loads for components not provided by building manufacturer.
Wall Edge Zones (within @U653 ' of corner)
@U620 psf pressure
@U622 psf suction
Other Wall Zones @U620 psf pressure
@U621 psf suction
These values are the maximum values required based on a 10 square foot area.
Components with larger areas may have lower wind loads.
Zones per ASCE 7-16; FIG. 30.3-1
Zones pressures shown are Un-Factored

Seismic
Seismic Importance Factor (Ie)..... @F315
Seismic Design Category..... @F311
Soil Site Class..... @U647
Ss..... @U601 g Sds..... @U607 g
S1..... @U602 g Sd1..... @U608 g
Analysis Procedure..... Equivalent Lateral Force

Location...	Int RF	Front SW	Back SW	Left EW	Right EW	
System.....	@J190	@J191	@J192	@J193	@J194	
R.....	@F368	@F374	@F376	@F370	@F372	
Cs.....	@F369	@F375	@F377	@F371	@F373	

Design Base Shear in kips (V) Transverse @F366
Design Base Shear in kips (V) Longitudinal @F365

System –Basic Force Resisting System
H –Steel System not Specifically Detailed for Seismic Resistance
C4 –Steel Ordinary Moment Frames
B3 –Steel Ordinary Concentric Braced Frames
G2 –Steel Ordinary Cantilevered Column Systems
R –Response Modification Coefficient
Cs –Seismic Response Coefficient
Transverse –Direction Parallel to the Rigid Frames
Longitudinal –Direction Perpendicular to the Rigid Frames

Building Descriptions				
Building ID	Width(ft)	Length(ft)	Height(ft)	Slope
Building A	@F201	@F202	@F204	@F206

Drawing Index	
Page	Description
C1	COVER SHEET
F1	ANCHOR BOLT PLAN
F2	ANCHOR BOLT REACTIONS
F3	ANCHOR BOLT DETAILS
E1	ROOF FRAMING PLAN
E2	ROOF SHEETING PLAN
E3	FRONT SIDEWALL
E4	BACK SIDEWALL
E5	LEFT ENDWALL
E6	RIGHT ENDWALL
E7	FRAME CROSS SECTION
DET1-5	STANDARD DETAILS
R1-R3	INSTALLATION SHEETS

DRAWING STATUS

☐ FOR APPROVAL
These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.

☐ FOR CONSTRUCTION PERMIT
These drawings, being for Permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered as complete.

☒ FOR ERECTOR INSTALLATION
Final drawings for construction.

For questions or assistance
Concerning Erection call or Email:
1-844-840-4603
Monday-Friday 7:30am to 5:00pm
FIELD.SERVICES@CORNERSTONE-BB.COM

ENGINEERING SEAL

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
@J024	@DATE	FOR @J041	@J012	@J014	@J011

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD. STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT: @J007
CUSTOMER: @J004 OWNER: @J038
LOCATION: @J009

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	@DATE	N.T.S.	1	A	@J010	C1	@J024

GN-1

HERITAGE
BUILDING SYSTEMS

ACCREDITED® AC 472

MBMA METAL BUILDING MANUFACTURERS ASSOCIATION

BUILDER/CONTRACTOR RESPONSIBILITIES

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1/2"Ø A325 BOLT GRIP TABLE (UNLESS NOTED)	
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0 TO 9/16"	1 1/4" F.T.
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Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
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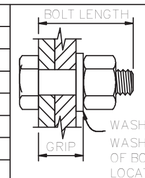
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS	
F.T. DENOTES FULLY THREADED	



NOTE:
FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

Rev. 11/15/2021

PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacture nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacture's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1, The fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-436

DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum defection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS...: Building A

Roof Limits	Rafters	Purlins	Panels
Live L/	180	180	60
Snow L/	180	180	60
Wind L/	180	180	60
Total Gravity L/	180	180	60

Frame Limits	Sidesway	Portal Frame Sidesway	
Live H/	60		
Snow H/	60		
Wind H/	60		
Seismic H/	195		
Crane H/	100		
Total Gravity H/	60		
Total Wind H/	60	60	
Total Seismic H/	195	195	

Wall Limits	Limit
Total Wind Panels L/	60
Total Wind Girts L/	90
Total Wind EW Columns L/	120

The Service Seismic limit as shown here is at service level loads.

ENGINEERING DESIGN CRITERIA

Building Code..... CBC 19
Building Risk Category..... II –Normal

Roof Dead Load
Superimposed..... 2.500 psf
Collateral..... 6 psf (Total)
(0.00 psf Ceiling 6 psf Other)
Roof Live Load.....20.00 psf No reduction

Snow
Ground Snow Load (Pg)..... 0.00 psf
Snow Load Importance Factor (Is) 1.00
Snow Exposure Factor (Ce)..... 1.00
Thermal Factor (Ct)..... 1.00
Flat Roof Snow Load (Pf)..... 0 psf
Minimum Roof Snow Load (Pm)..... 0.00 psf

Wind
Ultimate Wind Speed (Vult)..... 92 mph
Nominal Wind Speed (Vasd)..... 71 mph
Serviceability Wind Speed..... 64 mph
Ground Elevation Factor..... 1.00 (53.64 ASL)
Wind Exposure Category..... C
Internal Pressure Coefficient (GCpi) 0.18 /-0.18
Loads for components not provided by building manufacturer.
Wall Edge Zones (within 5.00 ' of corner)
19.12 psf pressure
-25.61 psf suction
Other Wall Zones 19.12 psf pressure
-20.75 psf suction
These values are the maximum values required based on a 10 square foot area.
Components with larger areas may have lower wind loads.
Zones per ASCE 7-16; FIG. 30.3-1
Zones pressures shown are Un-Factored

Seismic
Seismic Importance Factor (Ie)..... 1.00
Seismic Design Category..... D
Soil Site Class..... D
Ss..... 1.500 g Sds..... 1.000 g
S1..... 0.600 g Sd1..... 0.680 g
Analysis Procedure..... Equivalent Lateral Force

Location...	Int RF	Front SW	Back SW	Left EW	Right EW
System.....	C4	C4	C4	C4	C4
R.....	3.5	3.5	3.5	3.5	3.5
Cs.....	0.286	0.286	0.286	0.286	0.286

Design Base Shear in kips (V) Transverse 12.74
Design Base Shear in kips (V) Longitudinal 12.76

System –Basic Force Resisting System
H –Steel System not Specifically Detailed for Seismic Resistance
C4 –Steel Ordinary Moment Frames
B3 –Steel Ordinary Concentric Braced Frames
G2 –Steel Ordinary Cantilevered Column Systems
R –Response Modification Coefficient
Cs –Seismic Response Coefficient
Transverse –Direction Parallel to the Rigid Frames
Longitudinal –Direction Perpendicular to the Rigid Frames

Building Descriptions				
Building ID	Width(ft)	Length(ft)	Height(ft)	Slope
Building A	50	70	14	4.0:12

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G1	COVER SHEET
F1	ANCHOR BOLT PLAN
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E4	BACK SIDEWALL
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Concerning Erection call or Email:
1-844-840-4603
Monday-Friday 7:30am to 5:00pm
FIELD.SERVICES@CORNERSTONE-BB.COM

ENGINEERING SEAL

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

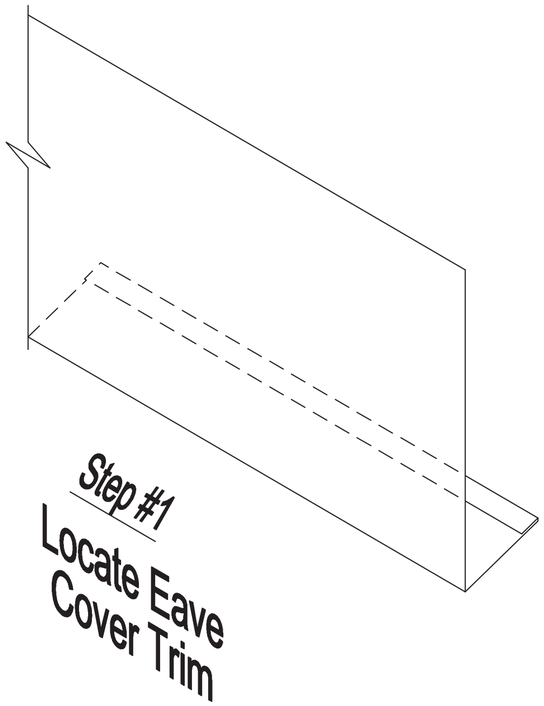
HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD. STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:
CUSTOMER: OWNER:
LOCATION:

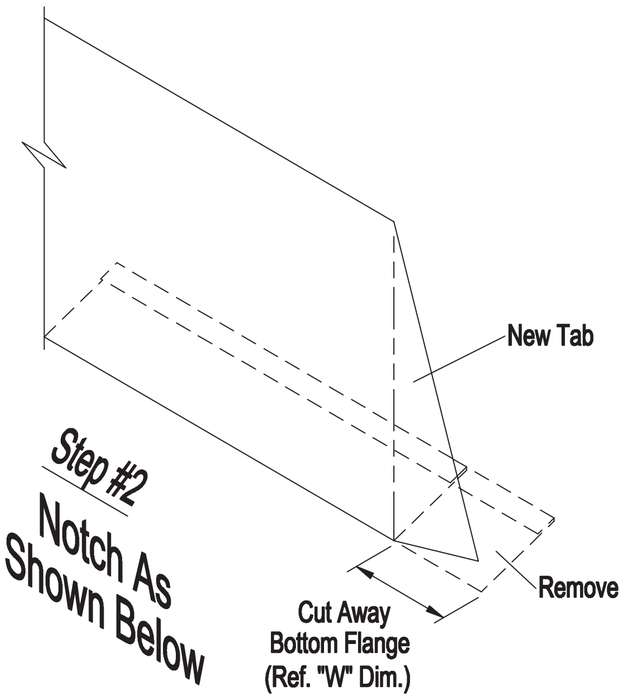
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		C1	0

GN-2

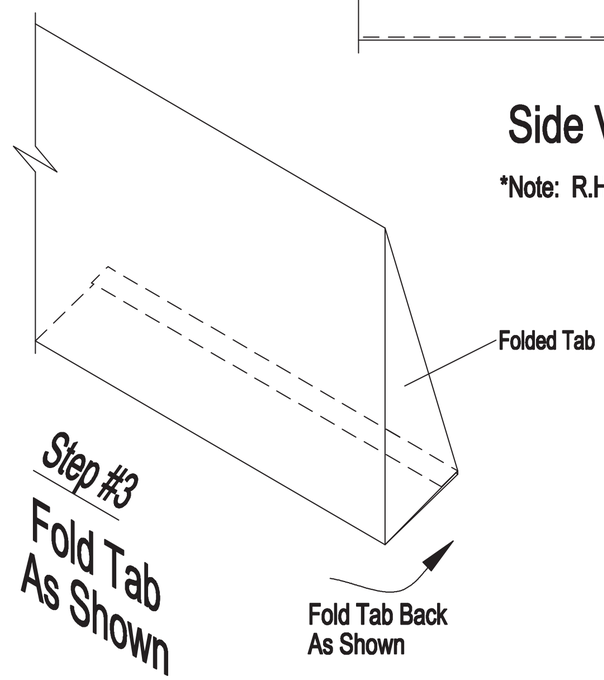
Cut To Roof Pitch		
Roof Pitch	"H" =	"W" =
1:12	- -	9/16"
2:12	3/16"	1 1/8"
3:12	7/16"	1 11/16"
4:12	11/16"	2 1/8"
5:12	1 1/16"	2 1/2"
6:12	1 7/16"	2 7/8"



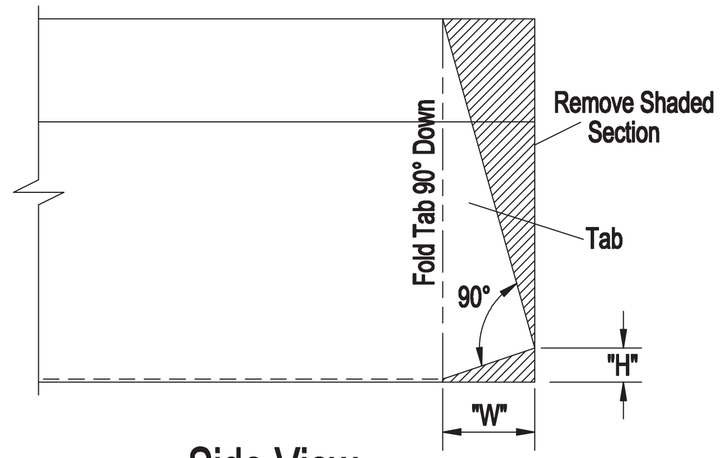
Step #1
Locate Eave Cover Trim



Step #2
Notch As Shown Below

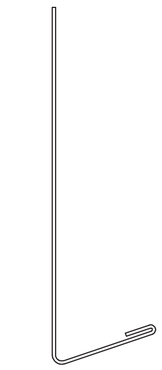


Step #3
Fold Tab As Shown



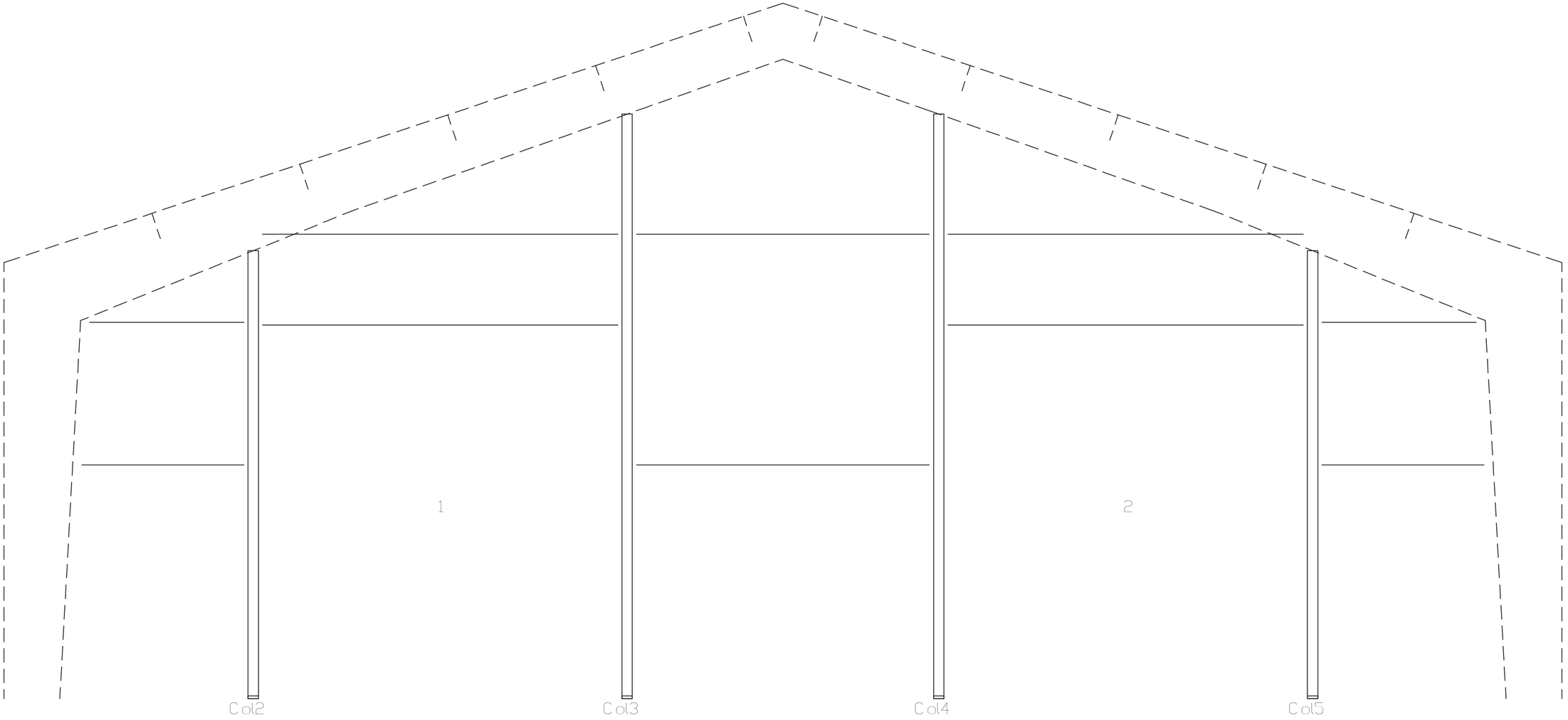
Side View

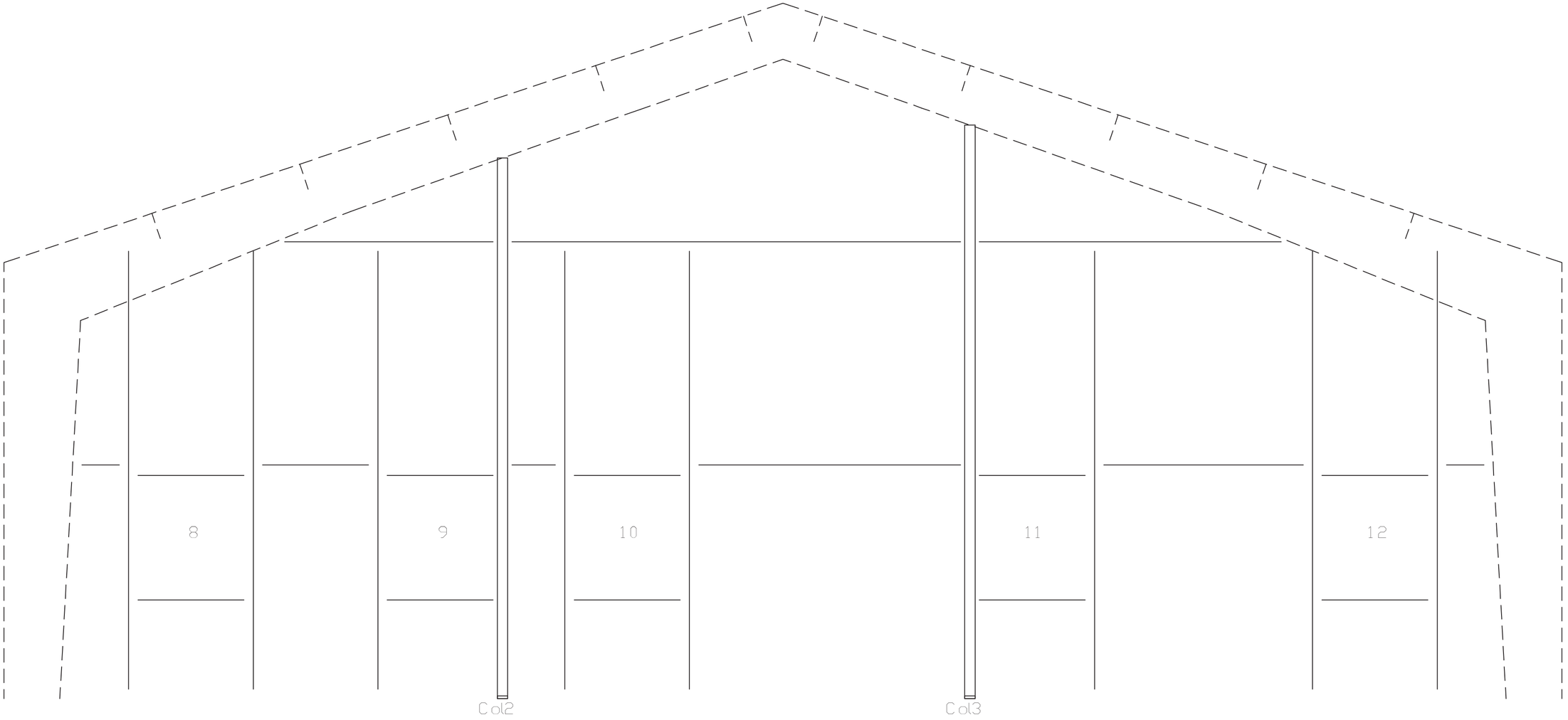
*Note: R.H. Shown
L.H. Opposite



End View

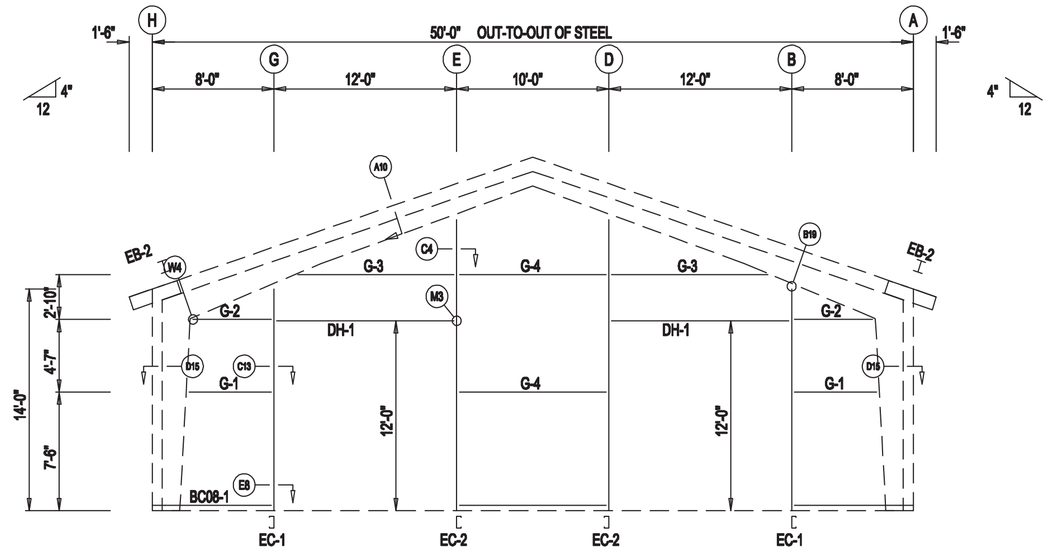
Instructions: Field Trimed Tab For Extension Cover Trim


1
2
Col2
Col3
Col4
Col5

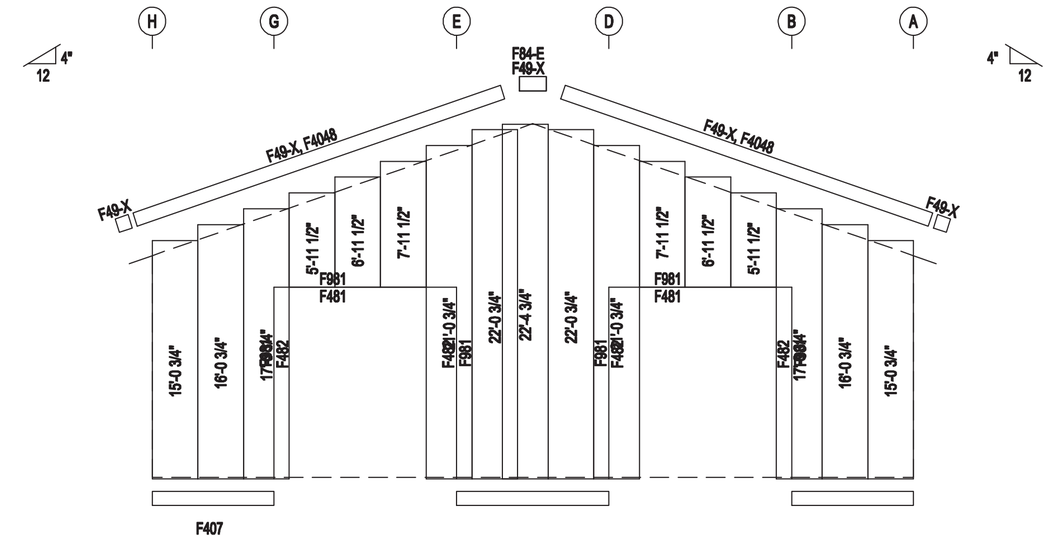


SE-2

2380395 RightEndwall Design 3/29/22



ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR - Light Stone

BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 1

LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
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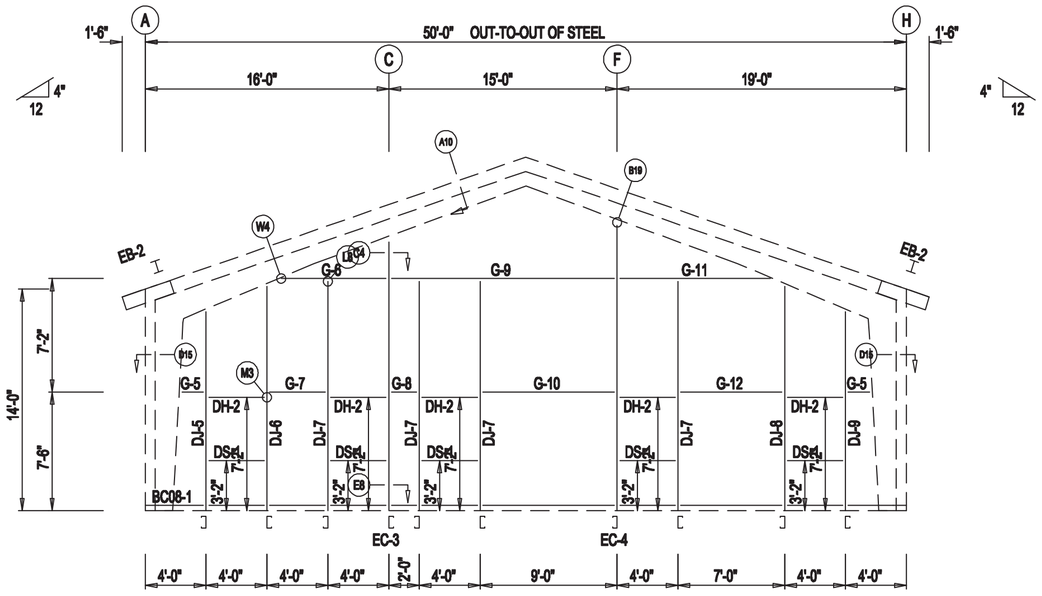
HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

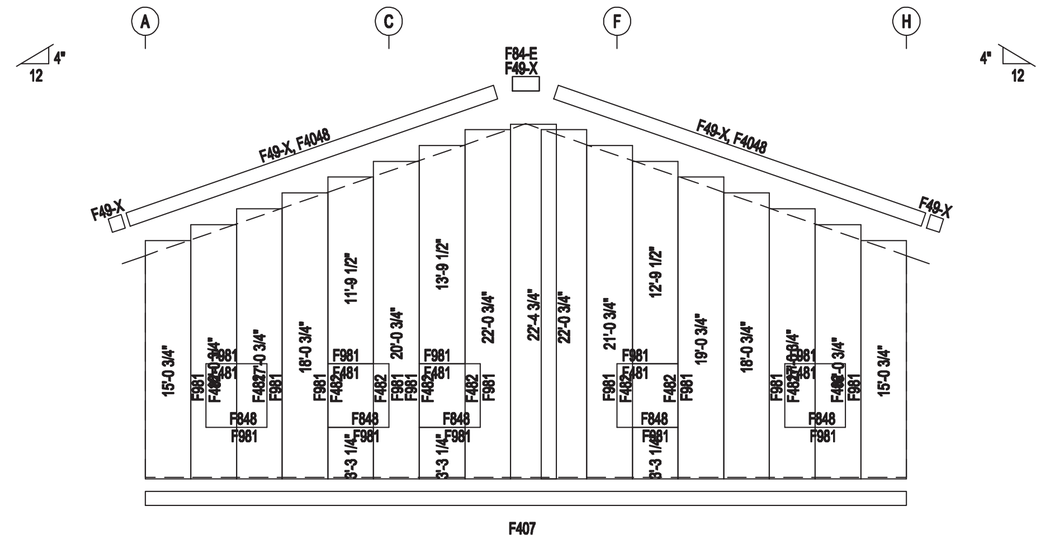
PROJECT:
CUSTOMER: OWNER:
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E5	0

S-23



ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Gauge PBR - Light Stone

BEARING FRAME ONLY!

WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE
FRAME LINE 4

LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

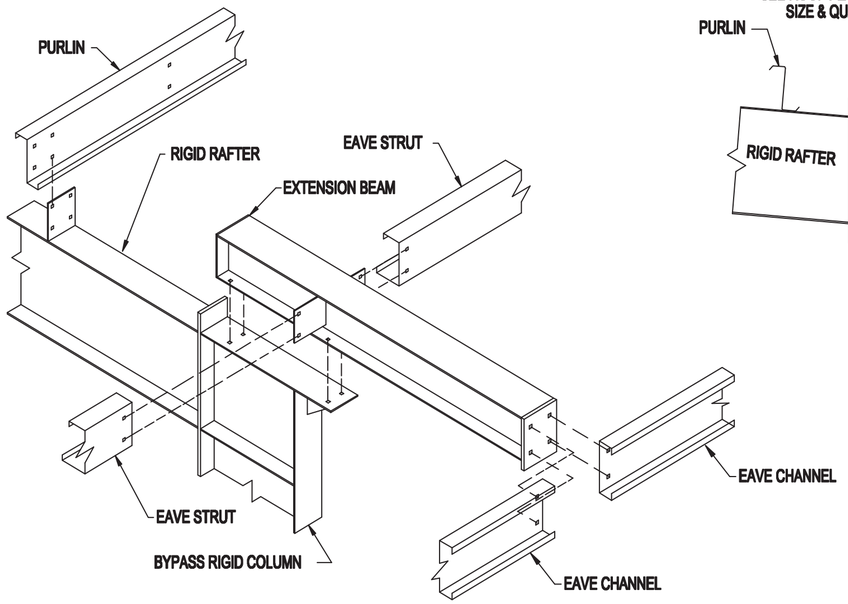
ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

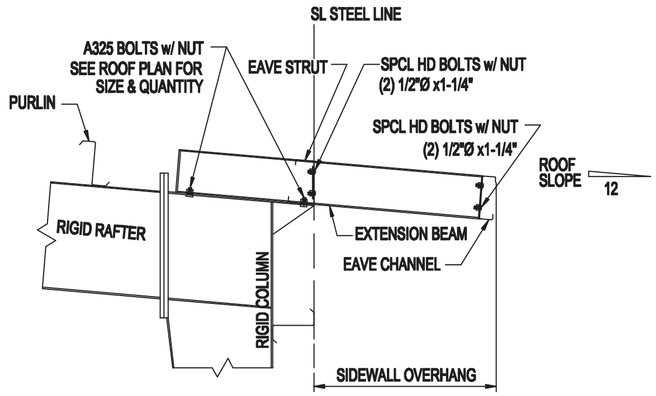
PROJECT:
CUSTOMER: OWNER:
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E6	0

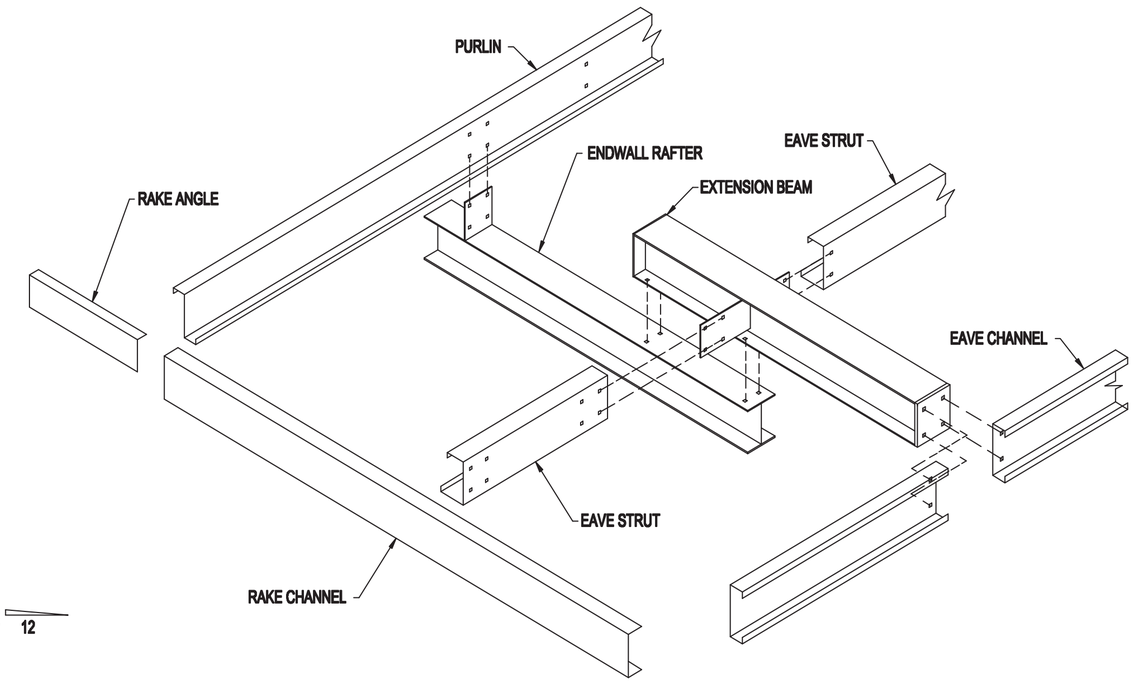
S-24



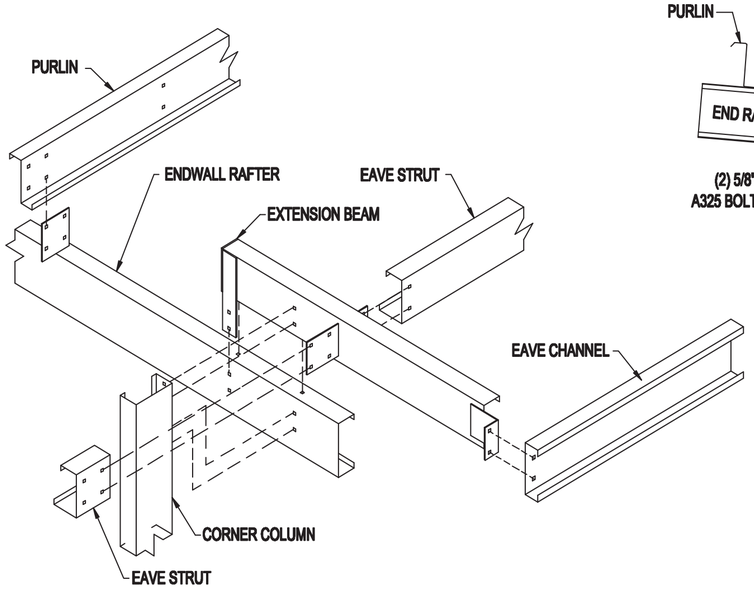
EXPLODED ISOMETRIC VIEW S/W OVERHANG
REFER TO SECTION FOR FASTENERS



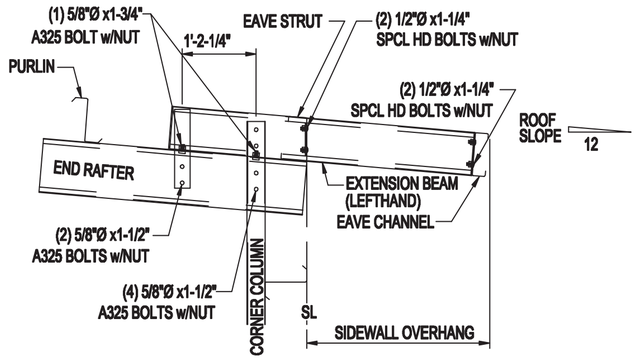
SECTION THRU RIGID FRAME



EXPLODED ISOMETRIC VIEW S/W TO E/W OVERHANG
REFER TO SECTION FOR FASTENERS



EXPLODED ISOMETRIC VIEW
REFER TO SECTION FOR FASTENERS



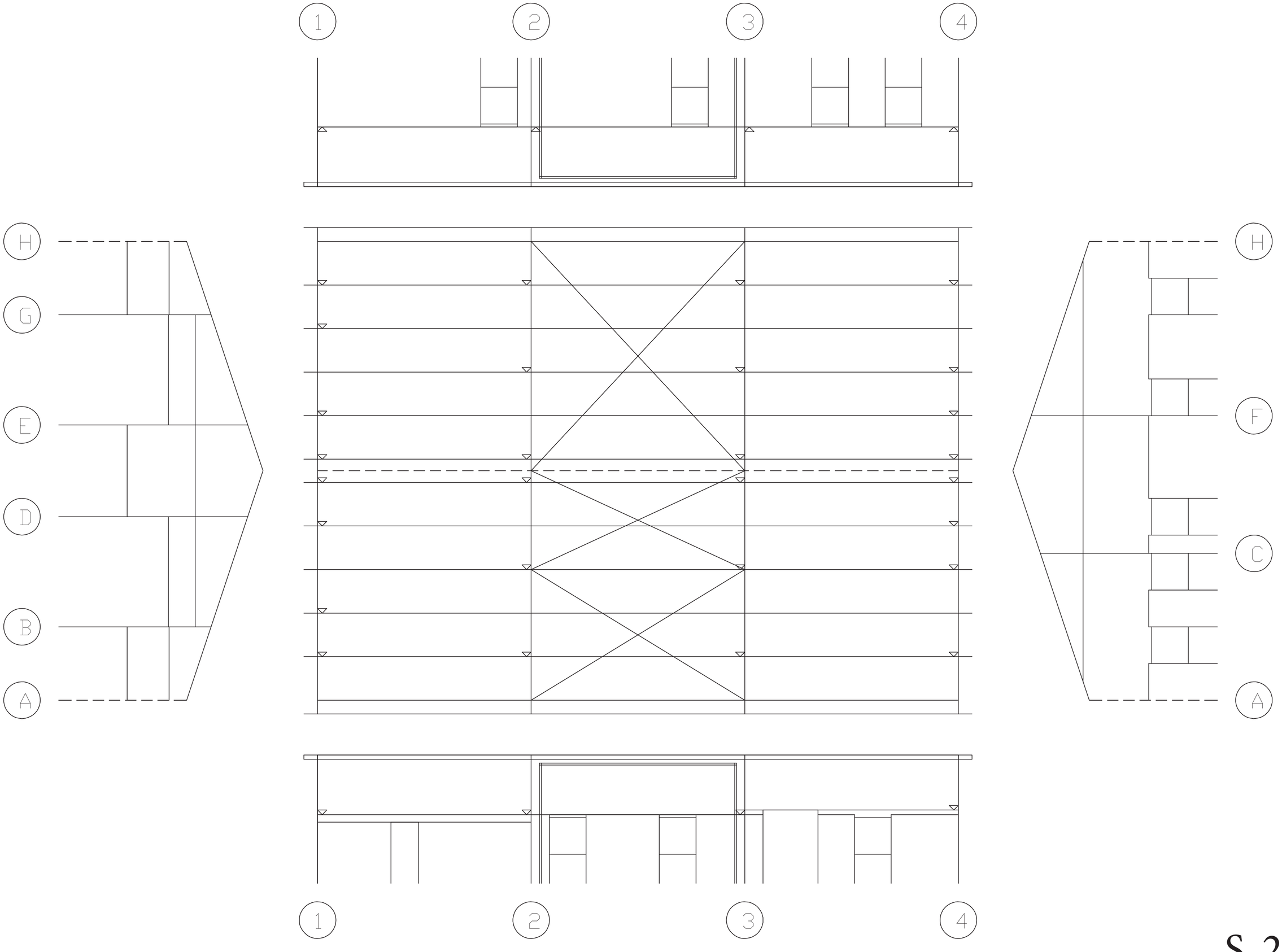
SECTION THRU BEARING FRAME
POST AND BEAM ENDWALL

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	9/15/16	FOR ERECTOR INSTALLATION	X	X	X

PROJECT: X
CUSTOMER: X
OWNER:
LOCATION: X

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	10/07/2016	N.T.S.	1	A	X		0

S-25

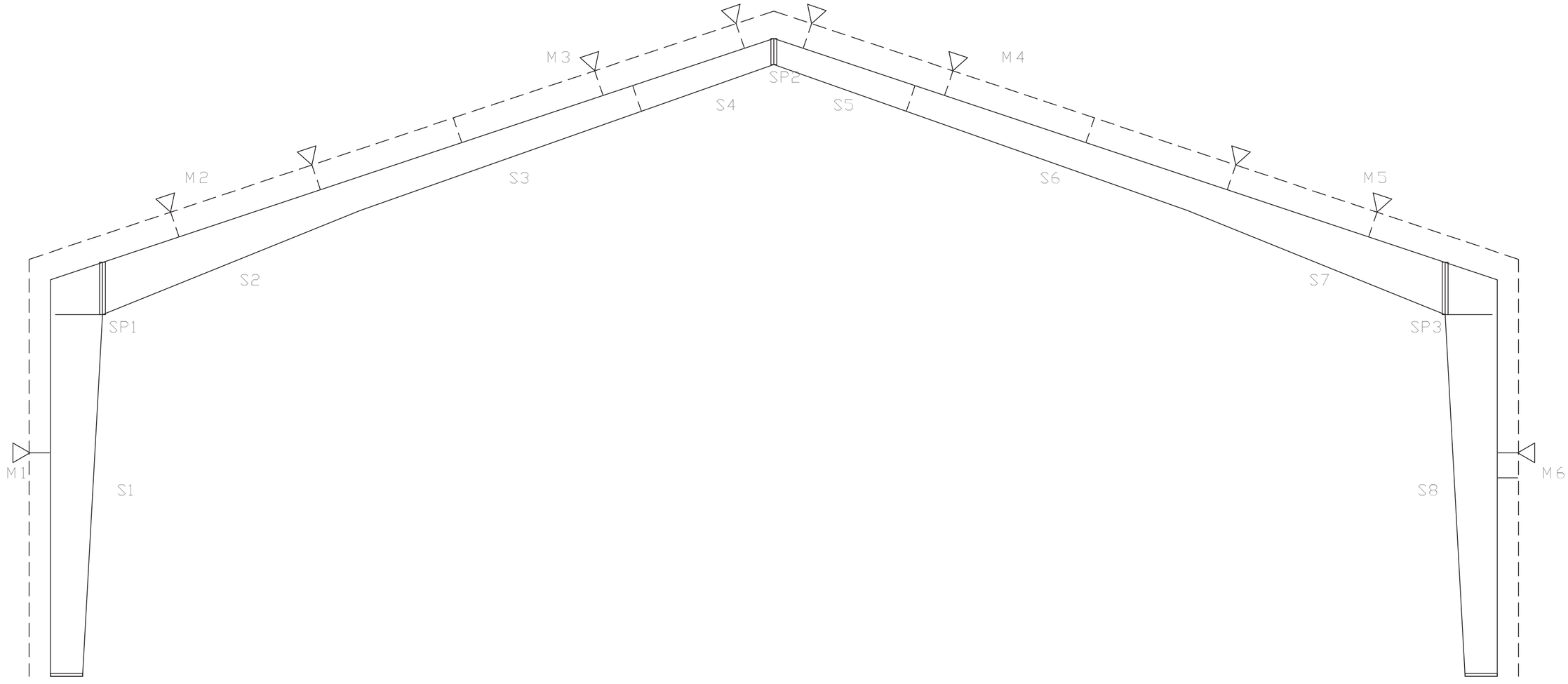


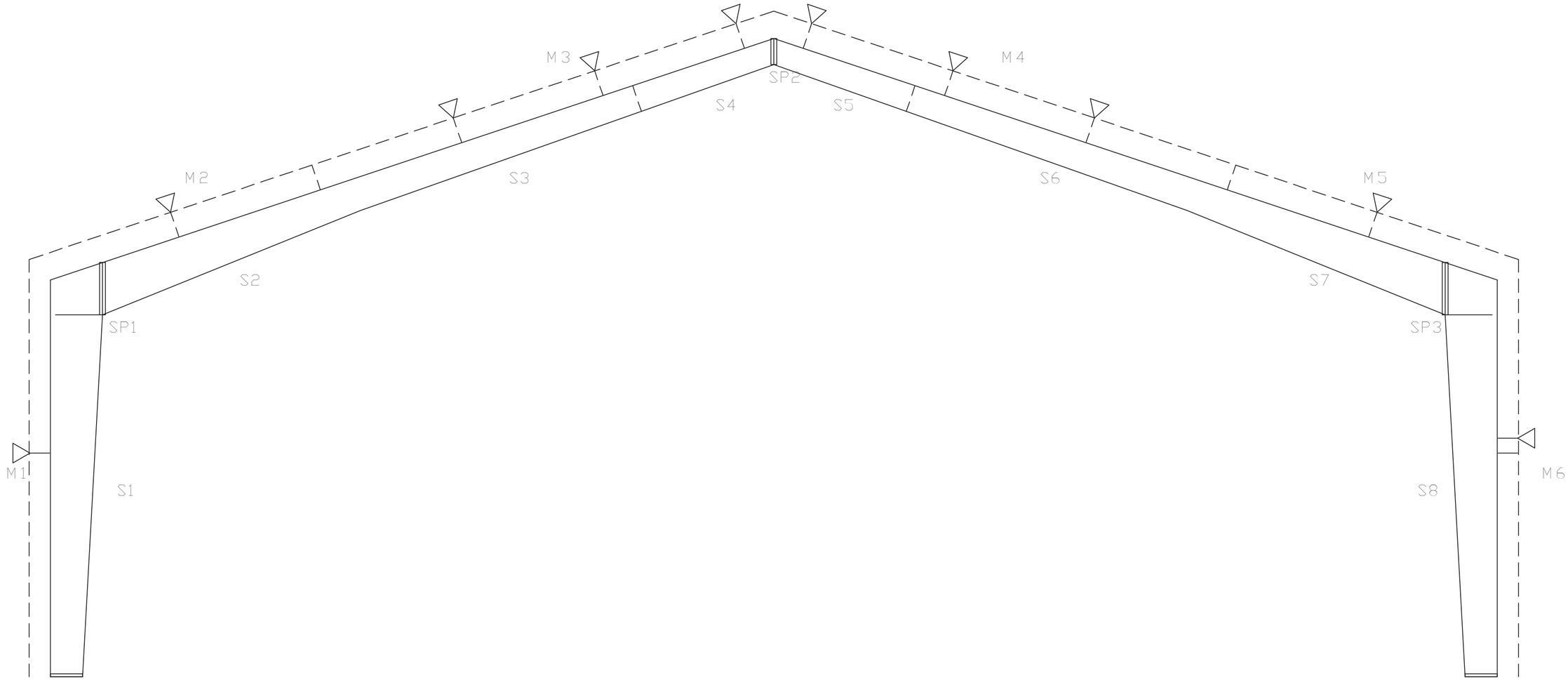
2380395

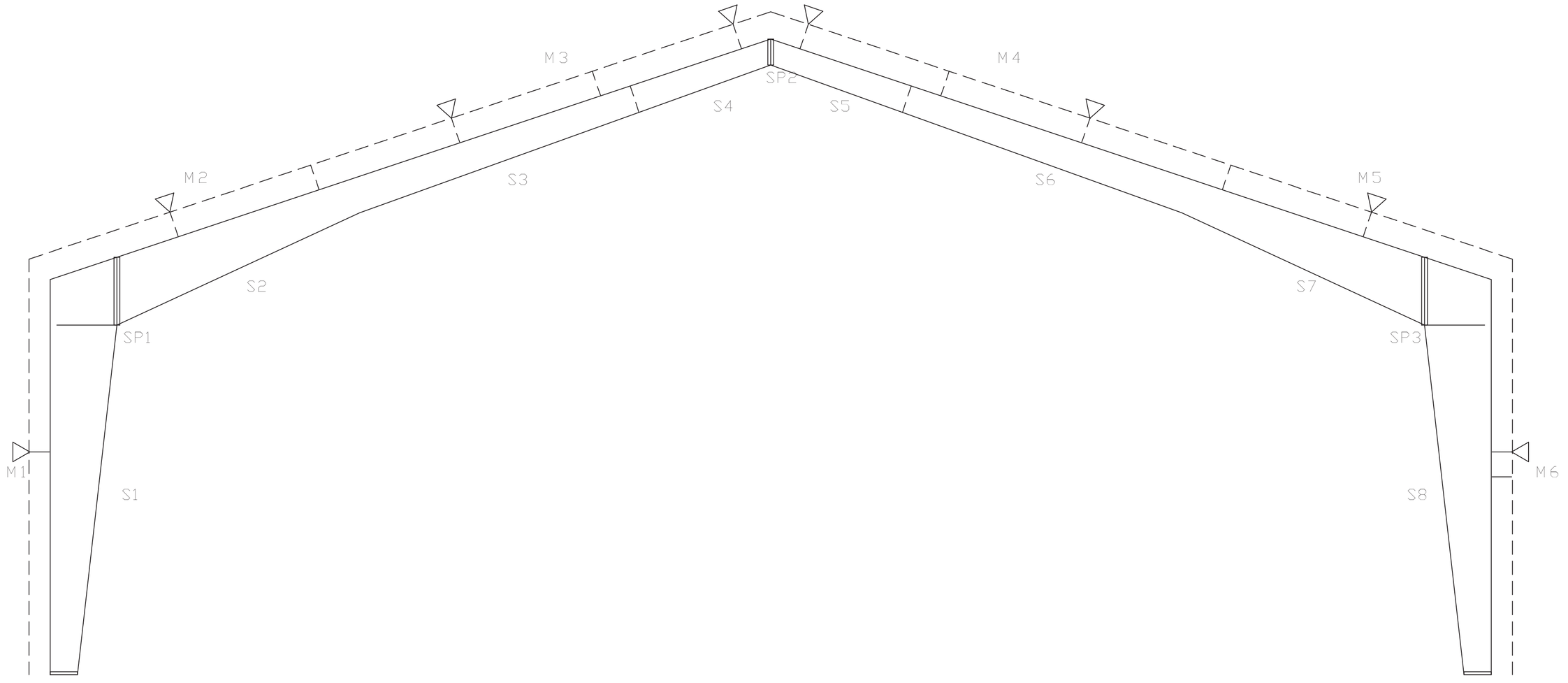
Framing Layout

3/29/22

S-26


M3
M4
SP2
S4
S5
M2
S3
S6
M5
S2
S7
SP1
SP3
M1
S1
S8
M6


M3
M4
SP2
S4
S5
M2
S3
S6
M5
S2
S7
SP1
SP3
M1
S1
S8
M6

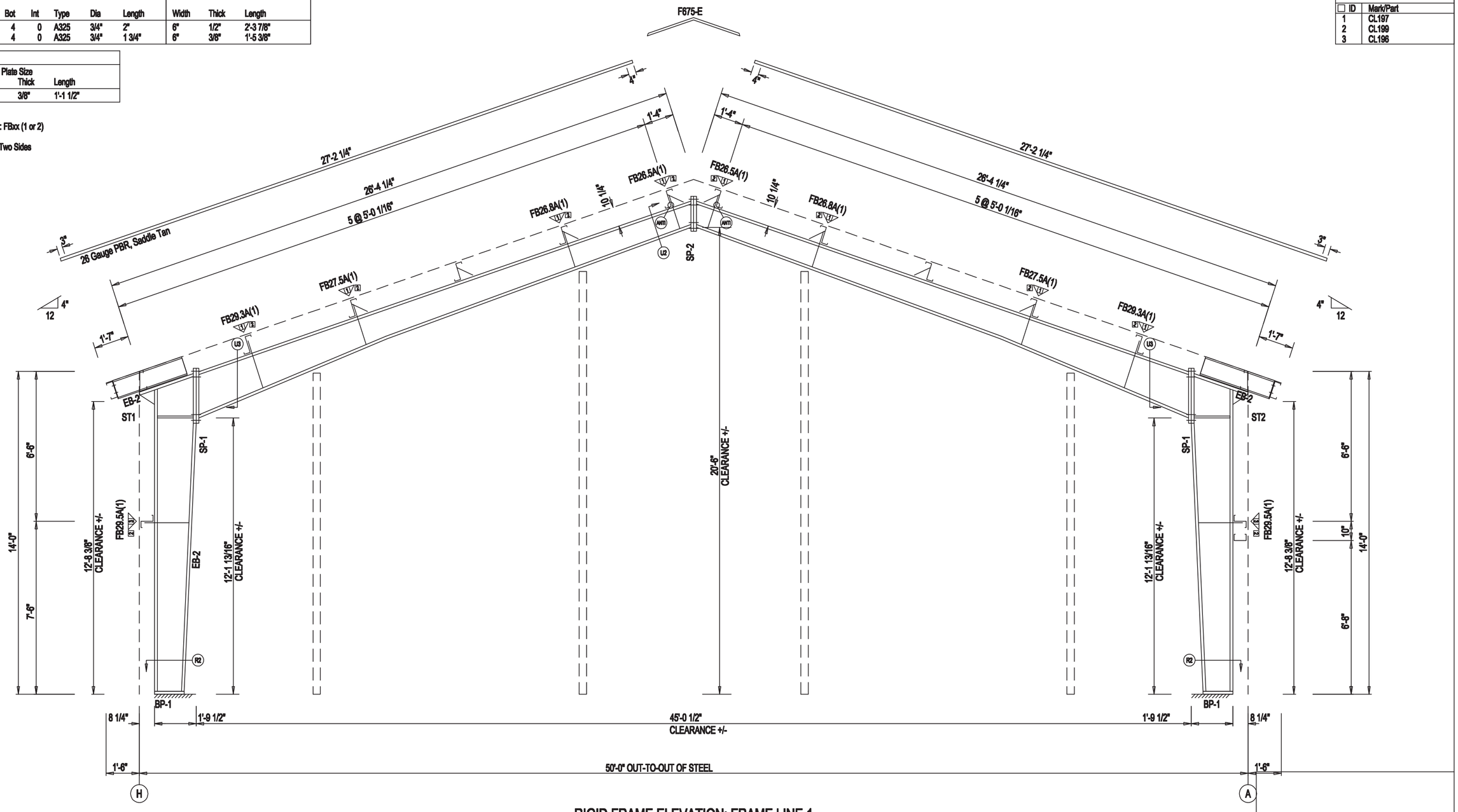

M3
M4
SP2
S4
S5
M2
S3
S6
M5
S2
S7
SP1
SP3
M1
S1
S8
M6

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2"	6"	1/2"	2'-3 7/8"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-5 3/8"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	6"	3/8"	1'-1 1/2"

CONNECTION PLATES	
ID	Mark/Part
1	CL197
2	CL199
3	CL196

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - L2X2X14G



RIGID FRAME ELEVATION: FRAME LINE 1

GENERAL NOTES:

1. BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD. STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:
CUSTOMER: OWNER:
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E7	0

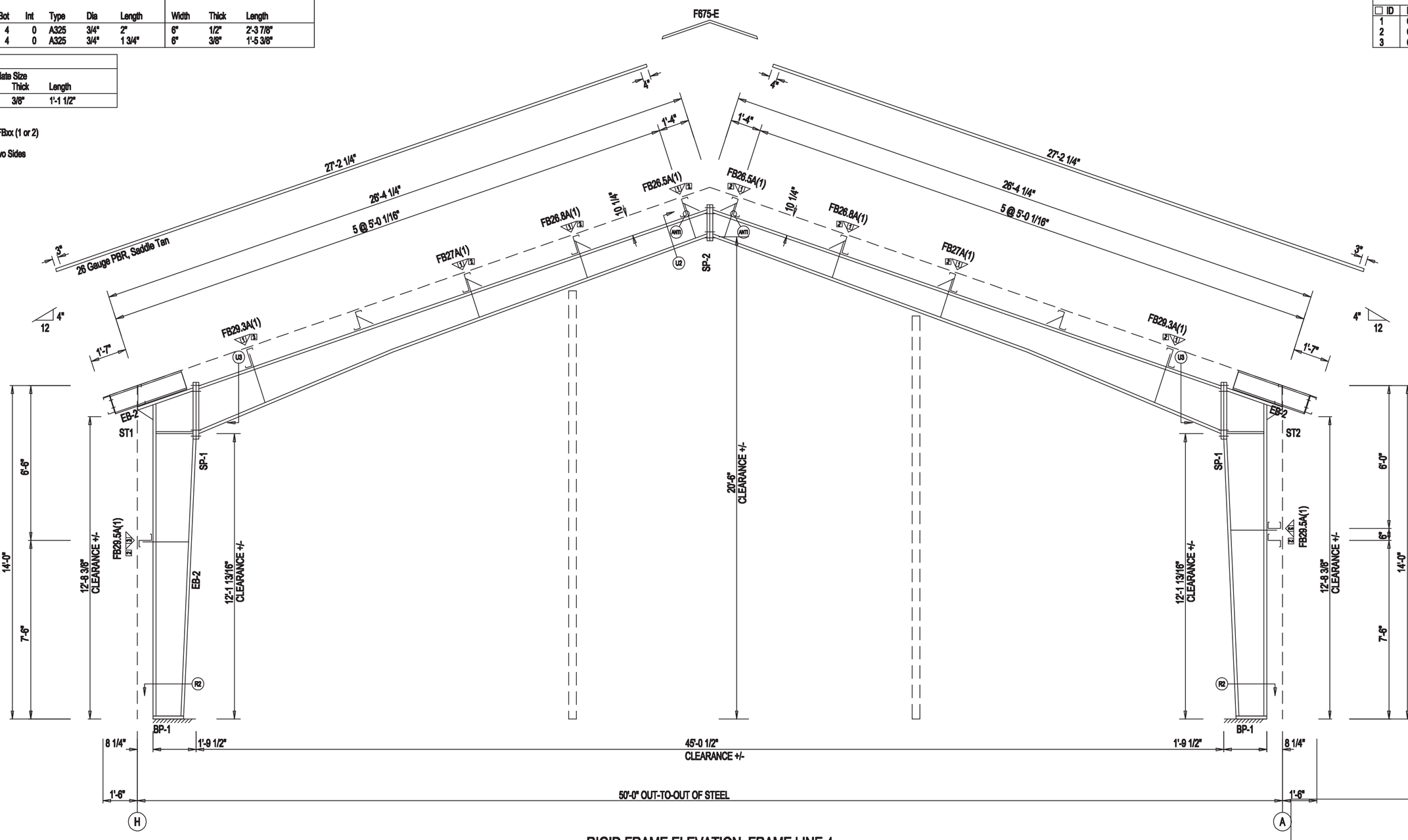
S-30

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0	A325	3/4"	2"	6"	1/2"	2'-3 7/8"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-5 3/8"

BASE PLATE TABLE			
Col Mark	Plate Size Width	Thick	Length
BP-1	6"	3/8"	1'-1 1/2"

CONNECTION PLATES	
ID	Mark/Part
1	CL197
2	CL199
3	CL196

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - L2X2X14G



RIGID FRAME ELEVATION: FRAME LINE 4

GENERAL NOTES:

1. BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
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1-800-643-5555

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LOCATION:

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	4/12/22	N.T.S.	1	A		E8	0

S-31

SPLICE PLATE & BOLT TABLE

Mark	Qty Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	3/4"	2"	6"	1/2"	2'-10 1/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-5 3/8"

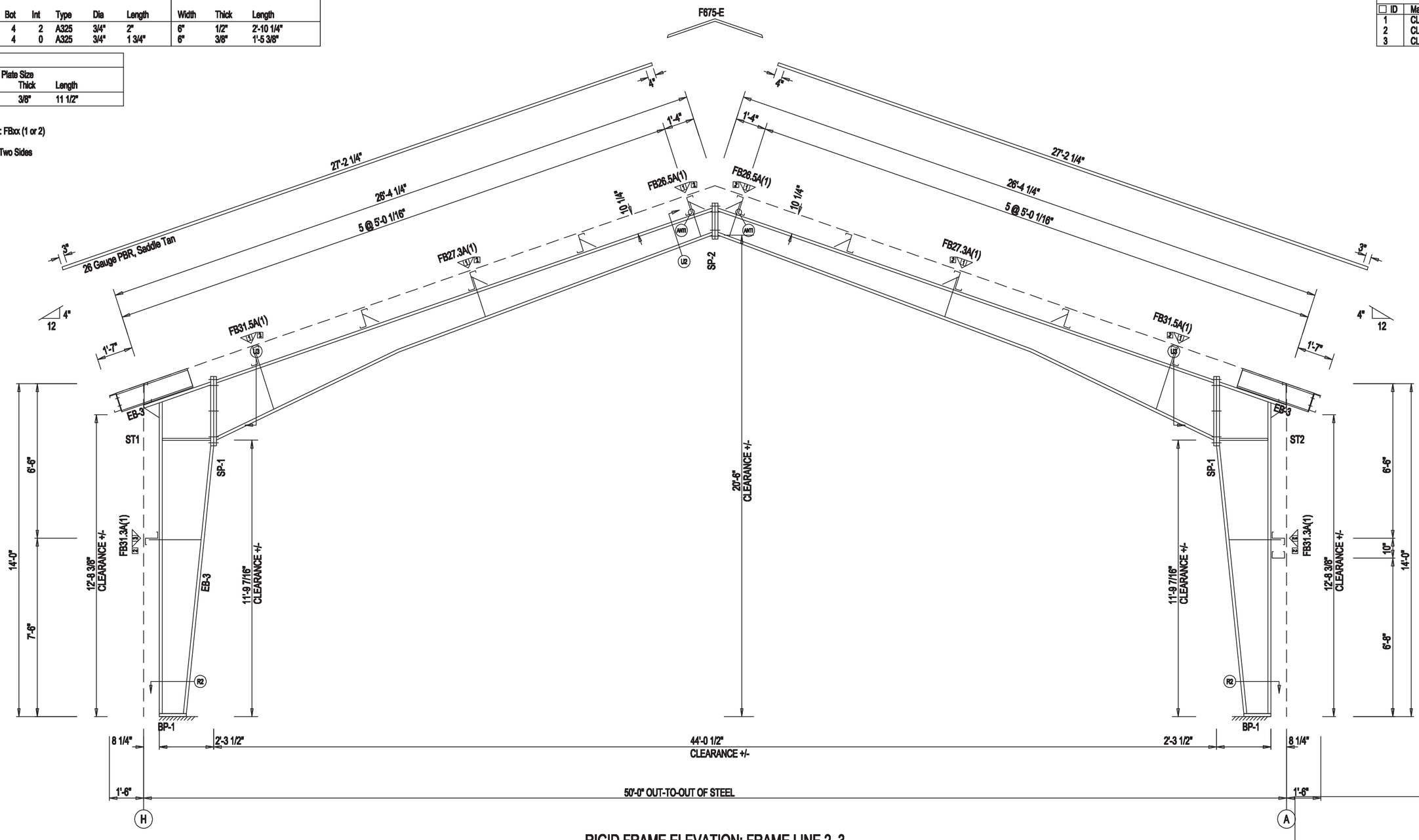
BASE PLATE TABLE

Col Mark	Plate Size Width	Thick	Length
BP-1	6"	3/8"	11 1/2"

CONNECTION PLATES

ID	Mark/Part
1	CL197
2	CL199
3	CL196

FLANGE BRACES: FBxx (1 or 2)
xx=length(in)
(1) One Side; (2) Two Sides
A - L2X2X14G



RIGID FRAME ELEVATION: FRAME LINE 2 3

GENERAL NOTES:

1. BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
2. ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
3. INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

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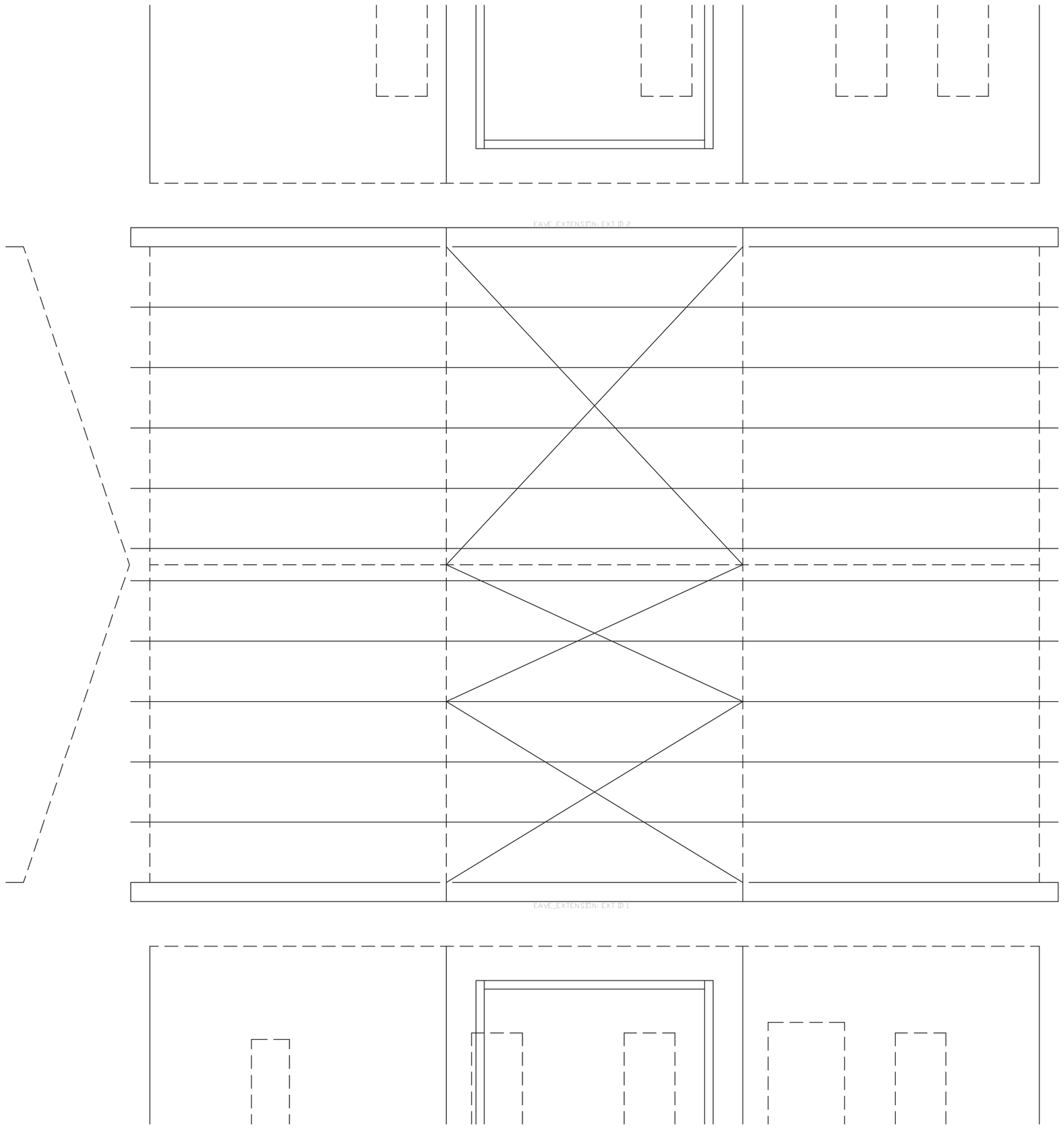
	4/12/22	N.T.S.	1	A		E9	0

S-32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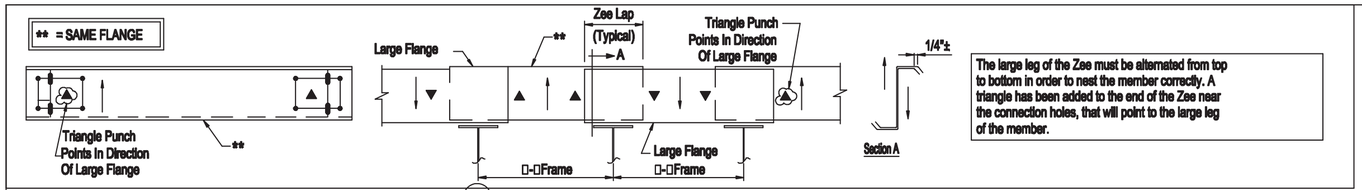
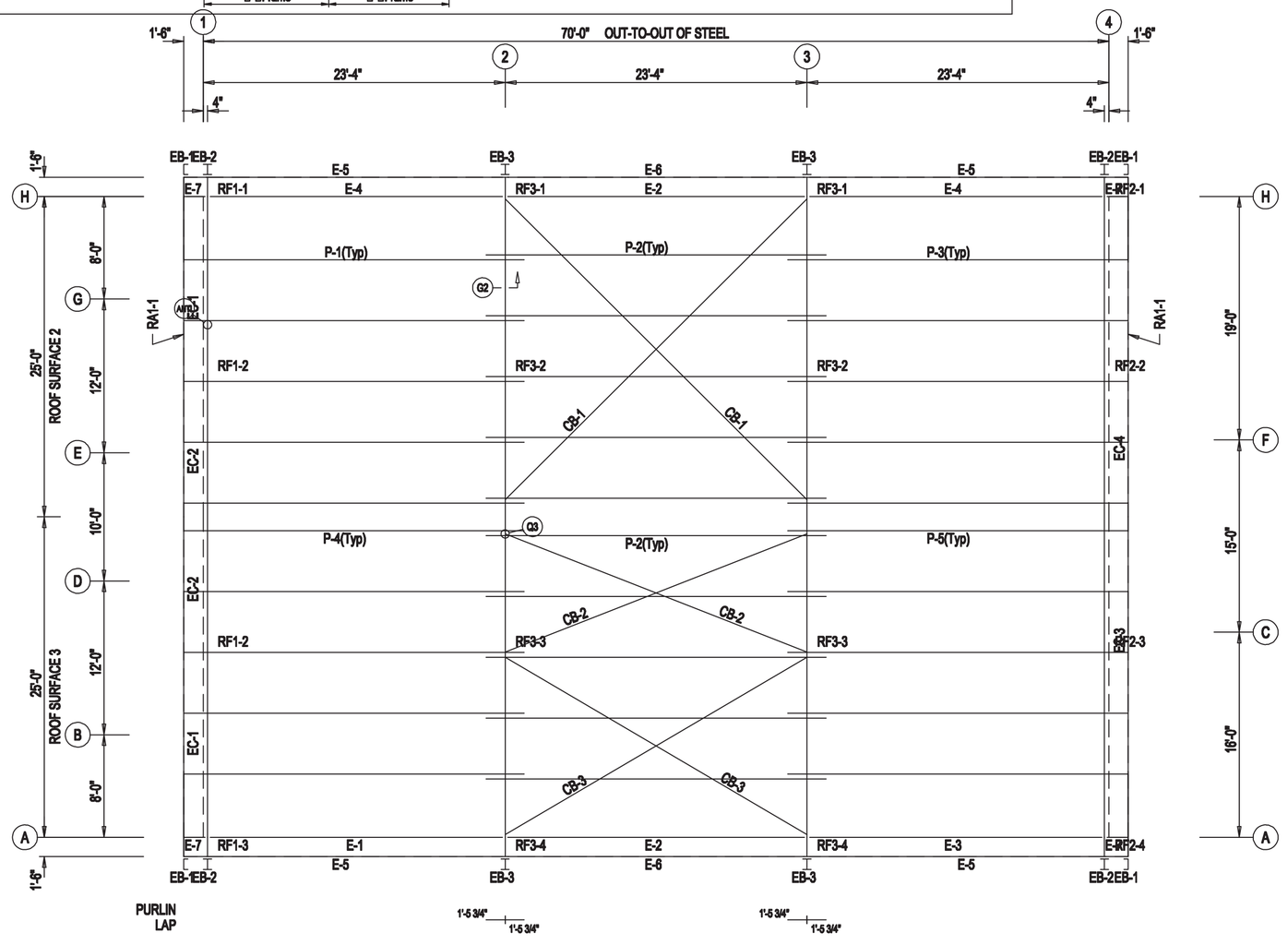
EAVE_EXTENSION: EXT ID 1



EXTENSION/CANOPY BOLTS ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	1/2"	1 1/4"
EB-3	4	A325	1/2"	1 1/4"



ROOF FRAMING PLAN

GENERAL NOTES:

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE
BUILDING SYSTEMS
2513 MCCAIN BLVD. STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:

CUSTOMER: OWNER:

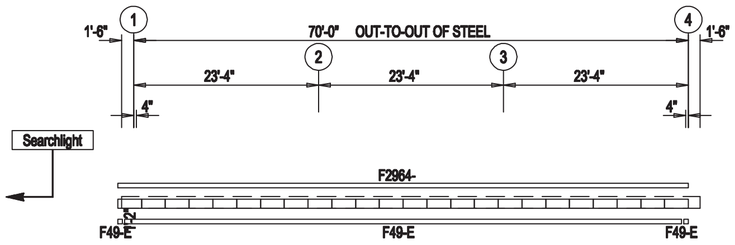
LOCATION:

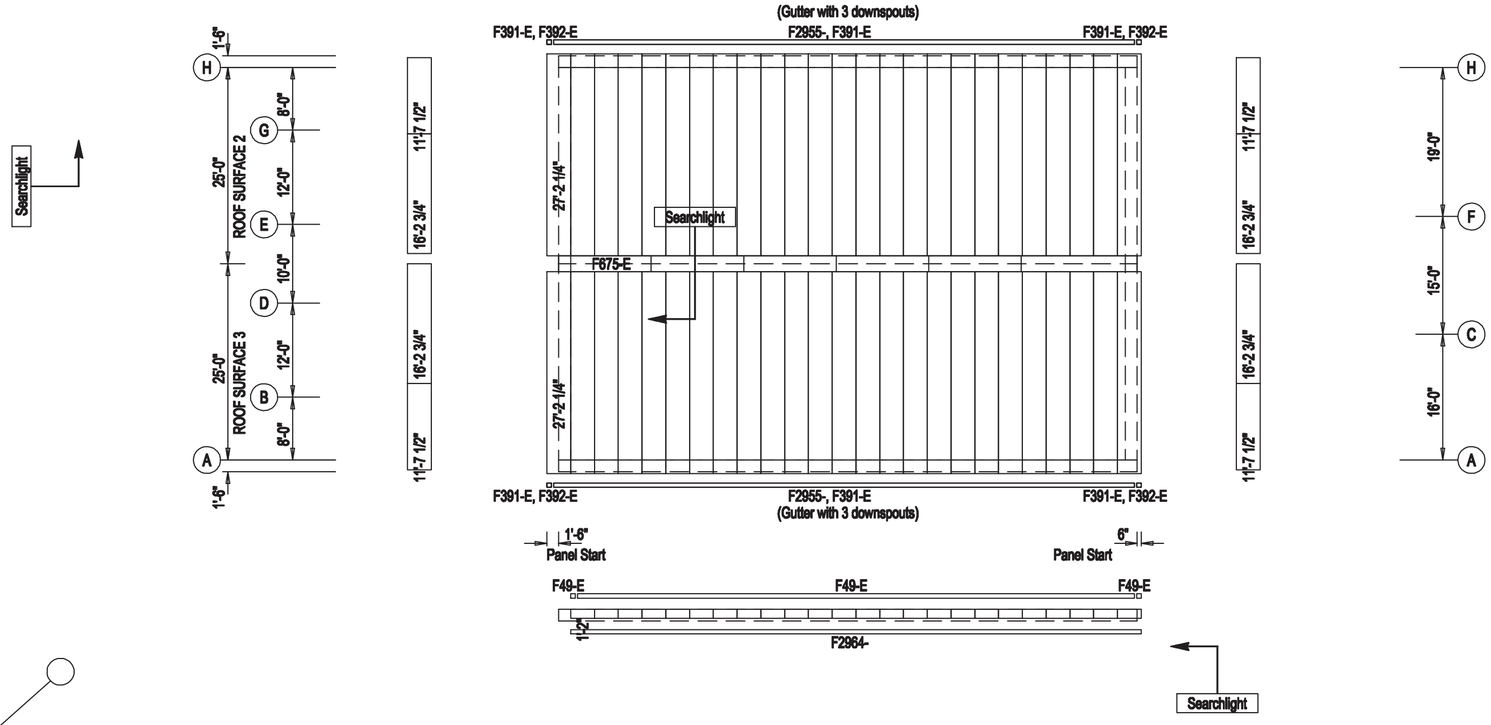
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E1	0

S-34

PBR ROOF SHEETING NOTE:

PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.





ROOF SHEETING PLAN

PANELS: 26 Gauge PBR - Saddle Tan

GENERAL NOTES:

1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:

CUSTOMER: OWNER:

LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E2	0

S-35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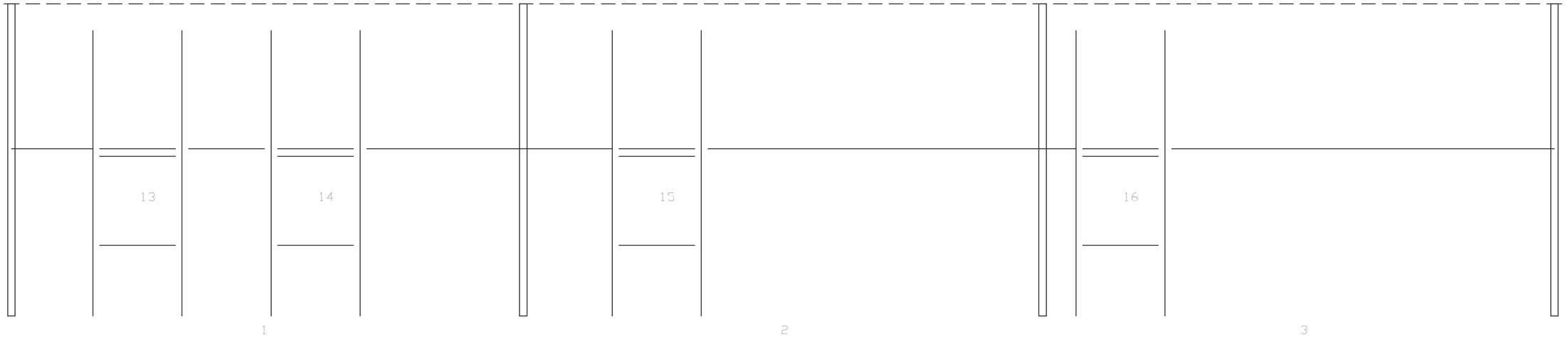
S-37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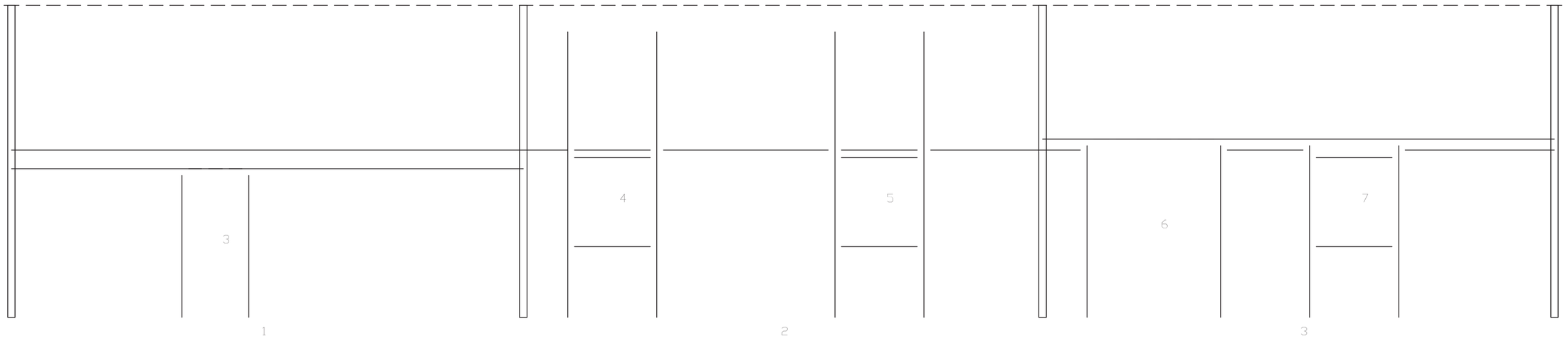
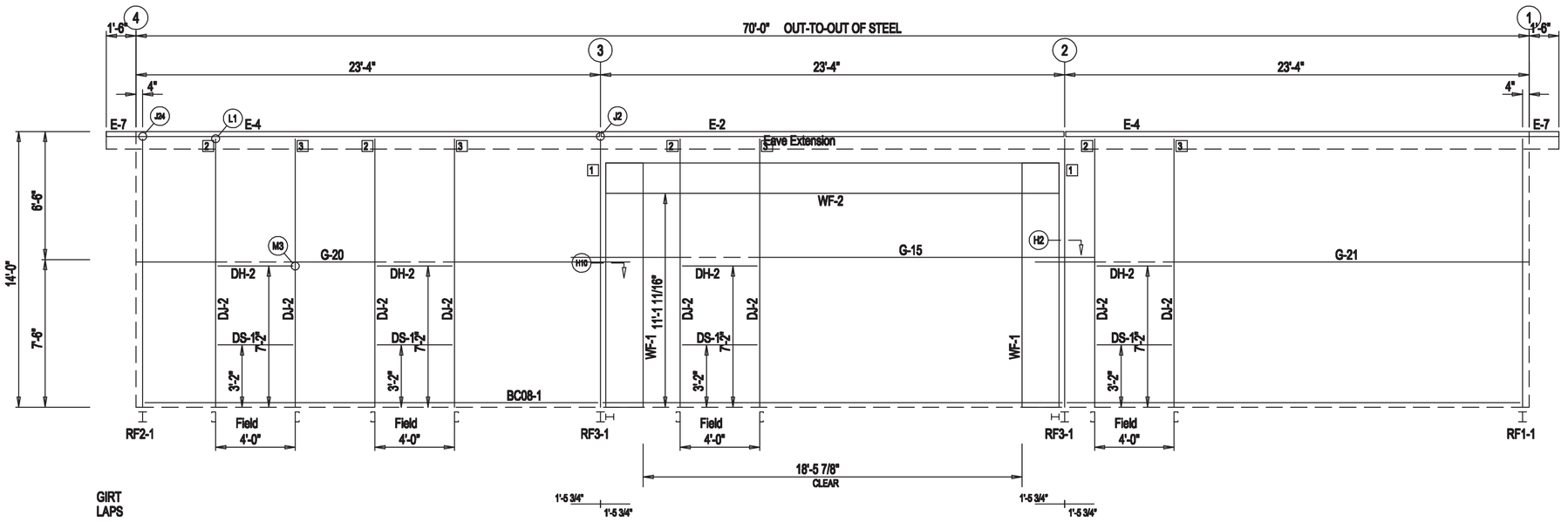


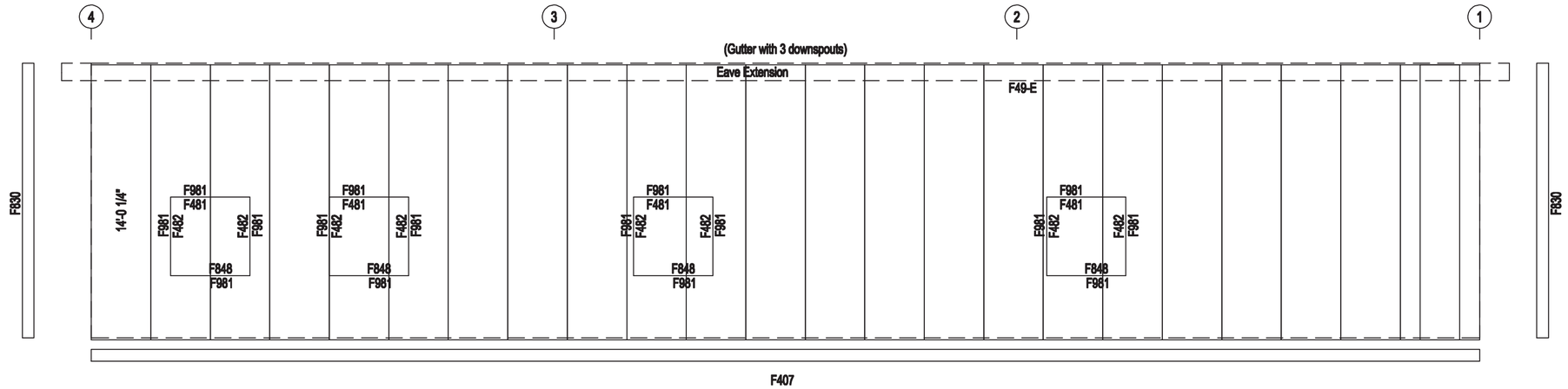
S-39

BOLT TABLE FRAME LINE H				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RF3-1	8	A325	3/4"	1 1/2"

CONNECTION PLATES FRAME LINE H	
ID	MARK/PART
1	SC479
2	SC587_L
3	SC587_R



SIDEWALL FRAMING: FRAME LINE H



SIDEWALL SHEETING & TRIM: FRAME LINE H

PANELS: 26 Gauge PBR - Light Stone

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

PROJECT:

CUSTOMER: OWNER:

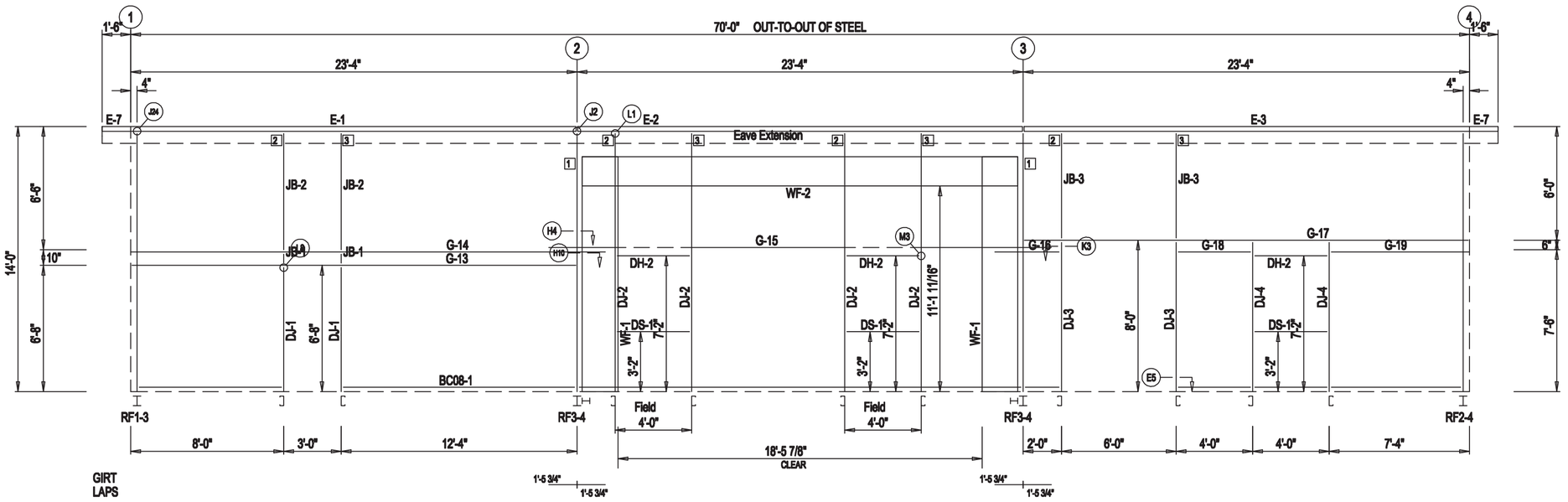
LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E4	0

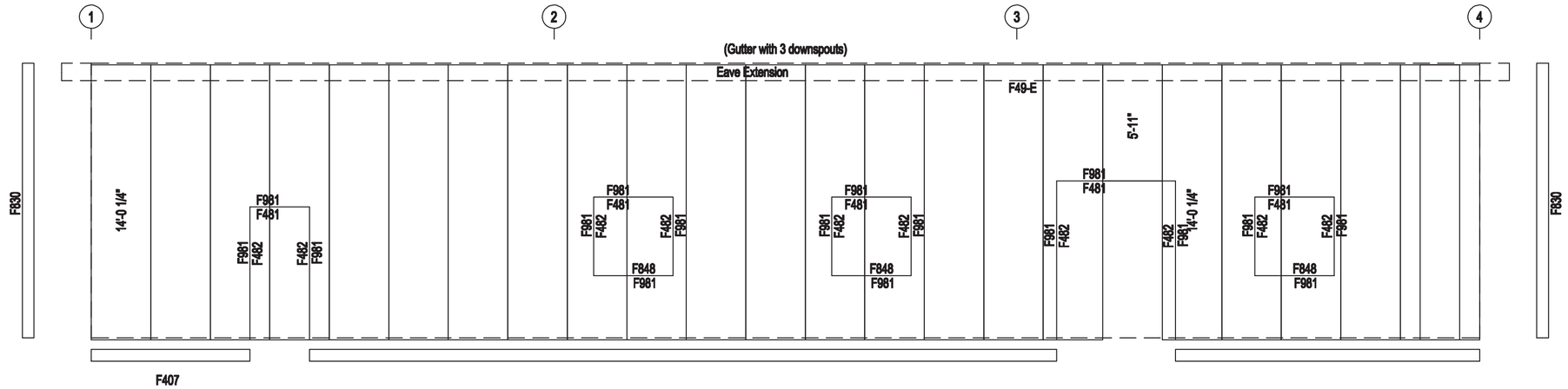
S-40

BOLT TABLE FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RF3-4	8	A325	3/4"	1 1/2"

CONNECTION PLATES FRAME LINE A	
ID	MARK/PART
1	SC479
2	SC587_L
3	SC587_R



SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR - Light Stone

DOWNSPOUT SPACING LOCATIONS

DOWNSPOUTS ARE TO BE PLACED AT A SPACING NOT TO EXCEED ?? FT. WITH A DOWNSPOUT WITHIN ?? FT. OF EACH END OF THE GUTTER RUN. GUTTER STRAPS TO BE 2'-0" ON CENTER.

GENERAL NOTES:

1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CK'D	DSN
0	4/12/22	FOR QUOTE			

HERITAGE BUILDING SYSTEMS
2513 MCCAIN BLVD, STE 2 #385
NORTH LITTLE ROCK, AR 72116-7606
1-800-643-5555

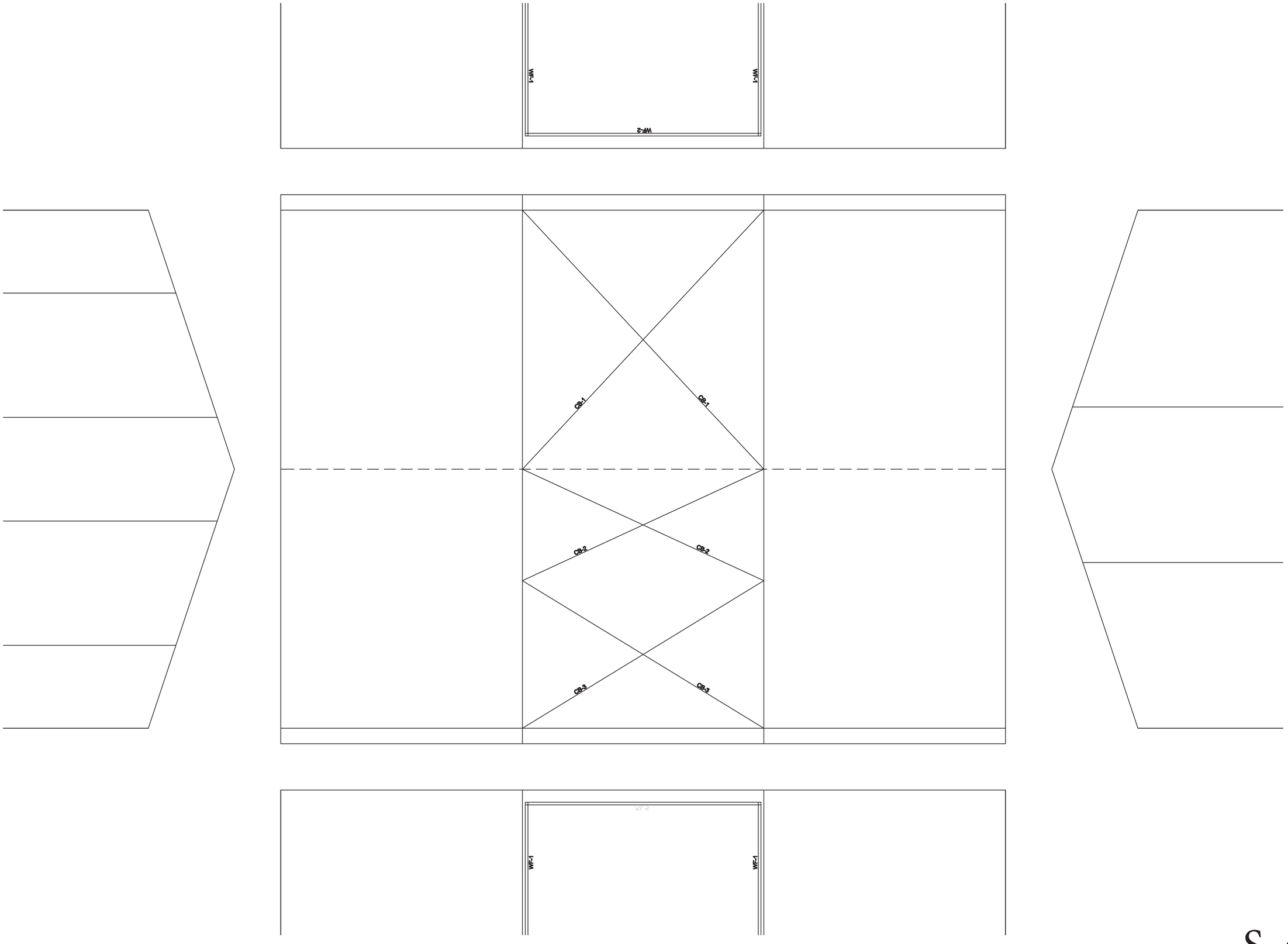
PROJECT:

CUSTOMER: OWNER:

LOCATION:

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E3	0

S-41



S-42

STRUCTURAL GENERAL NOTES

STRUCTURAL OBSERVATIONS

1. STRUCTURAL OBSERVATION IS REQUIRED FOR THE STRUCRURAL SYSTEM IN ACCORDANCE WITH CITY OF SAN JOSE ORDINANCES. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE BUILDING INSPECTOR OR THE DEPUTY INSPECTOR.

2. THE OWNER SHALL EMPLOY A CIVIL OR STRUCTURAL ENGINEER OR ARCHITECT TO PERFORM THE STRUCTURAL OBSERVATION. THE ENGINEER OR ARCHITECT SHALL BE REGISTERED OR LICENSED IN THE STATE OF CALIFORNIA. THE DEPARTMENT OF BUILDING & SAFETY RECOMMENDS THE USE OF THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN WHEN THEY ARE INDEPENDENT OF THE CONTACTOR.

3. THE STRUCTURAL OBSERVER SHALL PROVIDE EVIDENCE OF EMPLOYMENT BY THE OWNER. A LETTER FROM THE OWNER OR A COPY OF THE AGREEMENT FOR SERVICES SHALL BE SENT TO THE BUILDING INSPECTOR BEFORE THE FIRST SITE VISIT. THE STRUCTURAL OBSERVER SHALL ALSO INFORM THE OWNER OF THE REQUIREMENTS FOR A PRE-CONSTRUCTION MEETING AND SHALL PRESIDE OVER THAT MEETING.

4. THE OWNER OR OWNER'S REPRESENTATIVE SHALL COORDINATE AND CALL FOR A MEETING BETWEEN THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN, STRUCTURAL OBSERVER, CONTRACTOR, AFFECTED SUBCONTRACTORS AND DEPUTY INSPECTORS. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT THE VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS. A RECORD OF THE MEETING SHALL BE INCLUDED IN THE FIRST OBSERVATION REPORT AND SUBMITTED TO THE BUILDING INSPECTOR.

5. THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM, THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL ENGINEER.

STRUCTURAL OBSERVATION & DESIGNATION OF THE STRUCTURAL OBSERVER			
PROJECT ADDRESS: 10818 CROTHERS ROAD, SAN JOSE, CA 95127			PERMIT APPL. NO.:
DESCRIPTION OF WORK: PROPOSED NEW BARN			
OWNER: LARSON BARN	ARCHITECT: ECO-STRUCTION		ENGINEER: THANG LE, SE
STRUCTURAL OBSERVATION (ONLY CHECKED ITEMS ARE REQUIRED)			
FIRM OR INDIVIDUAL TO BE RESPONSIBLE FOR THE STRUCTURAL OBSERVATIONS: THANG LE, S.E.			
NAME: THANG LE	PHONE: 626-731-1539		CALIF. REGISTRATION: S4978
FOUNDATION	WALL	FRAME	DIAPHRAGM
☑ FTG., STEM WALLS, PIERS	☐ CONCRETE	☐ STL. MMNT. FRM.	☐ CONCRETE
☐ MAT FOUNDATION	☐ MASONRY	☐ STL. BRACED FRM.	☐ STEEL DECK
☐ CAISSON, PILES, GRD. BMS.	☐ WOOD SHEAR	☐ CONC. MMNT. FRM.	☐ WOOD
☐ STEPPED FTG./RETAINING FND. HILLSIDE SPECIAL ANCHORS	☐ WALL GREATER THAN 350 PLF	☐ MAS. WALL FRM.	☐ OTHERS
☐ OTHERS:	☐ RASTRA	☐ OTHERS:	

6. THE STRUCTURAL OBSERVER SHALL PREPARE A REPORT ON THE DEPARTMENT FORM B&S 261 FOR EACH SIGNIFICANT STAGE OF CONSTRUCTION OBSERVED. THE ORIGINAL OF THE OBSERVATION REPORT SHALL BE SENT TO THE BUILDING INSPECTOR'S OFFICE AND SHALL BE SIGNED AND SEALED (WET STAMPED) BY THE RESPONSIBLE STRUCTURAL OBSERVER. ONE COPY OF THE OBSERVATION REPORT SHALL BE ATTACHED TO THE APPROVED PLANS. COPIES OF THE REPORT SHALL ALSO BE GIVEN TO THE OWNER, CONTRACTOR AND DEPUTY INSPECTOR.

7. A FINAL OBSERVATION REPORT MUST BE SUBMITTED WHICH SHOWS THAT ALL OBSERVED DEFICIENCIES WERE RESOLVED AND THE STRUCTURAL SYSTEM GENERALLY CONFORMS WITH THE APPROVED PLANS AND SPECIFICATIONS. THE DEPARTMENT OF BUILDING AND SAFETY WILL NOT ACCEPT STRUCTURAL WORK WITHOUT THIS FINAL OBSERVATION REPORT AND THE CORRECTION OF SPECIFIC DEFICIENCIES NOTED DURING NORMAL BUILDING AND DEPUTY INSPECTION.

8. WHEN THE OWNER ELECTS TO CHANGE THE STRUCTURAL OBSERVER OF RECORD, THE OWNER SHALL:
   a) NOTIFY THE BUILDING INSPECTOR IN WRITING BEFORE THE NEXT INSPECTION;
   b) CALL AN ADDITIONAL PRE-CONSTRUCTION MEETING AND
   c) FURNISH THE REPLACEMENT STRUCTURAL OBSERVER WITH A COPY OF ALL PREVIOUS OBSERVATION REPORTS.

   THE REPLACEMENT STRUCTURAL OBSERVER SHALL APPROVE THE CORRECTION OF THE ORIGINAL OBSERVED DEFICIENCIES UNLESS OTHERWISE APPROVED BY PLAN CHECK SUPERVISION. THE POLICY OF THE DEPARTMENT SHALL BE TO CORRECT ANY PROPERLY NOTED DEFICIENCIES WITHOUT CONSIDERATION OF THEIR SOURCE.

9. THE ENGINEER OR ARCHITECT OF RECORD SHALL DEVELOPE ALL CHANGES RELATING TO THE STRUCTURAL SYSTEMS. THE BUILDING DEPARTMENT SHALL REVIEW AND APPROVE ALL CHANGES TO THE APPROVED PLANS AND SPECIFICATIONS.

B. STRUCTURAL STEEL

1. PROVIDE STRUCTURAL STEEL COMPLYING WITH THE 2020 LABC CHAPTER 22 AND THE FOLLOWING ASTM STANDARD SPECIFICATIONS, UNLESS OTHERWISE NOTED:
   a. ALL STRUCTURAL STEEL UNLESS OTHERWISE NOTED BELOW ..... ASTM A36
   b. ANCHOR BOLTS OR UNFINISHED MACHINE BOLTS ..... ASTM A307
   c. PIPES ..... ASTM A53, GRADE B (35 KSI)
   d. TUBES ..... ASTM A500, GRADE B (46 KSI)
   e. W SHAPES ..... ASTM A992 (50 KSI)
   f. THREADED ROUND STOCK ..... ASTM A36
   g. REINFORCING STEEL ..... SEE REINFORCING STEEL SECTION

2. HIGH STRENGTH BOLTS
   a. PROVIDE HIGH STRENGTH BOLTS, NUTS, AND WASHERS COMPLYING WITH ASTM A325 UNLESS OTHERWISE NOTED. ALL HIGH STRENGTH BOLTS SHALL BE BEARING TYPE WITH THREADS INCLUDED IN SHEAR PLANE (A325-N), UNLESS OTHERWISE NOTED. PROVIDE SLIP-CRITICAL HIGH STRENGTH BOLTS (A325-SC) ONLY WHERE SPECIFICALLY INDICATED ON PLANS.
   b. ASSEMBLE HIGH STRENGTH BOLTS IN COMPLIANCE WITH "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 FOR A490 BOLTS" AND THE IBC STANDARD 27-1 AND 27-7.

3. FABRICATE AND ERECT STRUCTURAL STEEL IN COMPLIANCE WITH THE LATEST EDITION OF AISC "LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS".

4. WELD STRUCTURAL STEEL IN COMPLIANCE WITH ANSI / AWS D1.1 AND AISC "SPECIFICATIONS," CHAPTER J.
   a. WELDERS SHALL CERTIFIED AS REQUIRED BY GOVERNING CODE AUTHORITY
   b. WELDING SHALL BE DONE BY ELECTRONIC ARC PROCESS USING E70XX ELECTRODES UNLESS OTHERWISE NOTED.
   c. WELDING MAY BE PERFORMED USING SUBMERGED ARC PROCESS WITH AUTOMATIC WELDING (SAW-1).
   d. PERFORM SHOP WELDING AND BY A FABRICATOR APPROVED BY GOVERNING CODE AUTHORITY.
   f. PROVIDE SPECIAL INSPECTION FOR ALL FIELD WELDING.

5. FIELD WELDING TO BE DONE IN WELDERS CERTIFIED BY THE LADBS FOR STRUCTURAL STEEL, REINFORCING STEEL AND LIGHT GAGE STEEL. CONTINUOUS INSPECTION BY A DEPUTY INSPECTOR IS REQQUIRED.

6. SHOP WELDS MUST BE PERFORMED IN THE LADBS LICENSED FABRICATOR SHOP.

7. LADBS LICENSED FABRICATOR IS REQUIRED FOR STRUCTURAL STEEL.

C. METAL STUD – LIGHT GAUGE STEEL (ICC-ES ESR-1538)

1. ALL WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:
   a. AMERICAN IRON AND STEEL INSITUTE (AISI) DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS.
   b. AMERICAN WELDING SOCIETY (AWS) D1.1 AND D1.3 SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURE.
   c. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM)

2. ALL STUD AND TRACK MATERIAL TO COMFORM TO THE FOLLOWING:
   a. 16 GAUGE AND HEAVIER:
      + 50 KSI MINIMUM YIELD, 65 KSI MINIMUM TENSILE STRENGTH
      + PAINTED STEEL ..... ASTM A570 – GRADE 50
      + GALVANIZED STEEL ..... ASTM A653 – GRADE 50
   b. 18 GAUGE AND LIGHTER:
      + 33 KSI MINIMUM YIELD, 45 KSI MINIMUM TENSILE STRENGTH
      + PAINTED STEEL..... ASTM A611 – GRADE C
      + GALVANIZED STEEL ..... ASTM A653 – GRADE 33

3. ALL WELDING TO BE PERFORMED BY CERTIFIED LIGHT GAUGE WELDERS CERTIFIED FOR ALL APPROPRIATE DIRECTION COMPLYING WITH AWS D1.2.

4. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY OR ON AN ANGLE SUCH AS BRACING TO SQUARELY FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD FIRMLY IN POSITION UNTIL PROPERLY FASTENED.

5. LOW VELOCITY FASTENERS (SHOTPINS) WHERE CALLED OUT ON PLAN SHALL BE RAMSET 1500 SERIES INSTALLED IN ACCORD LOCAL BUILDING CODE OR OTHER LISTED MAKE, APPROVED BY BUILDING OFFICIAL. LOW VELOCIT FASTENERS SHALL BE 0.14" DIAMETER WITH 1-1/4" MINIMUM EMBEDMENT, UNLESS OTHERWISE NOTED.

6. EXPANSION ANCHOR SHALL BE RAMSET/REDHEAD TRUBOLTS INSTALLED IN ACCORD WITH BUILDING DEPARMENT OR OTHER LISTED MAKE, APPROVE BY BUILDING OFFICIAL.

7. SCREWS SHALL BE "DART" BRAND SELF DRILLING/SELF-TAPPING STEEL SCREWS INSTALL ED IN ACCORD WITH BUILDING DEPARTMENT. SCEWS SHALL BE SUFFICIENT LENGTH TO ENSURE PENETRATION INTO STEEL STUD BY AT LEAST 2 FULL DIAMETER THREADS.

STATEMENT OF SPECIAL INSPECTIONS

1. THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTION DURING CONSTRUCTION. THE SPECIAL INSPECTOR(S) SHALL BE QUALIFIED TO THE SATISFACTION OF THE BUILDING OFFICIAL TO INSPECT THE KIND OF CONSTRUCTION BEING EMPLOYED IN THIS PROJECT. THE SPECIAL INSPECTOR SHALL SUBMIT REPORTS INDICATING RESULTS AND OBSERVATIONS OF TESTS AND INSPECTIONS AND STATING COMPLIANCE OR NONCOMPLIANCE WITH CONTRACT DOCUMENTS TO STRUCTURAL ENGINEER AND TO GOVERNING CODE AUTHORITY.

2. TESTING LABORATORY SHALL PROVIDE SPECIAL INSPECTION, COMPLYING WITH LABC SECTION 1701 (UNLESS OTHERWISE NOTED), FOR THE FOLLOWING:

A. EPOXY ANCHORS
B. BOLTS INSTALLED IN CONCRETE
C. CONCRETE STRENGTH f'c > 2,500 PSI
D. SHEATHED SHEAR WALL WHEN SHEAR EXCEEDS 350 POUNDS PER LINEAR FOOT WHERE THE FASTENER SPACING OF THE SHEATHING IS 4 INCHES ON CENTER OR LESS.

3. CONTRACTORS RESPONSIBLE FOR THE CONSTRUCTION OF A WIND OR SEISMIC FORCE RESISTING SYSTEM/COMPONENT LISTED IN THE "STATEMENT OF SPECIAL INSPECTIONS" SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE LADBS INSPECTORS AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON SUCH SYSTEM OR COMPONENT PER SECTION 1704.4.

4. CONTINUOUS SPECIAL INSPECTOR BY A REGISTERED DEPUTY INSPECTOR IS REQUIRED FOR FIELD WELDING, POST-INSTALLED ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR UPWARDLY INCLINED TO RESIST SUSTAINED TENSION LOADS, SHOTCRETE PLACEMENT, CONCRETE STRENGTH f'c > 2,500 PSI, HIGH STRENGTH BOLTING, SPRAYED-ON FIREPROOFING, ENGINEERED MASONRY, HIGH-LIFT GROUTING, PRE-STRESSED CONCRETE, HIGH LOAD DIAPHRAGMS, SPECIAL MOMENT-RESISTING CONCRETE FRAMES, AND HELICAL PILE FOUNDATIONS.

5. PERIODIC SPECIAL INSPECTION IS REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING, AND OTHER FASTENING TO COMPONENTS OF THE SEISMIC FORCE RESISTING SYSTEM. SPECIAL INSPECTION BY A DEPUTY INSPECTOR IS REQUIRED WHERE THE FASTENER SPACING OF THE SHEATHING IS 4 INCHES ON CENTER OR LESS.

B. FOUNDATION

1. PERFORM FOUNDATION WORK COMPLYING WITH REPORT AND ADDENDA. GEOTECHNICAL REPORT AND ADDENDA HERERY BECOME PART OF THESE CONTRACT DOCUMENTS AND SHALL BE KEPT ON JOB SITE AT ALL TIMES.

   FOUNDATION DESIGN IS BASED ON RECOMMENDATIONS OF
   BUTANO GEOTECHNICAL ENGINEERING, INC.
   231 GREEN VALLEY ROAD, SUITE E
   FREEDOM, CALIFORNIA 95019
   REPORT NO. 19-150-SCL
   DATED MAY 8, 2020

   ALLOWABLE SOIL BEARING = 1,500 PSF
   MAXIMUM SOIL BEARING = 3,000 PSF
   MINIMUM FOOTING DEPTH = 24 INCHES
   MINIMUM FOOTING WIDTH = 12 INCHES FOR CONTINUOUS FOOTINGS
   = 24 INCHES FOR PAD FOOTINGS

2. FOUNDATION EXCAVATIONS ARE TO BE OBSERVED BY AND ACCEPTABLE TO A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE PRIOR TO PLACEMENT OF FILL, REINFORCING STEEL, OR CONCRETE.

3. PERFORM FILLING, BACKFILLING, COMPACTION, ETC... AS INDICATED IN GEOTECHNICAL REPORT AND ONLY UNDER SUPERVISION OF A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE.

5. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS PRIOR TO COMPLETION AND INSPECTION OF WATERPROOFING. ADEQUATELY SHORE RETAINING WALLS DURING BACKFILL OPERATION. UNLESS ADEQUATELY SHORED, DO NOT PLACE BACKFILL BEHIND BUILDING STRUCTURE RETAINING WALLS, EXCLUDING SITE RETAINING WALLS, UNTIL CONCRETE AT ELEVATED FLOOR LEVELS ADJACENT TO WALLS ARE COMPLETELY POURED AND HAVE CURED FOR AT LEAST 7 DAYS.

6. THE APPROVED SOILS REPORT SHALL BE A PART OF THE PLANS AND SHALL BE KEPT AT THE JOB SITE AT ALL TIMES.

C. REINFORCING STEEL

1. REINFORCING STEEL COMPLYING WITH ASTM A615, GRADE 60 DEFORMED BARS, EXCEPT #3 BAR CAN BE GRADE 40 OR STRONGER.

2. WELDED REINFORCING STEEL COMPLYING WITH ASTM A706, GRACE 60 DEFORMED BARS.

3. SMOOTH WELDED WIRE FABRIC COMPLYING WITH ASTM A185. LAP FABRIC 1-1/2 SPACES (12" MINIMUM). PROVIDE DEFORMED WIRE STIRRUPS, SIZE D4 AND LARGER ONLY, COMPLYING WITH ASTM 497.

2. SPLICE REINFORCING STEEL WHERE INDICATED. IF SPLICE LOCATIONS ARE NOT SPECIFICALLY SHOWN OR INDICATED, VERIFY SPLICE LOCATIONS WITH ARCHITECT/ ENGINEER PRIOR TO DEVELOPING REINFORCING STEEL SHOP DRAWINGS.

3. LAP REINFORCING STEEL AT SPLICES TO THE FOLLOWING MINIMUM LENGTHS, UNLESS OTHERWISE NOTED, (APPLICABLE TO 3,000 PSI OR HIGHER, NORMAL WEIGHT CONCRETE ONLY):

BAR SIZE	TOP BARS	OTHER BARS	BAR SIZE	TOP BARS	OTHER BARS
#3	1'-9"	1'-4"	#8	6'-10"	5'-3"
#4	2'-4"	1'-10"	#9	8'-8"	6'-8"
#5	2'-11"	2'-3"	#10	11'-0"	8'-6"
#6	3'-10"	2'-11"	#11	13'-6"	10'-6"
#7	5'-3"	4'-0"			

TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW REBAR.

OTHER BARS ARE HORIZONTAL BARS WITH LESS THAN 12 INCHES OF CONCRETE CAST BELOW BARS AND ALL VERTICAL BARS.

4. MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL, INCLUDING SPLICED REINFORCING STEEL, SHALL BE 1 INCH OR 1 BAR DIAMETER, WHICHEVER IS GREATER. FOR BUNDLED BARS, MINIMUM CLEAR DISTANCES BETWEEN UNITS OF BUNDLED BARS SHALL BE SAME AS SINGLE BARS EXCEPT BAR DIAMETER IS DERIVED FROM EQUIVALENT TOTAL AREA OF BUNDLE.

5. MAINTAIN THE FOLLOWING MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL AND FACE OF CONCRETE UNLESS OTHERWISE NOTED:
   a. SLAB-ON-GRADE ..... C/L OF SLAB
   b. CONCRETE BELOW GRADE, FORMED ..... 2 INCHES
   c. CONCRETE BELOW GRADE, UNFORMED ..... 3 INCHES
   d. WALLS ABOVE GRADE, EXPOSED TO WEATHER ..... 2 INCHES
   e. WALLS ABOVE GRADE, NOTE EXPOSED TO WEATHER ..... 1 INCHES
   f. COLUMNS, CLEAR TO FACE OF TIES ..... 1-1/2 INCHES
   g. BEAMS, CLEAR TO FACE OF TIES ..... 1-1/2 INCHES

6. BEND REINFORCING STEEL COLD UNLESS OTHERWISE ACCEPTED BY ARCHITECT OR ENGINEER.

7. CHAIRS OR SPACERS FOR REINFORCING SHALL BE PLASTIC OR PLASTIC COATED WHEN RESTING ON EXPOSED SURFACES.

8. WELD REINFORCING STEEL COMPLYING WITH AWS D1.4. DO NOT WELD REINFORCING STEEL OTHER THAN THOSE CONFORMING TO ASTM A706.

9. SECURELY TIE ANCHOR BOLTS, REINFORCING STEEL, INSERTS, ETC... IN PLACE PRIOR TO PLACING CONCRETE OR GROUT.

10. SUBMIT REINFORCING STEEL SHOP DRAWINGS INDICATING REINFORCING PLACEMENT, INCLUDING SPLICE LOCATIONS AND LENGTHS, TO ARCHITECT/ENGINEER FOR REVIEW AND ACCEPTANCE.

D. CAST-IN-PLACE CONCRETE

1. NORMAL WEIGHT AGGREGATES OF NATURAL SAND AND ROCK COMPLYING WITH ASTM C33.

2. PORTLAND CEMENT CONFORMING TO ASTM C150, TYPE II.

3. NORMAL WEIGHT CONCRETE (145 PCF), WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.05%, ATTAINING MINIMUM COMPRESSIVE STRENGTHS (f'c) AT 28 DAYS AS FOLLOWS:
   FOUNDATIONS ..... 3,000 PSI
   CONCRETE WALLS ..... 3,000 PSI
   SLAB-ON-GRADE ..... 3,000 PSI
   UNLESS OTHERWISE NOTED ..... 3,000 PSI

4. SLUMP NOT TO EXCEED 4 INCHES.

5. DO NOT USE CONCRETE OR GROUT CONTAINING CHLORIDES

6. DO NOT EMBED CONDUITS, PIPES, OR SLEEVES OTHER THAN ELECTRICAL CONDUITS 1 INCH DIAMETER AND SMALLER IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY DETAILED OR ACCEPTED BY ARCHITECT OR ENGINEER.

7. FORM EXPOSED CORNERS OF COLUMNS, BEAMS, WALLS, ETC... WITH 3/4 INCH CHAMFERS UNLESS OTHERWISE DETAILED.

8. PROVIDE KEYS IN CONSTRUCTION JOINTS UNLESS OTHERWISE DETAILED.

9. ROUGHED CONCRETE SURFACE TO FULL AMPLITUDE OF 1/16 INCH WHERE MASONRY WALLS INTERSECT CONCRETE.

A. GENERAL

1. ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE CALIFORNIA BUILDING CODE 2019 EDITION (CBC 2019), REFERENCED STANDARDS OF CHAPTER 35 AND ALL APPLICABLE CODES AND ORDINANCES.

   BASIS OF DESIGN:
   a. SEISMIC LOADS
      + IMPORTANCE FACTOR, Ie = 1.0
      + Ss = 2.356g
      + S1 = 0.911g
      + SITE CLASS: C
      + Sds = 1.885g
      + Sd1 = 0.850g
      + Rho = 1.3 (REDUNDANCY FACTOR)
      + SEISMIC DESIGN CATEGORY: E
      + BASIC SEISMIC-FORCE-RESISTING SYSTEM: SHEATHED SHEAR BEARING WALLS
      + SEISMIC RESPONSE COEFFICIENT, Cs = 0.290 (STRENGTH) = 0.207 (SERVICE)
      + RESPONSE MODIFICATION FACTOR, R = 6.5
      + ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE
   b. WIND LOAD
      + BASIC WIND SPEED = 110 MPH (ULTIMATE)
      + EXPOSURE C
      + IMPORTANCE FACTOR, Iw=1.0
      + INTERNAL PRESSURE COEFFICIENT = 0.18
      + DESIGN WIND PRESSURE = 27.5 PSF
      + COMPONENTS AND CLADDING WIND PRESSURE = 39 PSF
   c. LIVE LOADS
      + ROOF = 20 PSF
      + FLOOR = 40 PSF
      + DECK/BALCONY = 60 PSF
   d. DEAD LOADS
      + ROOF = 18 PSF
      + FLOOR = 18 PSF

2. AISC – SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDING.

3. ACI-318 – BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.

4. ALL ASTM SPECIFICATIONS NOTED ON THESE DRAWINGS SHALL BE OF THE LATEST REVISION.

5. WRITTEN INFORMATION AND DIMENSIONS SHALL TAKE PRECEDENCE OVER GRAPHIC INFORMATION. DO NOT SCALE DRAWINGS.

6. ALL DIMENSIONS ARE TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, ELEVATIONS, SECTIONS, AND DETAILS.

7. ANY DISCREPANCIES ON THE PLANS OR ANY DEVIATIONS FROM THE PLANS WHICH ARE NECESSITATED BY FIELD CONDITIONS OR ANY CONDITION DIFFERENT FROM THOSE INDICATES ON THE PLANS, SHALL BE CALLED TO THE ATTENTION OF THANG LE, S.E. PRIOR TO CONTINUING CONSTRUCTION. ALL WORK IS TO BE COORDINATED SO THAT COOPERATION BETWEEN THE TRADES WHERE REQUIRED, IS ACCOMPLISHED.

8. SEE ARCHITECTURAL DRAWING FOR KINDS OF FLOOR FINISH, DEPRESSION IN SLAB, OPENINGS IN WALLS AND ROOF REQUIRED BY DOOR, WINDOWS, DUCTS, VENTS, HATCHES, PLUMBING, ETC...; ALL TYPE OF FLASHING, INSERTS, ANCHORS, HANGERS, ETC... EMBEDDED OR ATTACHED TO CONCRETE STRUCTURE; PAVING, WALKS, STAIRS, RAMPS, CURBS, PARAPETS, TERRACES, ETC...; EXTERIOR GRADES; ROOF SLABS, CRICKETS AND DRAINS.

9. THE CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS AS TO LAYOUT DIMENSIONS AND ELEVATIONS. ALL DISCREPANCIES SHALL BE REPORTED TO THANG LE, S.E. AND THE OWNER FOR PROPER ADJUSTMENT BEFORE PROCEEDING WITH THE WORK.

10. IN THE EVENT THAT CERTAIN FEATURES OF THE CONSTRUCTION ARE NOT FULLY SHOWN ON THE DRAWINGS OR CALLED FOR IN THE THE GENERAL NOTES, THEN THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS FOR CONDITIONS THAT ARE SHOWN OR CALLED FOR.

11. THE BUILDER SHALL TAKE FULL AND FINAL RESPONSIBILITY FOR CONSTRUCTING A FINAL PRODUCT OF APPROPRIATE QUALITY AND SERVICEABILITY CONSISTENT WITH THE INFORMATION AND REQUIREMENTS CONTAINED IN THE CONSTRUCTION DOCUMENTS OR REASONABLY INFERABLE THEREFROM, AND/OR CONTAINED IN THE REQUIREMENTS OF ANY GOVERNMENTAL ENTITY WITH JURISDICTION OVER THE PROJECT.

12. THE BUILDER SHALL TAKE FULL RESPONSIBILITY FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES INCLUDING WITHOUT LIMITATION DEMOLITION, EXCAVATION AND ERECTION PROCEDURES.

13. STRUCTURAL OBSERVATION VISITS TO SITE BY REPRESENTATIVES OF THANG LE, S.E. DO NOT INCLUDE INSPECTIONS OF CONSTRUCTION MEANS AND METHODS. OBSERVATIONS PERFORMED BY ENGINEER DURING CONSTRUCTION ARE NOT CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE PERFORMED BY OTHERS. OBSERVATIONS PERFORMED BY ENGINEER ARE PERFORMED SOLELY FOR THE PURPOSE OF DETERMINING IF THE CONTRACTOR UNDERSTAND DESIGN INTENT CONVEYED IN CONTRACT DOCUMENTS. OBSERVATIONS DO NOT GUARANTEE CONTRACTOR'S PERFORMANCE AND ARE NOT TO BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.

14. MODIFICATIONS OR SUBSTITUTIONS: DESIGN, MATERIALS, EQUIPMENT AND PRODUCTS OTHER THAN THOSE INDICATED OR SPECIFIED MAY BE CONSIDERED FOR USE PROVIDED A WRITTEN REQUEST, SUBJECT TO REVIEW, IS SUBMITTED TO OWNER, ARCHITECT, ENGINEER AND GOVERNING CODE AUTHORITY PRIOR TO ITS USE OR INCLUSION ON ANY SHOP DRAWING.

15. BRACE PIPING AND DUCTS COMPLYING WITH LATEST ADDITION OF GUIDELINES FOR SEISMIC RESTRAINTS OF MECHANICAL SYSTEMS BY THE SHEET METAL AND AIR CONDITIONING CONTRACTORS NATIONAL ASSOCIATION.

16. INSTALL AND ANCHOR MECHANICAL AND ELECTRICAL EQUIPMENT TO STRUCTURE COMPLYING ASCE/SEI 7-05, CHAPTER 13, AS MODIFIED BY CBC 1614.1.11 THROUGH 1614.1.16. ISOLATORS, FASTENERS AND ANY OTHER ELEMENT PROVIDING STABILITY FOR EQUIPMENT SHALL BE APPROVED BY ICC-ES OR EQUIVALENT TESTING PROCEDURE. PROVIDE SUSPENDED EQUIPMENT WITH APPROVED LATERAL OR SWAY BRACING.

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DATE ISSUED 10-15-2022 | DRAWN BY ML | CHECKED BY TL

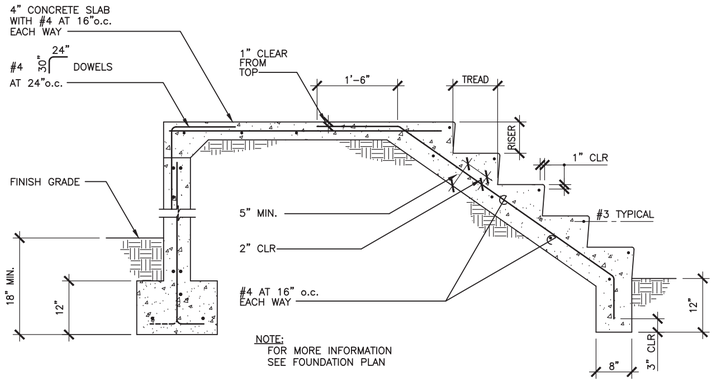
THANG LE & ASSOCIATES
STRUCTURAL ENGINEERS, INC.
319 E. FOOTHILL BLVD., SUITE C
ARCADIA, CALIFORNIA 91006
PHONE: (626) 731-1539

REGISTERED PROFESSIONAL ENGINEER
THANG H. LE
S 4978
EXP 06/30/24
STRUCTURAL
STATE OF CALIFORNIA
10.15.2022

LARSON BARN
10818 CROTHERS ROAD
SAN JOSE, CALIFORNIA 95127

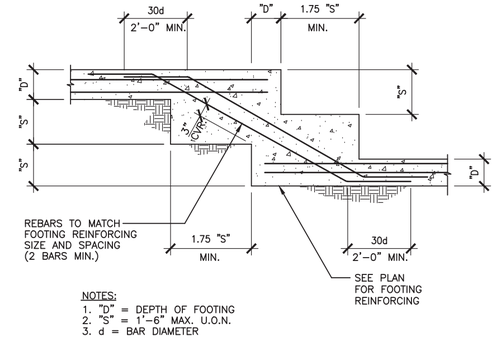
GENERAL NOTES

S1.1



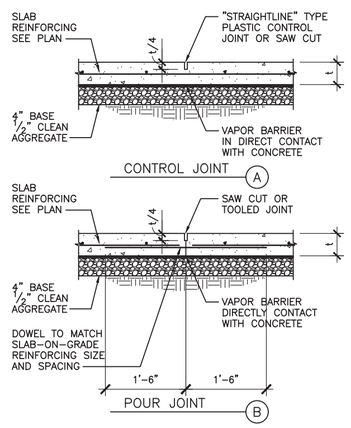
TYPICAL STAIR ON GRADE

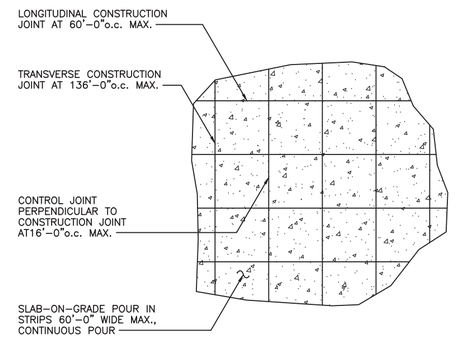
3/4"=1' — 6



TYPICAL STEPPED FOOTING

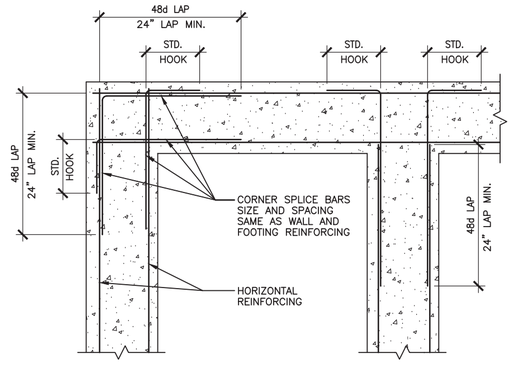
3/4"=1' — 3





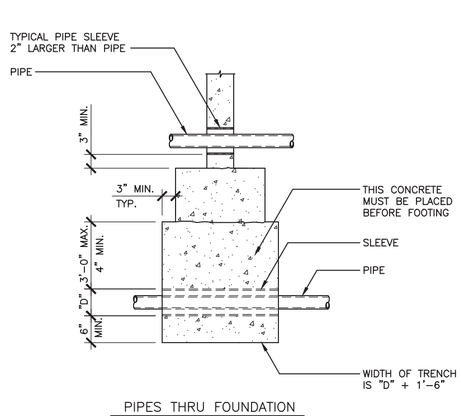
TYPICAL SLAB-ON-GRADE DETAIL

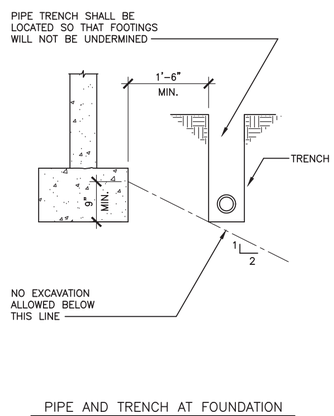
3/4"=1' — 5



TYPICAL FOOTING/WALL CORNER REINFORCING

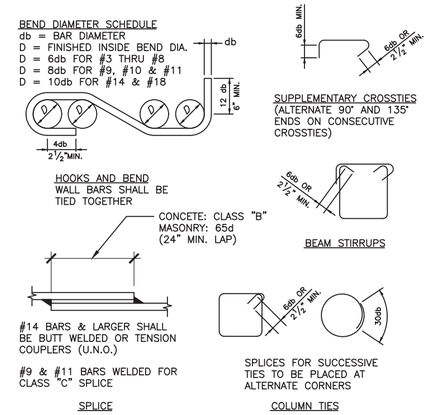
3/4"=1' — 2





TYPICAL PIPE AT FOOTING

3/4"=1' — 4



TYPICAL REBAR BENT

3/4"=1' — 1

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STRUCTURAL ENGINEERS, INC.
319 E. FOOTHILL BLVD., SUITE C
ARCADIA, CALIFORNIA 91006
PHONE: (626) 731-1539

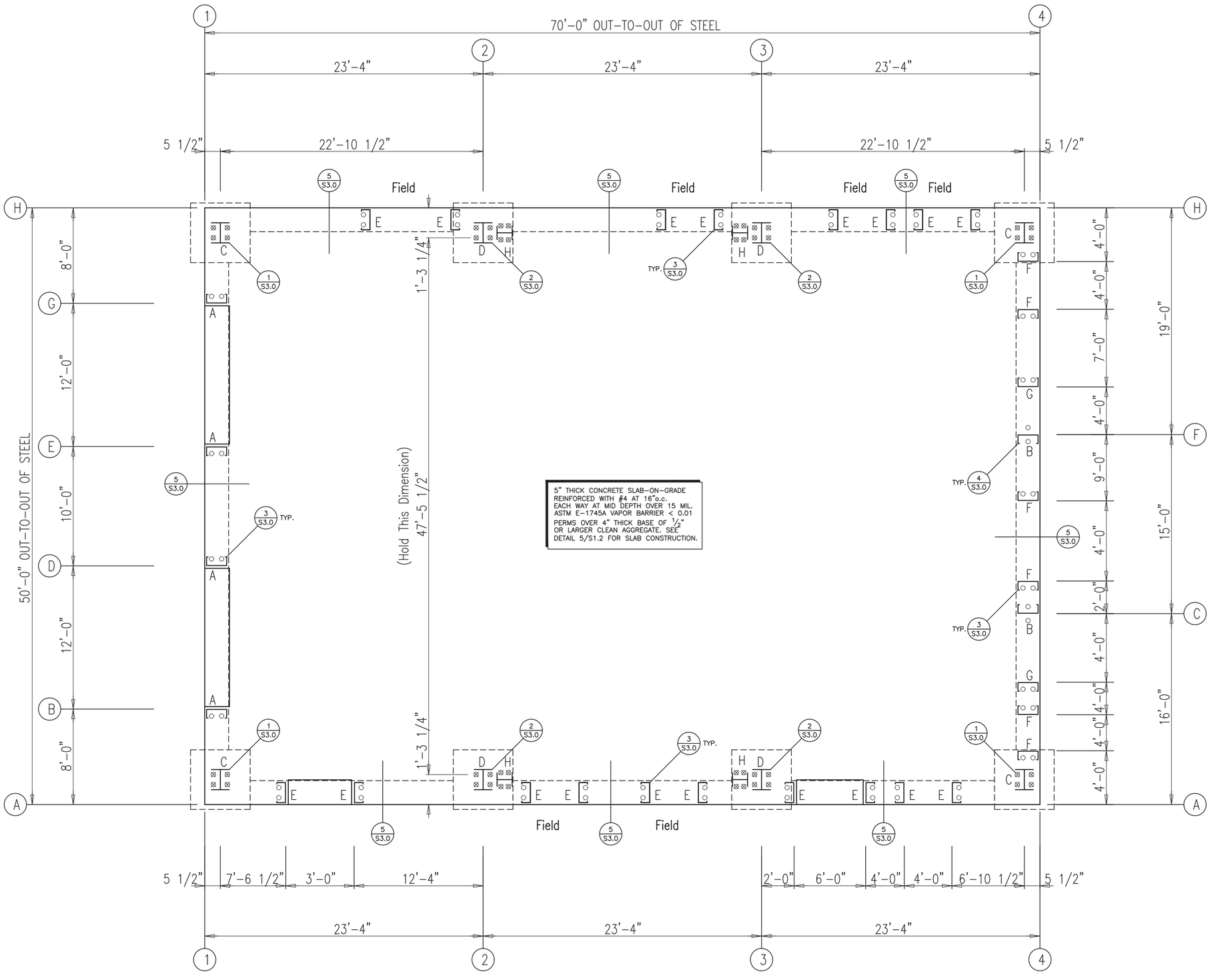
REGISTERED PROFESSIONAL ENGINEER — THANG H. LE — S 4978 EXP 06/30/24 — STRUCTURAL — STATE OF CALIFORNIA

10.15.2022

LARSON BARN
10818 CROTHERS ROAD
SAN JOSE, CALIFORNIA 95127

TYPICAL CONCRETE DETAILS

S1.2



FOUNDATION PLAN

SCALE: 1/4"=1'-0"

NOTES:

1. ALL COLUMN FOOTINGS ARE CENTERED ON COLUMNS AND COLUMNS ARE CENTERED ON GRIDS U.O.N. WALL FOOTINGS ARE CENTERED BENEATH WALLS U.O.N.
2. SEE SHEET S1.1 FOR GENERAL STRUCTURAL NOTES.
3. SEE SHEET S1.2 FOR TYPICAL CONCRETE DETAILS.
4. VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL
5. VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL & ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.
6. FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE 2/S1.2
7. - - - DESIGNATES GRADE BEAM
   - - - SEE PLAN FOR REINFORCEMENT
8. [ ] DESIGNATES NEW PAD FOOTING, SEE PLAN FOR REINFORCEMENT 4/S1.2
9. IF ADVERSE SOIL CONDITIONS ARE ENCOUNTERED, A SOILS INVESTIGATION REPORT MAY BE REQUIRED.

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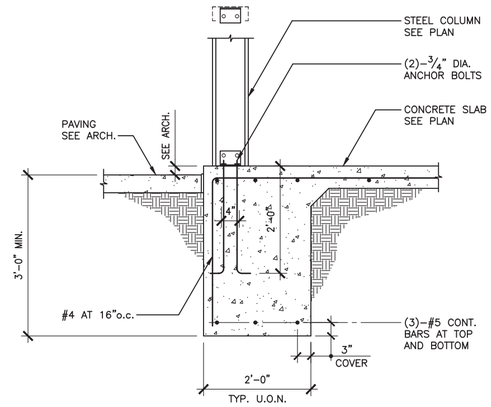
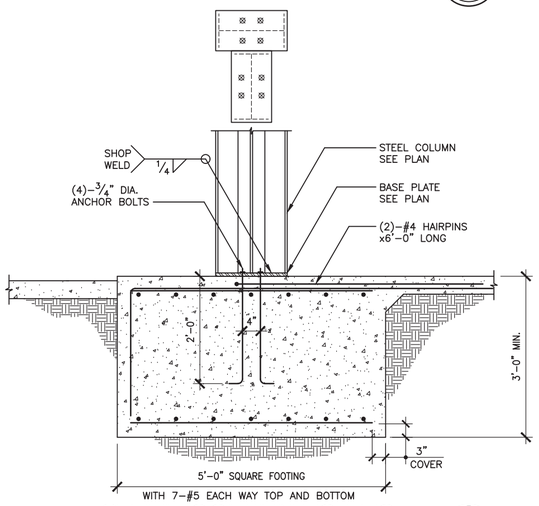
FOUNDATION PLAN

S2.0



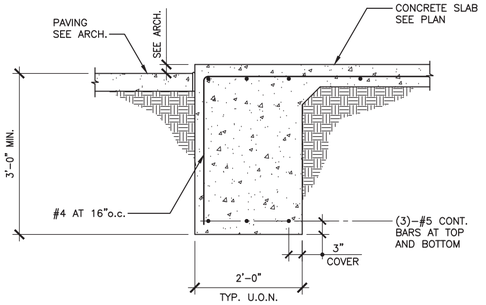
THICKENED SLAB EDGE 3

3/4"=1"



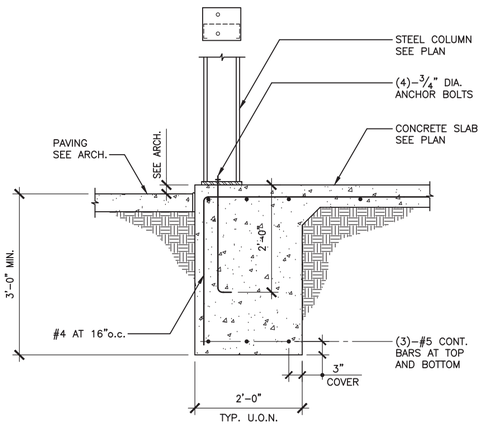
STEEL COLUMN TO FTG 2

3/4"=1"



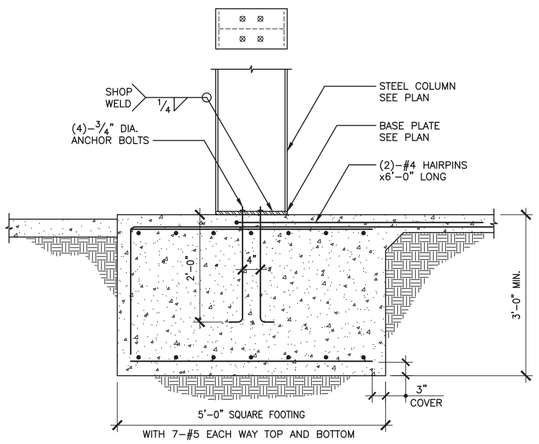
THICKENED SLAB EDGE 5

3/4"=1"



THICKENED SLAB EDGE 4

3/4"=1"



STEEL COLUMN TO FTG 1

3/4"=1"

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10.15.2022

LARSON BARN

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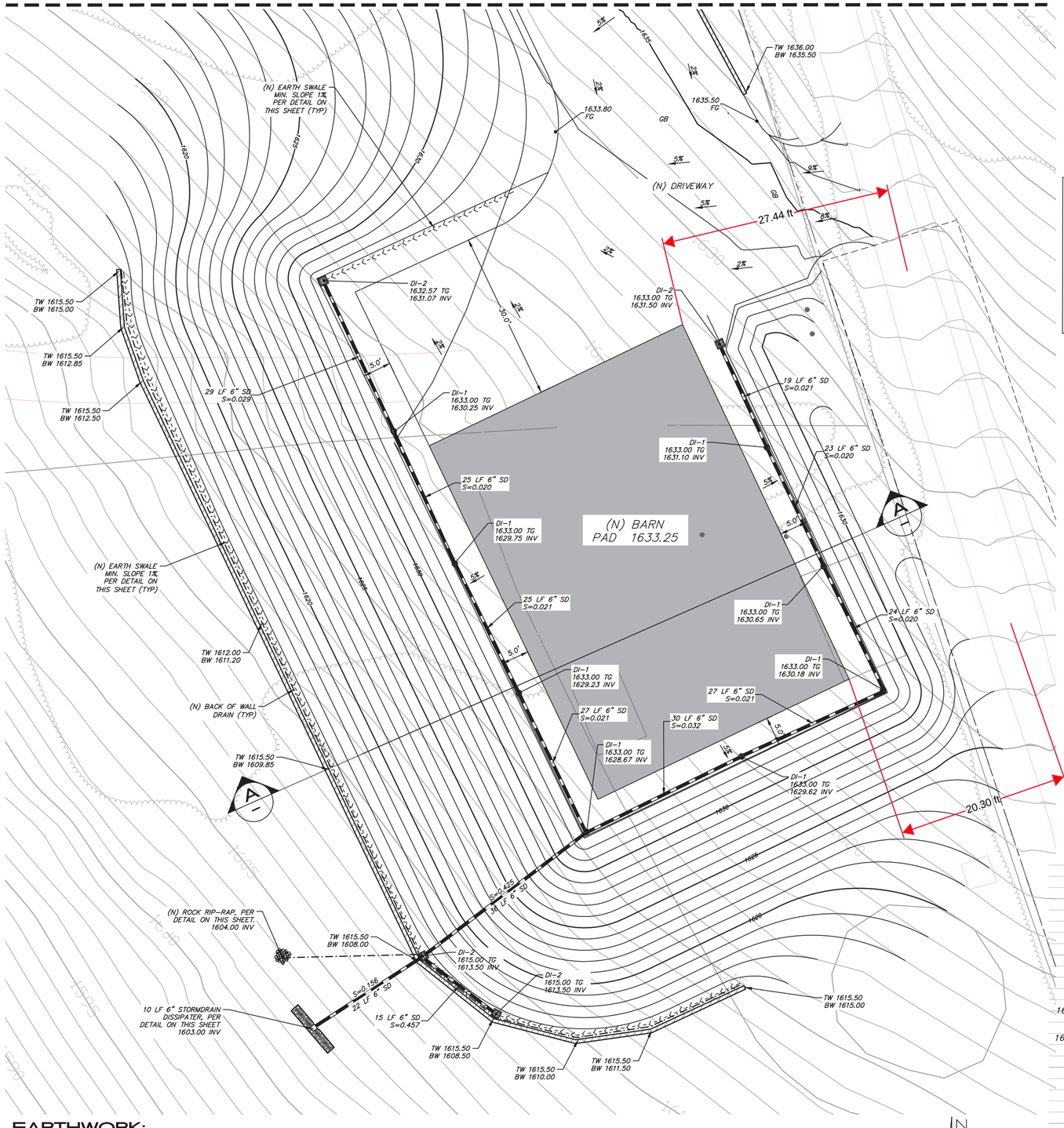
FOUNDATION DETAILS

S3.0

MATCH LINE SEE SHEET C7

MATCH LINE SEE SHEET C7



EARTHWORK:

	CUT	FILL	NET
BARN	10 CY	5520 CY	5510 CY FILL
TOTAL	10 CY	5520 CY	5510 CY FILL <IMPORT>

NOTE: NO DEDUCTION FOR PAD/CONCRETE WAS ASSUMED.

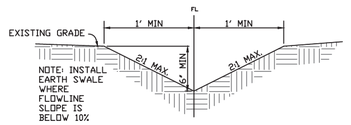
BARN ROUGH GRADING PLAN

SCALE: 1" = 10'

0 10 20 30

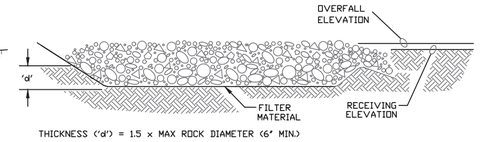
(GRAPHIC SCALE IN FEET)

N W E S



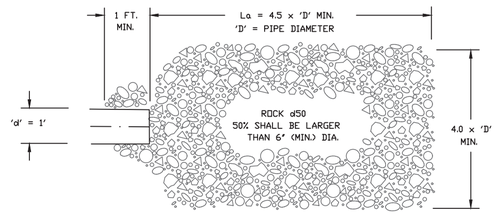
EARTH SWALE

NOT TO SCALE



THICKNESS ('d') = 1.5 x MAX ROCK DIAMETER (6" MIN.)

SECTION



PLAN

DRAINAGE SCHEDULE

DI-1: ◉	6" ROUND ATRIUM GRATE (OR APPROVED EQUAL)
DI-2: ▣	12"x12" OLDCASTLE PRECAST CONCRETE INLET (OR APPROVED EQUAL)
TD-1:	TRAFFIC RATTED TRENCH DRAIN (OR APPROVED EQUAL)

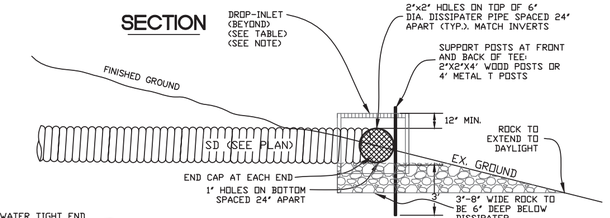
ALL 6" STORM DRAIN PIPE TO BE HDPE DUAL WALL.
ALL 4" STORM DRAIN AND ROOF LEADER DRAIN PIPE TO BE PVC SCHEDULE 40.

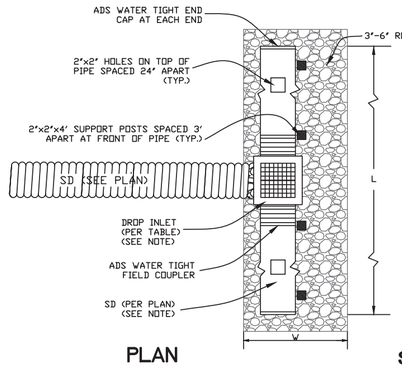
NOTES:

1. 'La' = LENGTH OF APRON. DISTANCE 'La' SHALL BE OF SUFFICIENT LENGTH TO DISSIPATE ENERGY.
2. APRON SHALL BE SET AT A ZERO GRADE AND ALIGNED STRAIGHT.
3. FILTER MATERIAL SHALL BE FILTER FABRIC OR 6' THICK (MIN.) GRADED GRAVEL LAYER.

ROCK OUTLET FOR BACK OF WALL DRAIN

NOT TO SCALE





PLAN

NOTE:
STORM DRAIN DISSIPATER SHALL BE SET AT ZERO GRADE (COMPLETELY FLAT) PARALLEL TO CONTOURS BY USE OF SURVEY STAKES

IF LENGTH > 20', A CLEANOUT SHALL BE INSTALLED ON EACH END

IF GROUND SLOPE ≥ 4H:1V, INLET SHALL BE PRECAST CONCRETE INLET PER TABLE DIMENSIONS

IF GROUND SLOPE ≤ 4H:1V AND SD 8" OR LESS, INLET SHALL BE NDS ROUND INLET (MATCHING DISSIPATER SIZE) WITH NDS ROUND GRATE

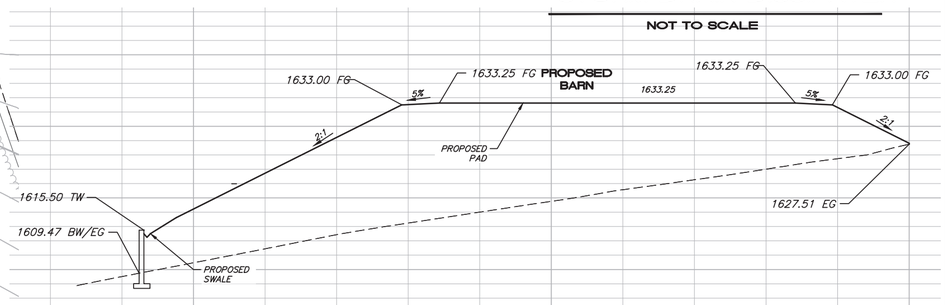
PRECAST CONCRETE INLET SIZING TABLE

SD PIPE SIZE (IN.)	INLET SIZE (IN.)	OLD CASTLE PRECAST MODEL NO.*
4" – 10"	12" X 12"	CB1212
12" – 15"	18" X 18"	CB1818
15" – 18"	24" X 24"	CB2424
18" – 24"	30" X 30"	CB3030

*OR APPROVED EQUAL

STORM DRAIN DISSIPATER

NOT TO SCALE



PROFILE A-A'

1" = 10' HORIZONTAL
1" = 10' VERTICAL

REVISION	DESCRIPTION	BY	DATE

DVC GROUP INC.
PLANNING • ENGINEERING • C.M.
513 CENTER STREET
HEALDSBURG, CA 95448
(707) 395-0988

REGISTERED PROFESSIONAL ENGINEER • DANIEL JOHN HUGHES • No. 60225 • 6-30-22 • STATE OF CALIFORNIA • CIVIL

DANIEL JOHN HUGHES DATE
RCE 60225

10818 CROTHERS ROAD
BARN ROUGH GRADING PLAN

612-44-033
10818 CROTHERS ROAD
SAN JOSE CA

NOVEMBER 28, 2022

JOB NO. 06-21

SHEET NO. C8

OF 8 SHEETS