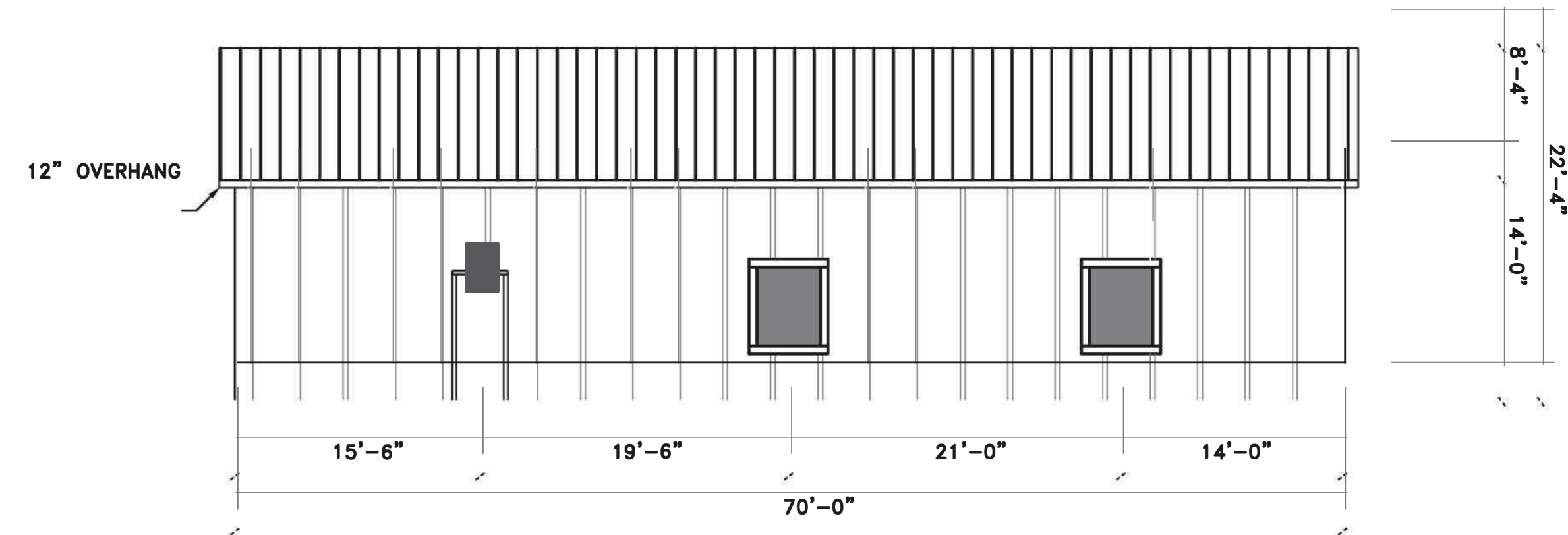
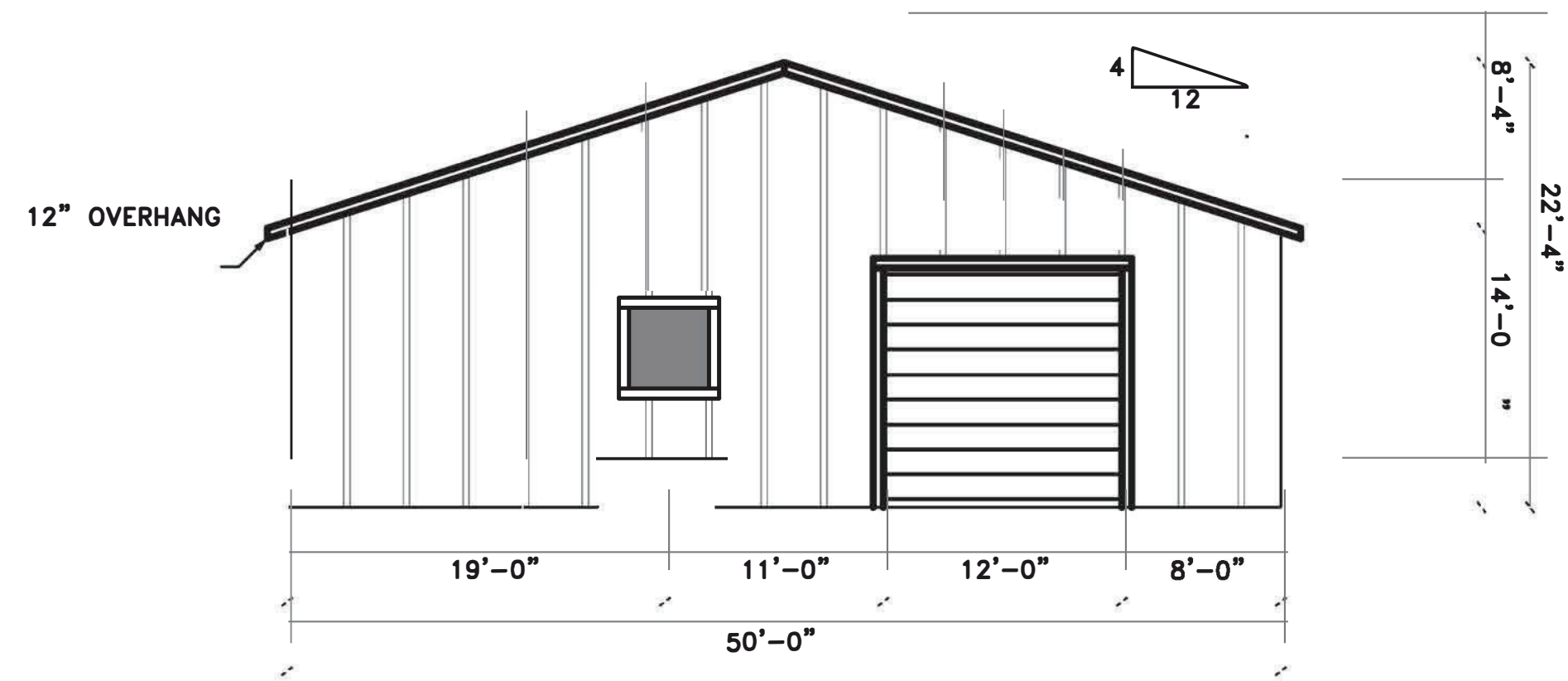


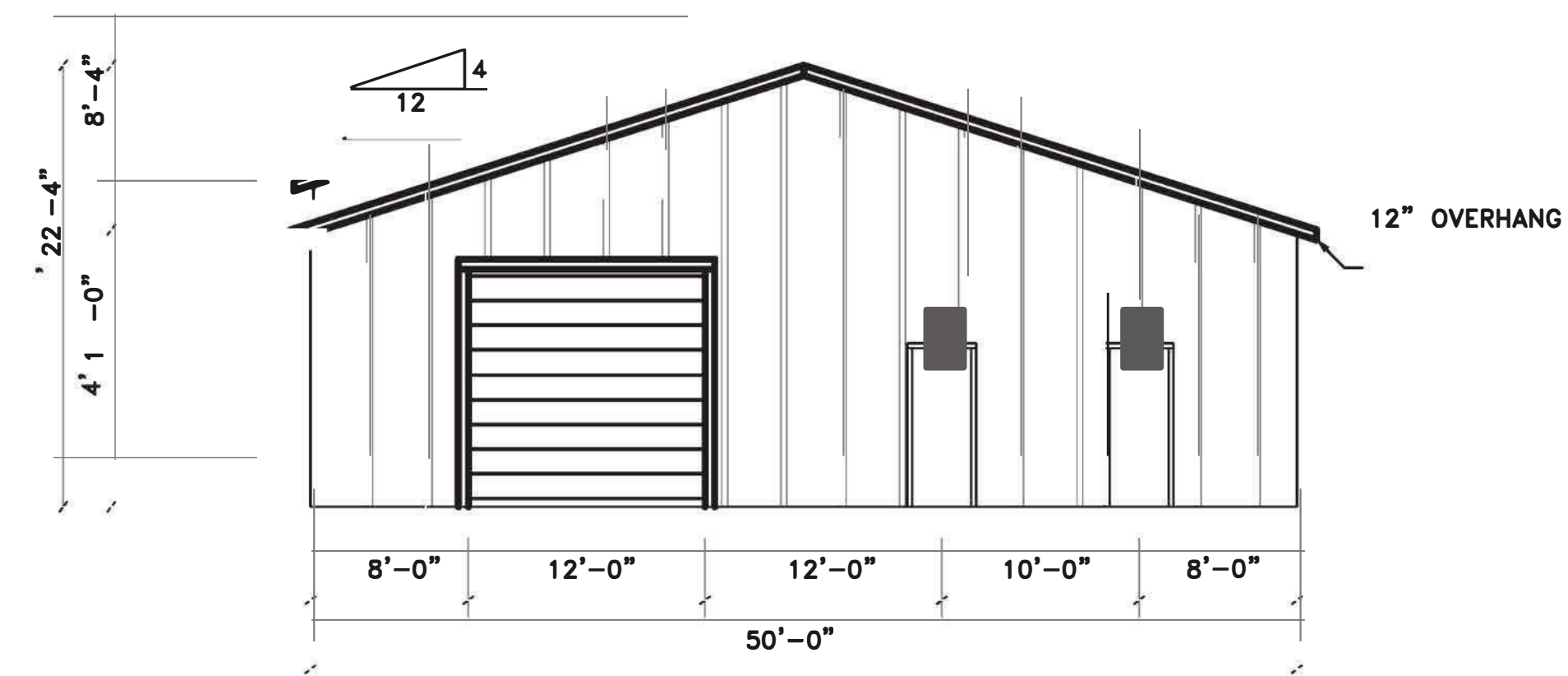
GARAGE-WEST ELEVATION  
SCALE 1/8" = 1'



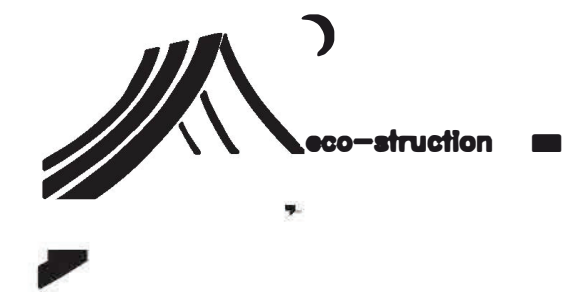
GARAGE-EAST ELEVATION  
SCALE 1/8" = 1'



GARAGE-SOUTH ELEVATION  
SCALE 1/8" = 1'



GARAGE-NORTH ELEVATION  
SCALE 1/8" = 1'



DATE	REVISION	BY

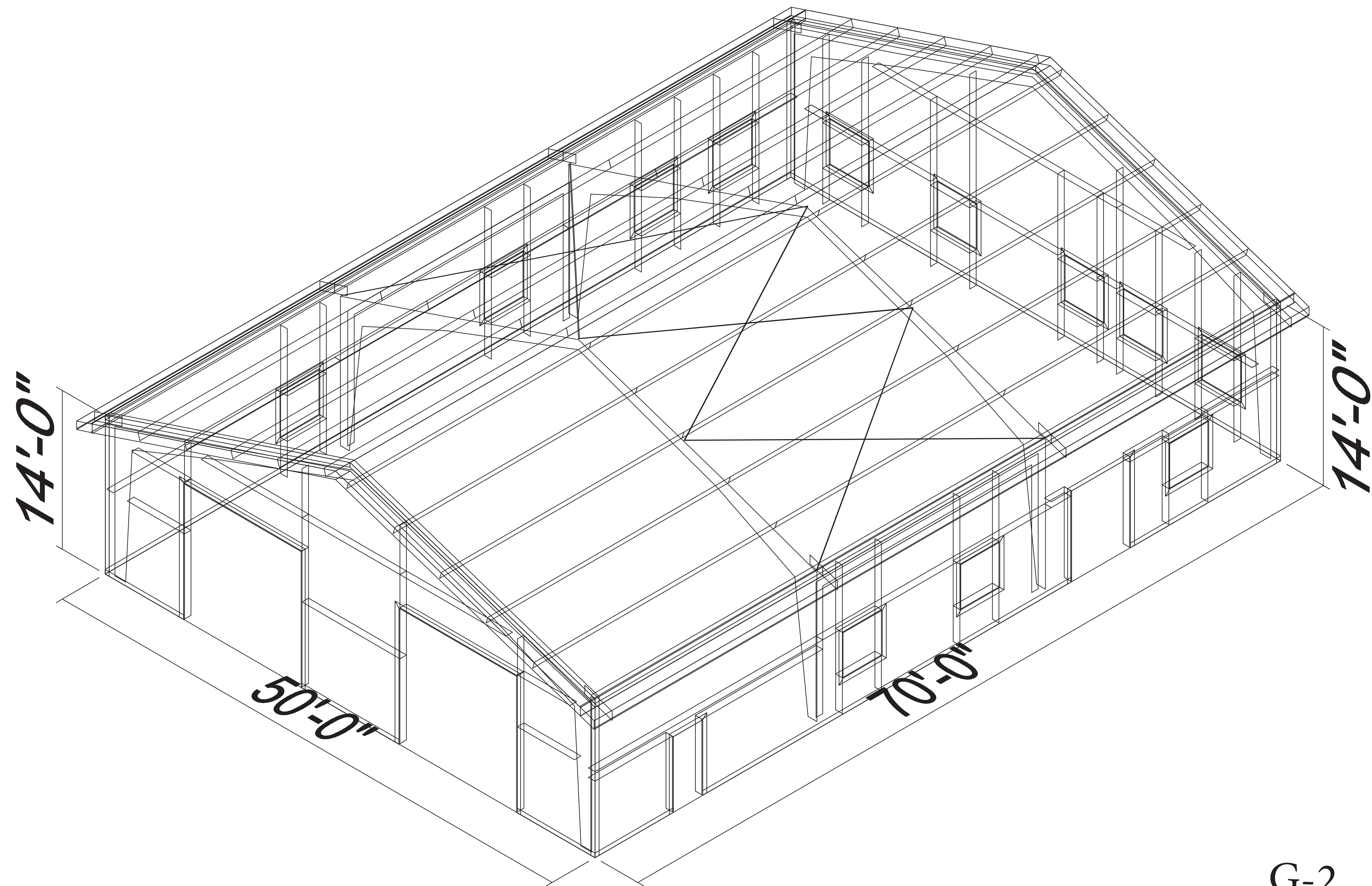


LARSON RESIDENCE  
10818 Crothers Road  
San Jose, CA 95127

GARAGE  
ELEVATIONS

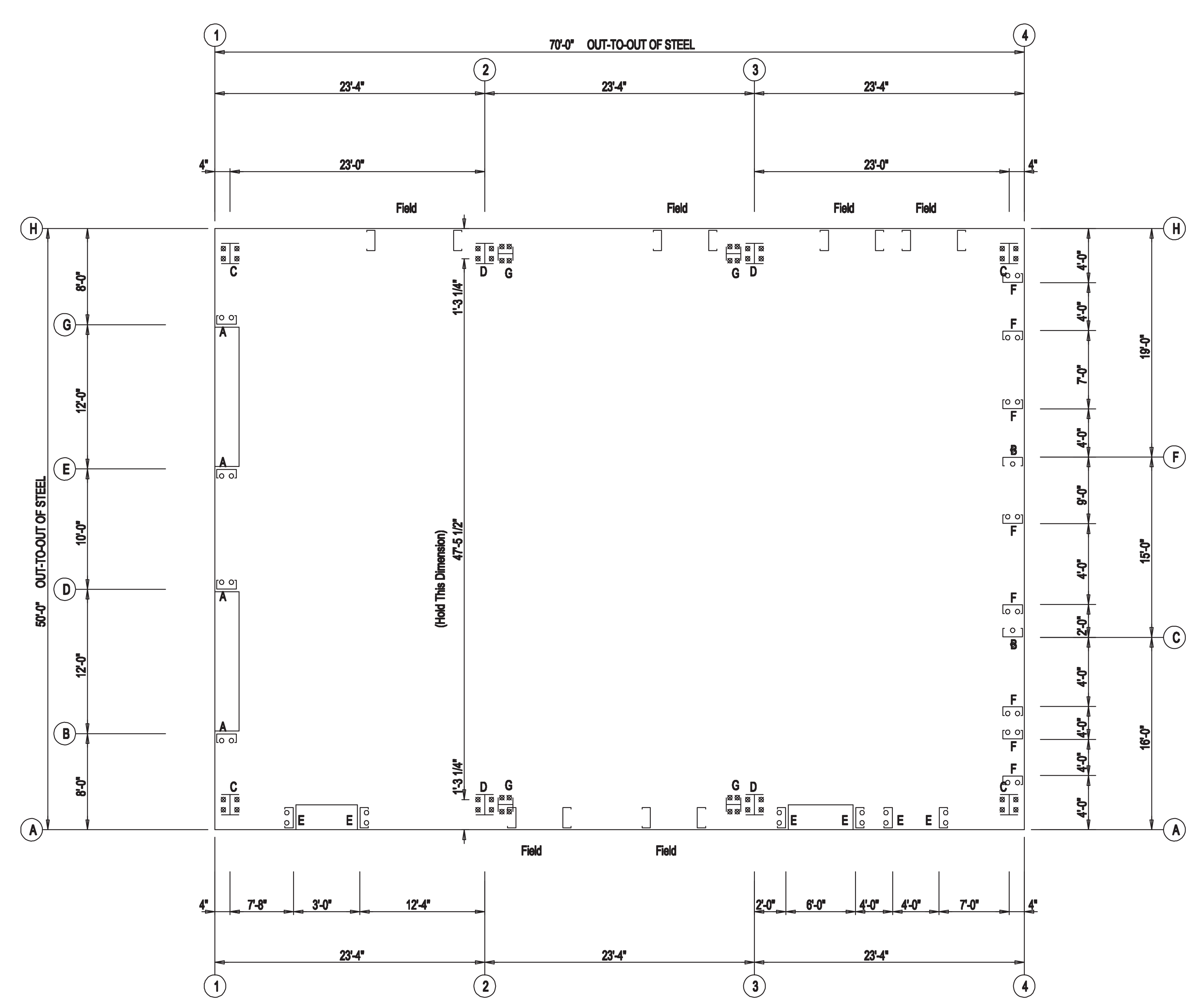
DRAWN: NN  
CHECKED: MS  
DATE: 1-6-23  
SCALE: 1/8"=1'

SHEET  
G1



G-2

○ Dia= 5/8"  
 ⊗ Dia= 3/4"



ANCHOR BOLT PLAN

NOTE: ALL BASE PLATES @ 100.0' (J.N.)  
 ASSUMED FINISH FLOOR @ 100.0' (J.N.)

ISSUE	DATE	DESCRIPTION	BY	CHKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE**  
 BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
 NORTH LITTLE ROCK, AR 72116-7606  
 1-800-643-5555

PROJECT:		CUSTOMER:		OWNER:			
LOCATION:							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		F1	0

A-3

**GENERAL NOTES**

1) THE REACTIONS PROVIDED ARE BASED ON THE ORDER DOCUMENTS AT THE TIME OF MAILING. ANY CHANGES TO BUILDING LOADS OR DIMENSIONS MAY CHANGE THE REACTIONS. THE REACTIONS WILL BE SUPERSEDED AND VOIDED BY ANY FUTURE MAILING.

2) THE REACTIONS PROVIDED HAVE BEEN CREATED WITH THE FOLLOWING LAYOUT (UNLESS NOTED OTHERWISE)

A) A REACTION TABLE IS PROVIDED WITH REACTIONS FOR EACH LOAD GROUP

B) RIGID FRAMES

(1) SEE NOTE 3.

C) ENDWALLS

(1) SEE NOTE 3.

D) X-BRACING

1) X-BRACING REACTIONS ARE INCLUDED IN VALUES SHOWN IN THE REACTION TABLES AS NOTED IN THE BRACING REACTIONS TABLE.

2) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE SIDEWALL, INDIVIDUAL LONGITUDINAL SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR, OMEGA<sub>o</sub>.

3) FOR IBC AND UBC BASED BUILDING CODES, WHEN X-BRACING IS PRESENT IN THE ENDWALL, INDIVIDUAL TRANSVERSE SEISMIC LOADS DO NOT INCLUDE THE AMPLIFICATION FACTOR, OMEGA<sub>o</sub>.

E) THE METAL BUILDING MANUFACTURER IS RESPONSIBLE ONLY FOR THE PORTION OF THE ANCHOR ROD DESIGN PERTAINING TO THE TRANSFER OF FORCES BETWEEN THE BASE PLATE BEARING AND THE ANCHOR ROD'S SHEAR AND TENSION. THE METAL BUILDING MANUFACTURER IS NOT RESPONSIBLE FOR THE ANCHOR ROD EMBEDMENT FOR TRANSFER OF FORCES TO THE FOUNDATION. THE METAL BUILDING MANUFACTURER DOES NOT DESIGN AND IS NOT RESPONSIBLE FOR THE DESIGN, MATERIAL, AND CONSTRUCTION OF THE FOUNDATION EMBEDMENT. THE END USE CUSTOMER SHALL ASSURE THAT ADEQUATE PROVISIONS ARE MADE TO THE FOUNDATION DESIGN FOR LOADS IMPOSED BY COLUMN REACTIONS OF THE BUILDING, OTHER IMPOSED LOADS, AND BEARING CAPACITY OF THE SOIL AND OTHER CONDITIONS OF THE BUILDING SITE. IT IS RECOMMENDED THAT THE ANCHORAGE AND FOUNDATION OF THE BUILDING BE DESIGNED BY A REGISTERED PROFESSIONAL ENGINEER COMPETENT IN THE DESIGN OF SUCH STRUCTURES.

I) (REF. APPENDIX A3 OF THE MBMA METAL BUILDING BUILDING SYSTEMS MANUAL)

F) ANCHOR RODS ARE ASTM F1554 GR. 36 MATERIAL UNLESS NOTED OTHERWISE ON THE ANCHOR ROD LAYOUT DRAWING.

3) REACTIONS ARE PROVIDED AS UN-FACTORED FOR EACH LOAD GROUP APPLIED TO THE COLUMN. THE FACTORS APPLIED TO LOAD GROUPS FOR THE STEEL COLUMN DESIGN MAY BE DIFFERENT THAN THE FACTORS USED IN THE FOUNDATION DESIGN. THE FOUNDATION ENGINEER SHALL APPLY THE APPROPRIATE LOAD FACTORS AND COMBINE THE REACTIONS IN ACCORDANCE WITH THE BUILDING CODE AND DESIGN SPECIFICATIONS FOR PROPER FOUNDATION DESIGN.

A) FOR PROJECTS USING ULTIMATE DESIGN WIND SPEEDS SUCH AS 2012 IBC, 2015 IBC, OR FLORIDA BUILDING CODE, THE WIND LOAD REACTIONS ARE AT A STRENGTH VALUE WITH A LOAD FACTOR OF 1.0.

B) FOR IBC CODES, THE SEISMIC REACTIONS PROVIDED ARE AT A STRENGTH LEVEL WITH A LOAD FACTOR OF 1.0, AND DO NOT CONTAIN THE RHO FACTOR.

THE MANUFACTURER DOES NOT PROVIDE "MAXIMUM" LOAD COMBINATION REACTIONS. HOWEVER, THE INDIVIDUAL LOAD REACTIONS PROVIDED MAY BE USED BY THE FOUNDATION ENGINEER TO DETERMINE THE APPLICABLE LOAD COMBINATIONS FOR HIGHER DESIGN PROCEDURES AND ALLOW FOR AN ECONOMICAL FOUNDATION DESIGN.

**ENDWALL COLUMN:**

Frm Line	Col Line	Anc. Bolt		Base Plate (in)			Grout (in)
		Qty	Dia	Width	Length	Thick	
1	G	2	0.625	3.500	8.000	0.250	0.0
1	E	2	0.625	3.500	8.000	0.250	0.0
1	D	2	0.625	3.500	8.000	0.250	0.0
1	B	2	0.625	3.500	8.000	0.250	0.0
4	C	2	0.625	7.000	8.000	0.250	0.0
4	F	2	0.625	7.000	8.000	0.250	0.0

**NOTES FOR REACTIONS**

BUILDING REACTIONS ARE BASED ON THE FOLLOWING BUILDING DATA:

WIDTH (FT)	= 50
LENGTH (FT)	= 70
EAVE HEIGHT (FT)	= 14 / 14
ROOF SLOPE (rise/run)	= 4.0:12 / 4.0:12
DEAD LOAD (psf)	= 2.500
COLLATERAL LOAD (psf)	= 6
ROOF LIVE LOAD (psf)	= 20.00
FRAME LIVE LOAD (psf)	= 20
ROOF SNOW LOAD (psf)	= 0
GROUND SNOW LOAD (psf)	= 0.00
WIND SPEED (MPH)	= 82
WIND CODE	= CBC 19
EXPOSURE	= C
CLOSED/OPEN	= Closed
IMPORTANCE - WIND	= 1.00
IMPORTANCE - SEISMIC	= 1.00
SEISMIC ZONE	= D

**REACTION KEY:**

WIND Left/Right 1 = (with +Gcpl Internal Pressure)  
WIND Left/Right 2 = (with -Gcpl Internal Pressure)  
Wind Long 1 = Wind Load Case B at Left EW  
Wind Long 2 = Wind Load Case B at Right EW  
MIN. SNOW = Minimum Snow (Pm) per code  
EiRiUNB\_SL\_L = Endwall Unbalanced Snow Left  
EiRiUNB\_SL\_R = Endwall Unbalanced Snow Right  
FIRiUNB\_SL\_L = Rigid Frame Unbalanced Snow Left  
FIRiUNB\_SL\_R = Rigid Frame Unbalanced Snow Right

**ANCHOR BOLT SUMMARY**

Qty	Locate	Dia (in)	Type	Proj (in)
28	Jamb	5/8"	F1554	2.00
12	Endwall	5/8"	F1554	2.00
32	Frame	3/4"	F1554	2.50
16	WindCol	3/4"	F1554	2.50

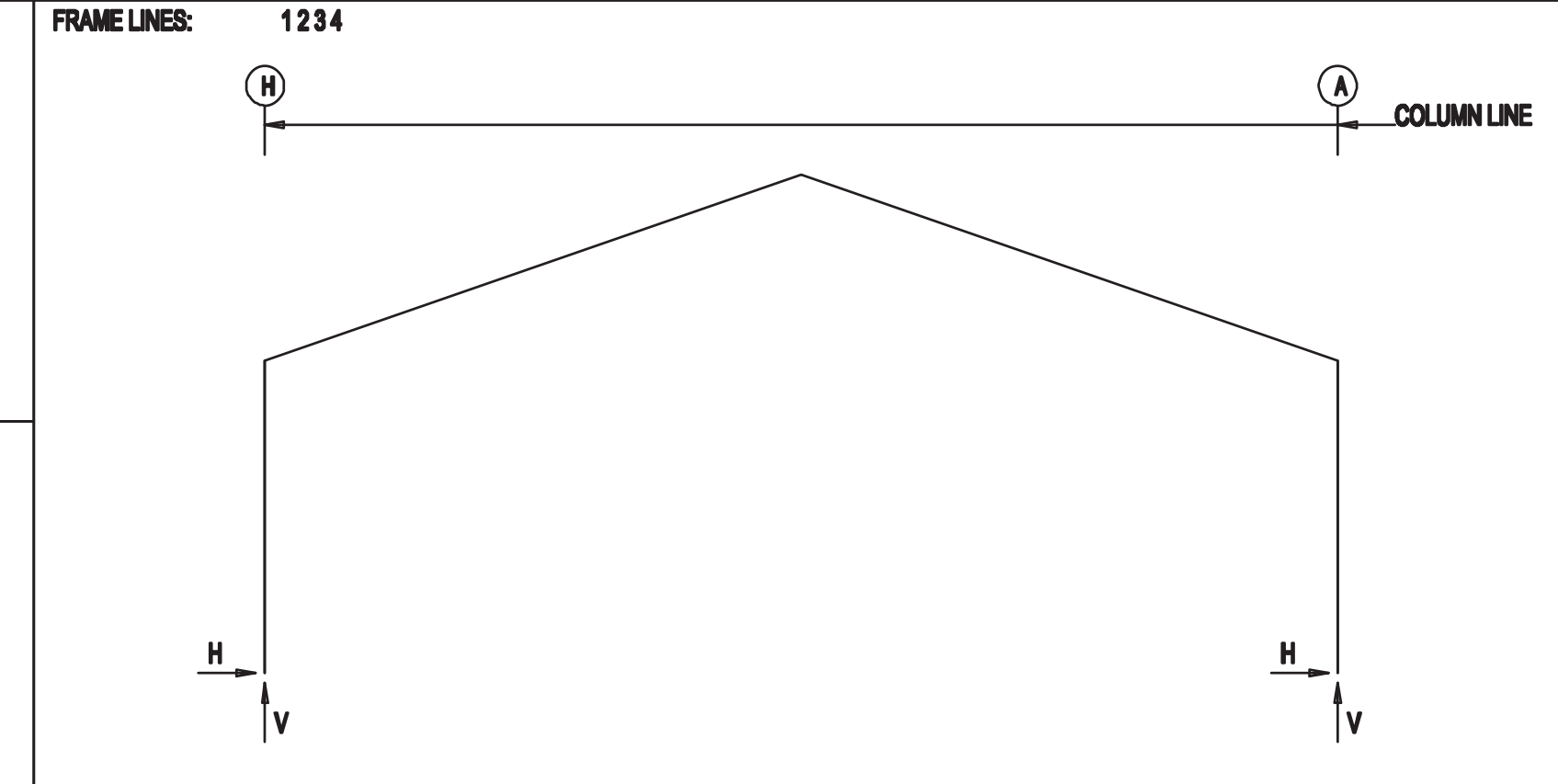
**BUILDING BRACING REACTIONS**

Wall Loc	Col Line	Reactions in plane of wall + Reactions (k)				Panel Shear (lb/ft)		Note
		Wind Horiz	Wind Vert	Seismic Horiz	Seismic Vert	Wind	Seis	
L_EW	1							(h)
F_SW	A	2.3						(e)
R_EW	4							(h)
B_SW	H	2.3						(e)

(a) Wind bent in bay  
(h) Rigid frame at endwall

**WIND BENT REACTIONS**

Wall Loc	Col Line	Wind(k)	Reactions		Seismic(k)		Base Plate(in)				
			Horz	Vert	Horz	Vert	Bolt Qty	Dia	Width	Length	Thick
F_SW	A	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375	
F_SW	A	3	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375
B_SW	H	3	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375
B_SW	H	2	1.9	2.3	3.2	3.7	4	0.750	6.000	22.500	0.375



**RIGID FRAME: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Dia	Base Plate (in)		Thick	Grout (in)
1	H	4	0.750	6.000	13.50	0.375	0.0
1	A	4	0.750	6.000	13.50	0.375	0.0

**RIGID FRAME: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Dia	Base Plate (in)		Thick	Grout (in)
2*	H	4	0.750	6.000	11.50	0.375	0.0
2*	A	4	0.750	6.000	11.50	0.375	0.0

2\* Frame lines: 2 3

**RIGID FRAME: ANCHOR BOLTS & BASE PLATES**

Frm Line	Col Line	Anc. Bolt Qty	Dia	Base Plate (in)		Thick	Grout (in)
4	H	4	0.750	6.000	13.50	0.375	0.0
4	A	4	0.750	6.000	13.50	0.375	0.0

**RIGID FRAME: BASIC COLUMN REACTIONS (k)**

Frame Line	Column Line	Dead		Collateral		Live		-Wind Left		-Wind Right		-Wind Left	
		Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert	Horz	Vert
1	H	0.6	1.5	1.1	2.1	3.3	7.1	-4.0	-6.6	-0.2	-4.5	-3.9	-4.7
1	A	-0.8	1.5	-1.1	2.1	-3.3	7.1	0.2	-4.5	4.0	-6.6	0.1	-2.5
4	H	0.6	1.5	1.1	2.1	3.3	7.1	-4.0	-6.6	-0.2	-4.5	-3.9	-4.7
4	A	-0.8	1.5	-1.1	2.1	-3.3	7.1	0.2	-4.5	4.0	-6.6	0.1	-2.5
2*	H	1.0	2.2	1.9	3.7	6.1	12.3	-6.0	-9.2	0.6	-6.0	-6.1	-5.9
2*	A	-1.0	2.2	-1.9	3.7	-6.1	12.3	0.6	-9.2	6.0	-9.2	-0.5	-2.6

**ENDWALL COLUMN: BASIC COLUMN REACTIONS (k)**

Frm Line	Col Line	Dead Vert	Wind Press Horiz	Wind Suct Horiz
1	E	0.1	-1.7	1.8
1	D	0.1	-1.7	1.8
1	B	0.1	-1.2	1.2
4	C	0.1	-2.2	2.3
4	F	0.1	-2.5	2.7

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE BUILDING SYSTEMS**

2513 MCCAIN BLVD. STE 2 #385  
NORTH LITTLE ROCK, AR 72116-7606  
1-800-643-5555

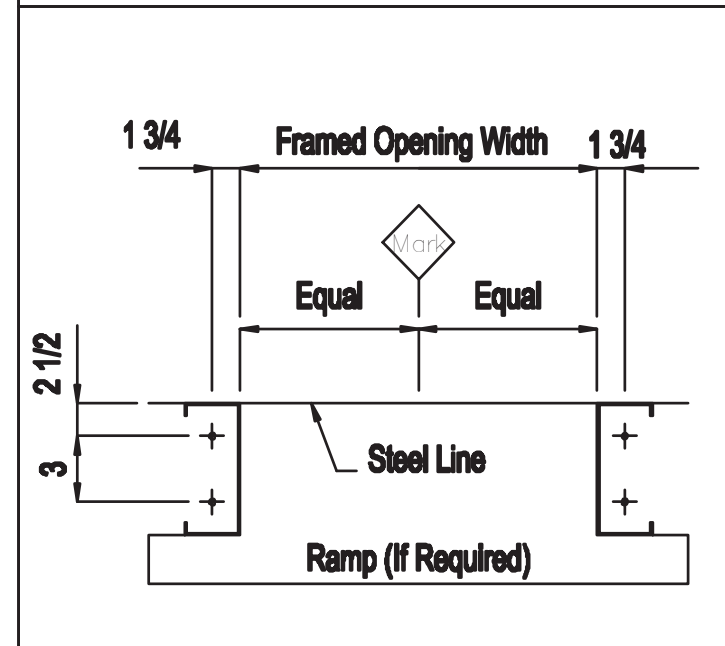
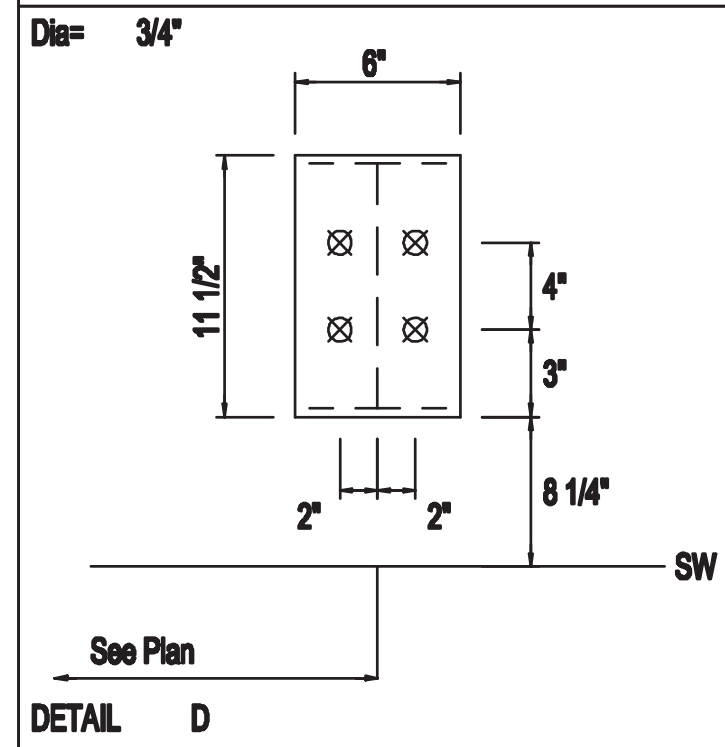
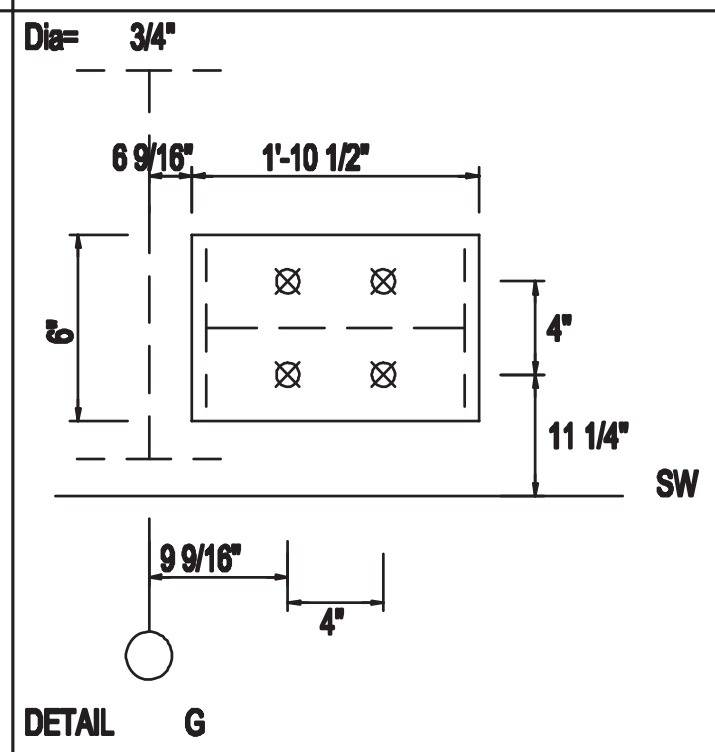
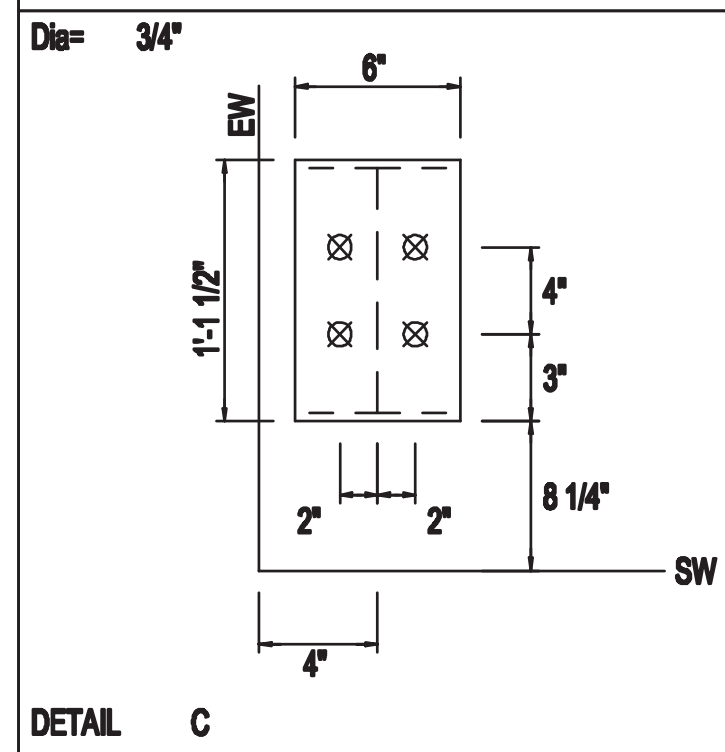
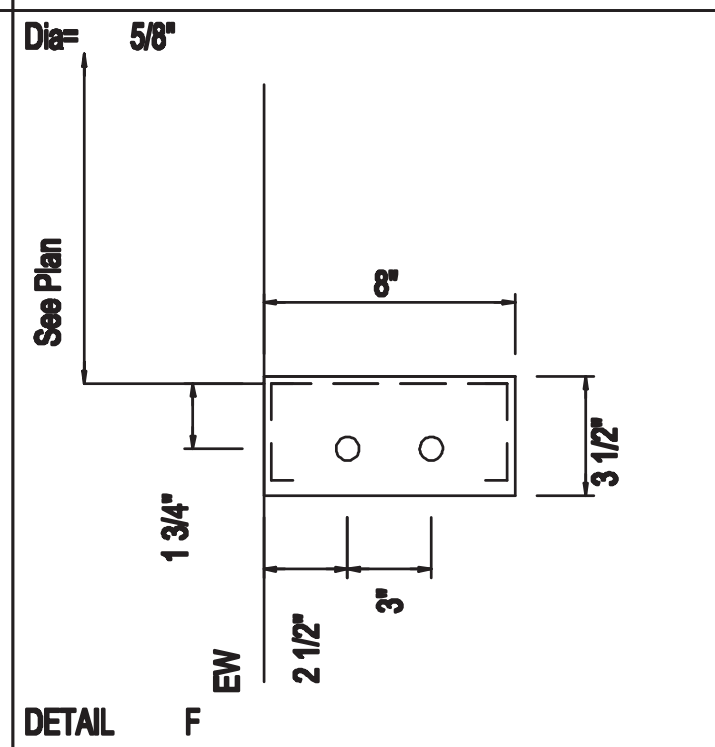
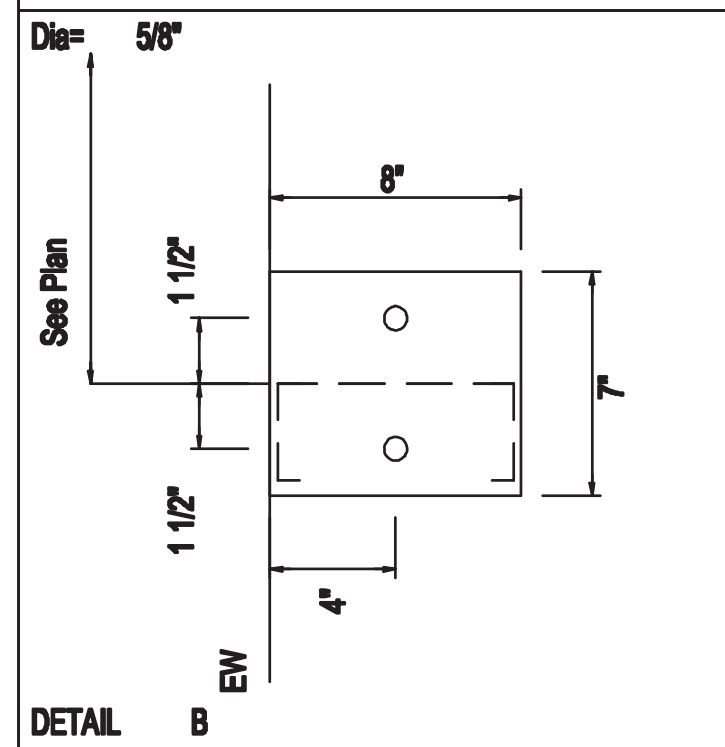
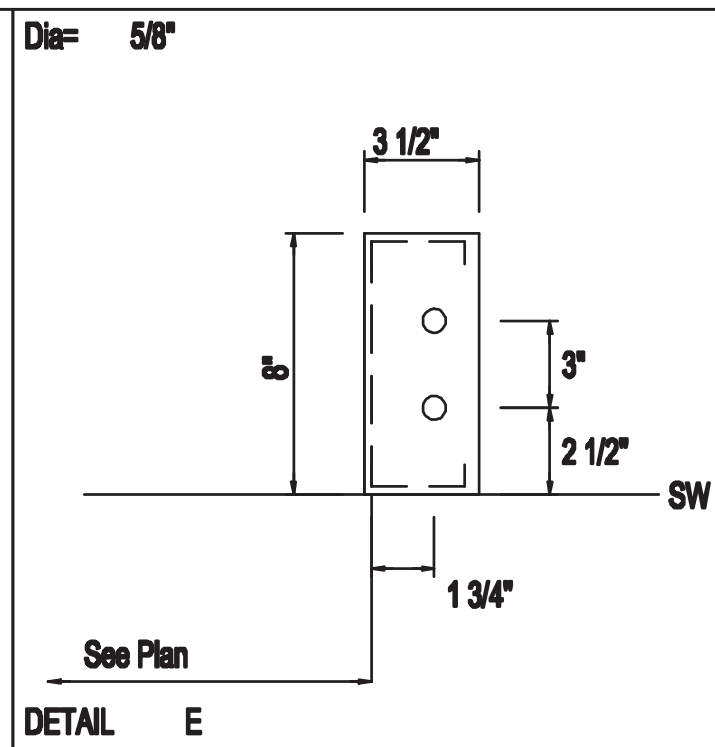
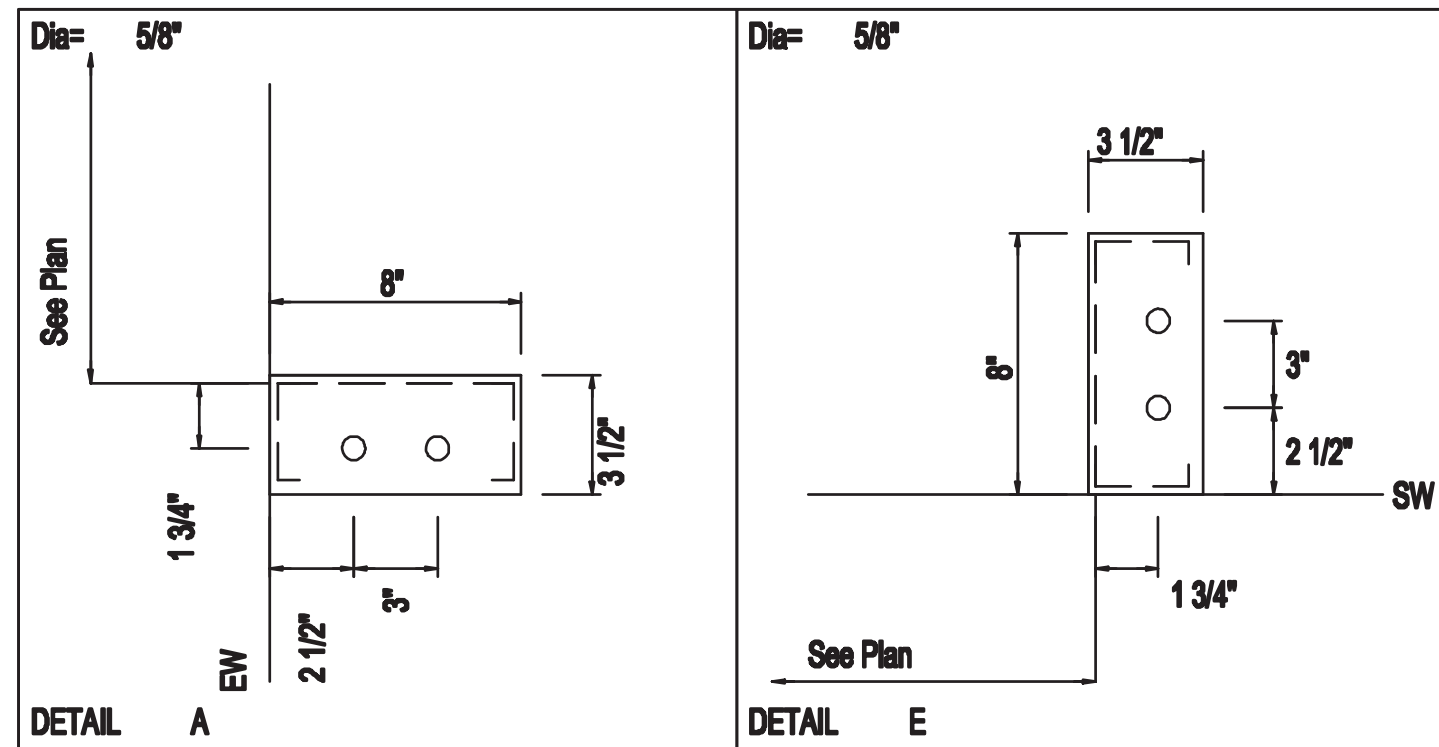
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CUSTOMER: \_\_\_\_\_ OWNER: \_\_\_\_\_

LOCATION: \_\_\_\_\_

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		F2	0

S-1



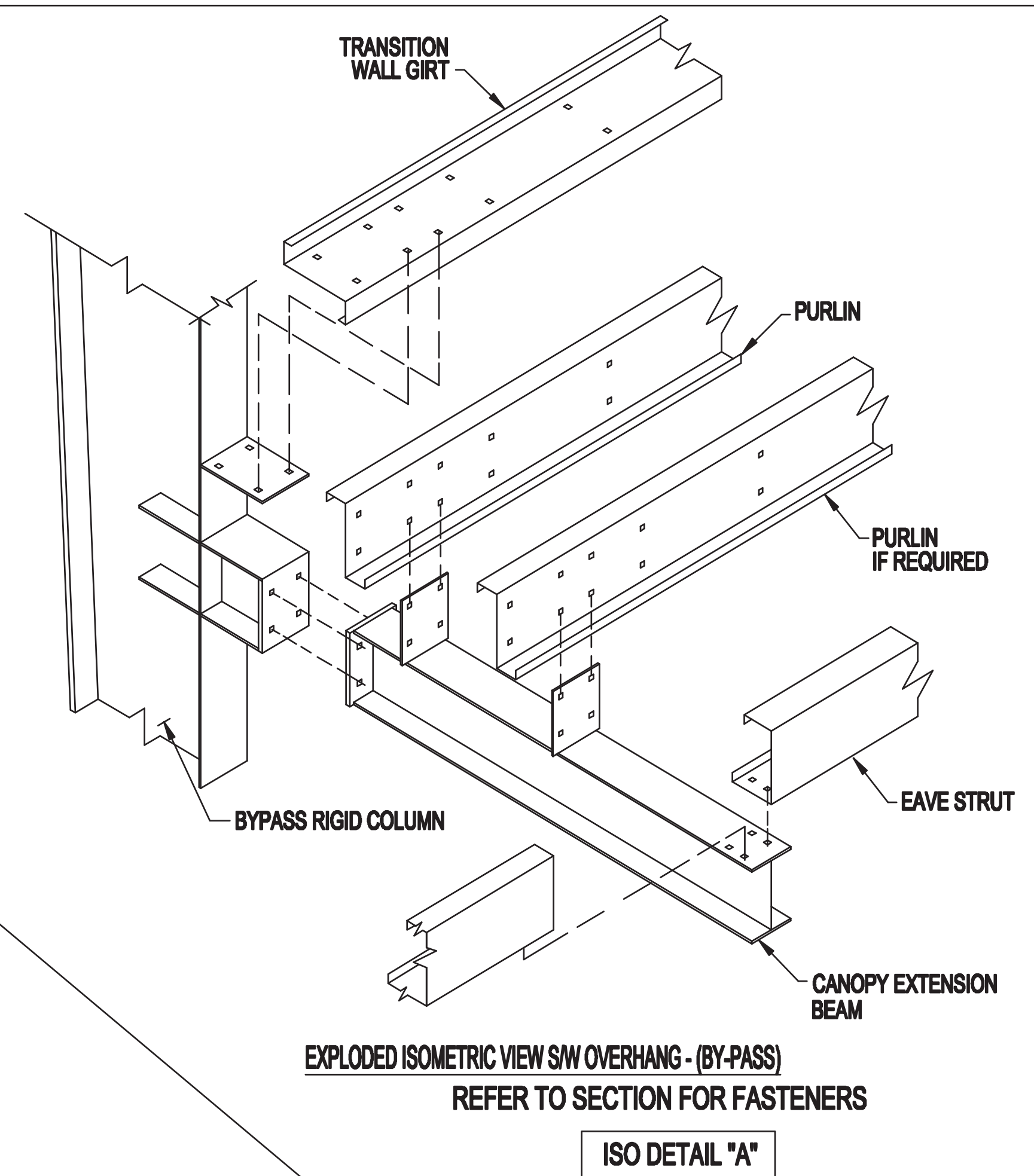
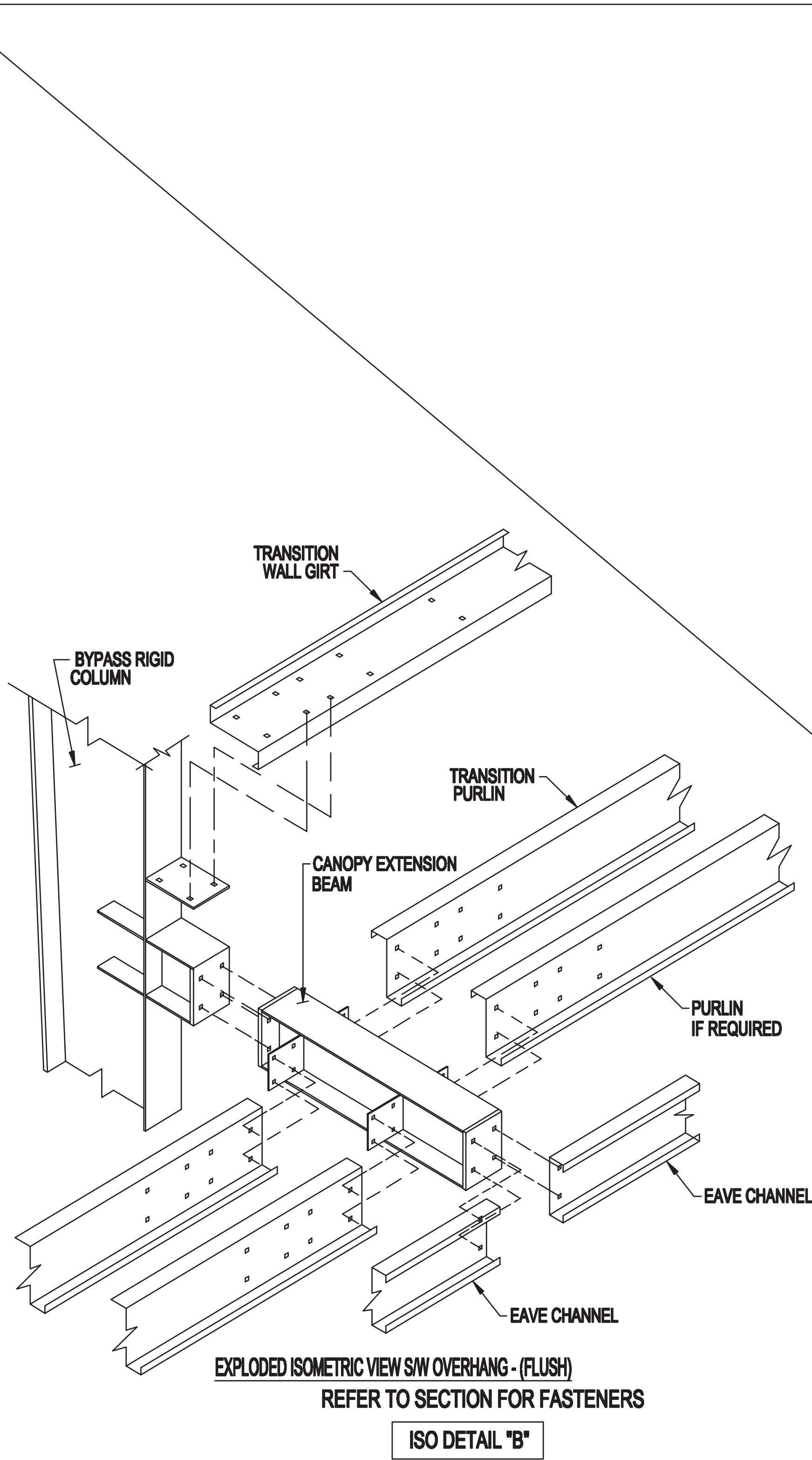
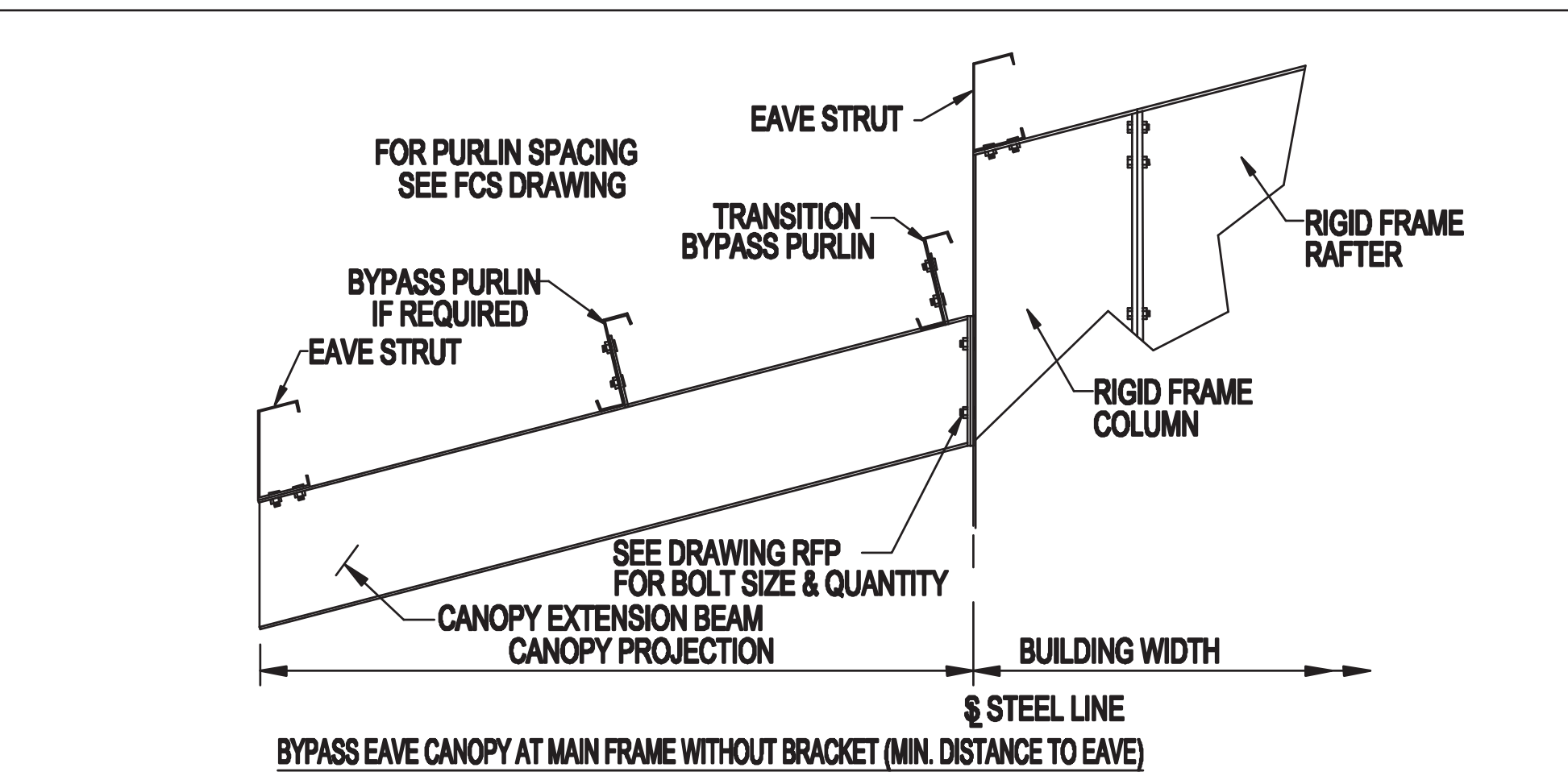
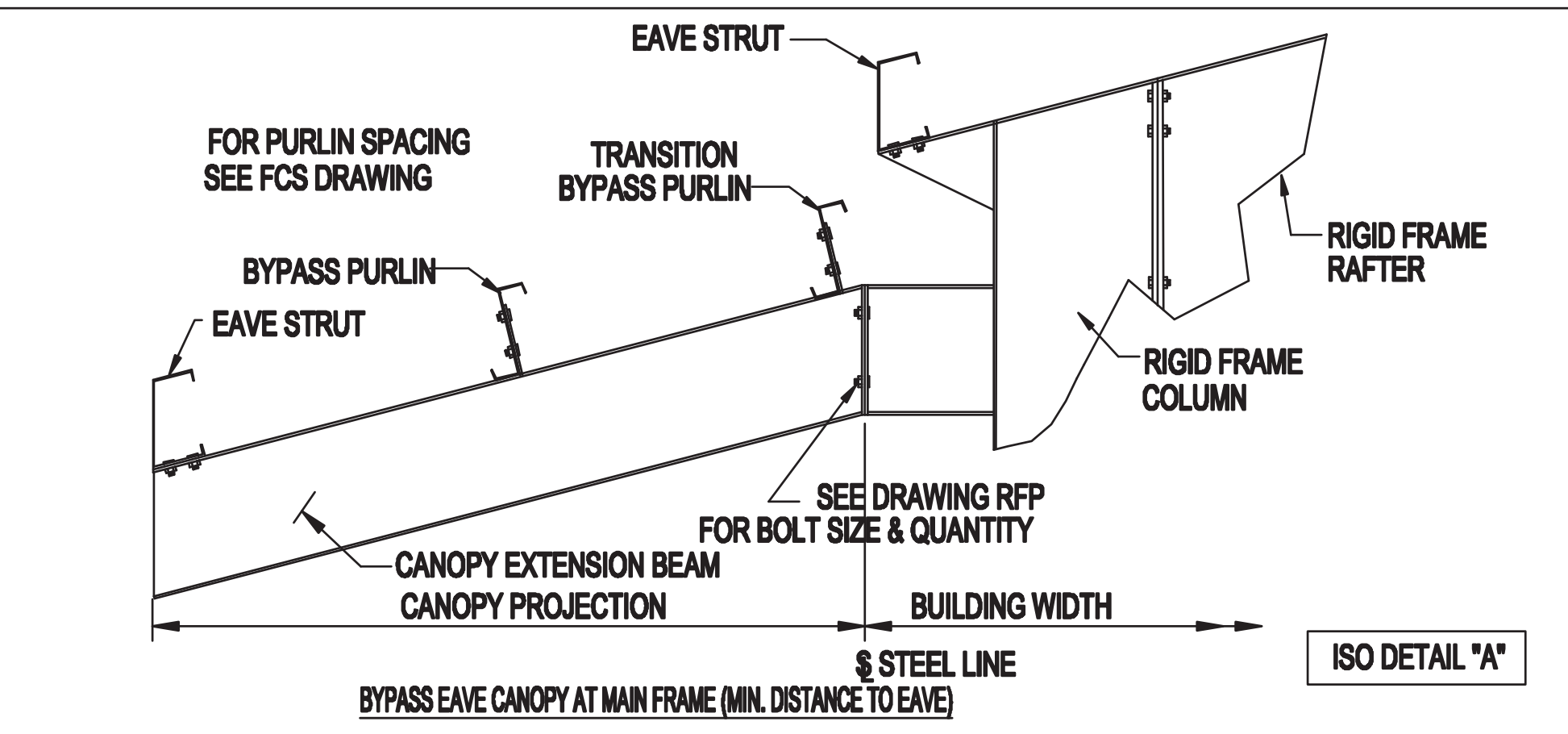
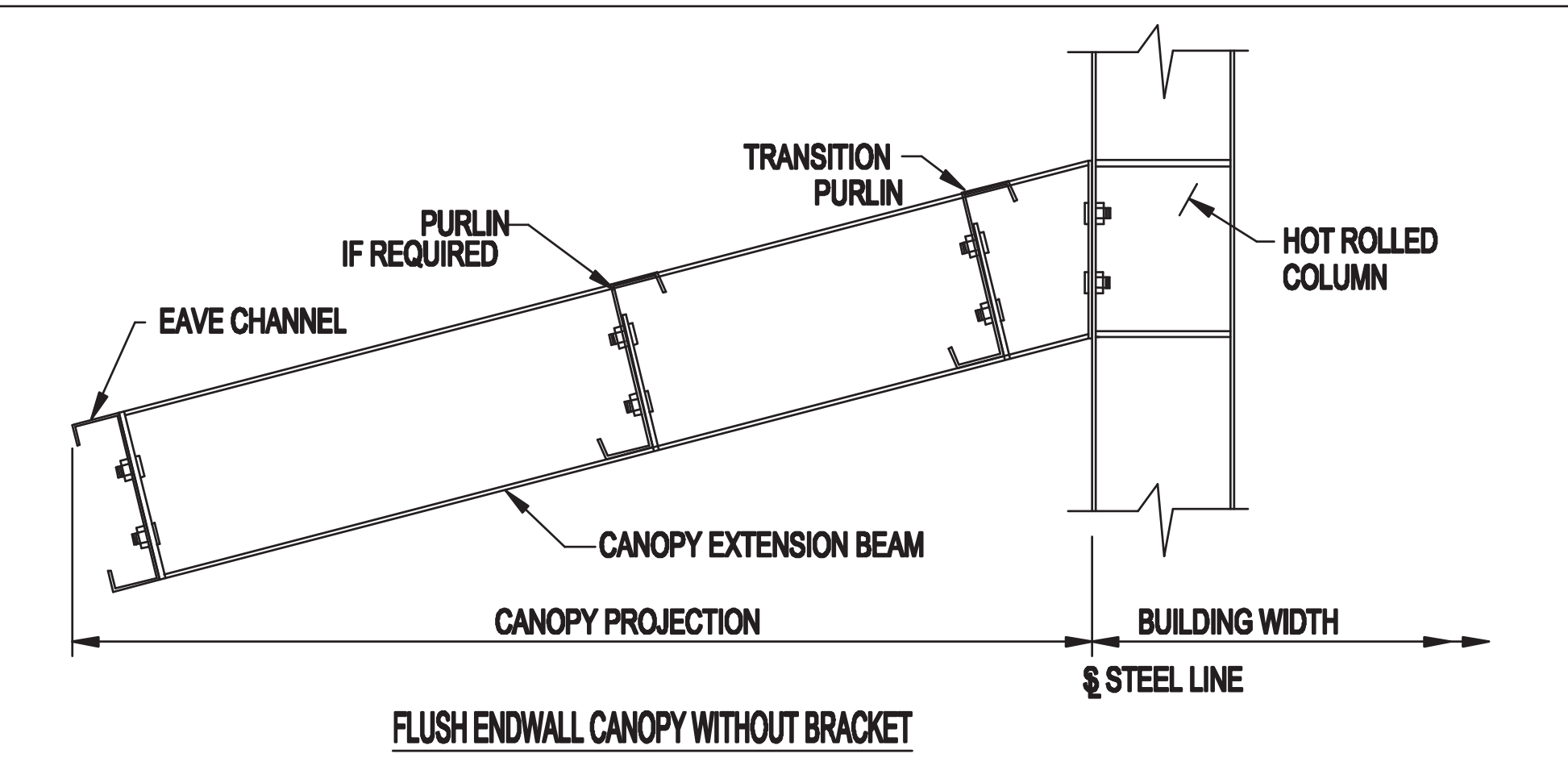
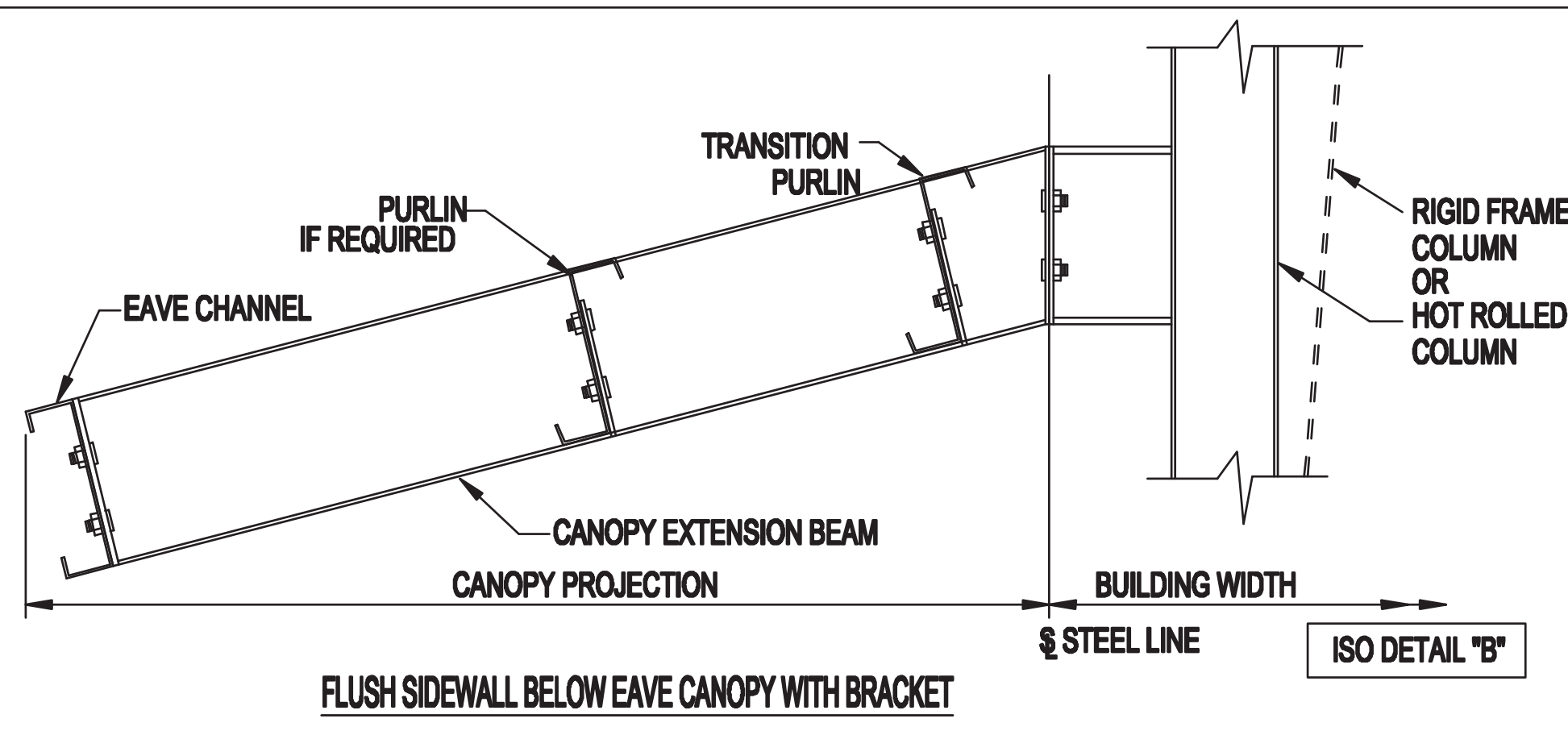
ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
NORTH LITTLE ROCK, AR 72116-7606  
1-800-643-8555

PROJECT:							
CUSTOMER:			OWNER:				
LOCATION:							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		F3	0

S-2



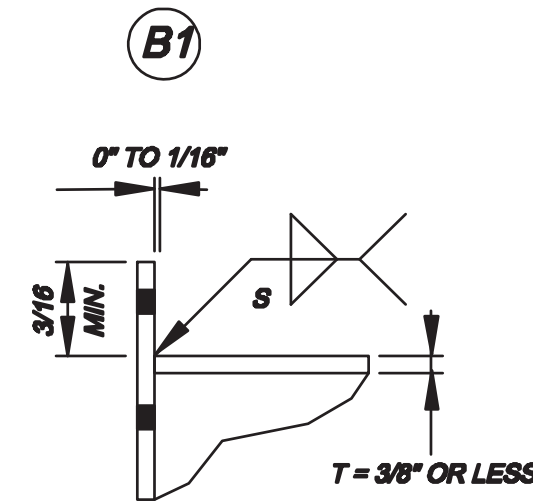
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						CUSTOMER:	X	OWNER:					
						LOCATION:	X						
						CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
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**GENERAL NOTES AND DETAILS**  
WS-1A Date Issued: July 31, 2020

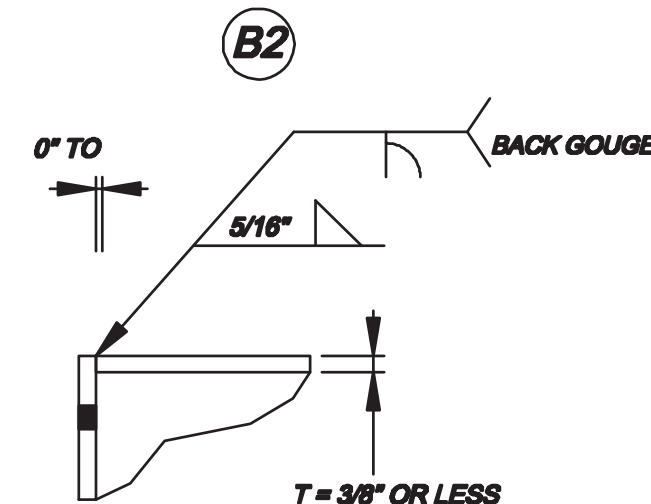
FLANGE THICK	FILLET S
1/4"	3/16"
5/16"	1/4"
3/8"	5/16"

FILLETS NOT ALLOWED ON CRANE BRACKETS EXCEPT AS NOTED ON WS6. USE DETAILS B2 AND B3 ON CRANE BRACKETS WHERE FILLET WELDS ARE NOT ALLOWED.

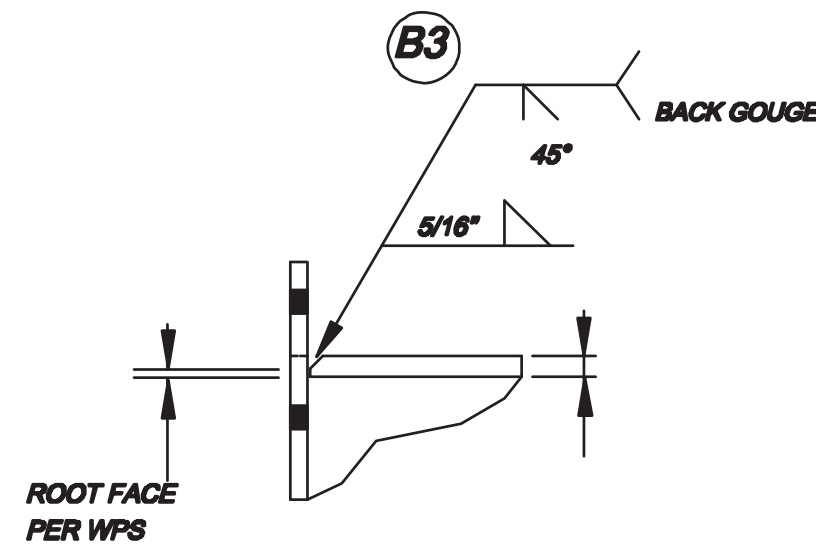
**EXTENDED CONNECTION PLATE AND BRACKET TO FLANGE**



**FLUSH OR RECESSED CONNECTION PLATE AND BRACKET TO FLANGE**



**EXTENDED, FLUSH OR RECESSED CONNECTION PLATE AND BRACKET TO FLANGE**

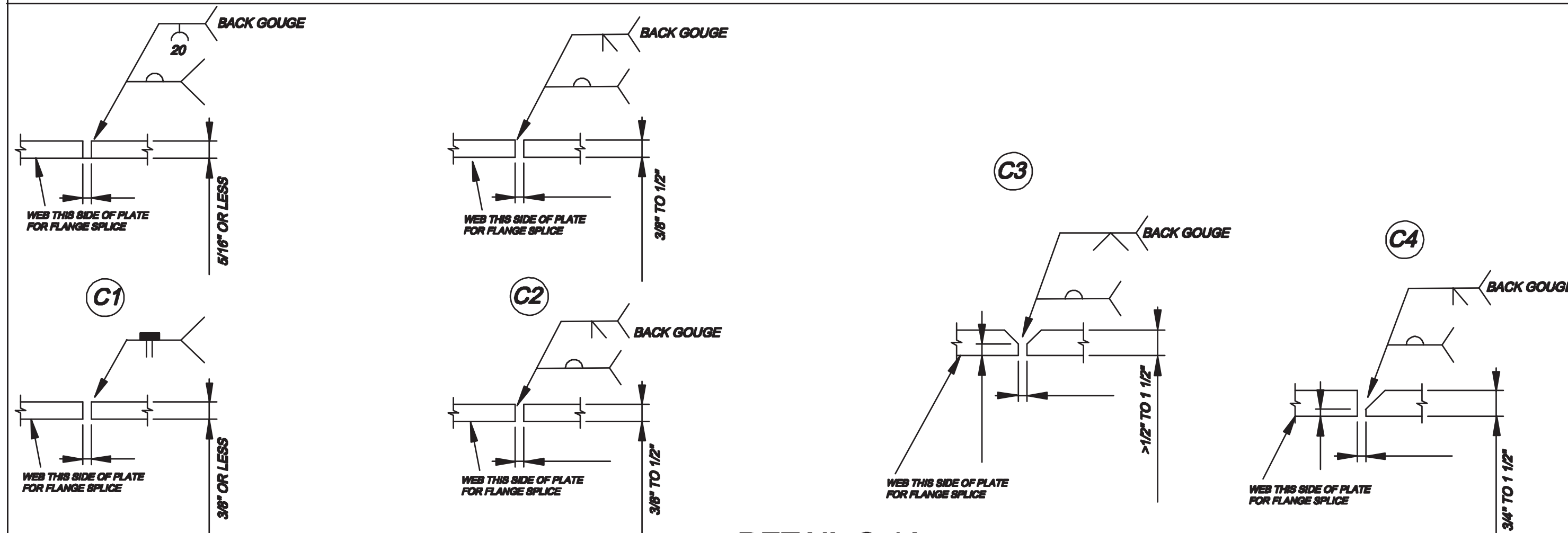


**DETAIL B-1A GENERAL USAGE GUIDELINES**

CONDITION	LIMIT(S)	DETAIL TO USE
FLANGE TO CONNECTION PLATE	FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8"	B1
	CONNECTION PLATE IS EXTENDED PAST FLANGE ENOUGH TO ALLOW REQUIRED FILLET SIZE	B1
	FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8"	B2
CRANE BRACKET TO FLANGE	BRACKET FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8"	B2
	BRACKET FLANGE THICKNESS GREATER THAN 3/8"	B3
NON-CRANE BRACKET TO FLANGE	NON-CRANE BRACKET FLANGE THICKNESS LESS THAN OR EQUAL TO 3/8"	B1
	NON-CRANE BRACKET FLANGE THICKNESS GREATER THAN 3/8"	B3
COLUMN FLANGE TO MOMENT BASE PLATE, STIFFENER TO CONNECTION PLATE OR ANY CONDITION CALLING FOR DETAIL B-1A BUT NOT MENTIONED ABOVE.	WELDED PLATE THICKNESS LESS THAN OR EQUAL TO 3/8"	B1
	PLATE IS EXTENDED PAST WELDED PLATE ENOUGH TO ALLOW REQUIRED FILLET SIZE	B1
	WELDED PLATE THICKNESS LESS THAN OR EQUAL TO 3/8"	B2
	WELDED PLATE THICKNESS GREATER THAN 3/8"	B3

**DETAIL B-1A** SEE NOTE C2 AND C3 IN DETAIL C-1A

**FLANGE TO CONNECTION PLATE AND BRACKET TO FLANGE**



**DETAIL C-1A**  
**SPLICE WELDS**

**NOTES**

C2 ANY PREQUALIFIED OR "PROCEDURE QUALIFIED" CJP WELD MAY ALSO BE USED FOR DETAILS B-1A AND C-1A, WITH A WRITTEN CBB APPROVED WPDS.  
C3 SEE GENERAL NOTE 7 FOR RUN TAB REQUIREMENTS

1:2 1/2 THICKNESS TRANSITION REQUIRED FOR CANADA PROJECTS

**GENERAL NOTES**

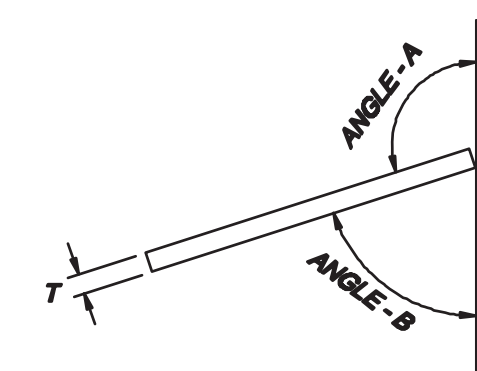
- ALL SAW WEB TO FLANGE WELDS TO BE CONTINUOUS ONE SIDE ONLY UNLESS NOTED ON FABRICATION DOCUMENTS.
- INCREASE FILLET SIZE BY 1/16 FOR EACH 1/16 OF GAP IF GAP AT ROOT IS GREATER THAN 1/16. MAXIMUM FILLET ROOT PERMITTED IS 3/16. SEE DETAIL D-1A ON THIS SHEET.
- FILLET WELD SIZE IS NO GREATER THAN THE SHORTEST LEG SIZE.
- FOLLOW APPROPRIATE WELDING PROCEDURE SPECIFICATIONS FOR ALL WELDS.
- ALL CLIPS SHOULD BE WELDED AT 90-DEG TO THEIR SUPPORTING SURFACE UNLESS OTHERWISE NOTED IN THESE DRAWINGS OR THE PROJECT SHOP DRAWINGS.
- SC2 AND SC280 FLANGE BRACE CLIPS SHOULD BE ALIGNED WITH THE PURLING/IRT CLIP ABOVE. REFER TO DETAIL ON SHEET WS-5. REFER TO DETAIL ON SHEET WS-5.
- WELD RUN-TABS (RUN-ON AND RUN-OFF) SHALL BE USED ON ALL CJP CONNECTIONS, EXCEPT WHERE JOINT GEOMETRY AND/OR INTERFERENCE PREVENT THE PLACEMENT OF A WELD TAB. THIS SHALL BE DONE BY USE OF WELD TABS ALIGNED IN SUCH A MANNER TO PROVIDE AN EXTENSION OF THE JOINT PREPARATION FOR PURPOSE OF WELD PASS INITIATION AND TERMINATION. STIFFENER WELDS ENDING AT OR THE INSIDE CORNER OF THE STIFFENER MEMBER OR A CLIP NEAR THAT CORNER ARE EXAMPLES OF WELDS THAT CANNOT BE TERMINATED ON A WELD TAB. WELD RUN-TABS SHALL BE REMOVED UPON COMPLETION OF THE JOINT. ENDS OF WELDED BUT JOINTS SHALL BE FINISHED 90 AS NOT TO REDUCE THE WIDTH BEYOND THE DETAILED WIDTH OR THE ACTUAL WIDTH FURNISHED, WHICHEVER IS THE GREATER, BY MORE THAN 1/8".

**DETAIL A-1A**

**FILLET INCREASE AT SKEWED WELD JOINTS**

THE FOLLOWING TABLES PROVIDE THE REQUIRED MODIFICATION TO THE REQUESTED/REQUIRED FILLET WELD BASED ON THE JOINT GEOMETRY.

NC - INDICATES THAT NO CHANGE TO THE REQUESTED/REQUIRED FILLET WELD IS REQUIRED.  
CJP - INDICATES THE REQUESTED/REQUIRED FILLET WELD MUST BE REPLACED BY A CJP WELD.  
+1/16 - INDICATES THE REQUESTED/REQUIRED FILLET WELD LEG SIZE MUST BE INCREASE BY 1/16" DUE TO THE SKEWED JOINT.  
+1/8 - INDICATES THE REQUESTED/REQUIRED FILLET WELD LEG SIZE MUST BE INCREASE BY 1/8" DUE TO THE SKEWED JOINT.  
+3/16 - INDICATES THE REQUESTED/REQUIRED FILLET WELD LEG SIZE MUST BE INCREASE BY 3/16" DUE TO THE SKEWED JOINT.  
GENERAL NOTE 2 IS IN ADDITION TO ANY INCREASE SHOWN IN THE TABLES BELOW.  
WHEN THE FILLET SIZE MUST BE INCREASE THE FINAL FILLET WELD SIZE SHALL NOT EXCEED 3/8". IF THE FINAL FILLET WELD SIZE EXCEEDS 3/8" THEN USE CJP.

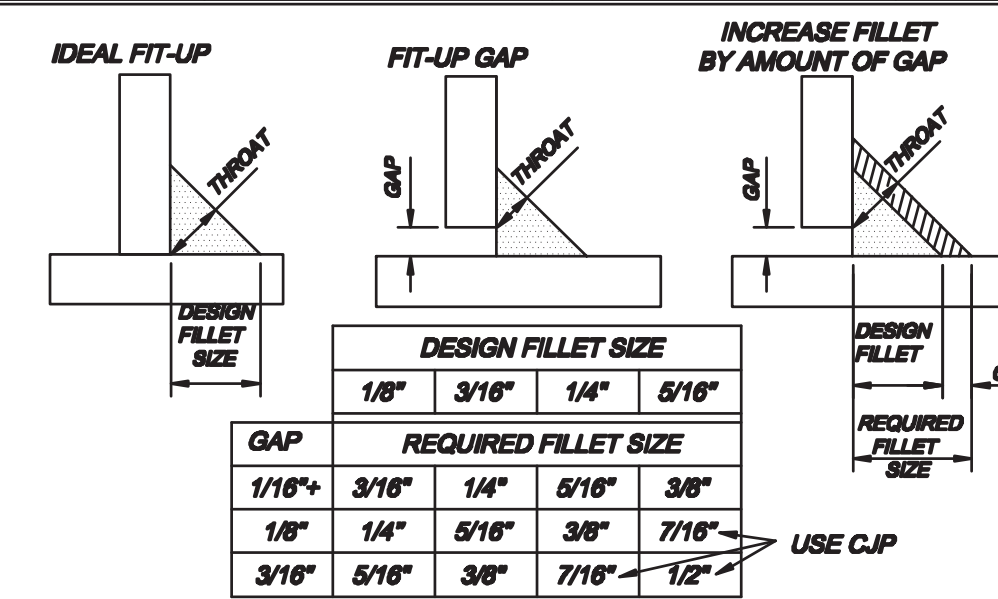


**TABLE D-1**  
REQUIRED MODIFICATION TO FILLET WELD ON ANGLE-B SIDE OF PLATE

T (IN)	ANGLE-B (DEGREES)			
	60 < A < 90	45 < ANGLE-B < 60	30 < ANGLE-B < 45	
0.1340	NC	+1/8	CJP	
0.1560	NC	+1/8	CJP	
0.1850	NC	+1/8	CJP	
3/8"	NC	+1/8	CJP	
1/2"	NC	+1/8	CJP	
5/8"	NC	+1/8	CJP	

**TABLE D-2**  
REQUIRED MODIFICATION TO FILLET WELD ON ANGLE-A SIDE OF PLATE

T (IN)	ANGLE-A (DEGREES)			
	90 < ANGLE-A < 96	106 < ANGLE-A < 118	125 < ANGLE-A < 125	135 < ANGLE-A < 135
0.1340	NC	+1/8	+1/8	+3/8
0.1560	NC	+1/8	+1/8	+3/8
0.1850	NC	+1/8	+3/8	+3/8
3/8"	NC	+1/8	+3/8	+3/8
1/2"	NC	+1/8	+3/8	CJP
5/8"	NC	+1/8	+3/8	CJP
3/8"	NC	+1/8	+3/8	CJP
1/2"	NC	+3/8	+3/8	CJP
5/8"	NC	+3/8	CJP	CJP



**DETAIL D-1A** FILLET INCREASE

**TABLE 1**  
MIN. FILLET WELD SIZES

THICKER PLATE	THINNER PLATE (USUALLY THE WEB)						
	< 1/4"	1/4"	5/16"	3/8"	1/2"	5/8"	3/4"
UNDER 1/4"	3/16"	-	-	-	-	-	-
1/4"	3/16"	3/16"	-	-	-	-	-
5/16 THRU 1/2"	3/16"	3/16"	3/16"	3/16"	3/16"	-	-
5/8 THRU 3/4"	3/16"	1/4"	1/4"	1/4"	1/4"	1/4"	1/4"
OVER 3/4"	3/16"	1/4"	5/16"	5/16"	5/16"	5/16"	5/16"

**TABLE 2**  
GENERAL FILLET WELDS

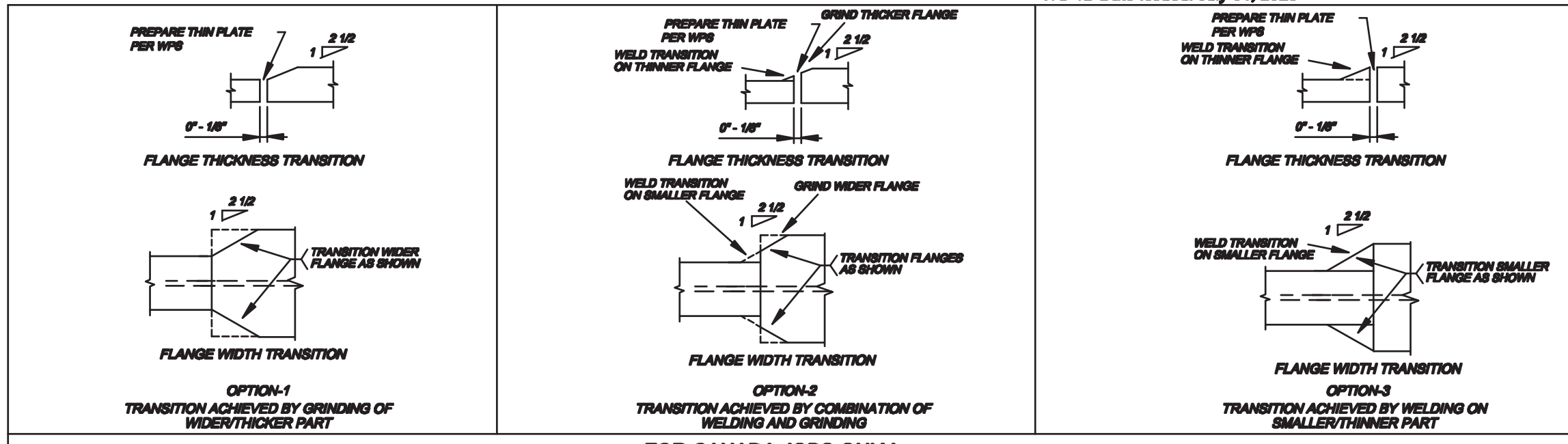
THINNER PLATE THICKNESS	FILLET SIZE	
	ONE SIDE	BOTH SIDES
UNDER 1/4"	3/16"	3/16"
1/4"	1/4"	3/16"
5/16"	5/16"	1/4"
OVER 5/16"	5/16"	5/16"

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN

PROJECT: \_\_\_\_\_  
CUSTOMER: \_\_\_\_\_ OWNER: \_\_\_\_\_  
LOCATION: \_\_\_\_\_

CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

**GENERAL NOTES AND DETAILS**  
WS-18 Date Issued: July 31, 2020



**FOR CANADA JOBS ONLY**

TRANSITION CAN BE ACHIEVED BY WELDING ON THE THINNER/SMALLER PLATE (OPTION 3), BY GRINDING THE THICKER/WIDER PLATE (OPTION 1) OR BY A COMBINATION OF BOTH (OPTION 2) TO GET THE REQUIRED 1/2 L2 TRANSITION. AT INRSE CONNECTION PLATES THIS TRANSITION MUST BE ACHIEVED BY USING OPTION 3, WELDING ON THE THINNER/SMALLER PLATE. DO NOT ACCEPT THE INRSE CONNECTION PLATE TO GET THE REQUIRED 1/2 L2 TRANSITION. FOR STEEPED THICKNESS, THINNER PLATE END PREPARATION MAY BE USED IF TRANSITION OF THICKER PLATE IS PREPARED PRIOR TO WELDING.

**END PLATE WELD GUIDANCE FOR GENERIC BEAMS BASED ON PARENT PIECE MARK**

PARENT MARK PREFIX	DESCRIPTION	SHEEL	DETAIL	SHEET DETAIL
BB*(see note 1)	Bracket	NA	W-S-E	Beam End Bracket
CB	Crane End Section	End Plate Weld Required unless provided by the design engineer		
BB*	Beam-DR Beam	W-S-G, M-S and L-S	W-S-G	Detail D-5
BM*	Mezzanine Beam	W-S-G, M-S and L-S	W-S-G	Detail D-5
BP*	Purlin, Post-tension Beam	W-S-G, M-S and L-S	W-S-G	Detail D-5
BR*	Roof Beam	W-S-G, M-S and L-S	W-S-G	Detail D-5
BS*	Sparrow Beam	W-S-G, M-S and L-S	W-S-G	Detail D-5
BL*	Wall Beam	W-S-G, M-S and L-S	W-S-G	Detail D-5
BE*	Underhung Crane Bracket Extension	NA	W-S-E	Underhung Crane Bracket Extension

**SEAL WELD NOTES:**

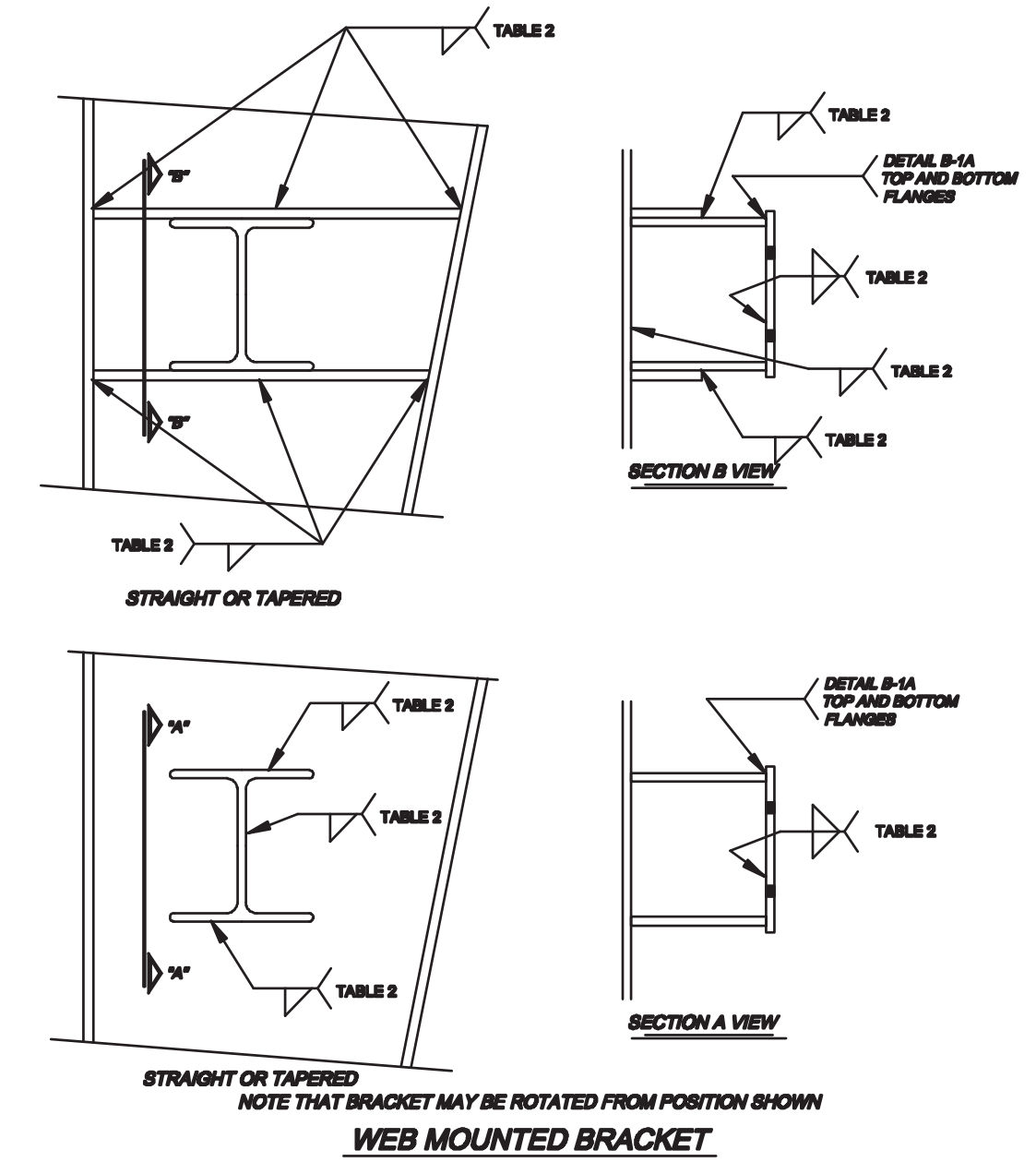
- SEAL WELDS SHALL ONLY BE PROVIDED WHEN SPECIFICALLY REQUESTED ON THE SHOP DRAWINGS.
- SEAL WELDS SHALL MEET ALL THE SAME QUALITY AND WORKMANSHIP REQUIREMENTS AS ANY OTHER WELD ON A MEMBER AND CRITERIA OF SECTION 1.8.8 OF THE CBS WELD MANUAL AND CLAUSE 6.4.10 OF CSA W59-12.
- SEAL WELD SIZE SHALL BE PER TABLE 2 SHOWN ON THE WELD STANDARD DRAWINGS.
- FOR MEMBERS REQUIRING SEAL WELDS CBS WILL PROVIDE MEMBERS TO MEET CLASS 1 OR CLASS II CRITERIA AS DEFINED BY AMERICAN GALVANIZERS ASSOCIATION (AGA) UNLESS OTHERWISE SPECIFICALLY REQUESTED BY THE CUSTOMER. PROPER VENTING FOR OVERLAPPING PARTS SHALL BE PROVIDED IN THE FORM OF UNWELDED PORTIONS OR HOLES PER TABLE SHOWN ON THIS SHEET FOR ALL MEMBERS TO BE HOT DIPPED GALVANIZED AFTER FABRICATION.

VENT HOLES FOR OVERLAPPED AREAS FOR STEELS 1/2 IN. (12.75 mm) OR LESS IN THICKNESS		
OVERLAPPED AREA IN. (25.4mm)	VENT HOLES	UNWELDED AREA
UNDER 18 (103)	NONE	NONE
18 (103) TO UNDER 84 (413)	ONE 3/8 IN. (1 cm)	1 IN. (2.5 cm)
84 (413) TO UNDER 400 (2280)	ONE 1/2 IN. (1.25 cm)	2 IN. (5.1 cm)
400 (2280) AND GREATER, EACH 400 (2280)	ONE 3/4 IN. (1.91 cm)	4 IN. (10.2 cm)

VENT HOLES FOR OVERLAPPED AREAS FOR STEELS GREATER THAN 1/2 IN. (12.75 mm) IN THICKNESS		
OVERLAPPED AREA IN. (25.4mm)	VENT HOLES	UNWELDED AREA
UNDER 18 (103)	NONE	NONE
18 (103) TO UNDER 84 (413)	NONE	NONE
84 (413) TO UNDER 400 (2280)	ONE 1/2 IN. (1.25 cm)	2 IN. (5.1 cm)
400 (2280) AND GREATER, EACH 400 (2280)	ONE 3/4 IN. (1.91 cm)	4 IN. (10.2 cm)

**SEAL WELDS**



PARTIAL DEPTH END PLATE FULL DEPTH END PLATE

ISSUE	DATE	DESCRIPTION	BY	CHK	DN

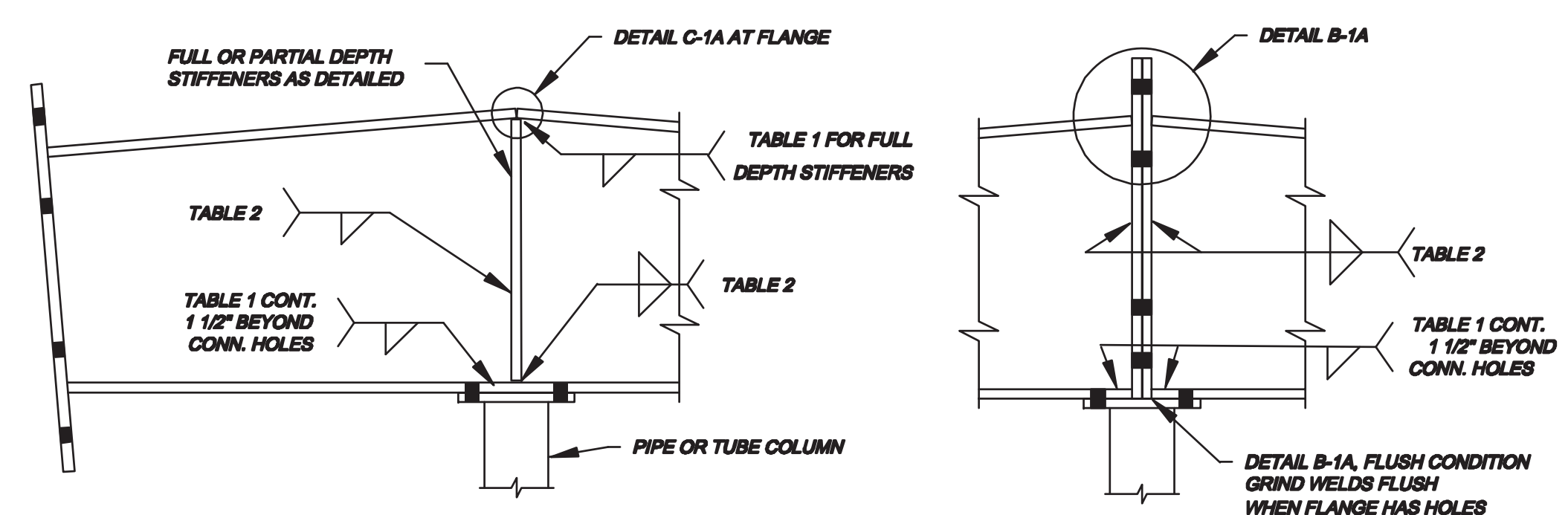
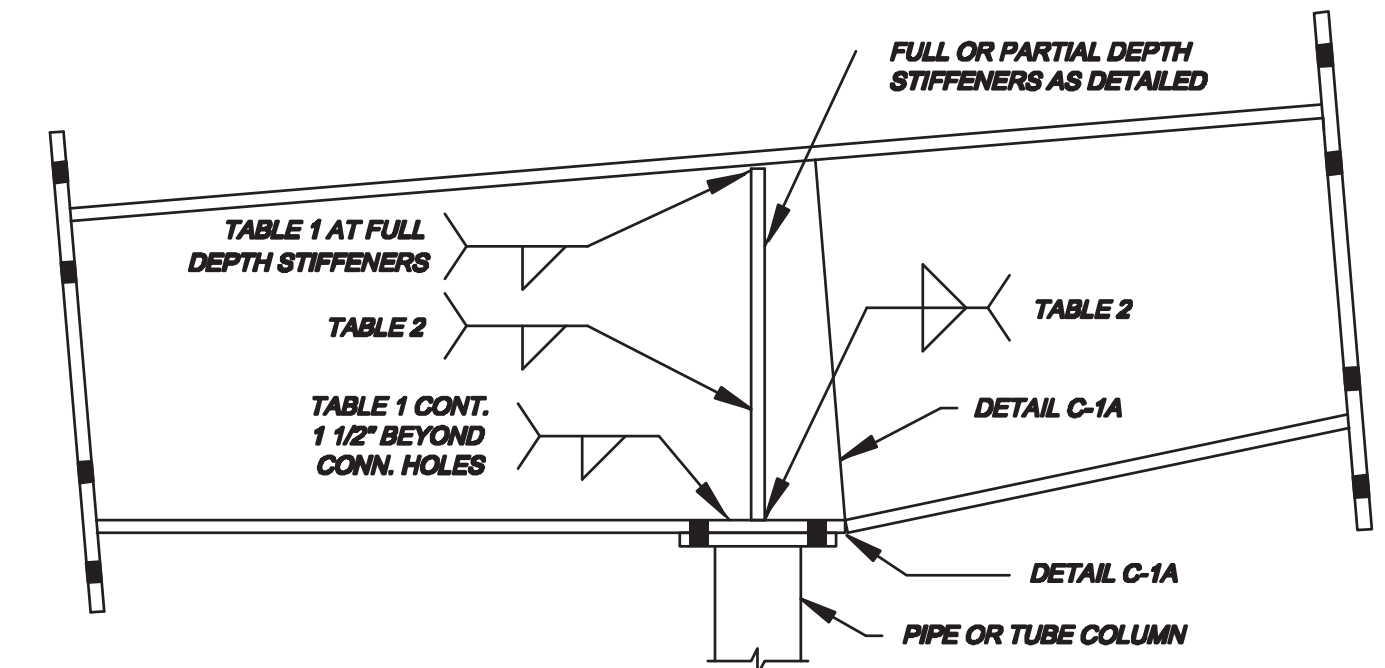
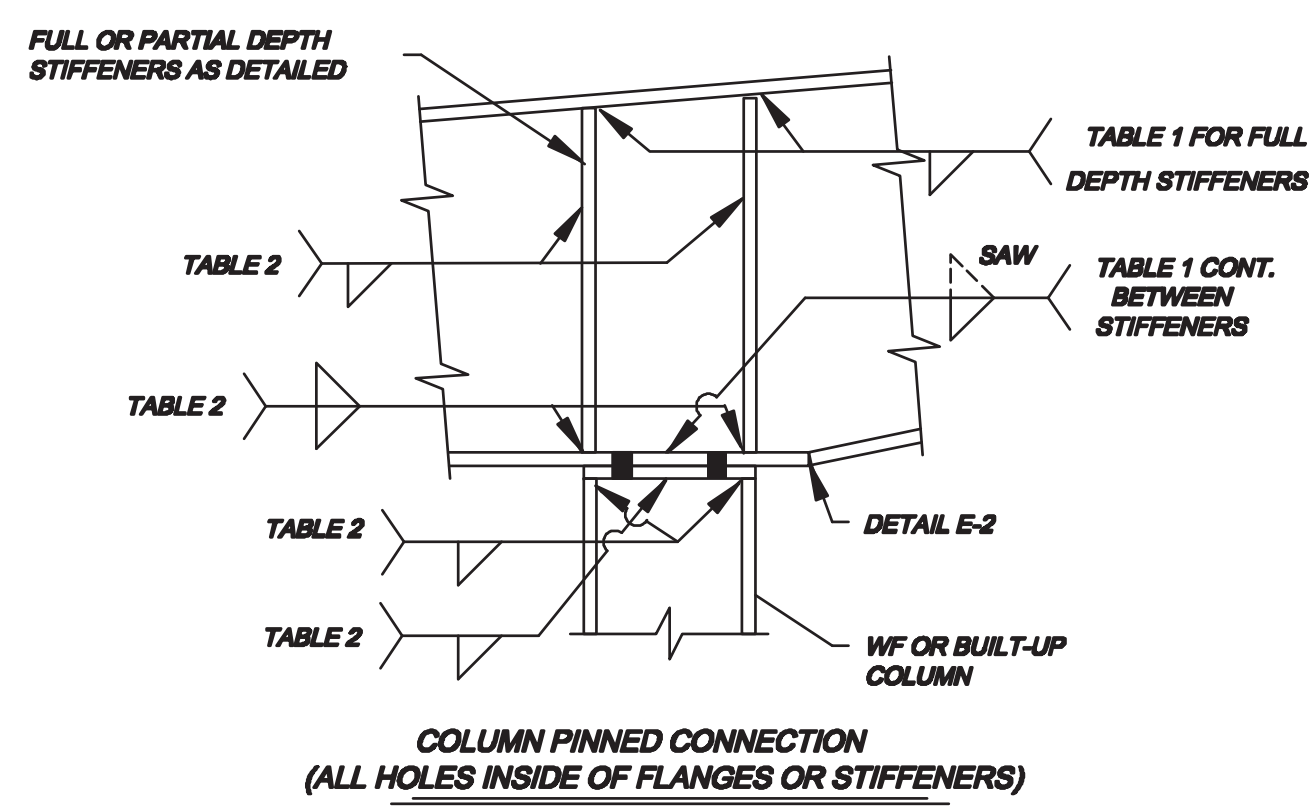
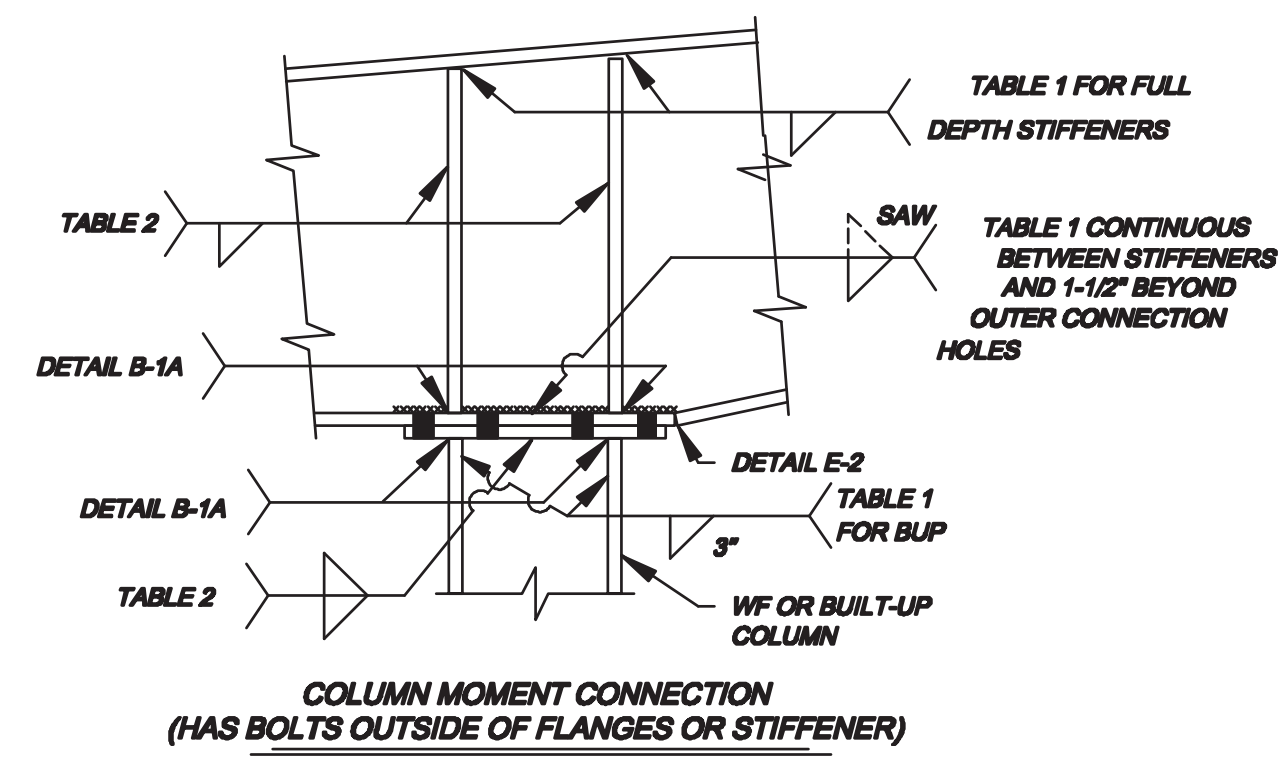
  

PROJECT:							
CUSTOMER:							
OWNER:							
LOCATION:							
CVD	DATE	SCALE	PSWE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.A.					

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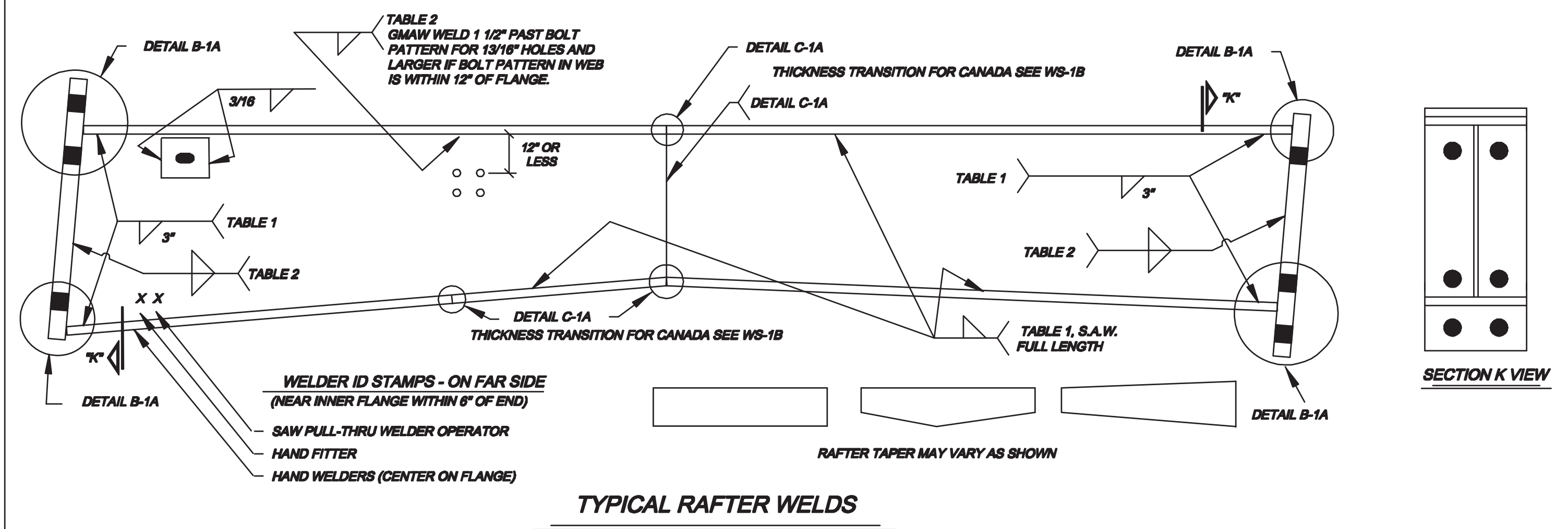




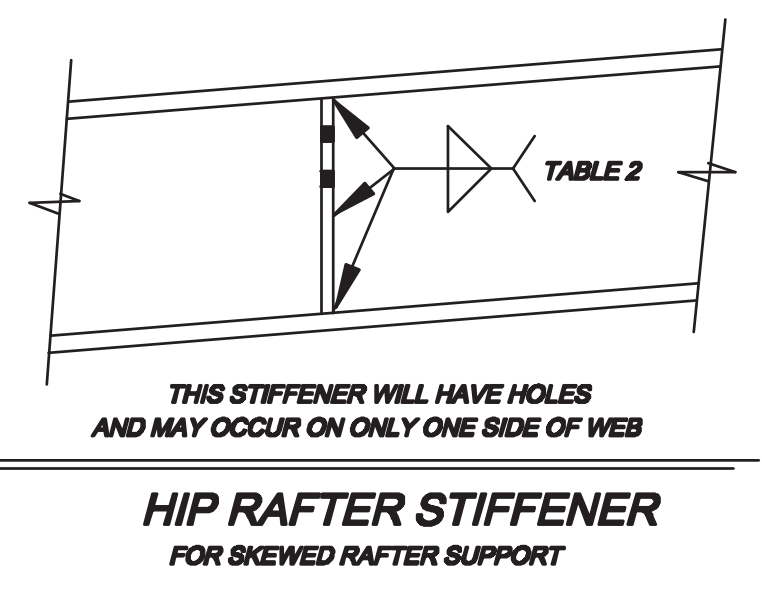
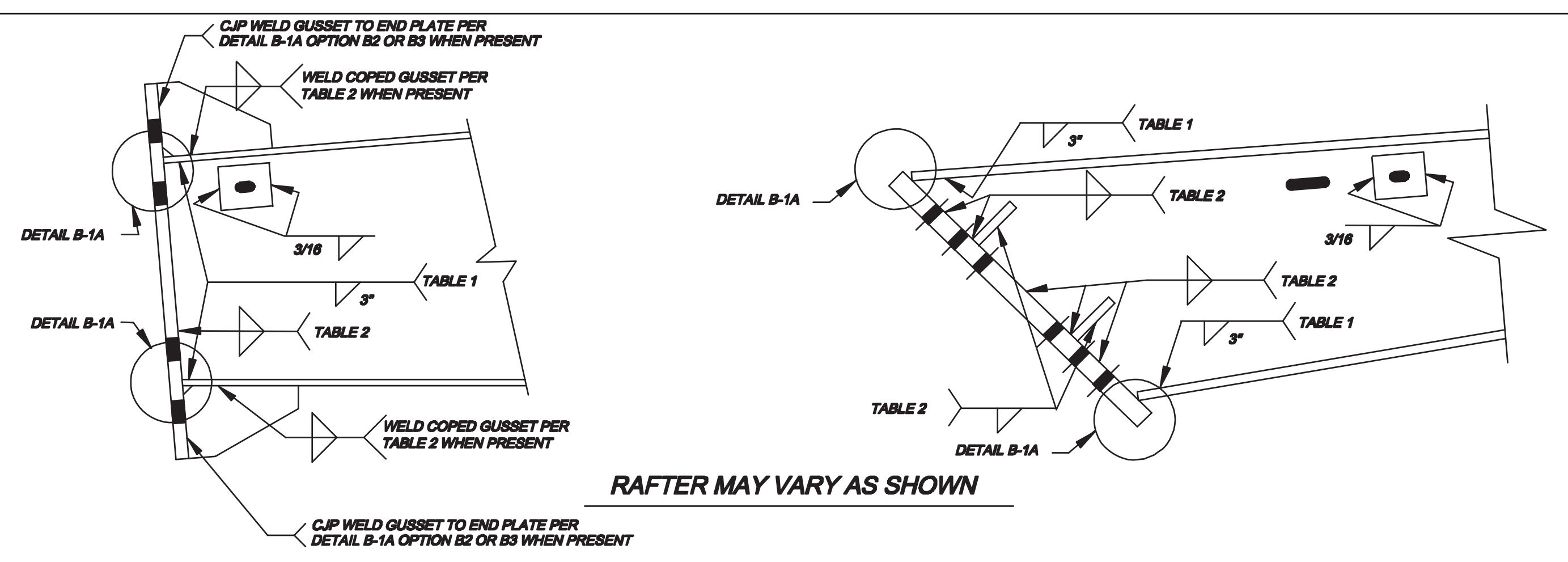


**RAFTER INTERIOR SUPPORTS**

**RIGID FRAME RAFTERS**  
WS-3 Date Issued: July 31, 2020

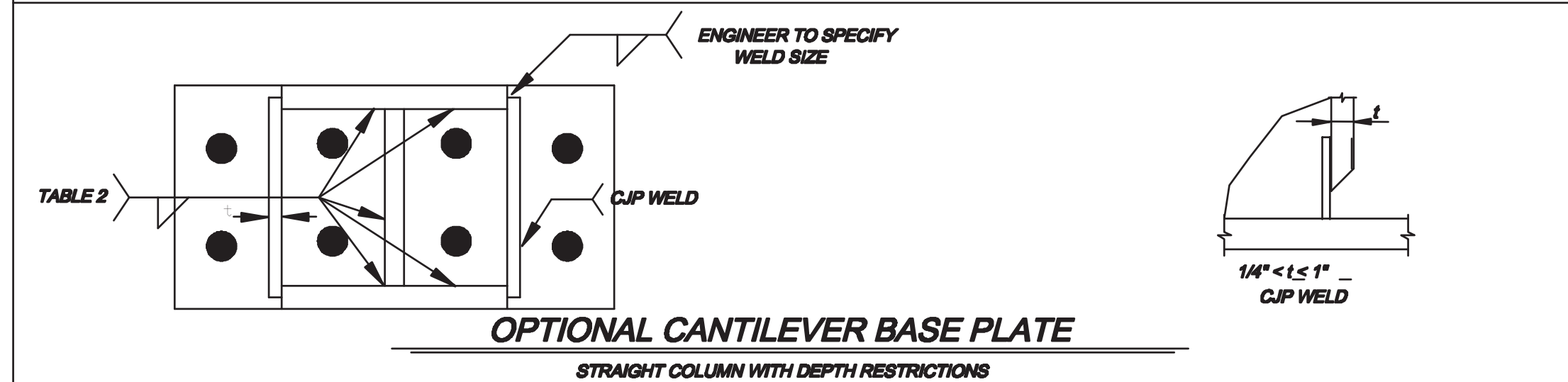
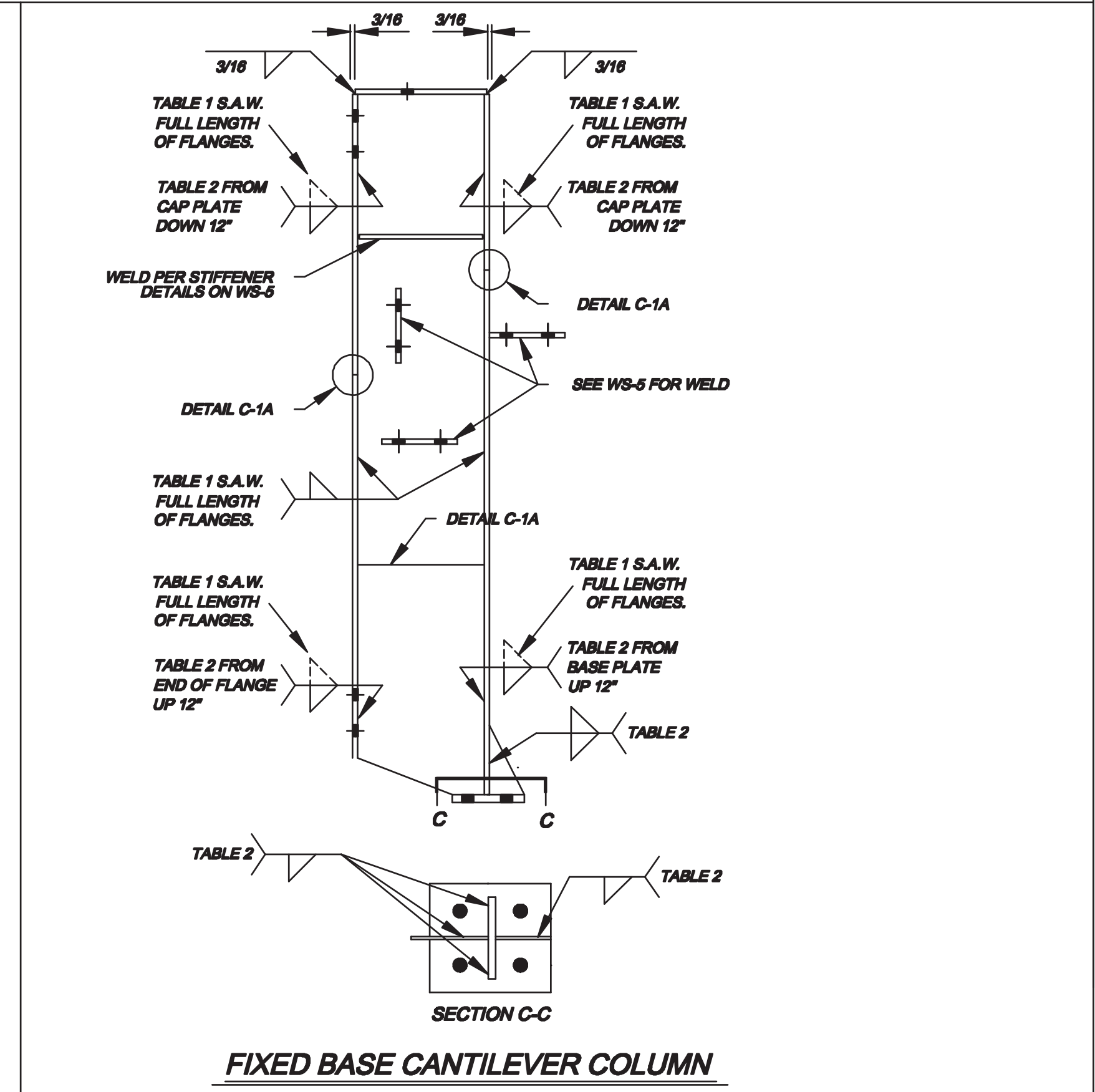
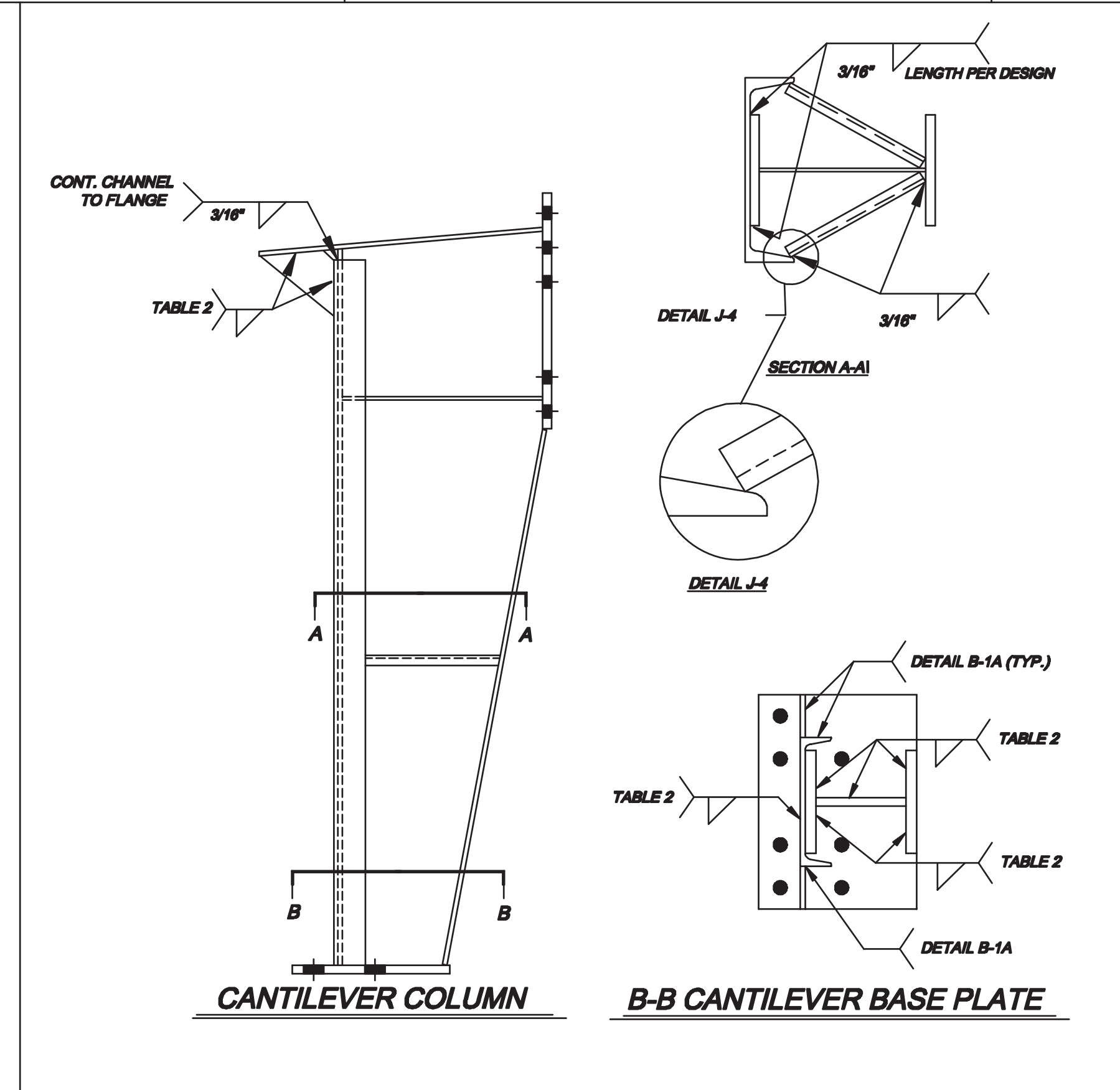
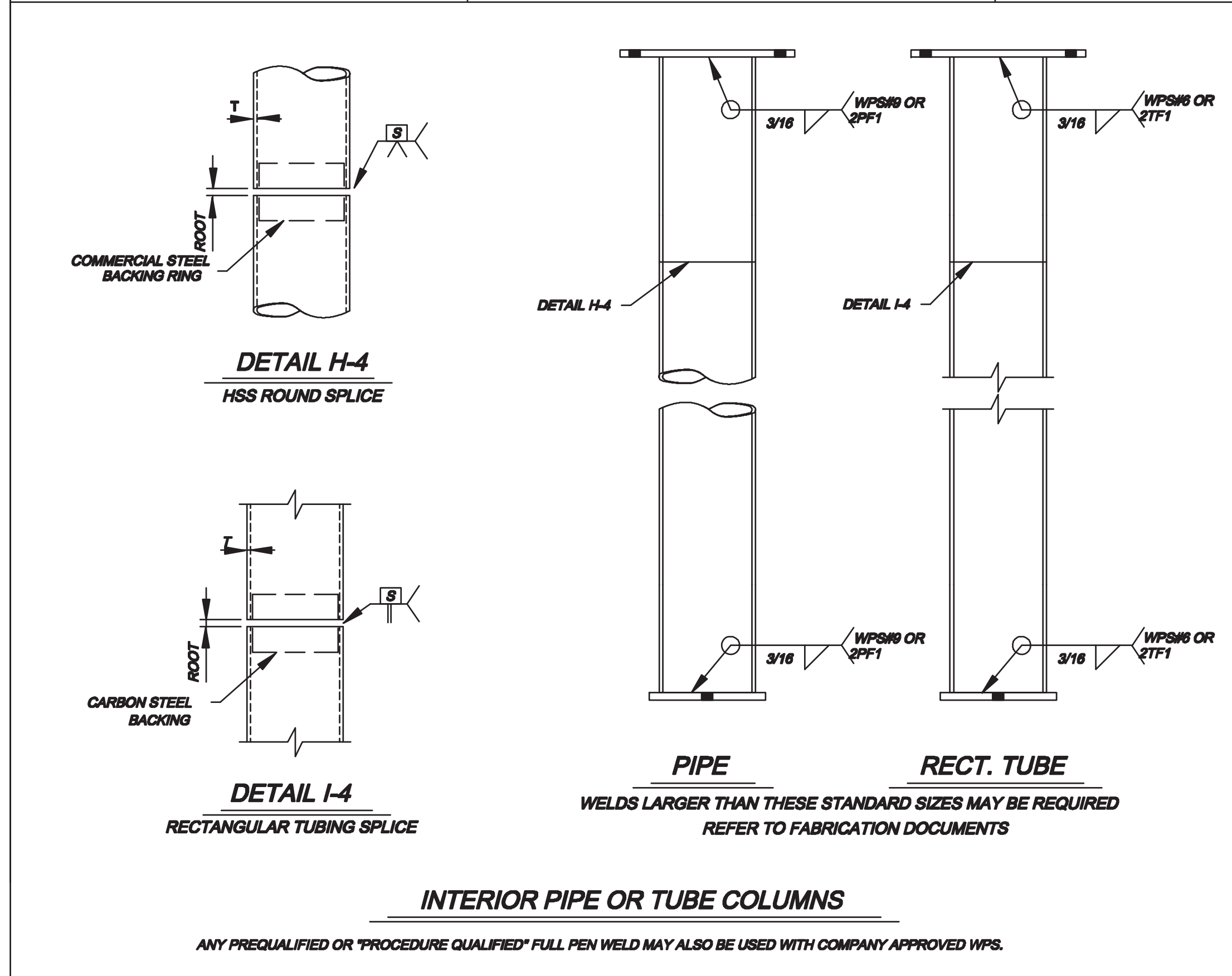
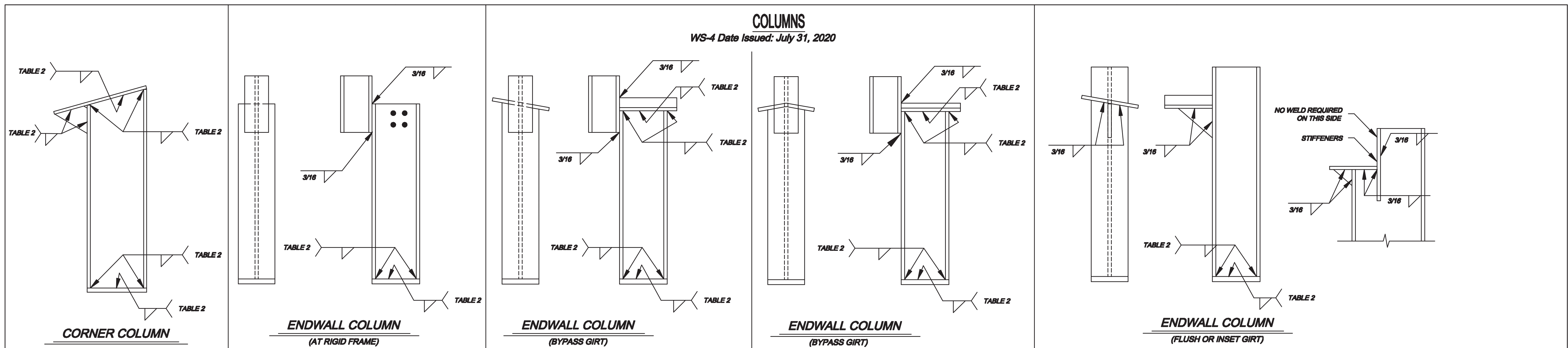


**TYPICAL RAFTER WELDS**



DESCRIPTION	BY	CK'D	DSN								
				PROJECT:							
				CUSTOMER:			OWNER:				
				LOCATION:							
				CAD	DATE	SCALE N.T.S.	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE

**COLUMNS**  
WS-4 Date Issued: July 31, 2020

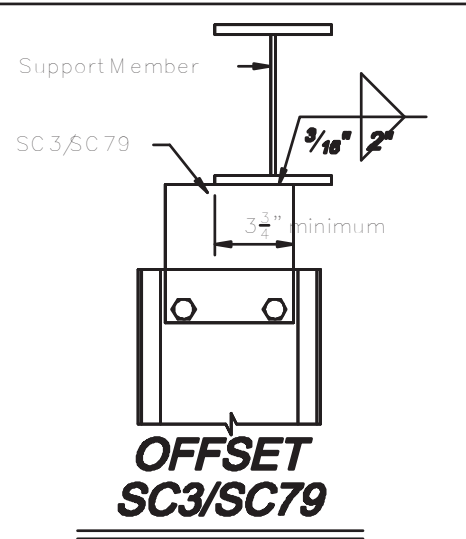
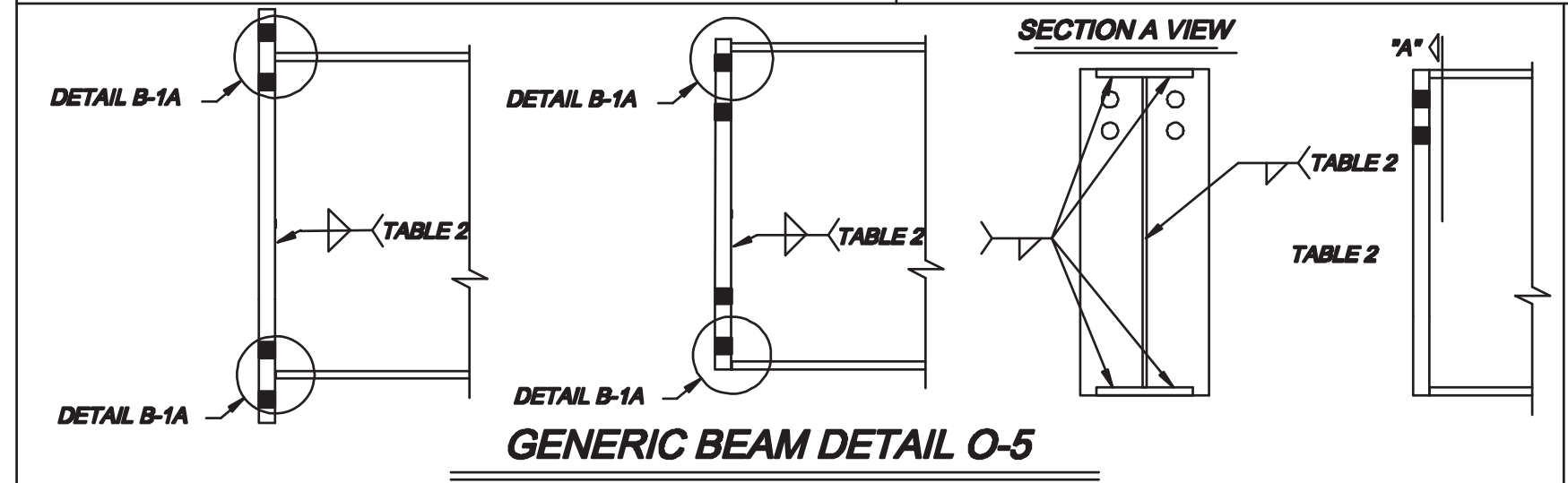
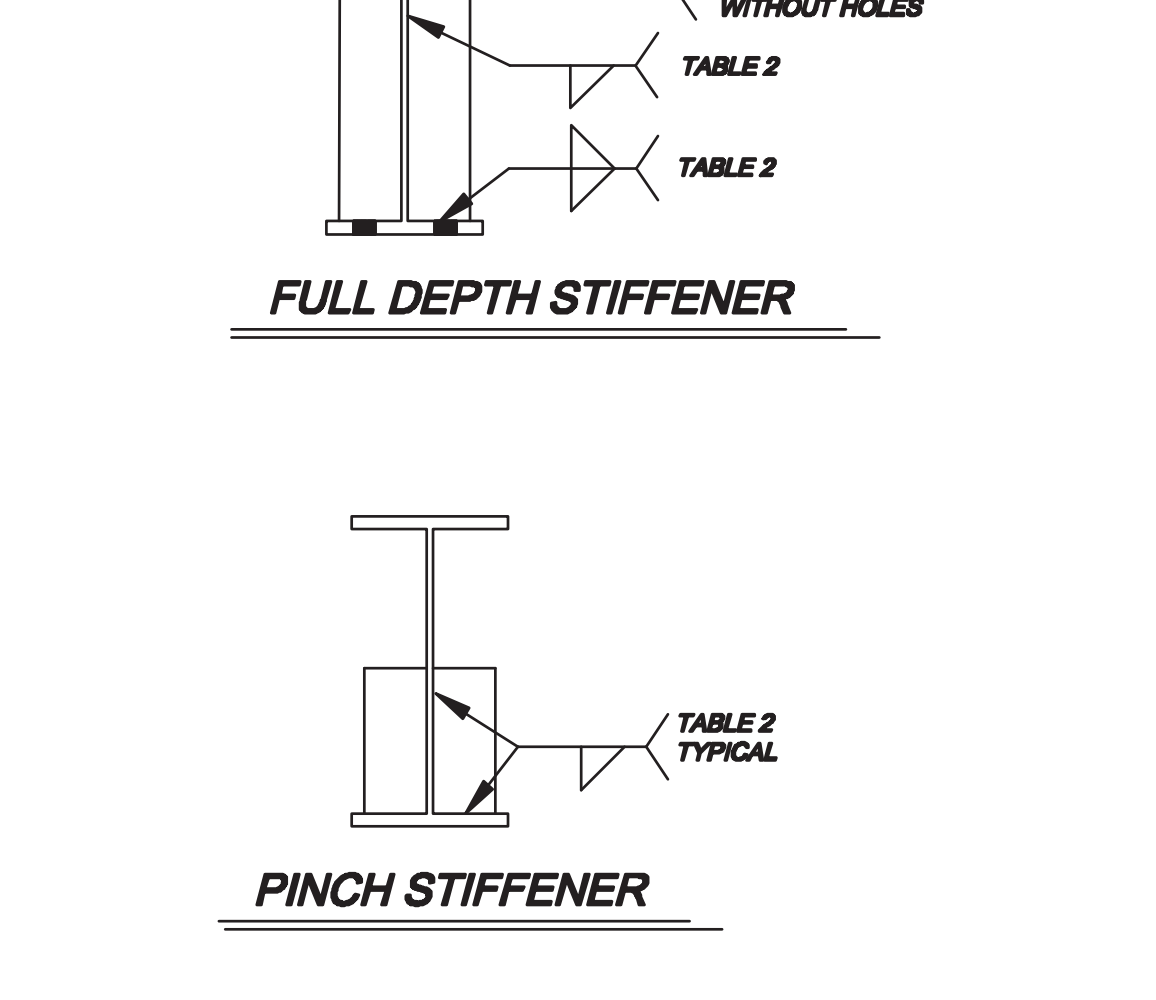
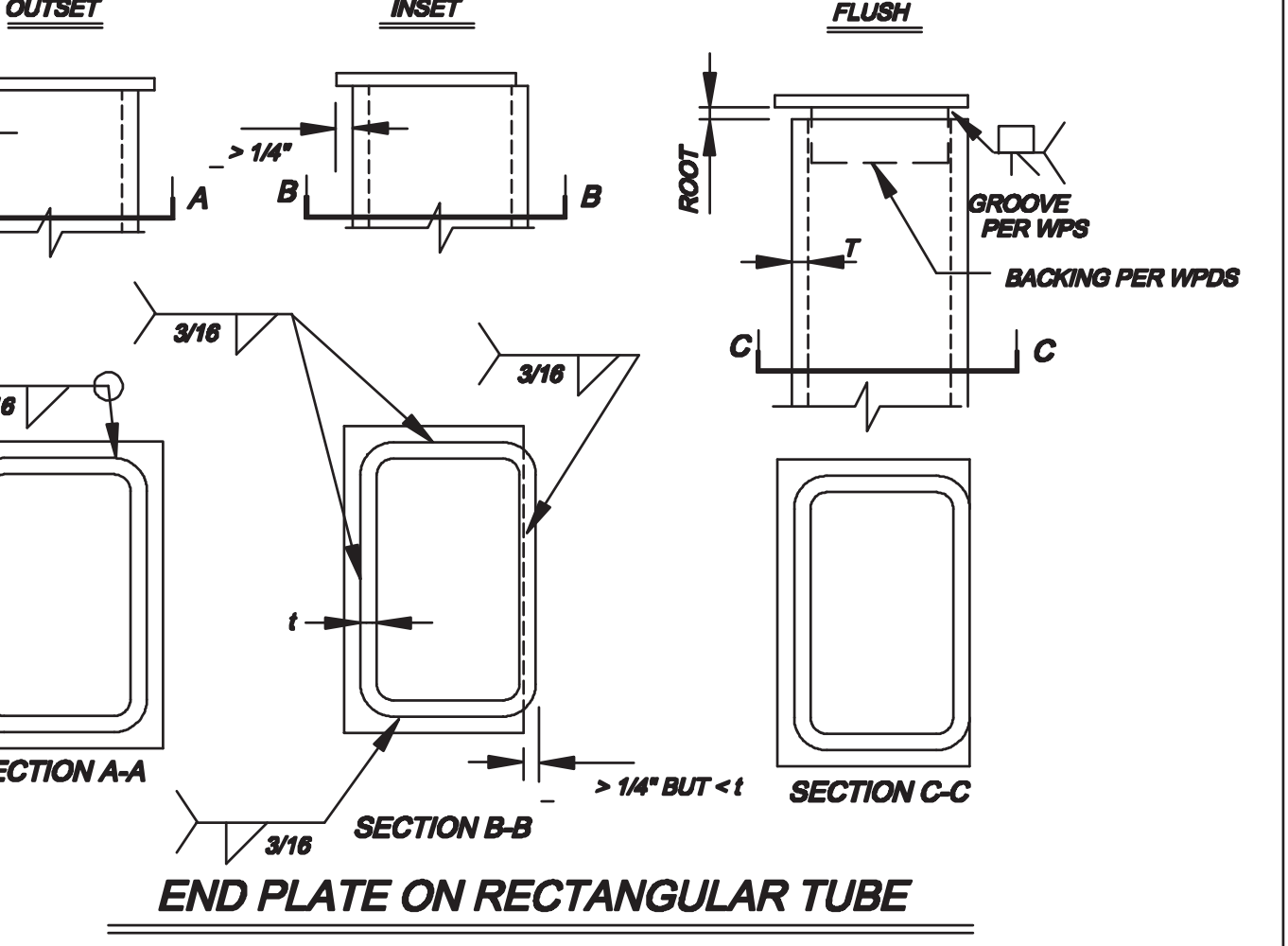
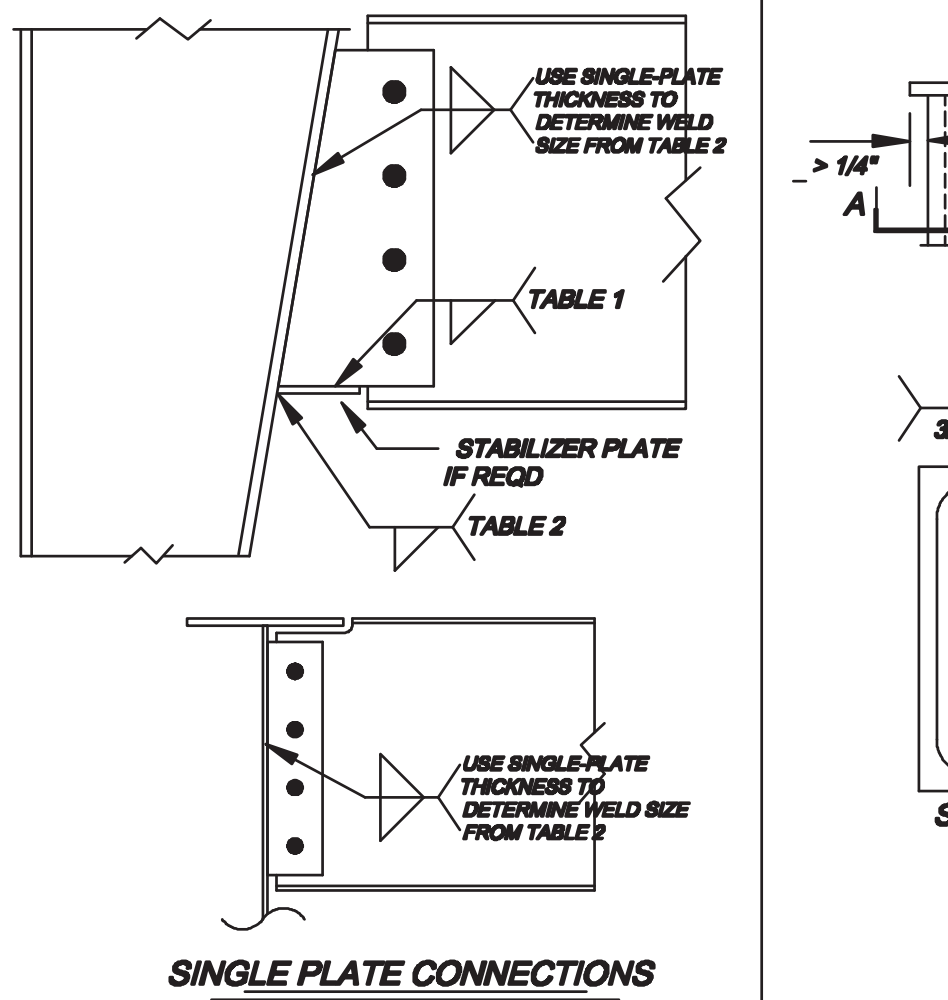
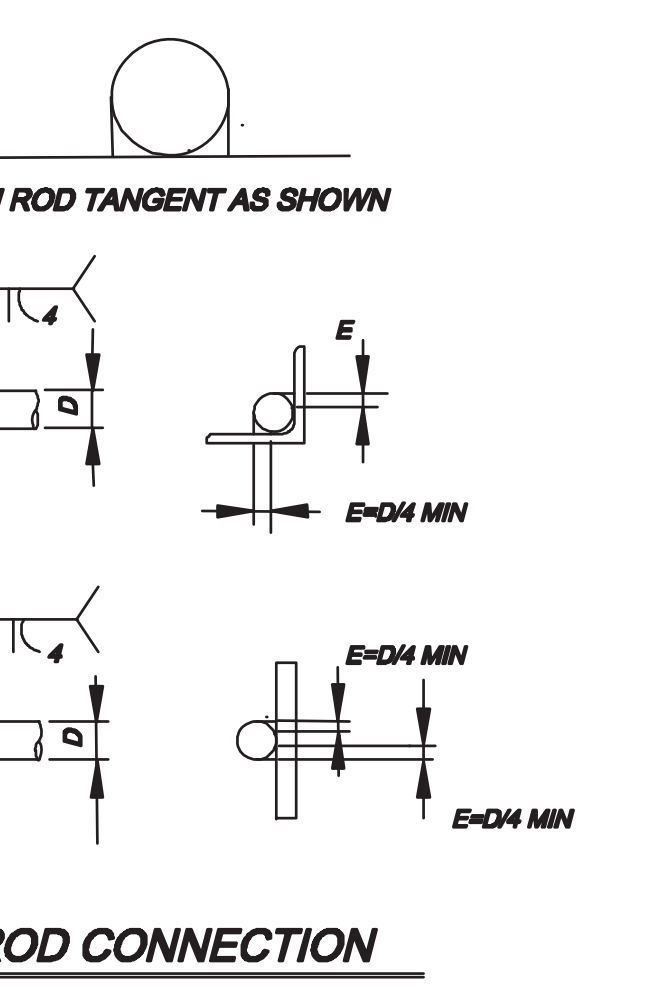
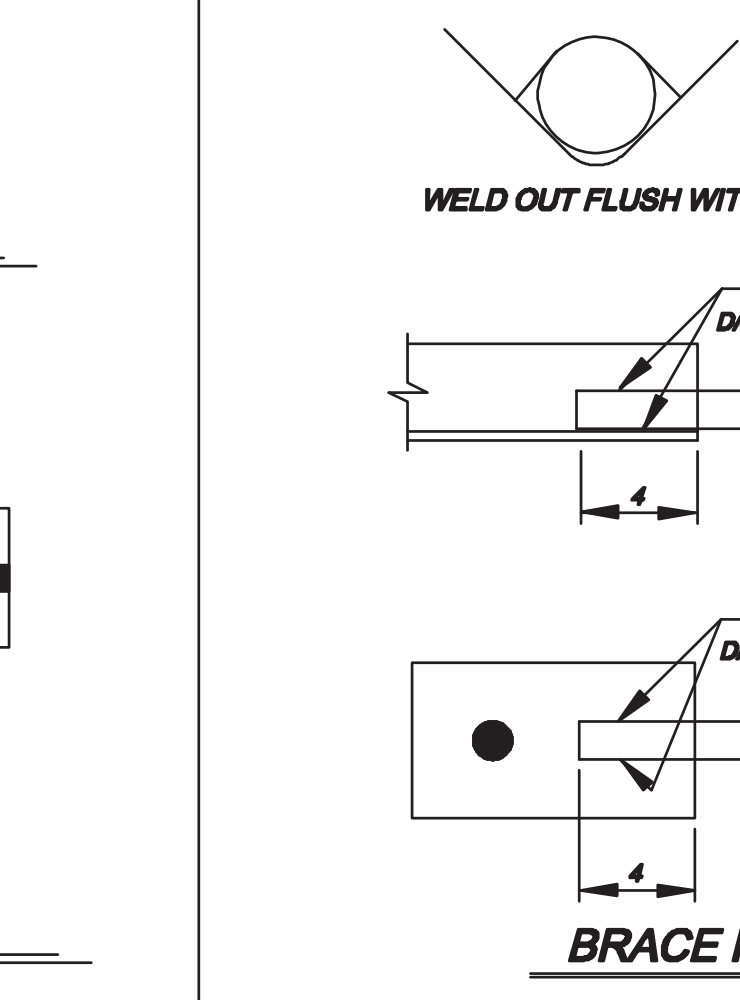
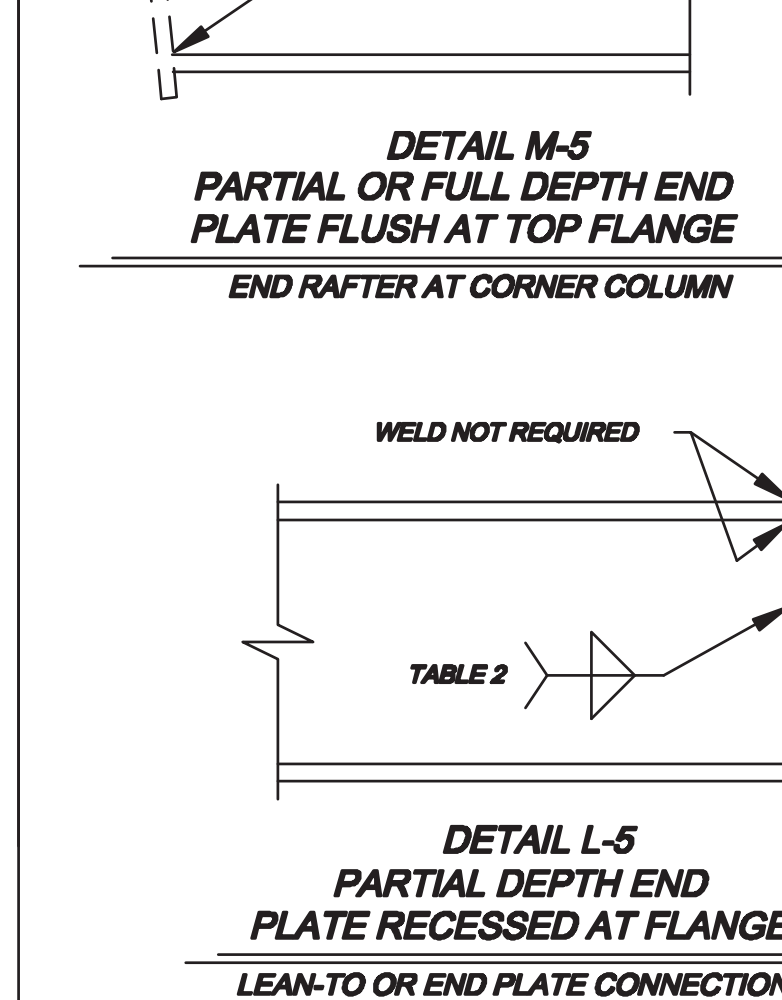
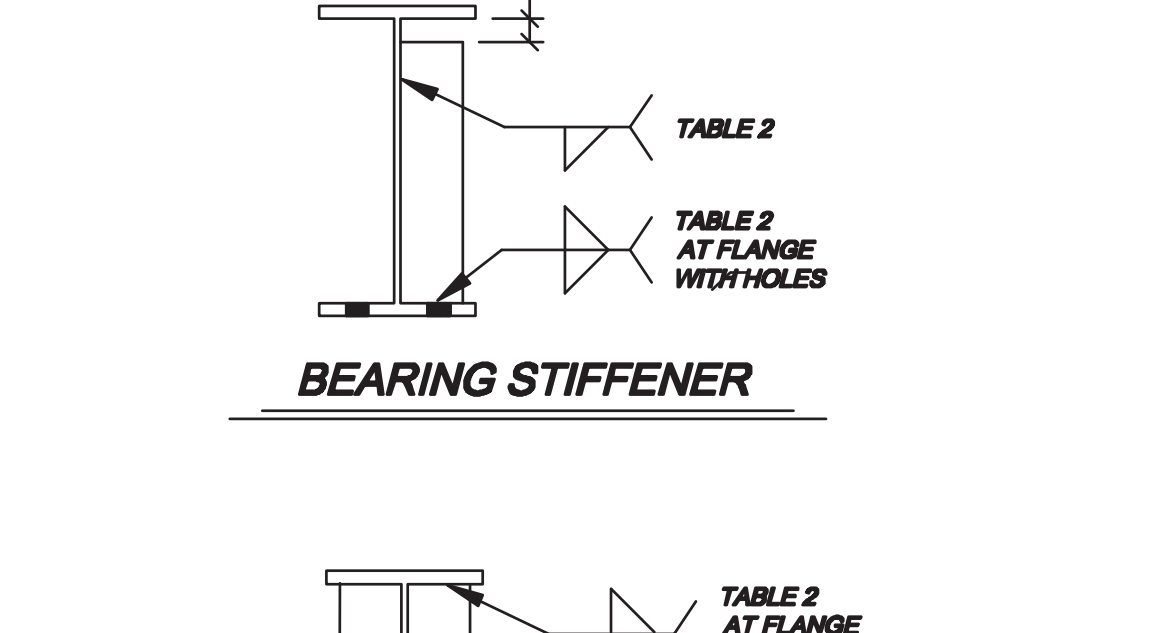
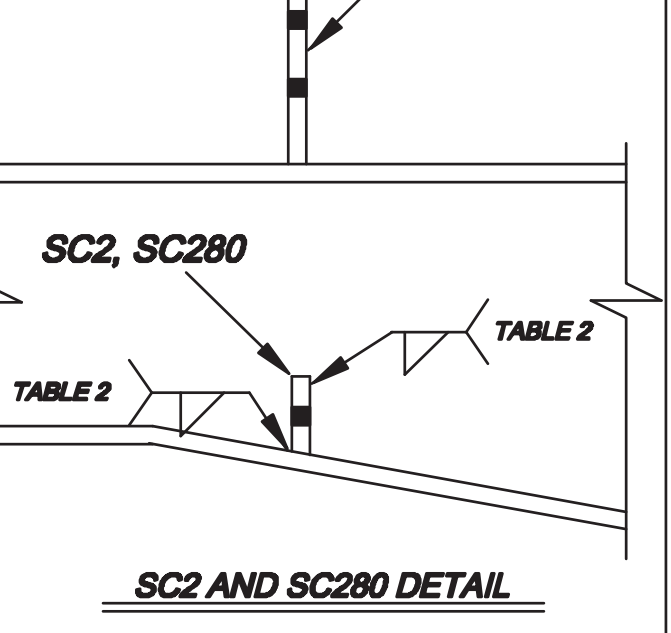
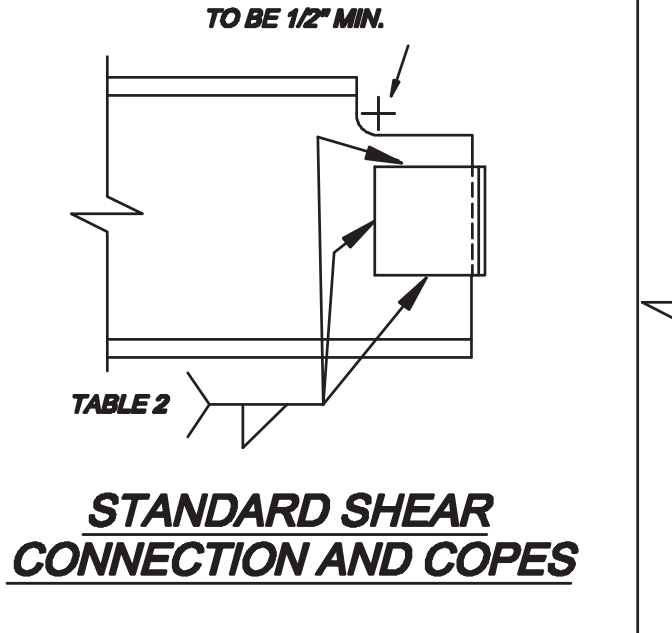
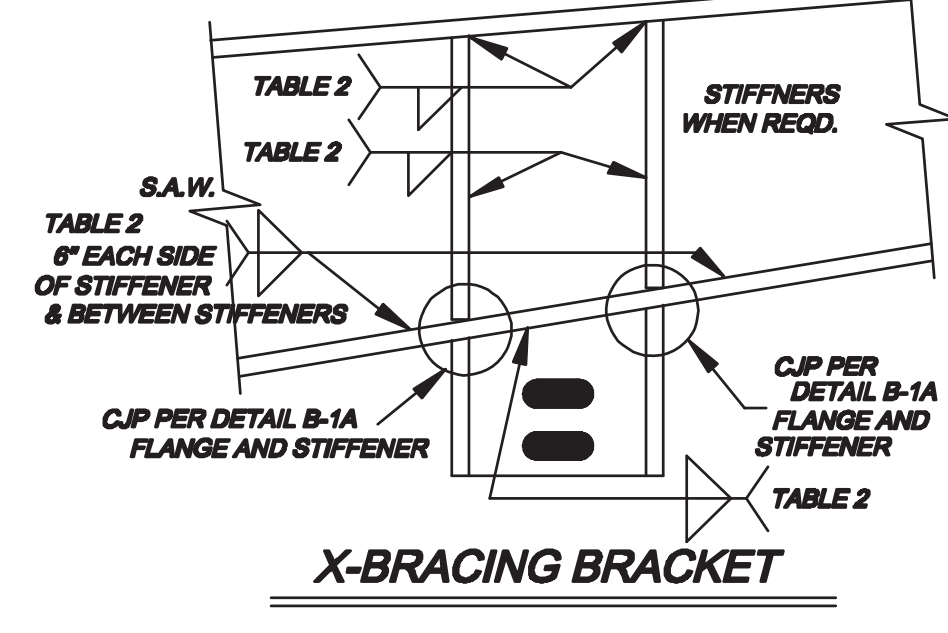
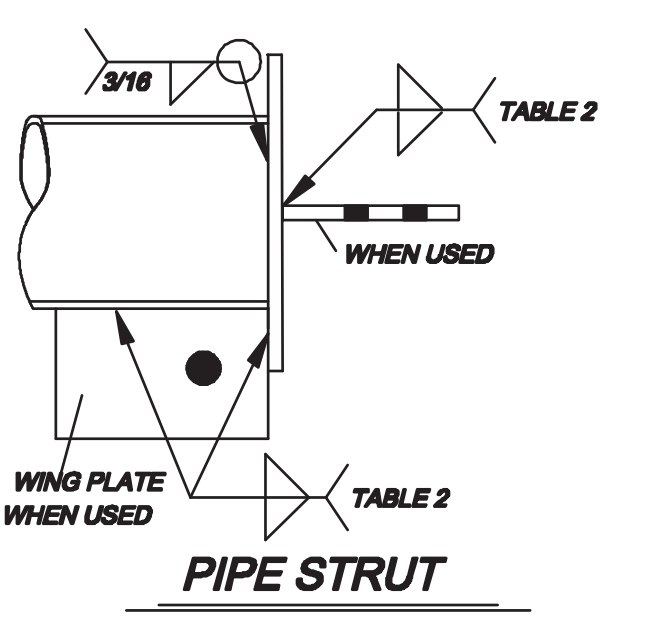
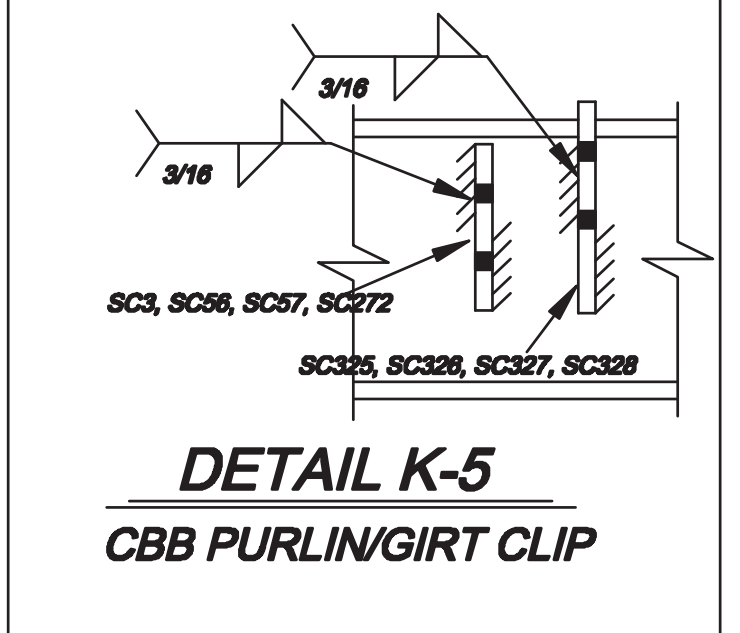
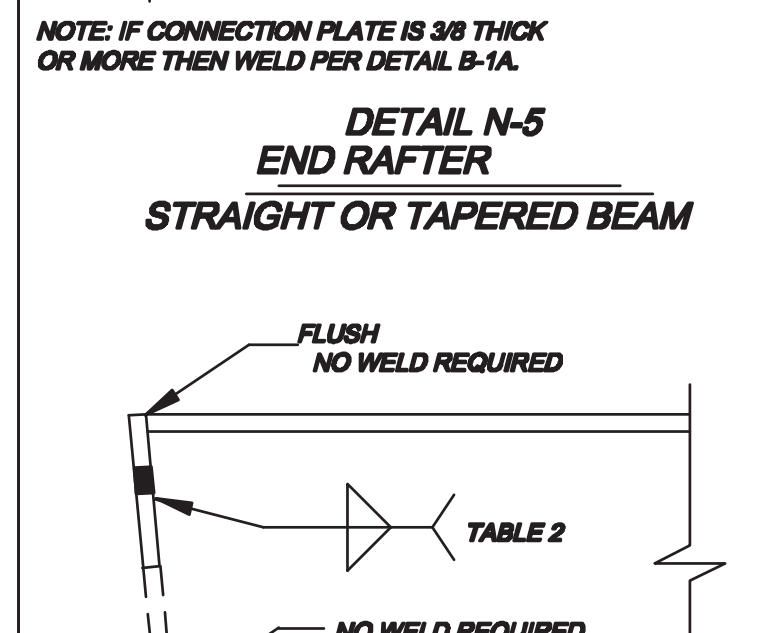
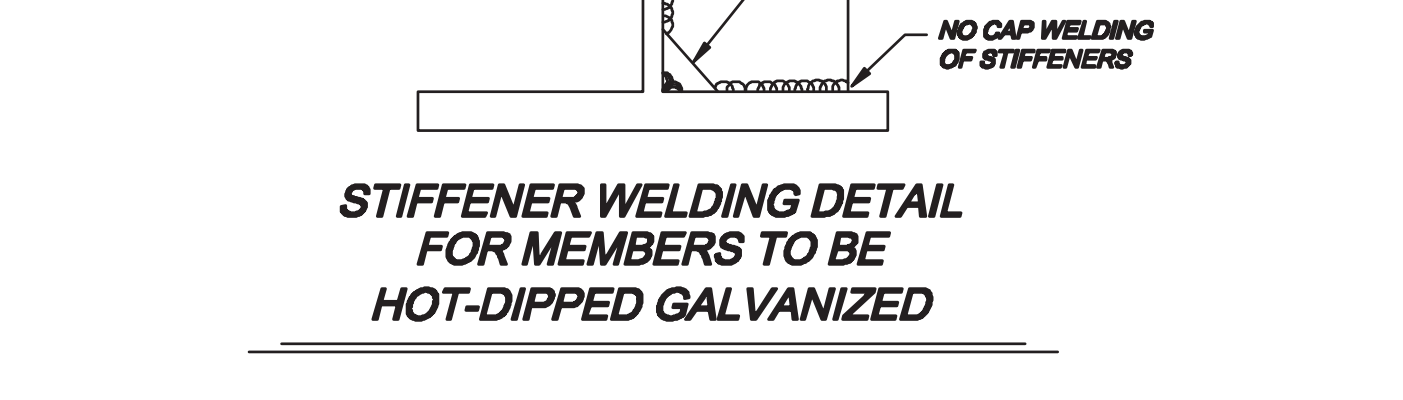
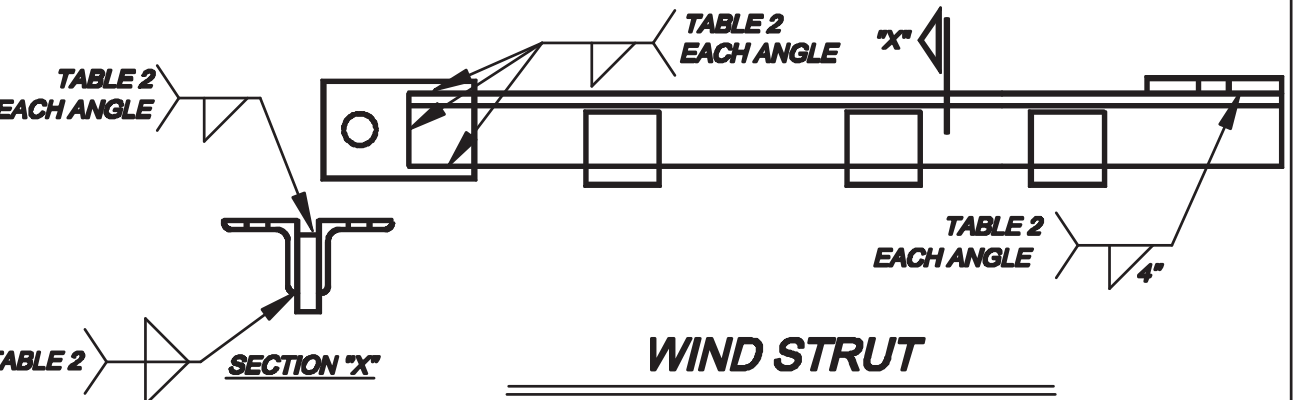
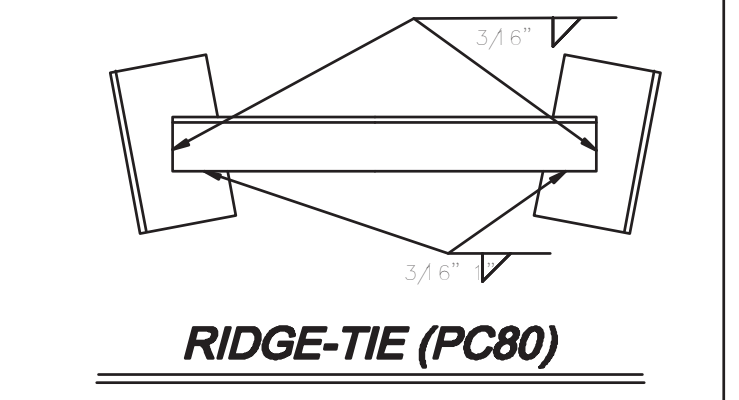
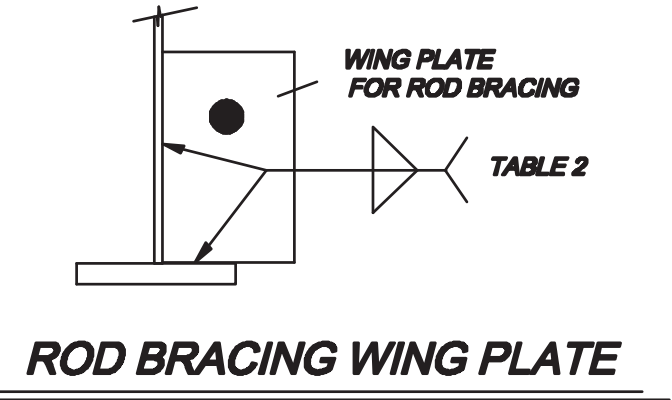
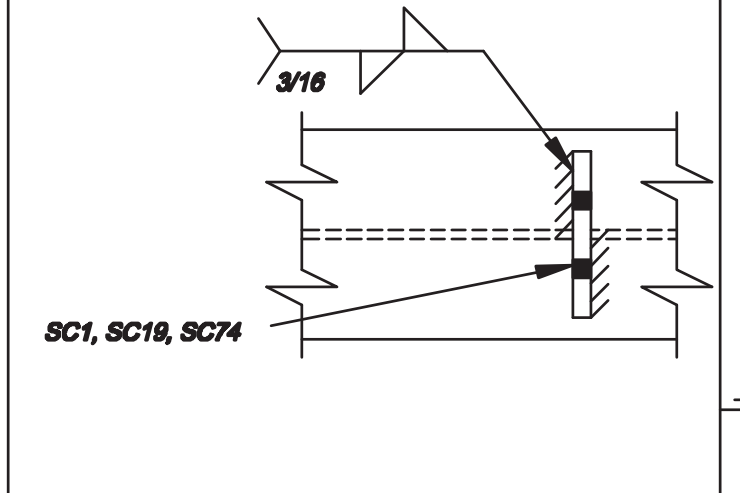
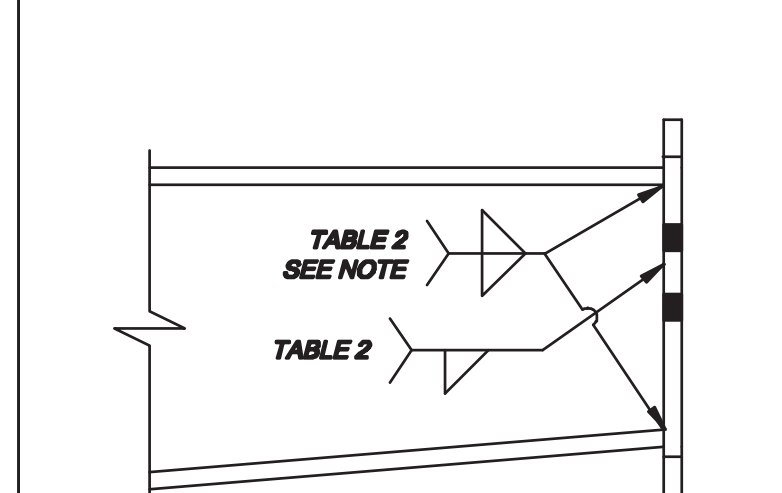
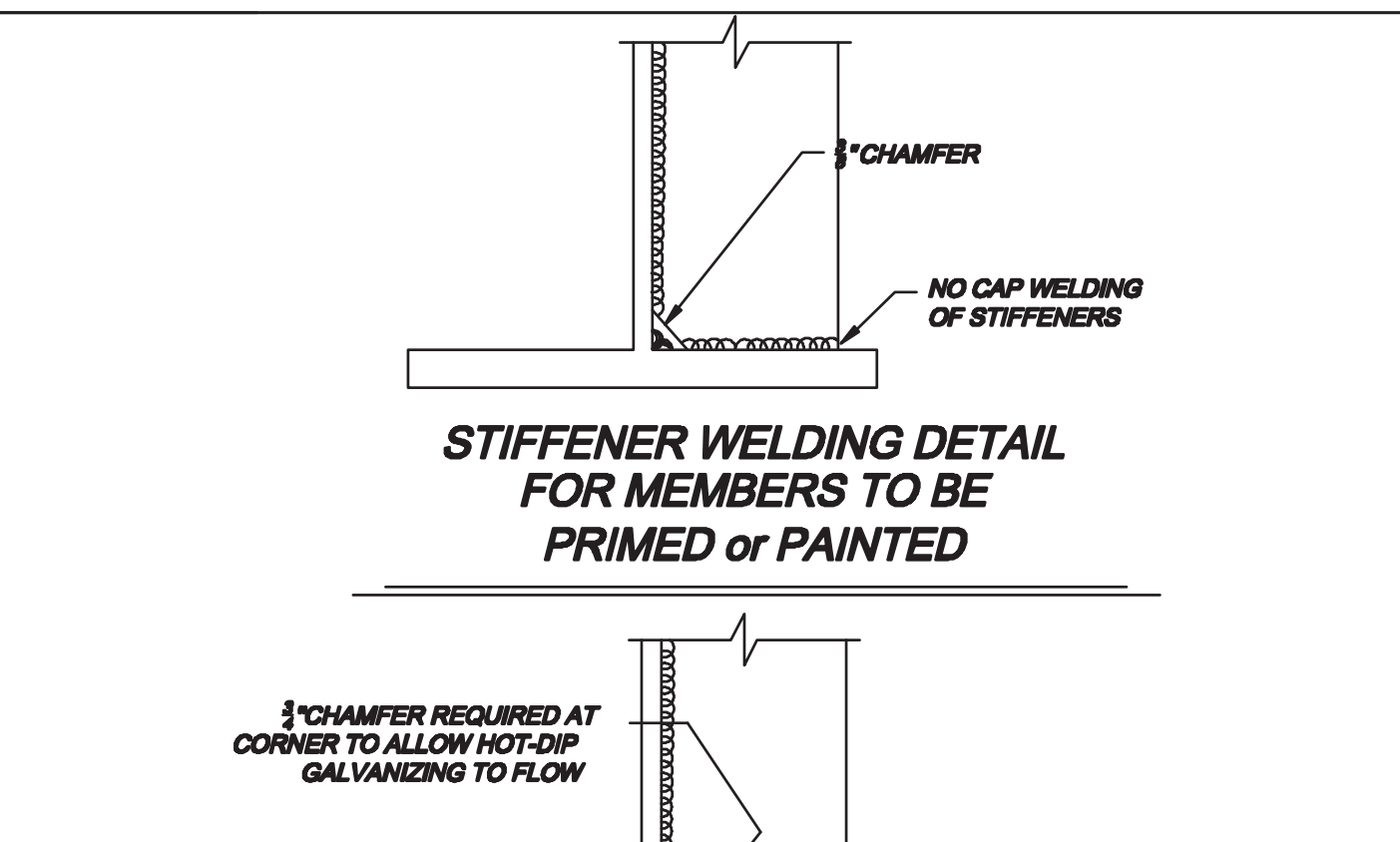
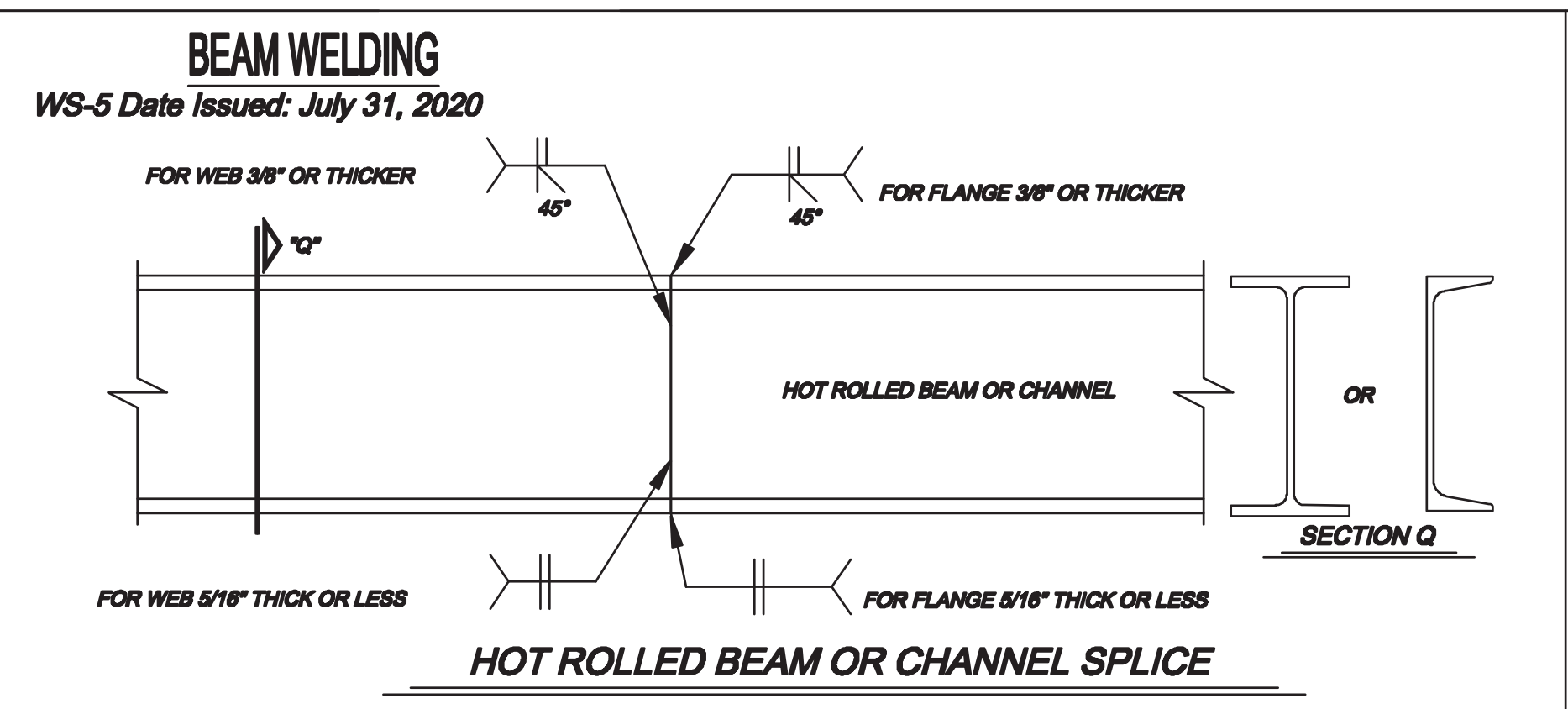
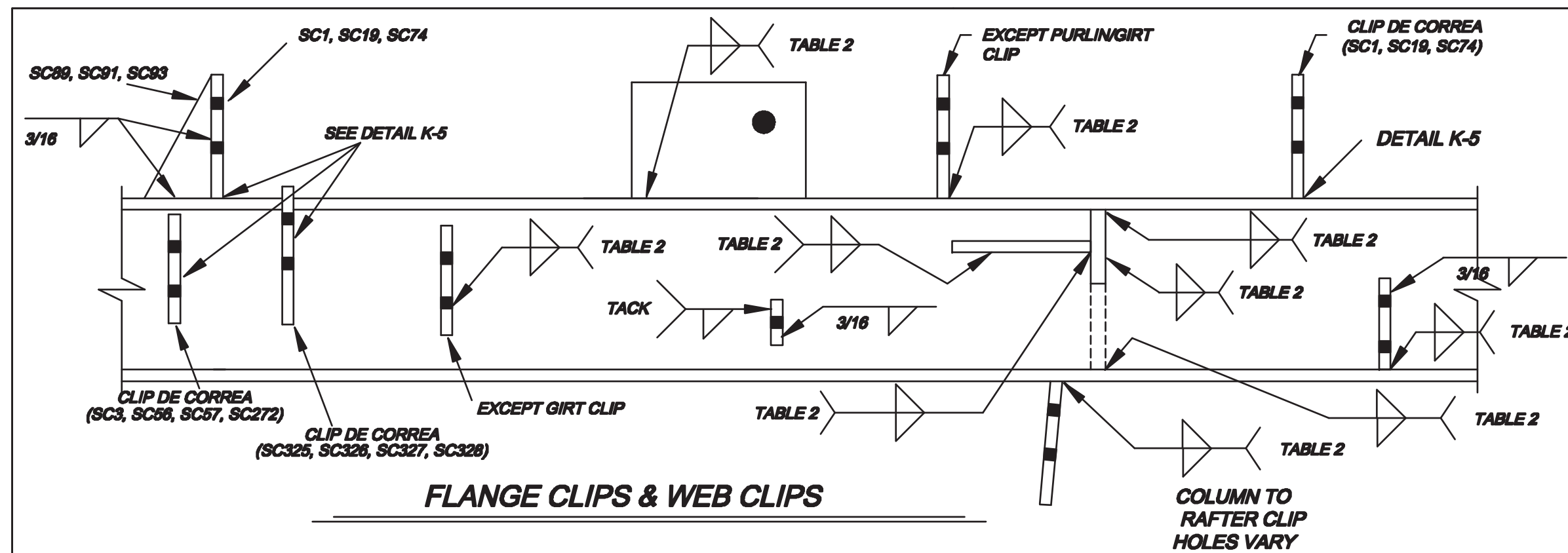


ISSUE	DATE	DESCRIPTION	BY	CHKD	DSN

PROJECT:							
CUSTOMER:							
OWNER:							
LOCATION:							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

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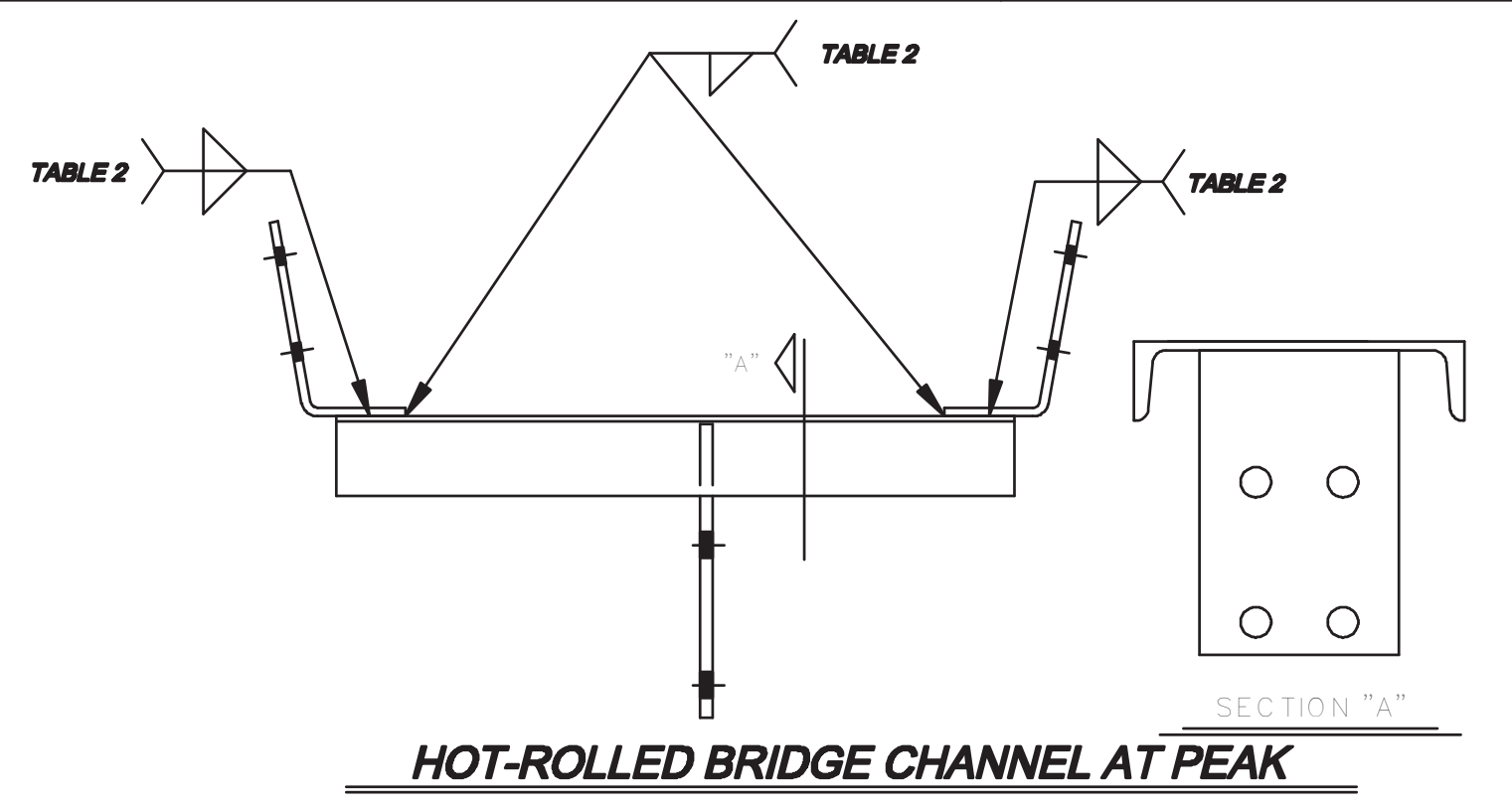
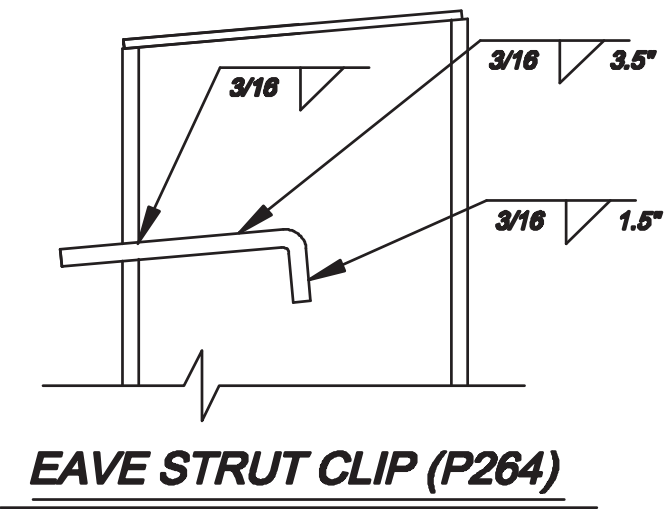


ISSUE	DATE	DESCRIPTION	BY	CHKD	DSN

PROJECT:		OWNER:					
CUSTOMER:							
LOCATION:							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
		N.T.S.					

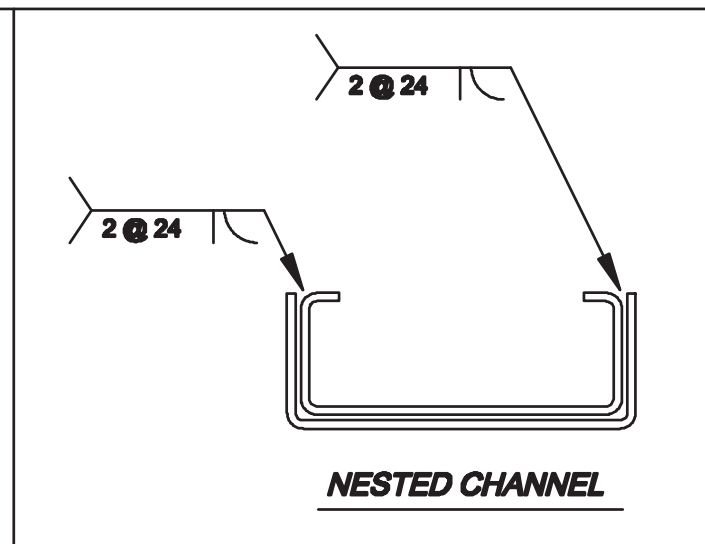
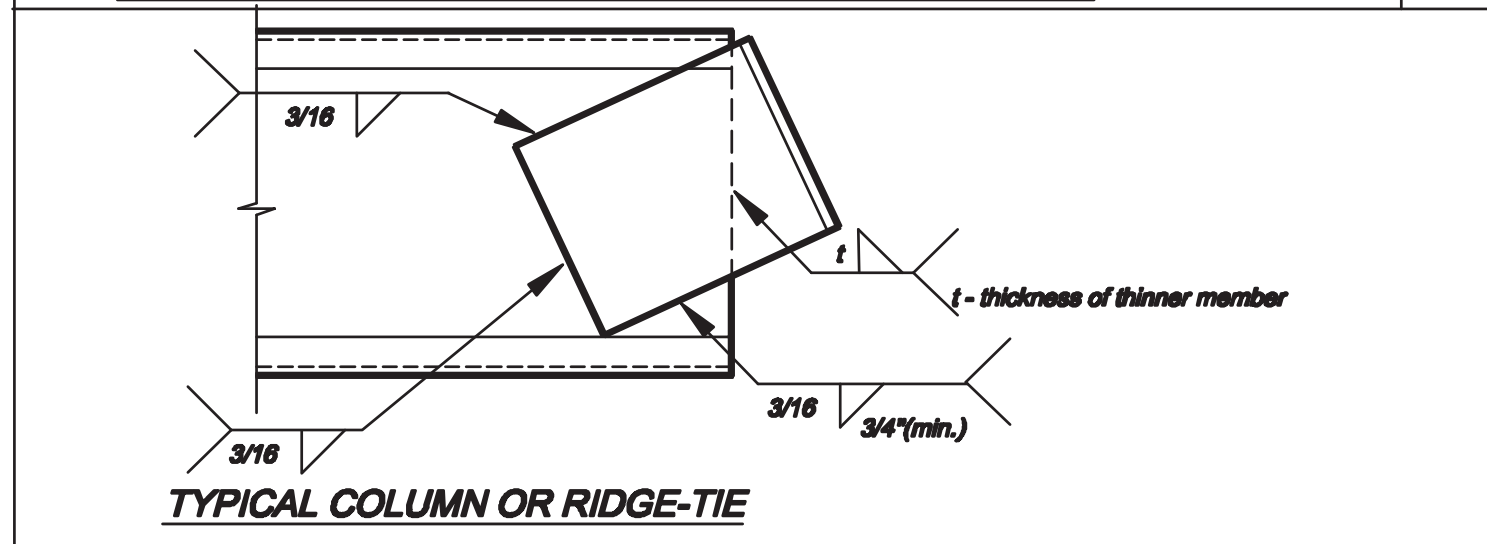
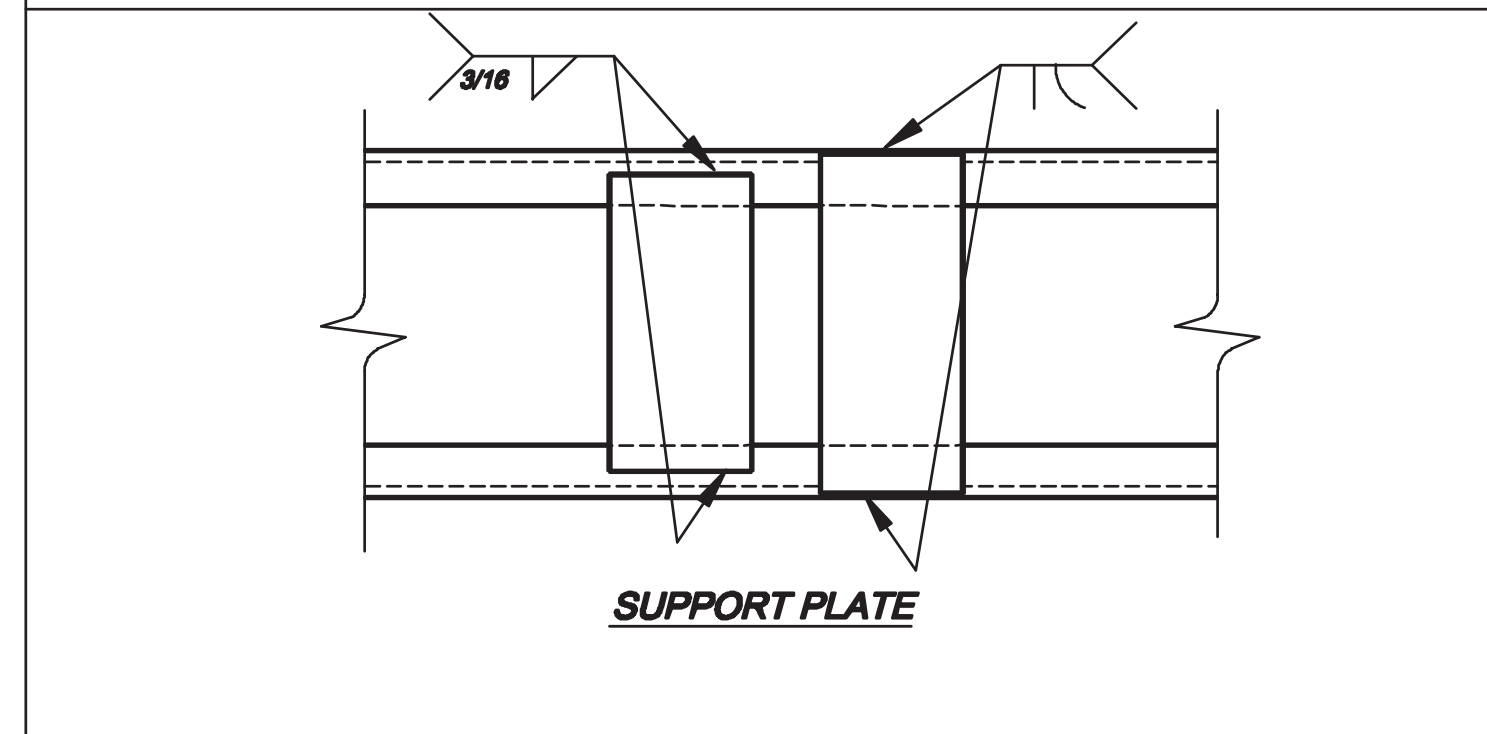
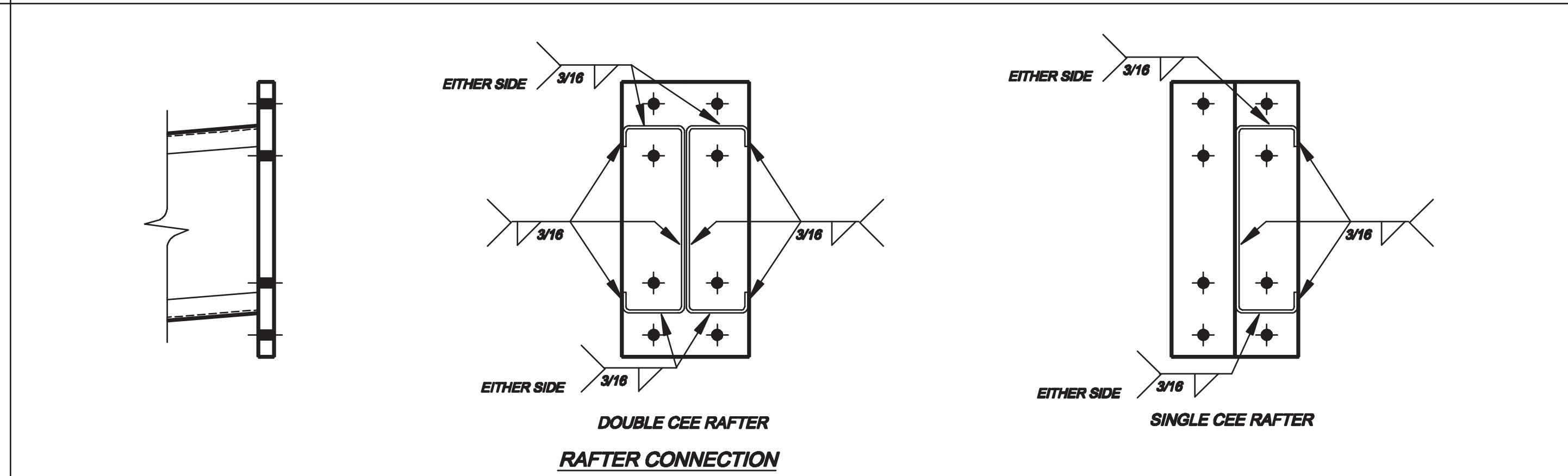
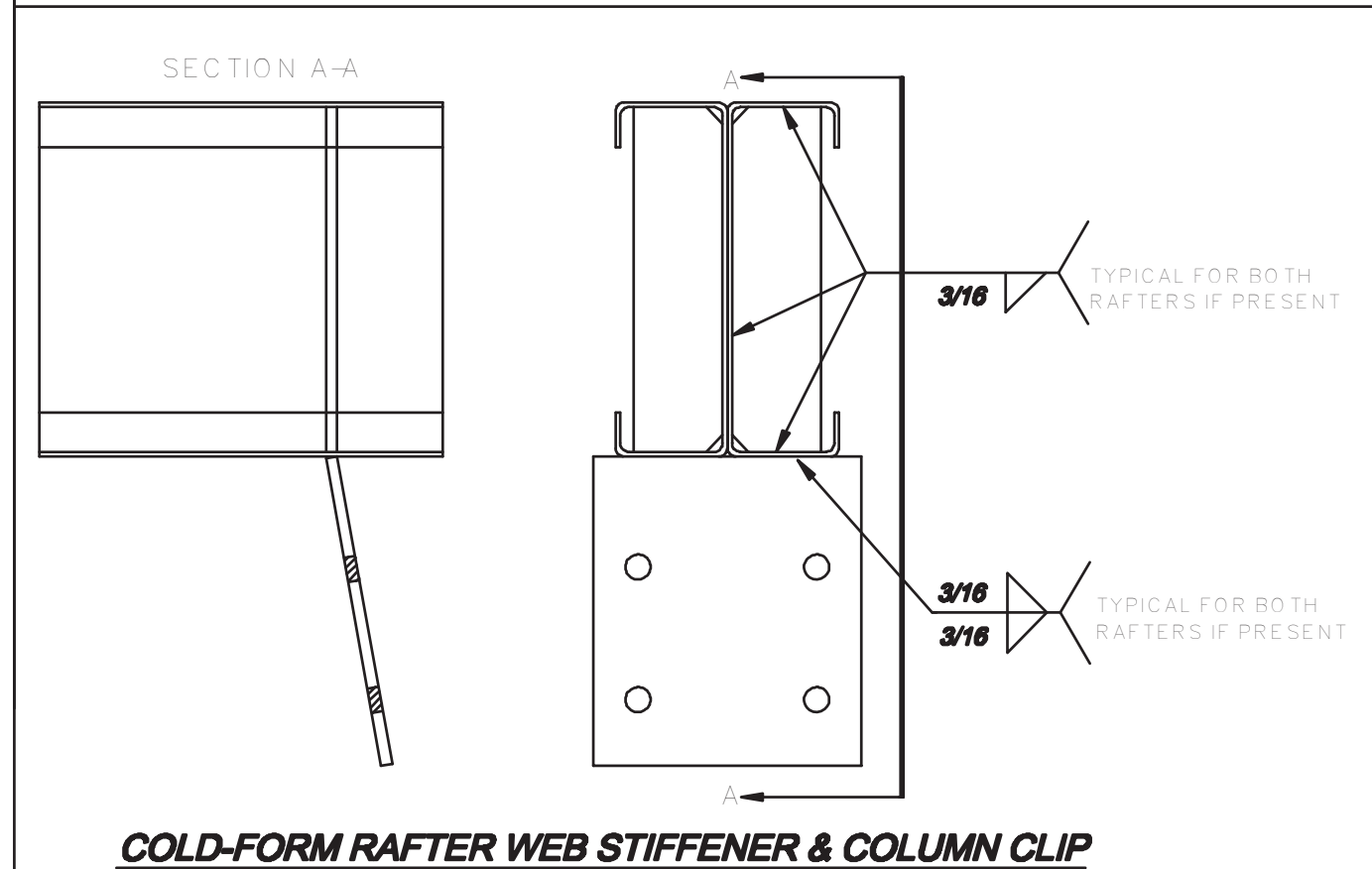
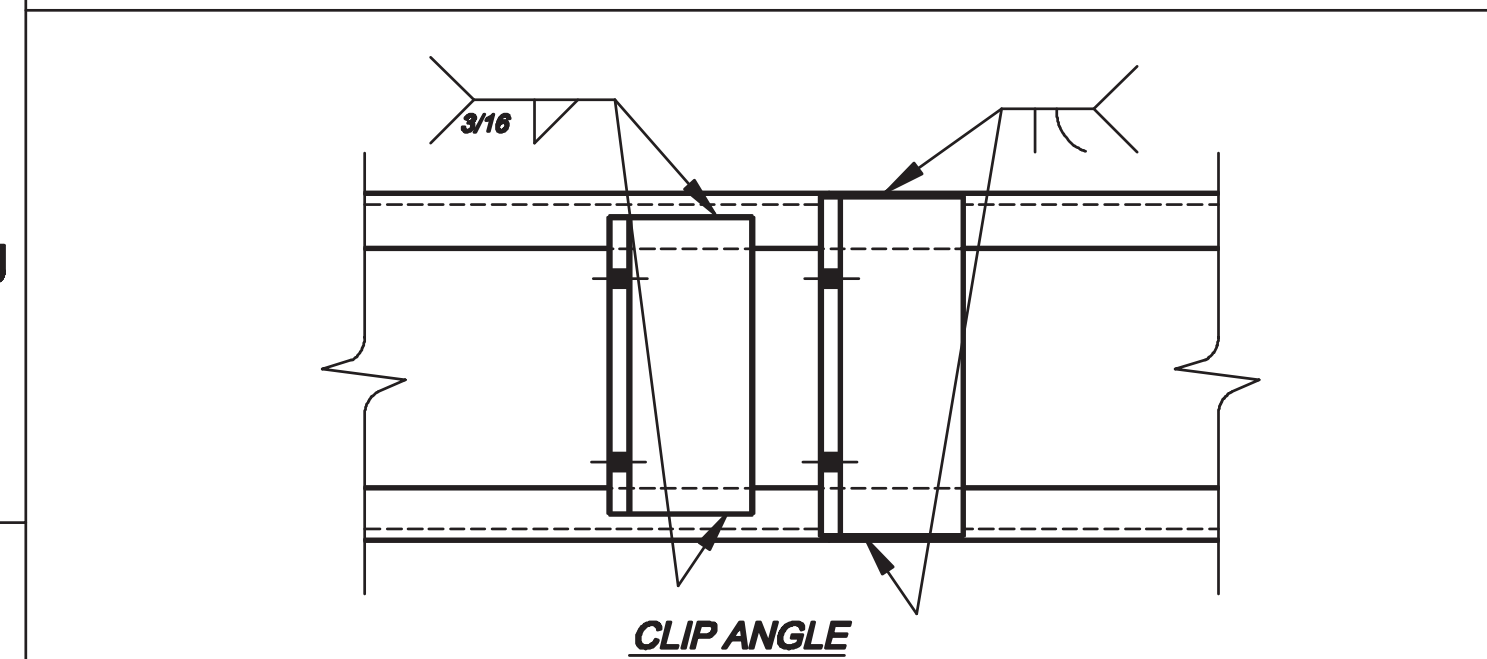
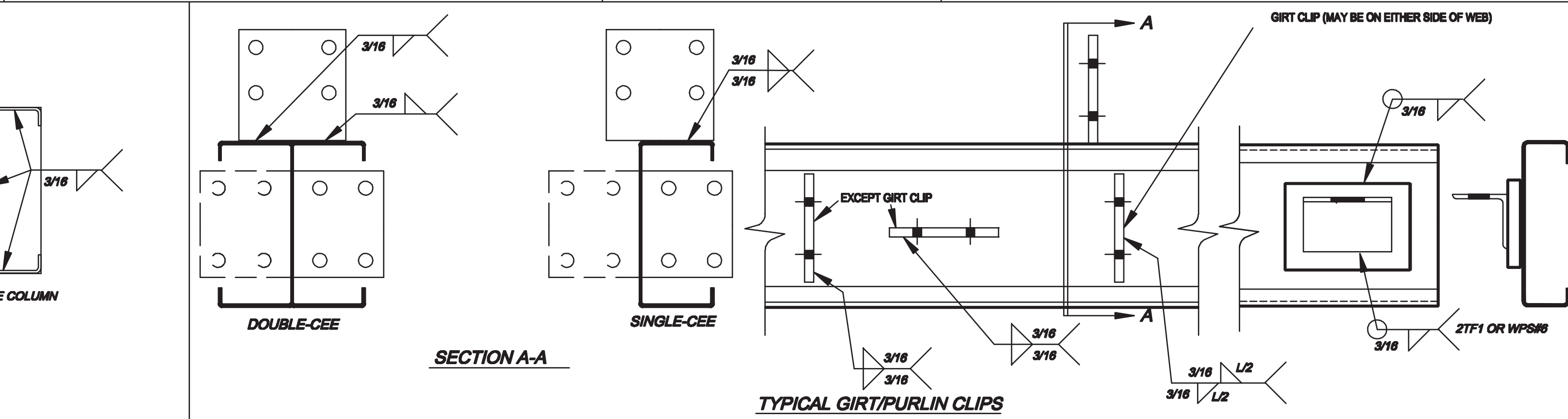
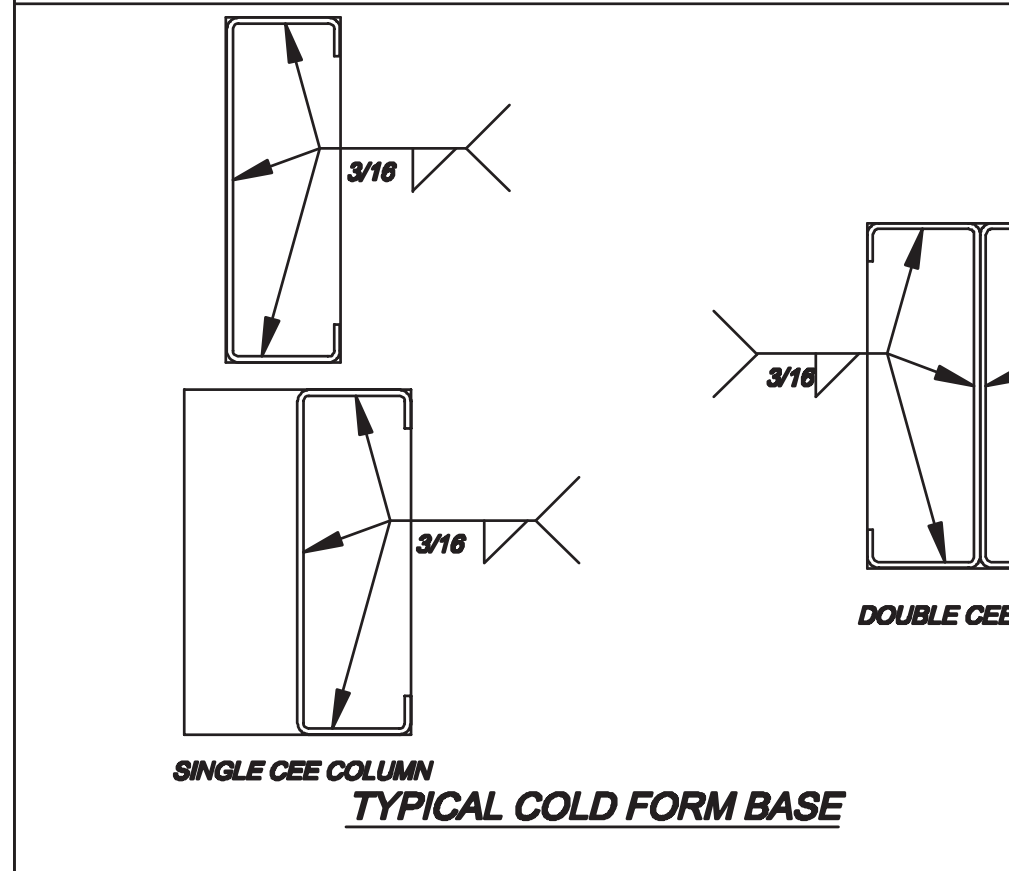
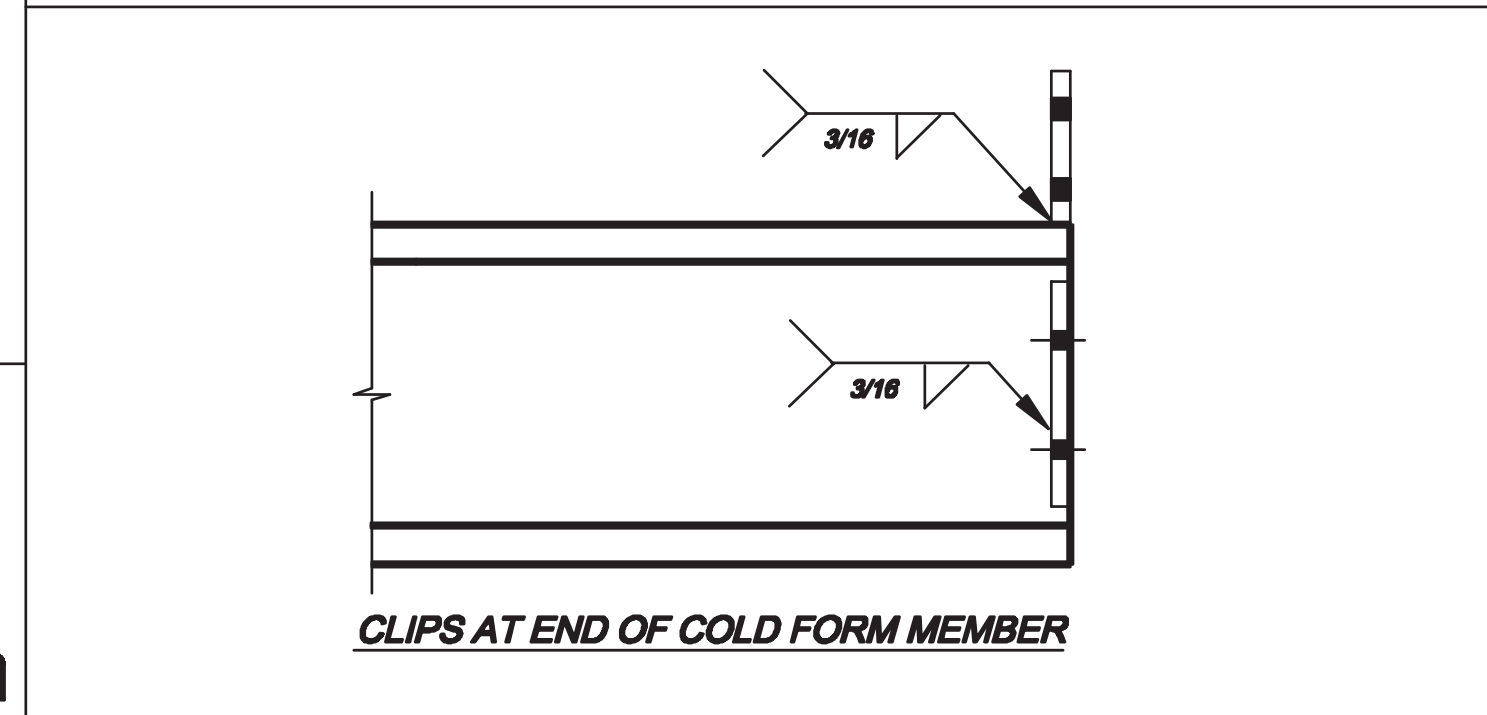
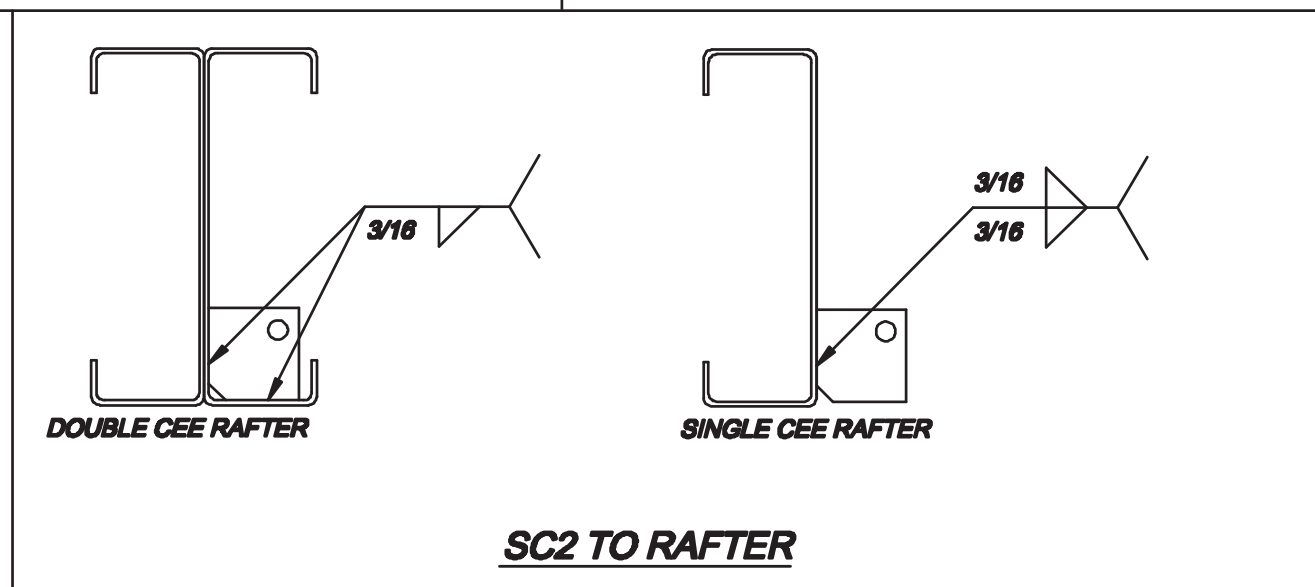
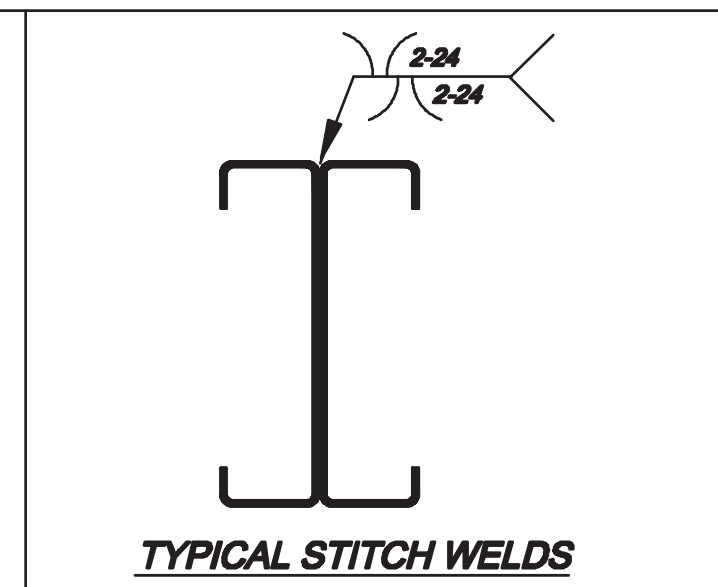
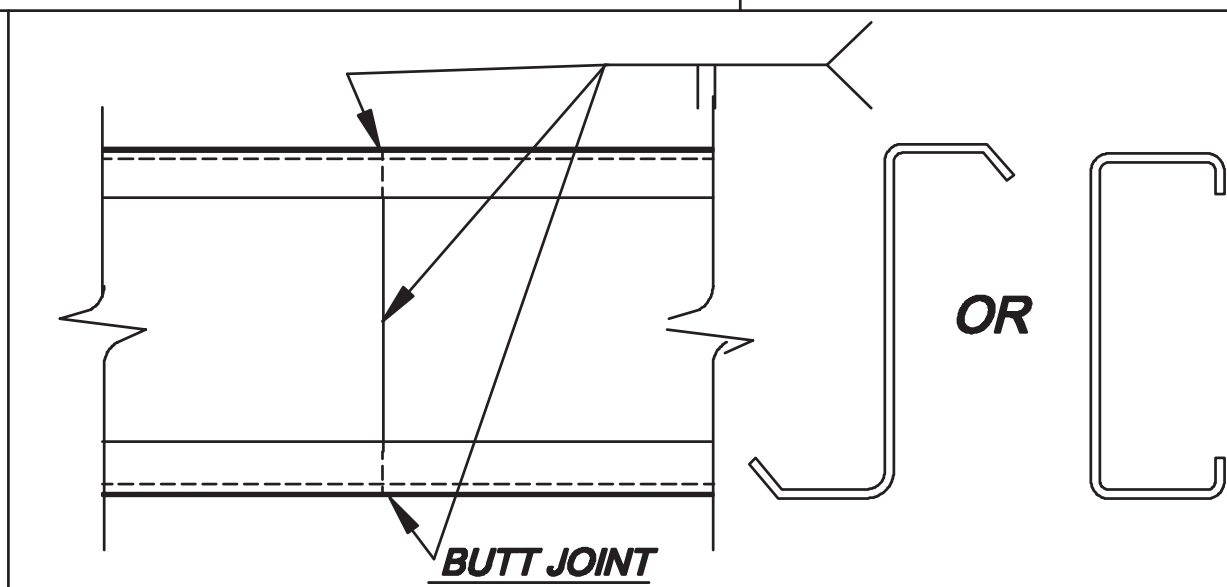
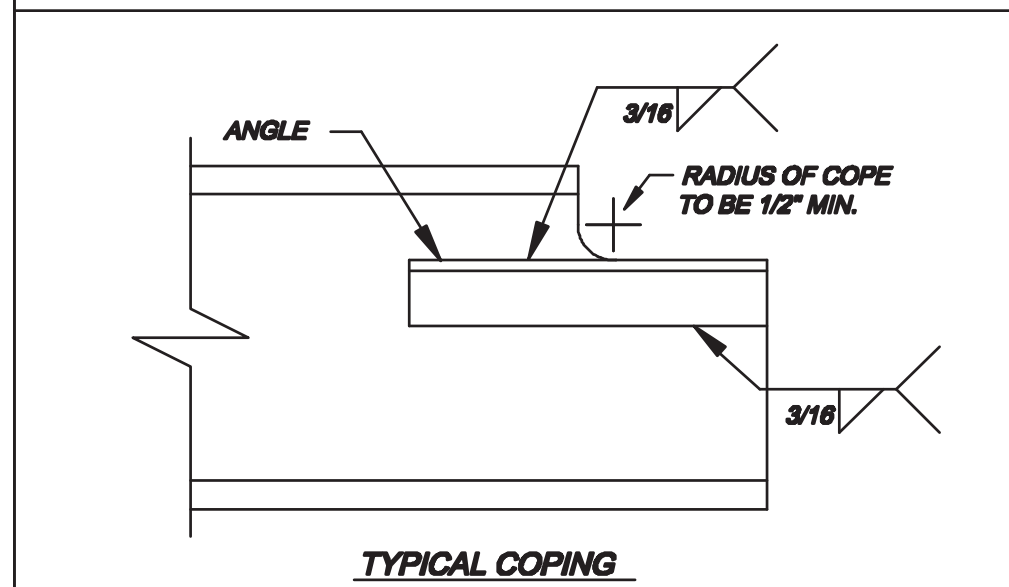
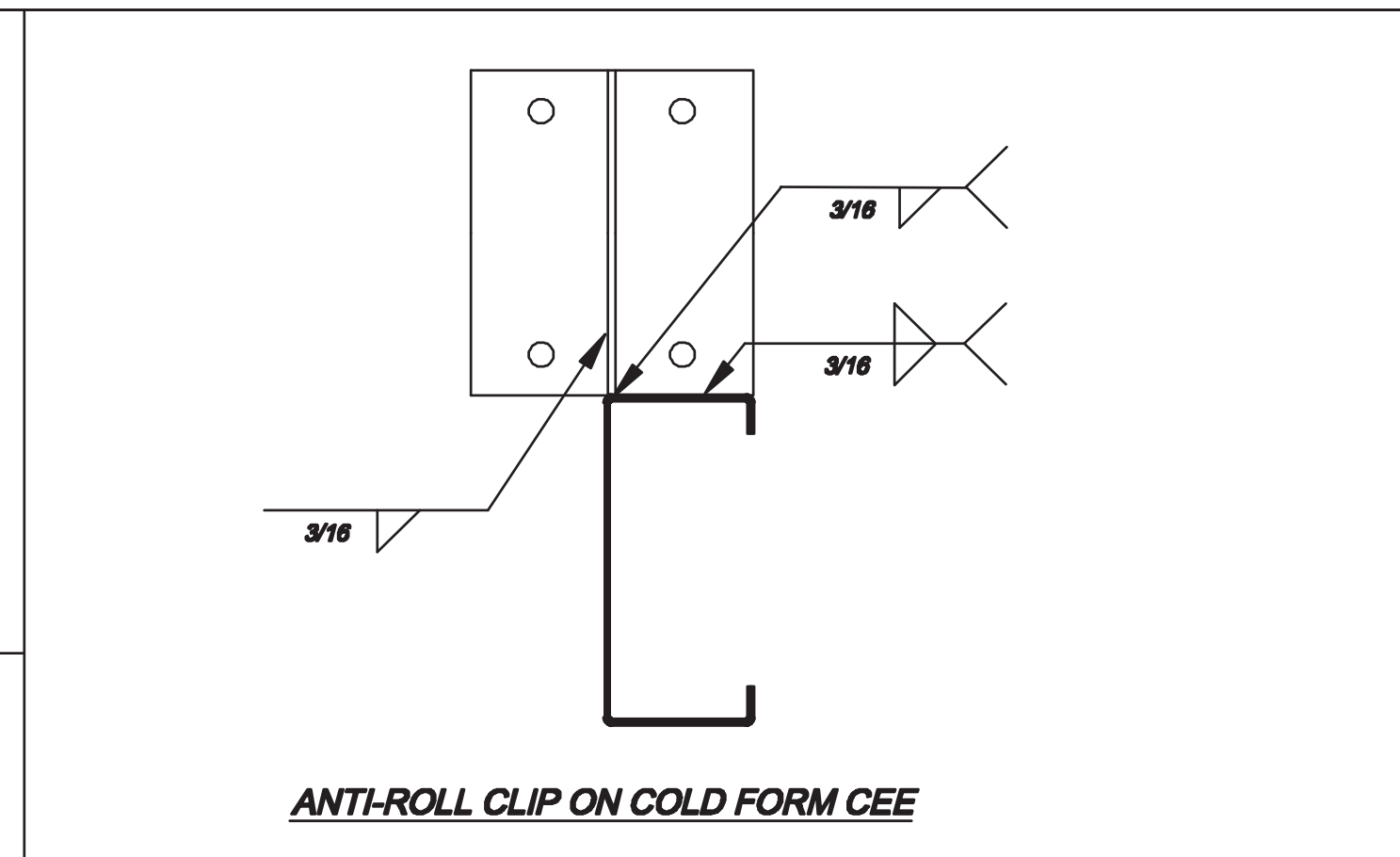
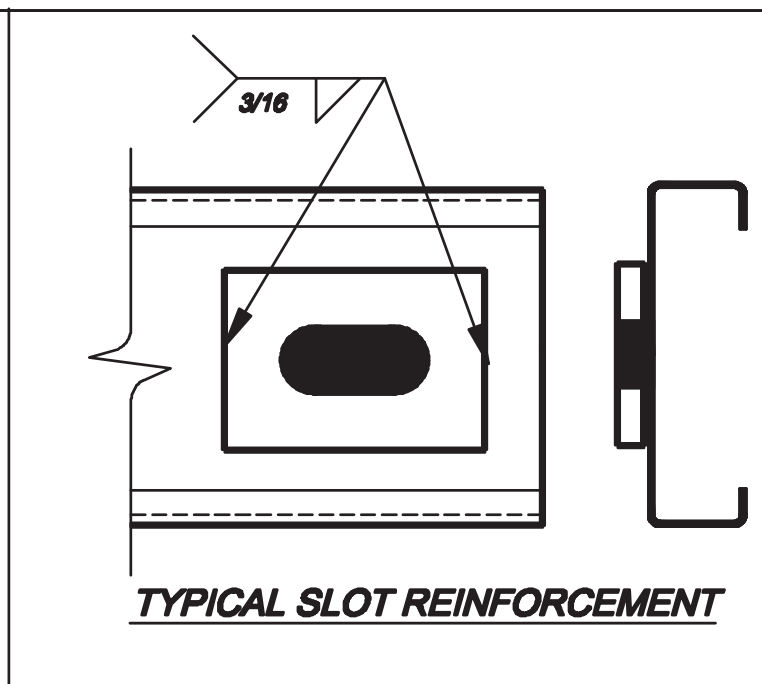
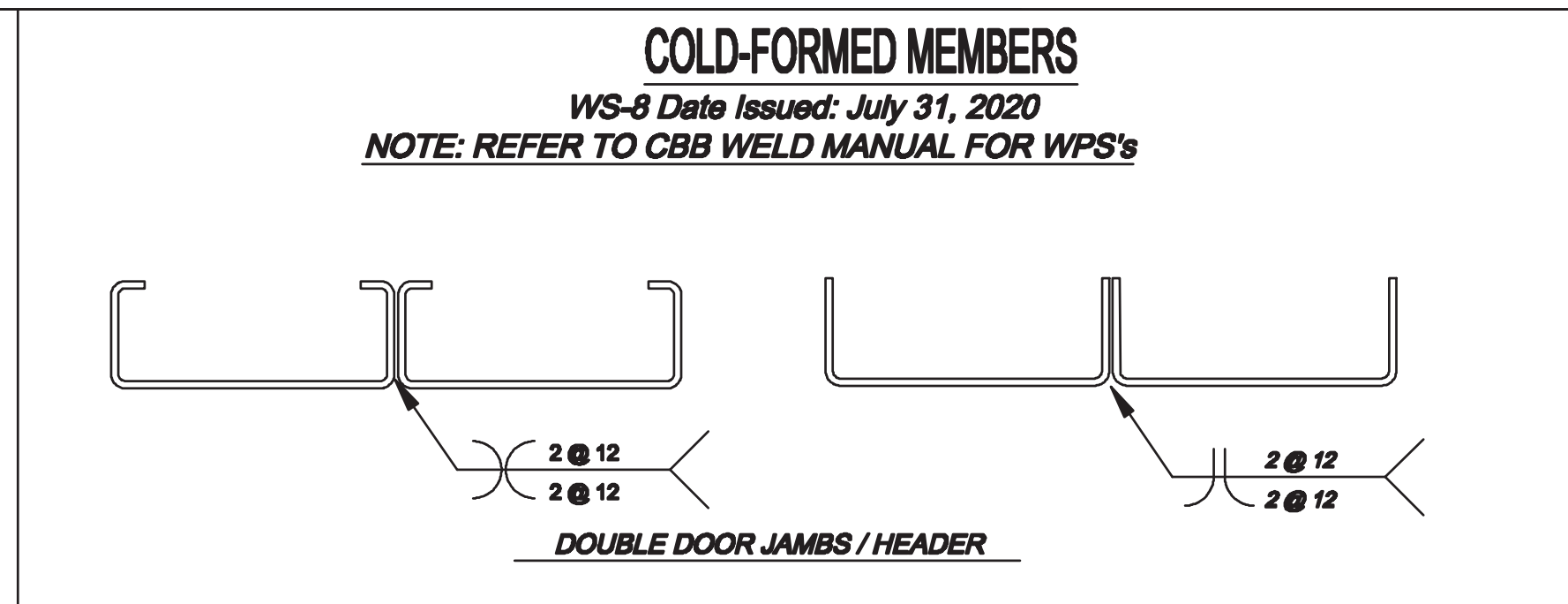
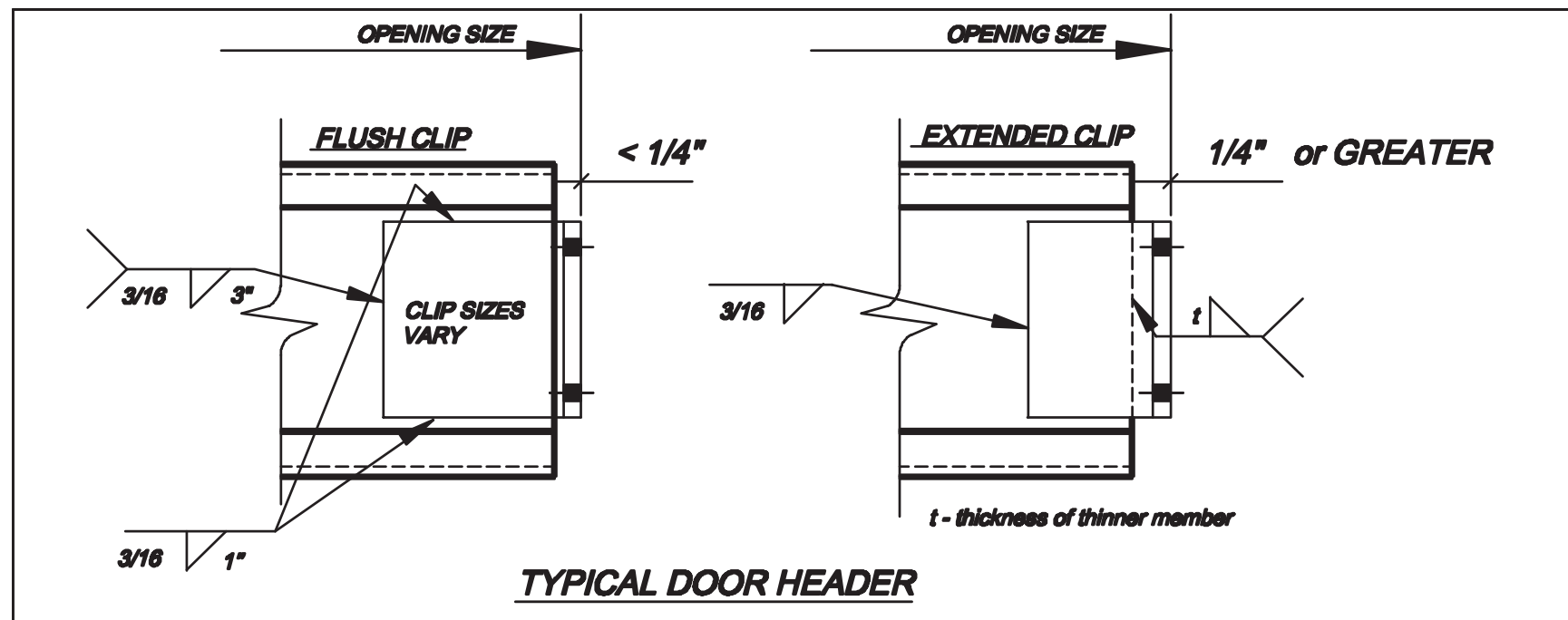
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**BEAM WELDING-2**  
 WS-5A Date Issued: July 31, 2020



ISSUE	DATE	DESCRIPTION	BY	CHK'D	DSN								
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						CUSTOMER:				OWNER:			
						LOCATION:							
						CAD	DATE	SCALE N.T.S.	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE

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ISSUE	DATE	DESCRIPTION	BY	CKD	DSN

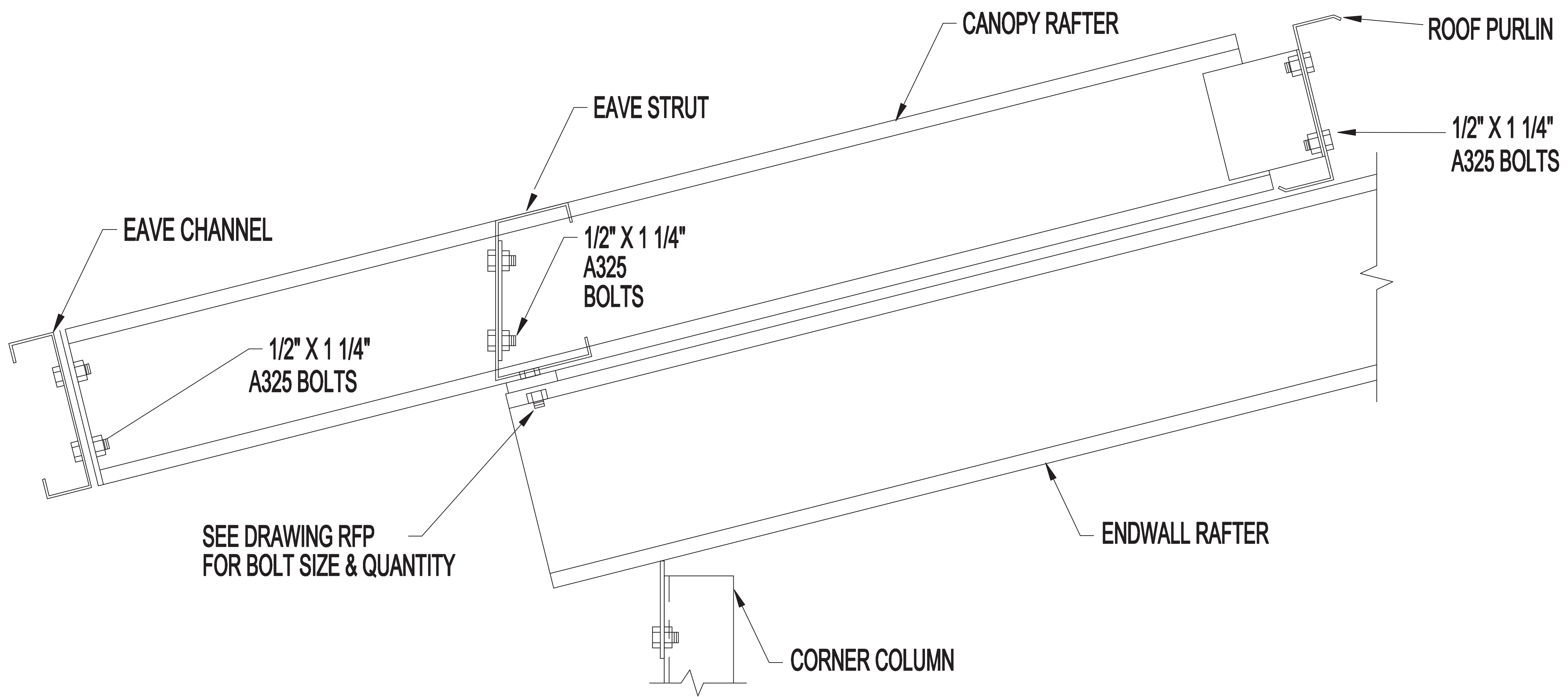
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CUSTOMER:								OWNER:	
LOCATION:									
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE		
		N.T.S.							

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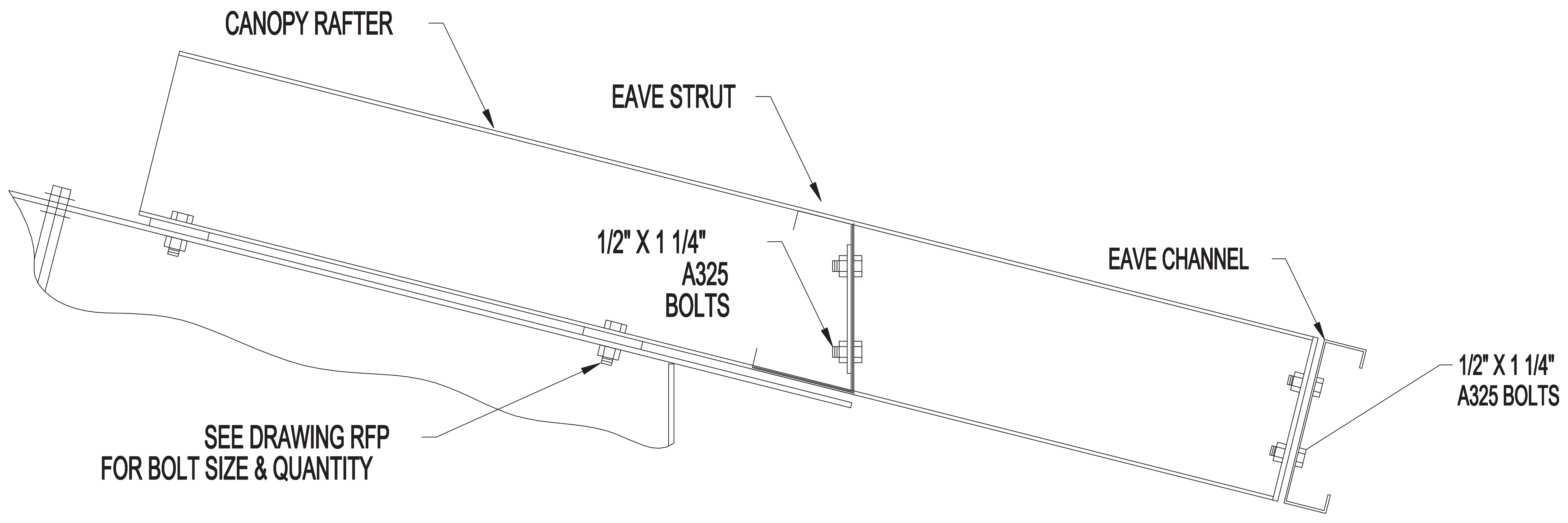






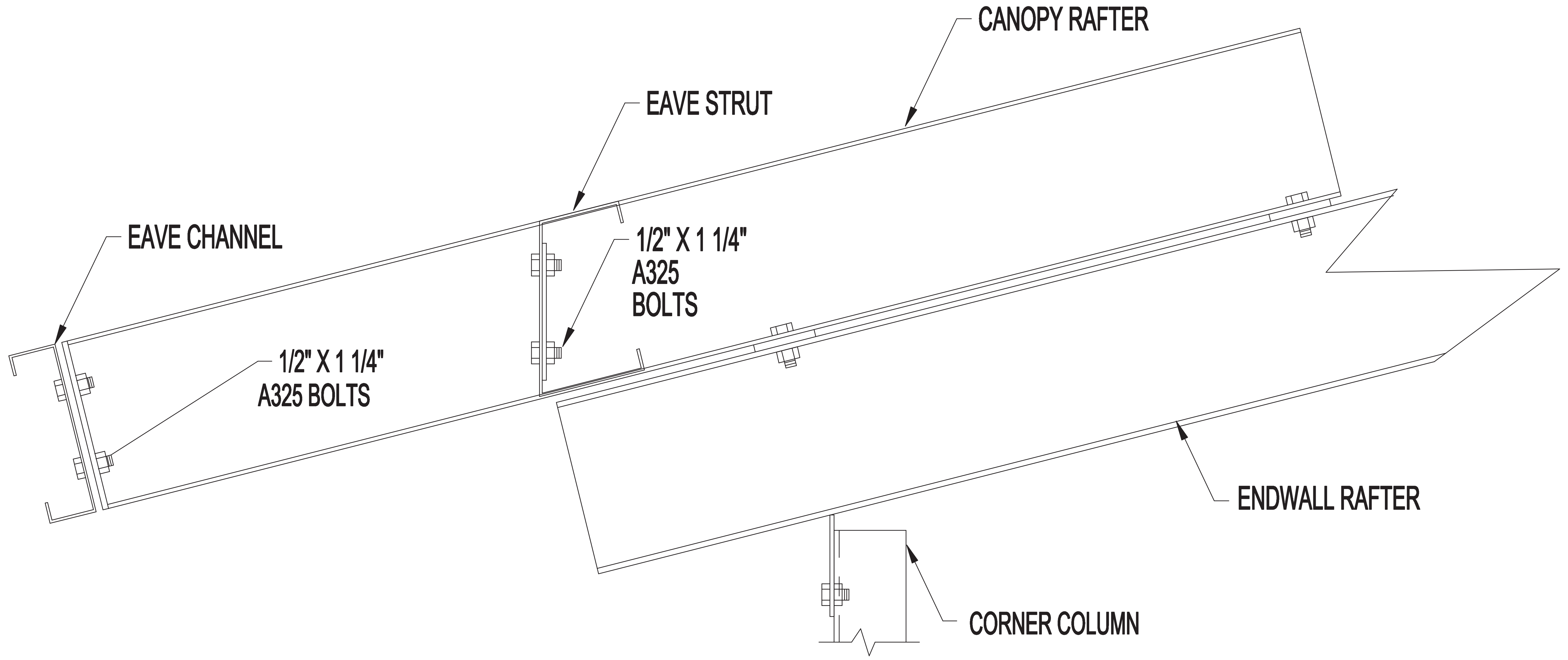
**EAVE CANOPY AT "C" ENDWALL RAFTER**

X2



EAVE CANOPY AT MAIN FRAME

X3



EAVE CANOPY AT HOT ROLLED ENDWALL RAFTER

X4

EXTENSION2

Beam12

Beam11

Beam10

Beam9

Beam8

Beam7

Beam1

Beam2

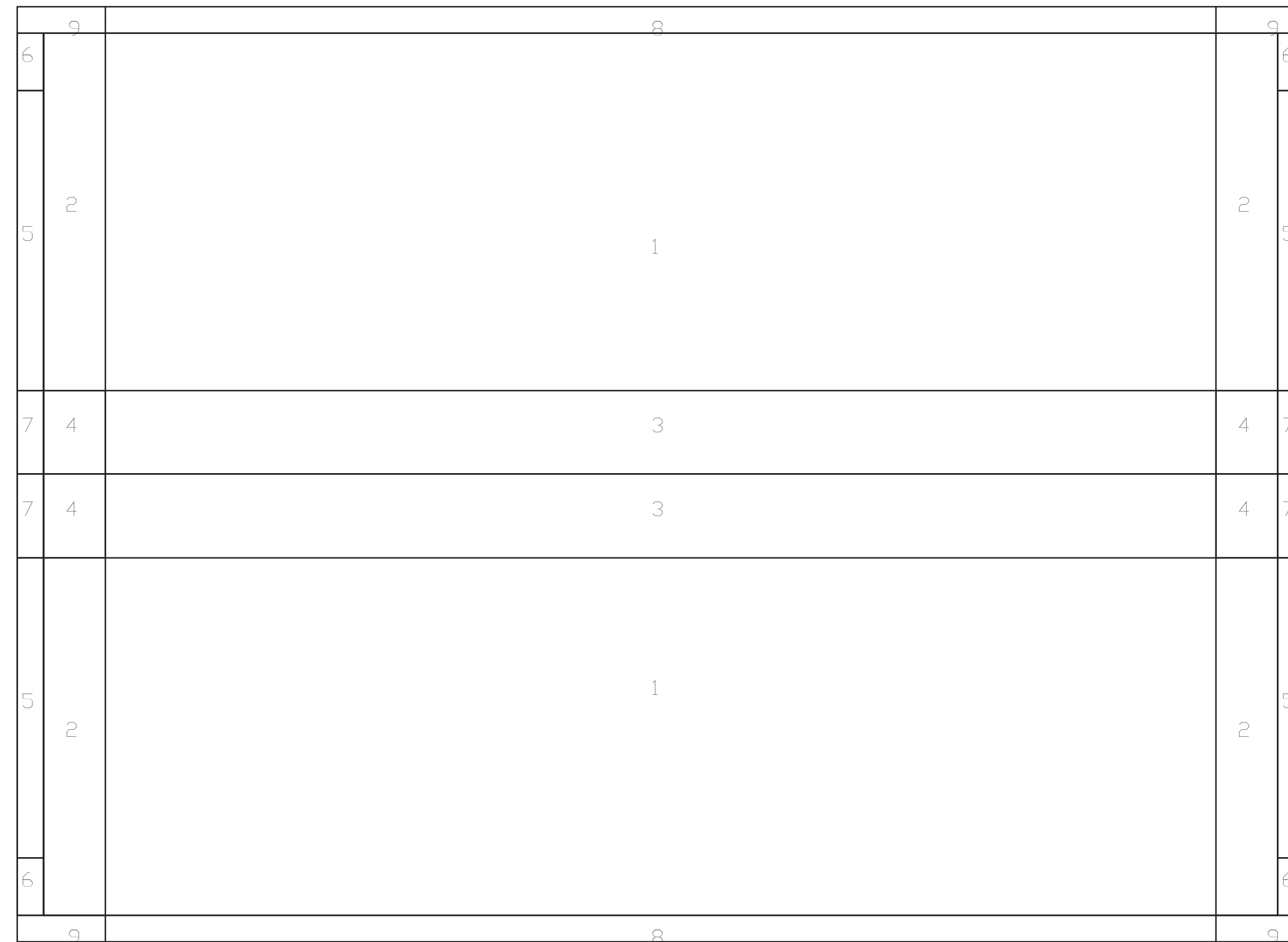
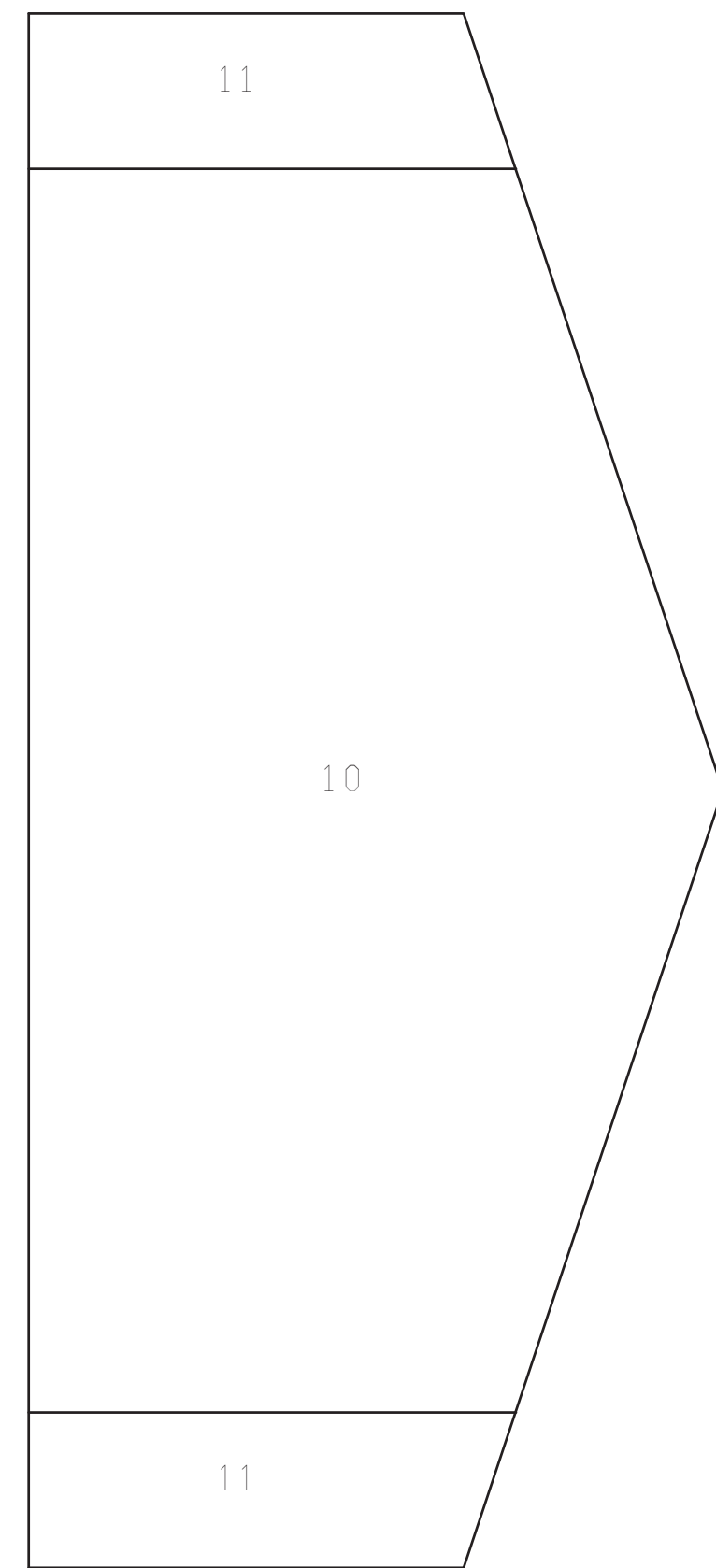
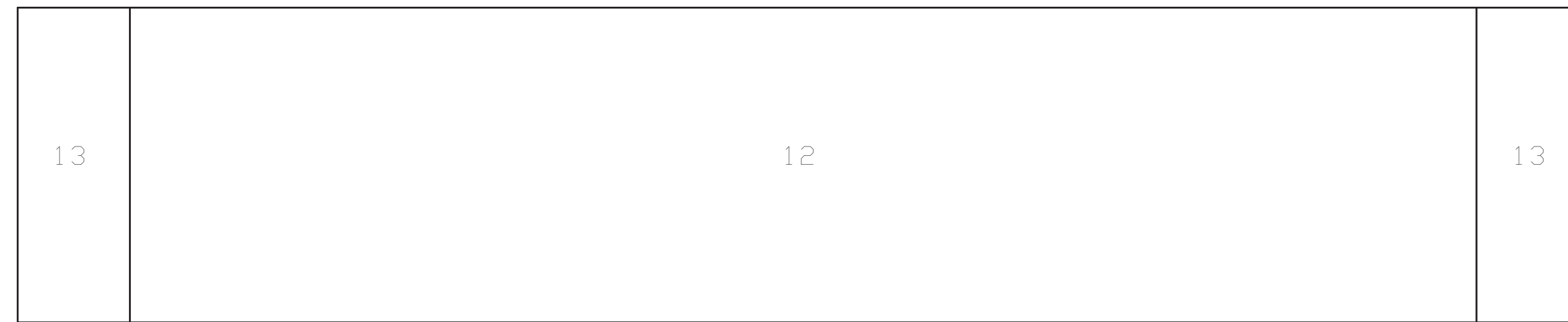
Beam3

Beam4

Beam5

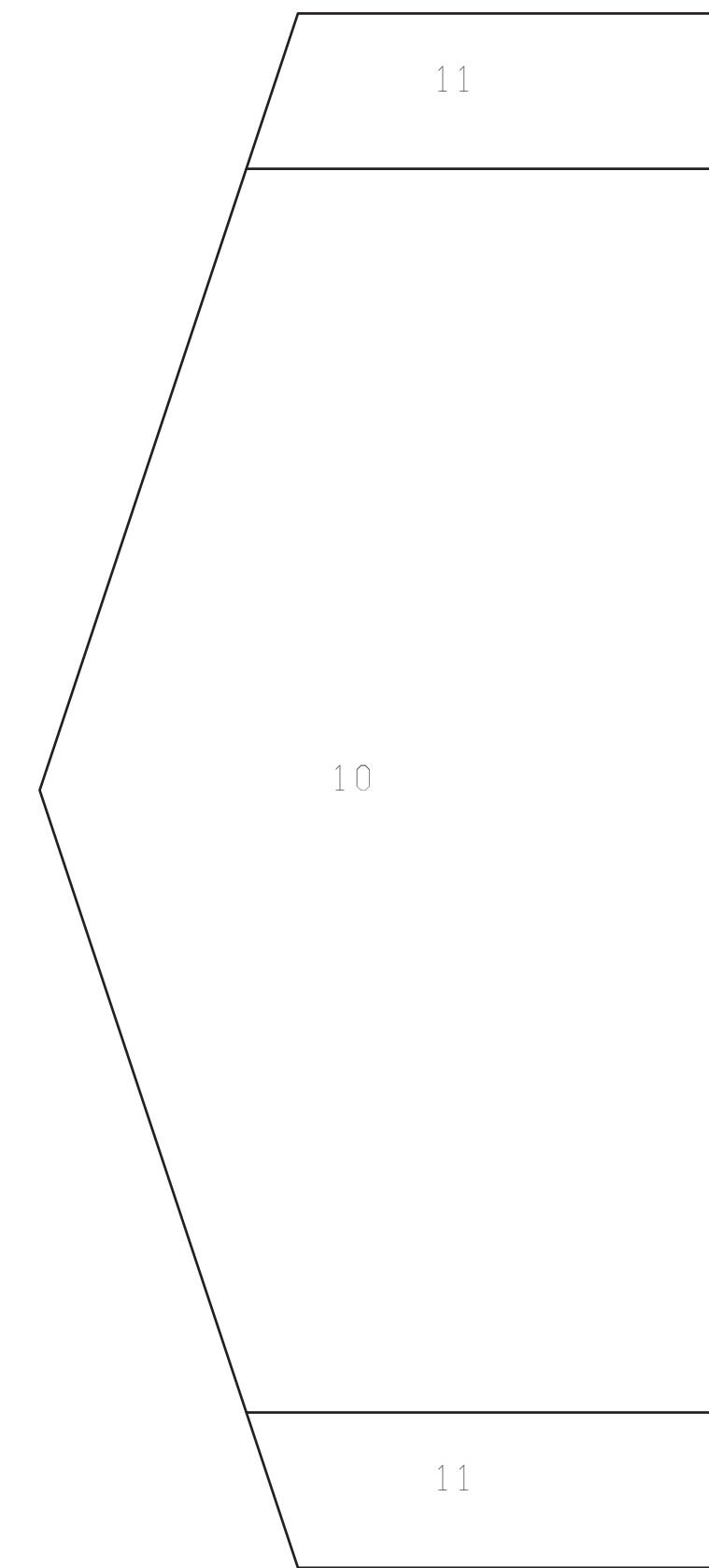
Beam6

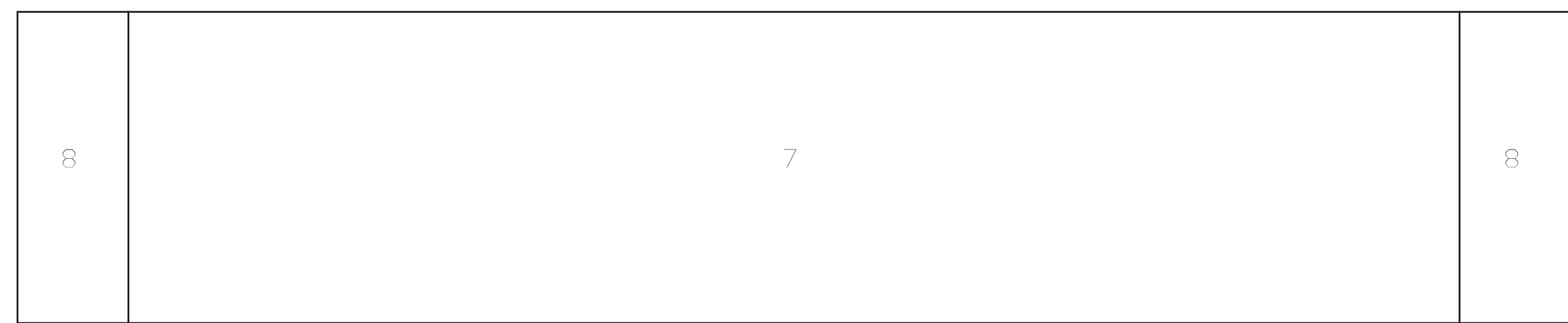
EXTENSION1



Zone	Width (ft)	Length (ft)	Components & Cladding		Suction (psf) (Factored)	
			Pressure (psf) Member	Panel	Member	Panel
1			7.20	7.20	-9.60	-21.20
2		3.50	7.20	7.20	-15.26	-30.95
3	5.00		7.20	7.20	-15.26	-30.95
4	5.00	3.50	7.20	7.20	-19.30	-36.67
5	17.93	1.50	7.20	7.20	-30.91	-34.13
6	3.42	1.50	7.20	7.20	-34.46	-39.85
7	5.00	1.50	7.20	7.20	-38.69	-45.79
8	1.50	63.00	7.20	7.20	-14.59	-24.31
9	1.50	5.00	7.20	7.20	-19.55	-39.87
10			9.60	11.47	-10.52	-12.45
11	5.00		9.60	11.47	-11.46	-15.31
12			9.60	11.46	-10.50	-12.42
13	5.00		9.60	11.46	-11.45	-15.28

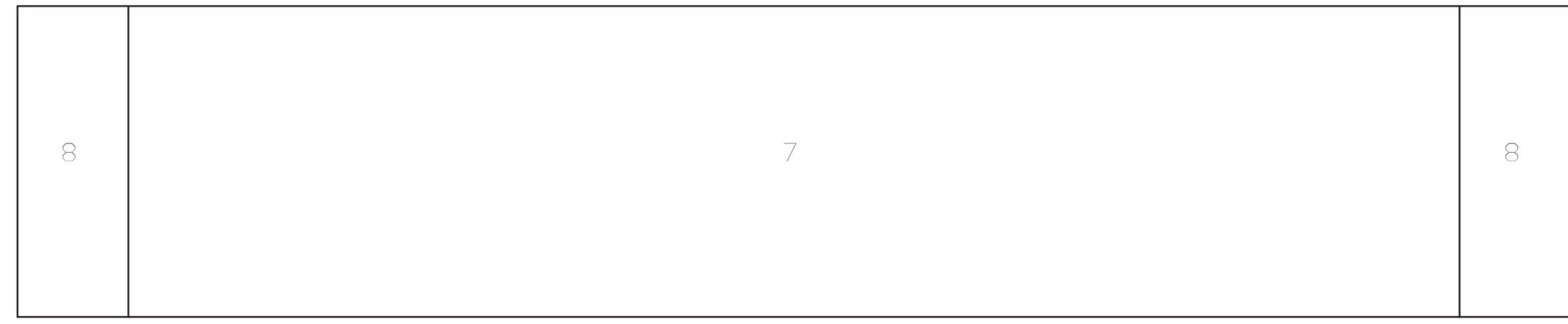
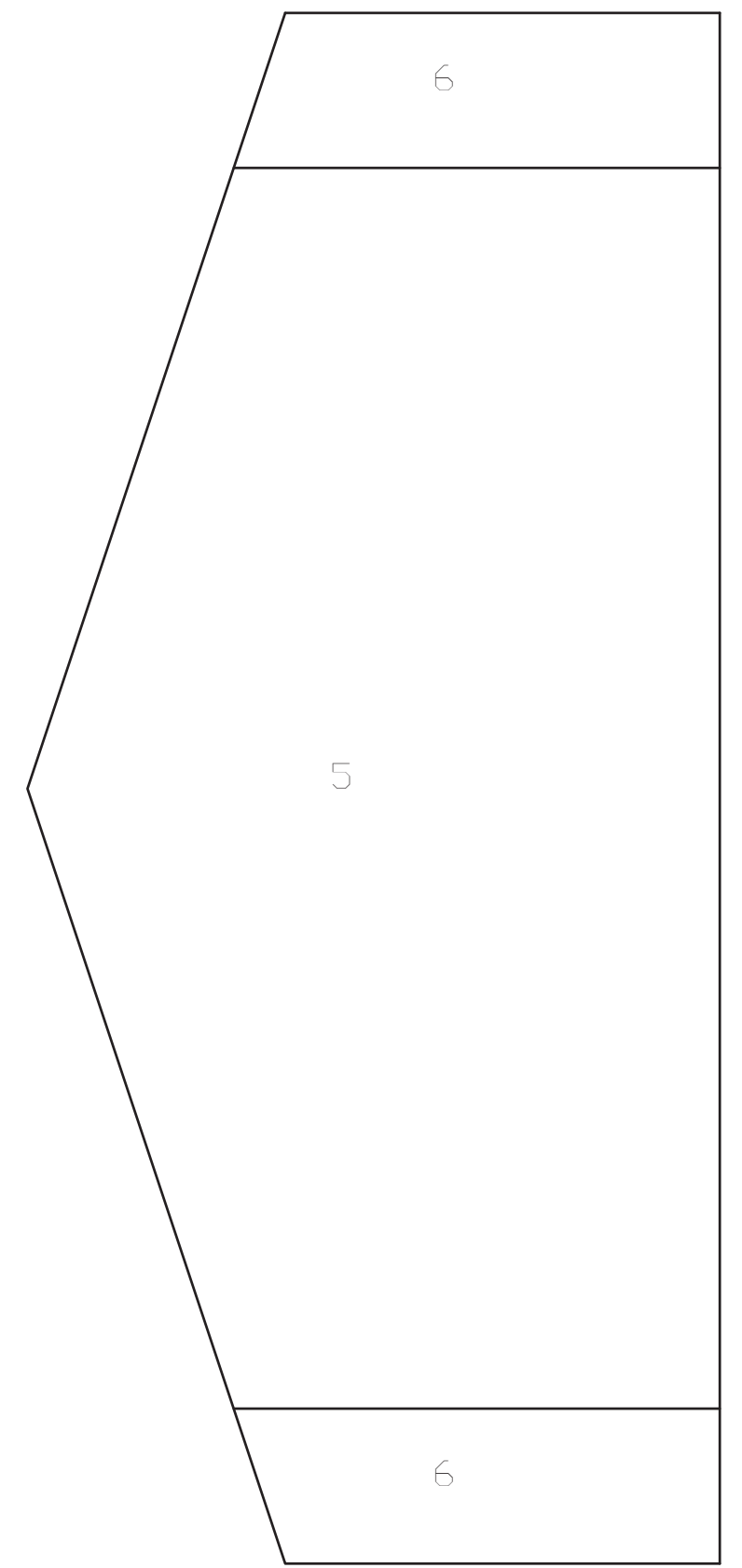
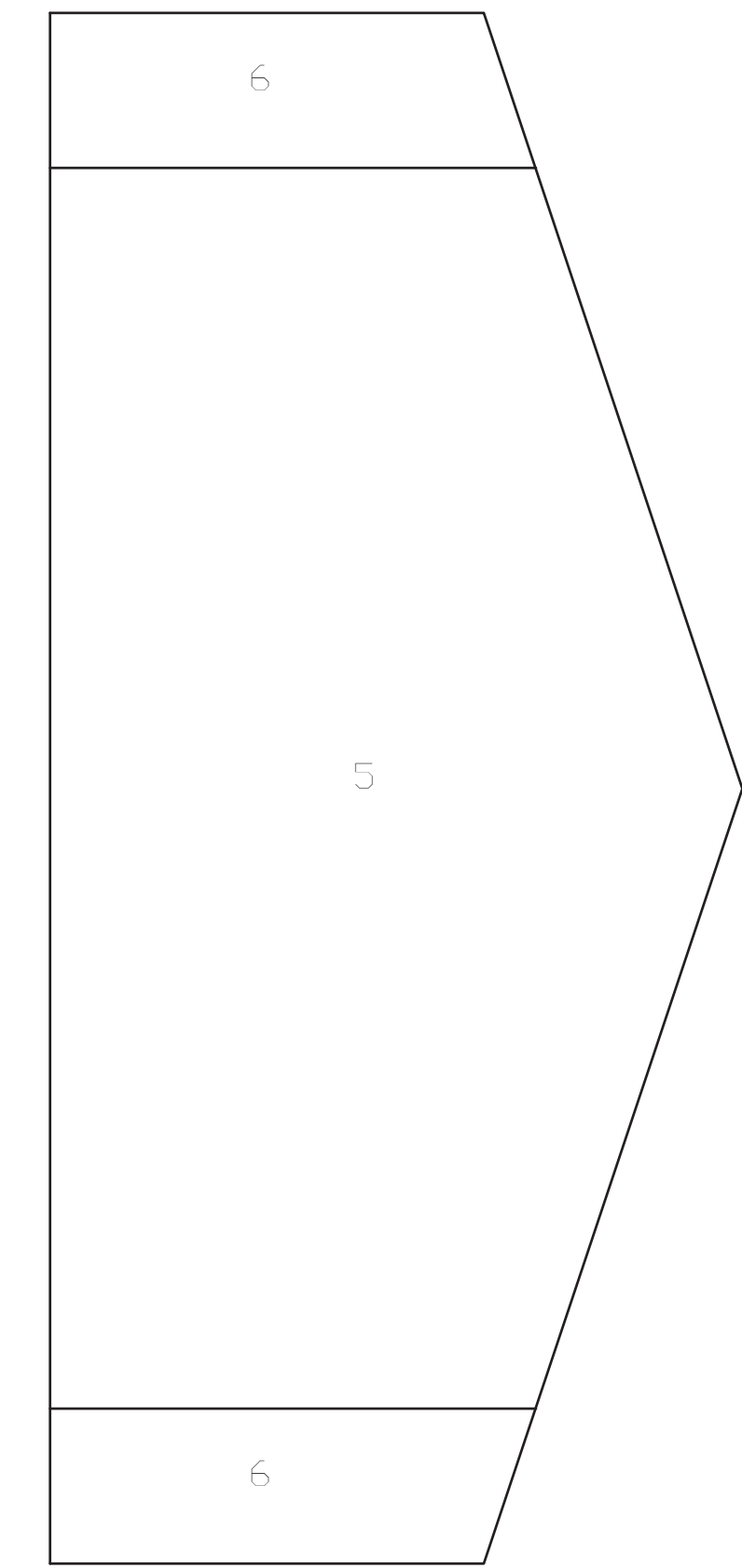
(+) wind towards surface  
 (-) wind away from surface

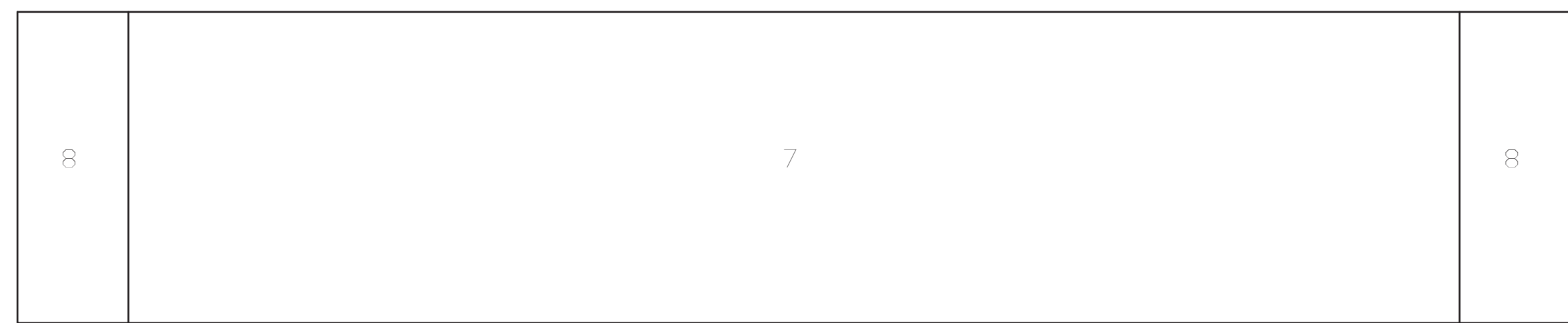




Zone	Width (ft)	Length (ft)	Components & Cladding (Factored)			
			Pressure (psf)		Suction (psf)	
			Member	Panel	Member	Panel
1			7.20	7.20	-9.60	-21.20
2	26.35		0.00	0.00	0.00	0.00
3	1.50	63.00	7.20	7.20	-14.59	-24.31
4	1.50	5.00	7.20	7.20	-19.55	-39.87
5			9.60	11.47	-10.52	-12.45
6	5.00		9.60	11.47	-11.46	-15.31
7			9.60	11.46	-10.50	-12.42
8	5.00		9.60	11.46	-11.45	-15.28

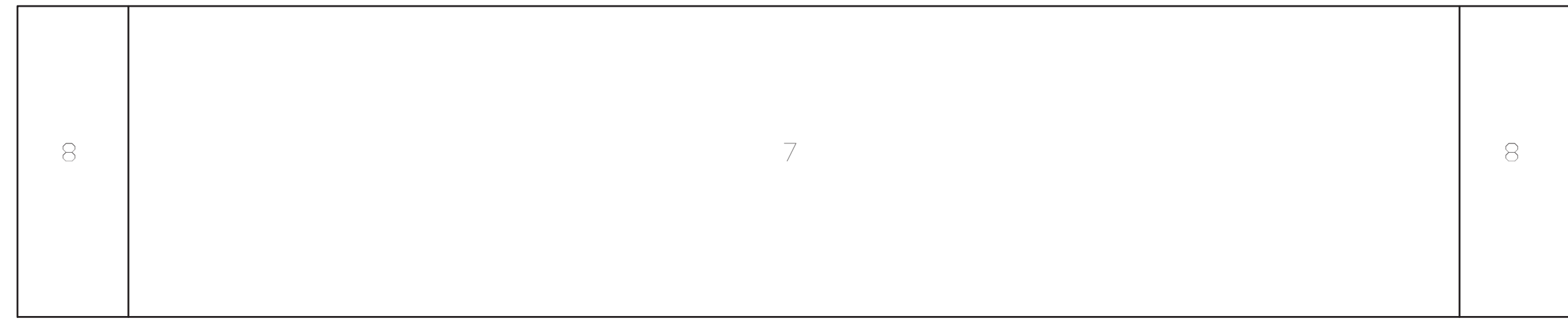
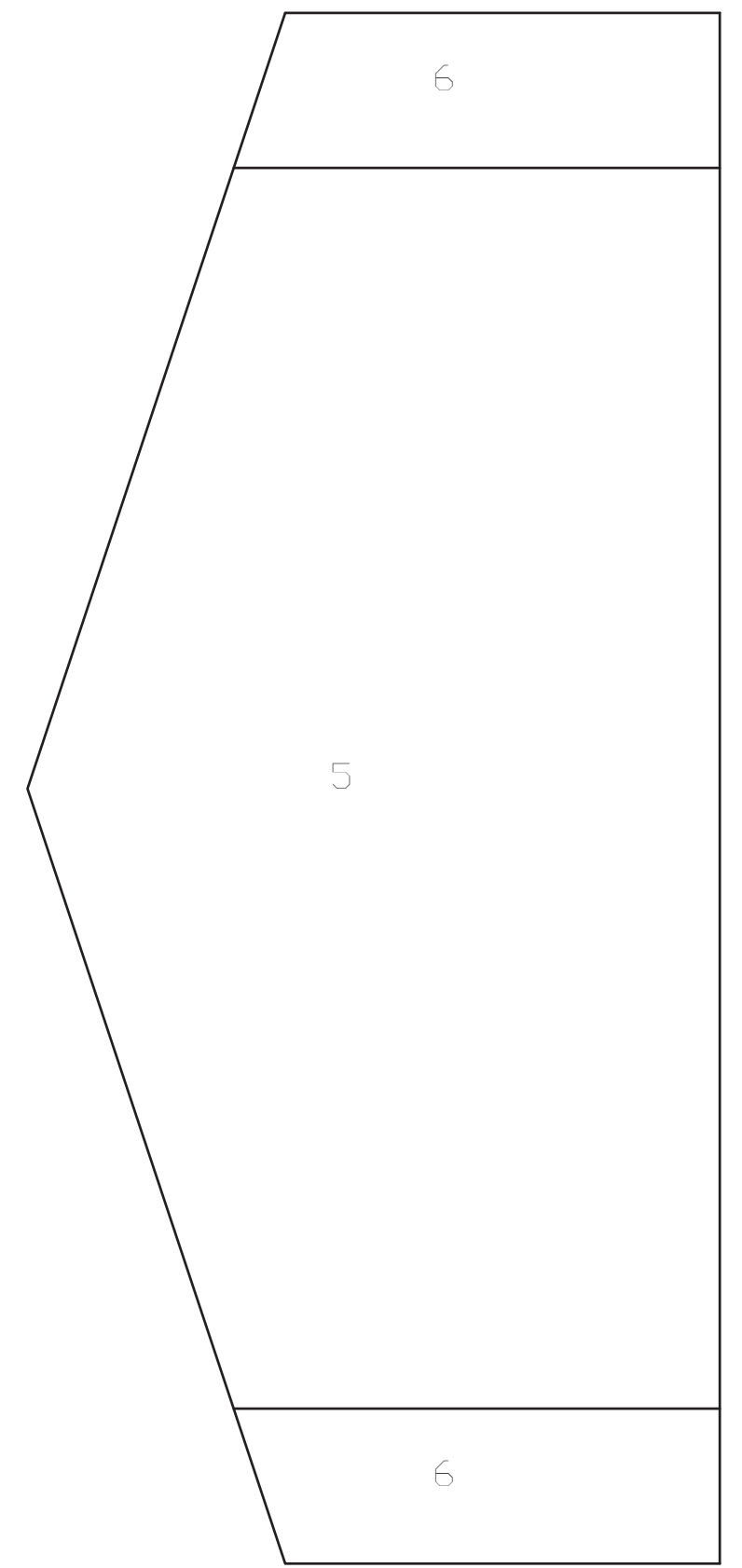
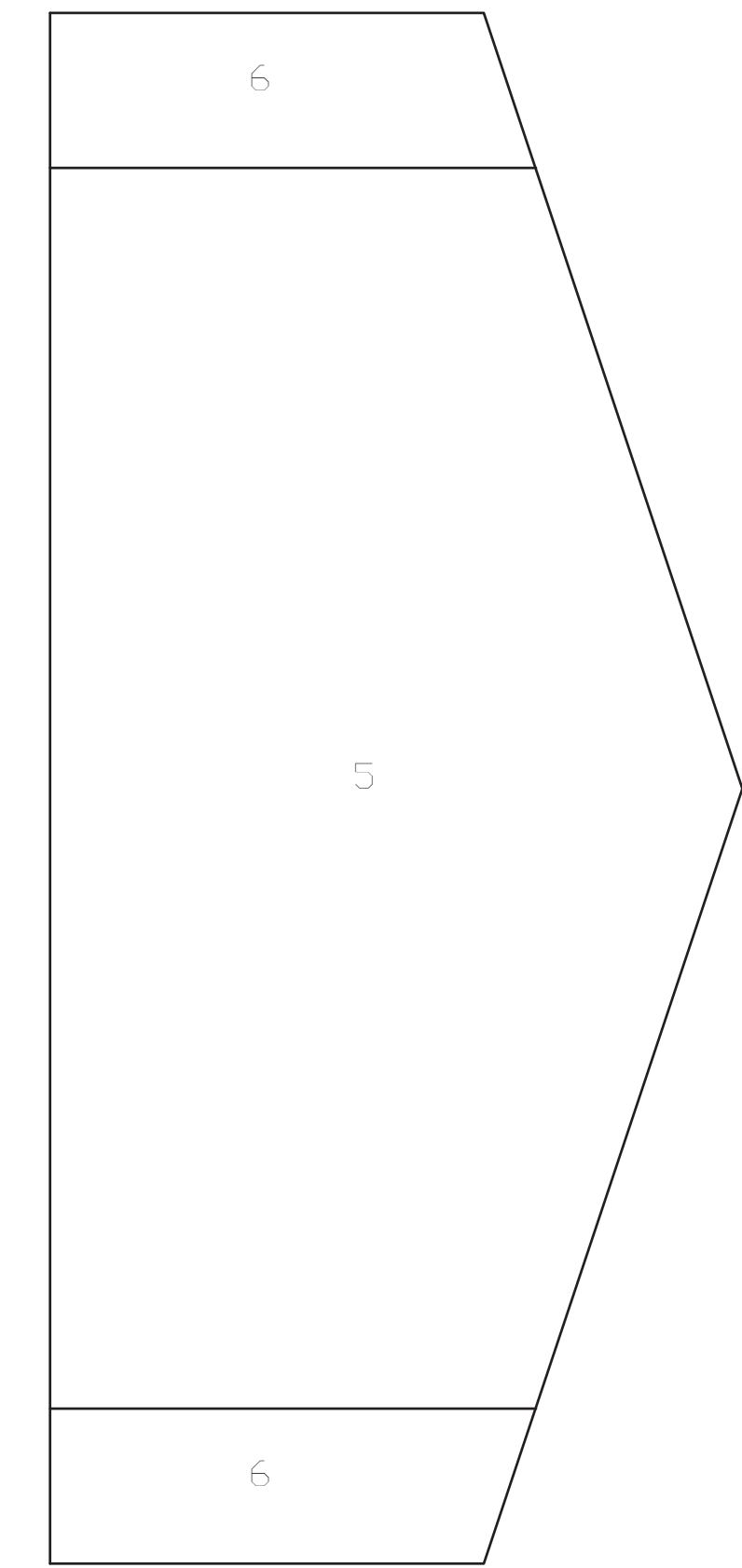
(+) wind towards surface  
 (-) wind away from surface

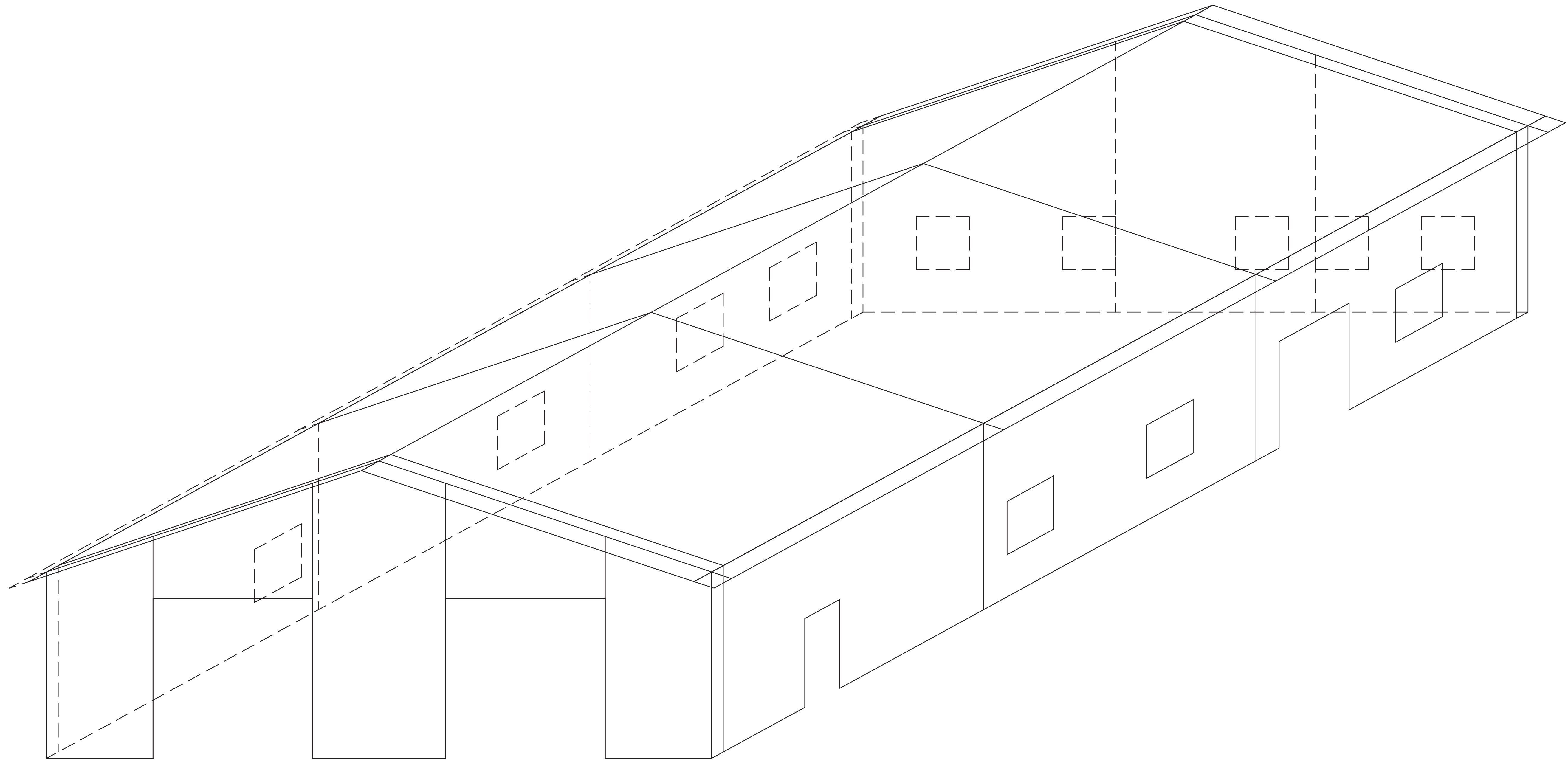




Zone	Width (ft)	Length (ft)	Components & Cladding (Factored)			
			Pressure (psf)		Suction (psf)	
			Member	Panel	Member	Panel
1			0.00	0.00	-9.60	0.00
2	26.35		0.00	0.00	0.00	0.00
3	1.50	63.00	7.20	7.20	-14.59	-24.31
4	1.50	5.00	7.20	7.20	-19.55	-39.87
5			9.60	11.47	-10.52	-12.45
6	5.00		9.60	11.47	-11.46	-15.31
7			9.60	11.46	-10.50	-12.42
8	5.00		9.60	11.46	-11.45	-15.28

(+) wind towards surface  
 (-) wind away from surface







# HERITAGE BUILDING SYSTEMS



## BUILDER/CONTRACTOR RESPONSIBILITIES

**Drawing Validity** –These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

**Builder Acceptance of Drawings** –Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice APR 10 Section 4.4.1)

**Code Official Approval** –It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

**Builder is responsible for State, Federal and OSHA safety compliance** –The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

**Building Erection** –The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice APR 10 Section 7.10.3)

**Discrepancies** –Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice APR 10 Section 3.3)

**Materials by Others** –All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturer's assumptions will govern.

**Modification of the Metal Building from Plans** –The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

**Foundation Design** –The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA MBMS Chapter 4 Section 3.2.2 and Section A.3)



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Descargue los manuales de instalación del panel desde:  
www.cornerstonebuildingbrands.com/installationmanuals/

1/2" Ø A325 BOLT GRIP TABLE (UNLESS NOTED)		BOLT LENGTH	NOTE: FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.  WASHER REQUIRED ONLY WHEN SPECIFIED. WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH AT LOCATIONS NOTED ON ERECTION DRAWINGS. ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.
GRIP	LENGTH		
0 TO 9/16"	1 1/4" F.T.		
Over 9/16" TO 1 1/16"	1 3/4" F.T.		
Over 1 1/16" TO 1 5/16"	2"		
Over 1 5/16" TO 1 9/16"	2 1/4"		
Over 1 9/16" TO 1 13/16"	2 1/2"		
Over 1 13/16" TO 2 1/16"	2 3/4"		
LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS			
F.T. DENOTES FULLY THREADED			

Rev. 11/15/2021

## PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacturer's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1. The fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-436

## DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS...: Building A

Roof Limits	Rafters	Purlins	Panels			
Live L/				@ F360	@ F354	@ F357
Snow L/				@ F360	@ F354	@ F357
Wind L/				@ F360	@ F355	@ F358
Total Gravity L/				@ F360	@ F354	@ F357
Frame Limits	Sideways	Portal Frame	Sideways			
Live H/				@ F359		
Snow H/				@ F359		
Wind H/				@ F359		
Seismic H/				@ F363		
Crane H/				@ F362		
Total Gravity H/				@ F359		
Total Wind H/				@ F359	@ F361	
Total Seismic H/				@ F363	@ F364	
Wall Limits	Limit					
Total Wind Panels L/				@ F356		
Total Wind Girts L/				@ F353		
Total Wind EW Columns L/				@ F350		

The Service Seismic limit as shown here is at service level loads.

## ENGINEERING DESIGN CRITERIA

Building Code.....	@ F324
Building Risk Category.....	@ U619
Roof Dead Load	
Superimposed.....	@ F301 psf
Collateral.....	@ F305 psf (Total)
(0.00 psf Ceiling @ F305 psf Other)	
Roof Live Load.....	@ F302 psf @ F304 reduction
Snow	
Ground Snow Load (Pg).....	@ U600 psf
Snow Load Importance Factor (Is).....	@ U614
Snow Exposure Factor (Ce).....	@ U615
Thermal Factor (Ct).....	@ U612
Flat Roof Snow Load (Pf).....	@ F303 psf
Minimum Roof Snow Load (Prn).....	@ U648 psf
Wind	
Ultimate Wind Speed (Vult).....	@ F307 mph
Nominal Wind Speed (Vnom).....	@ U636 mph
Serviceability Wind Speed.....	@ U646 mph
Ground Elevation Factor.....	@ U654 (@ U655L)
Wind Exposure Category.....	@ F309
Internal Pressure Coefficient (GCp) @ U623 / @ U624	
Loads for components not provided by building manufacturer.	
Wall Edge Zones (within @ U653 ' of corner)	@ U620 psf pressure
@ U622 psf suction	
Other Wall Zones	@ U620 psf pressure
@ U621 psf suction	
These values are the maximum values required based on a 10 square foot area.	
Components with larger areas may have lower wind loads.	
Zones per ASCE 7-16; FIG. 30.3-1	
Zones pressures shown are Un-Factored	
Seismic	
Seismic Importance Factor (Ie).....	@ F315
Seismic Design Category.....	@ F311
Soil Site Class.....	@ U647
Ss..... @ U601 g	Sds..... @ U607 g
S1..... @ U602 g	Sd1..... @ U606 g
Analysis Procedure.....	Equivalent Lateral Force
Location... Int RF Front SW Back SW Left EW Right EW	
System.....	@ J190 @ J191 @ J192 @ J193 @ J194
R.....	@ F368 @ F374 @ F376 @ F370 @ F372
Cs.....	@ F369 @ F375 @ F377 @ F371 @ F373
Design Base Shear in kips (V) Transverse @ F366	
Design Base Shear in kips (V) Longitudinal @ F365	

System –Basic Force Resisting System  
 H –Steel System not Specifically Detailed for Seismic Resistance  
 C4 –Steel Ordinary Moment Frames  
 R3 –Steel Ordinary Concentric Braced Frames  
 C2 –Steel Ordinary Cantilevered Column Systems  
 R –Response Modification Coefficient  
 Cs –Seismic Response Coefficient  
 Transverse –Direction Parallel to the Rigid Frames  
 Longitudinal –Direction Perpendicular to the Rigid Frames

Building Descriptions				
Building ID	Width(ft)	Length(ft)	Height(ft)	Slope
Building A	@ F201	@ F202	@ F204	@ F206

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
@J024	@DATE	FOR @J041	@J012	@J014	@J011

# HERITAGE BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #385  
 NORTH LITTLE ROCK, AR 72116-7606  
 1-800-643-5555

PROJECT:	@J007						
CUSTOMER:	@J004						
OWNER:	@J038						
LOCATION:	@J009						
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	@DATE	N.T.S.	1	A	@J010	C1	@J024

Drawing Index	
Page	Description
C1	COVER SHEET
F1	ANCHOR BOLT PLAN
F2	ANCHOR BOLT REACTIONS
F3	ANCHOR BOLT DETAILS
E1	ROOF FRAMING PLAN
E2	ROOF SHEETING PLAN
E3	FRONT SIDEWALL
E4	BACK SIDEWALL
E5	LEFT ENDWALL
E6	RIGHT ENDWALL
E7	FRAME CROSS SECTION
DET 1-10	HARD DETAILS
R1	INSTALLATION SHEETS

## DRAWING STATUS

FOR APPROVAL

These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.

FOR CONSTRUCTION PERMIT

These drawings, being For Permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered as complete.

FOR ERECTOR INSTALLATION

Final drawings for construction.

For questions or assistance  
 Concerning Erection call or Email:  
**1-844-840-4603**  
 Monday-Friday 7:30am to 5:00pm  
 FIELD.SERVICE@CORNERSTONE-BB.COM

## ENGINEERING SEAL

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

# GN-1

# HERITAGE BUILDING SYSTEMS



## BUILDER/CONTRACTOR RESPONSIBILITIES

**Drawing Validity** –These drawings, supporting structural calculations and design certification are based on the order documents as of the date of these drawings. These documents describe the material supplied by the manufacturer as of the date of these drawings. Any changes to the order documents after the date on these drawings may void these drawings, supporting structural calculations and design certification. The Builder/Contractor is responsible for notifying the building authority of all changes to the order documents which result in changes to the drawings, supporting structural calculations and design certification.

**Builder Acceptance of Drawings** –Approval of the manufacturer's drawings and design data affirms that the manufacturer has correctly interpreted and applied the requirements of the order documents and constitutes Builder/Contractor acceptance of the manufacturer's interpretations of the order documents and standard product specifications, including its design, fabrication and quality criteria standards and tolerances. (AISC code of standard practice APR 10 Section 4.4.1)

**Code Official Approval** –It is the responsibility of the Builder/Contractor to ensure that all project plans and specifications comply with the applicable requirements of any governing building authority. The Builder/Contractor is responsible for securing all required approvals and permits from the appropriate agency as required.

**Builder is responsible for State, Federal and OSHA safety compliance** –The Builder/Contractor is responsible for applying and observing all pertinent safety rules and regulations and OSHA standards as applicable.

**Building Erection** –The Builder/Contractor is responsible for all erection of the steel and associated work in compliance with the Metal Building Manufacturers drawings. Temporary supports, such as temporary guys, braces, false work or other elements required for erection will be determined, furnished and installed by the erector. (AISC Code of Standard Practice APR 10 Section 7.10.3)

**Discrepancies** –Where discrepancies exist between the Metal Building plans and plans for other trades, the Metal Building plans will govern. (AISC Code of Standard Practice APR 10 Section 3.3)

**Materials by Others** –All interface and compatibility of any materials not furnished by the manufacturer are the responsibility of and to be coordinated by the Builder/Contractor or A/E firm. Unless specific design criteria concerning any interface between materials if furnished as a part of the order documents, the manufacturer's assumptions will govern.

**Modification of the Metal Building from Plans** –The Metal Building supplied by the manufacturer has been designed according to the Building Code and specifications and the loads shown on this drawing. Modification of the building configuration, such as removing wall panels or braces, from that shown on these plans could affect the structural integrity of the building. The Metal Building Manufacturer or a Licensed Structural Engineer should be consulted prior to making any changes to the building configuration shown on these drawings. The Metal Building Manufacturer will assume no responsibility for any loads applied to the building not indicated on these drawings.

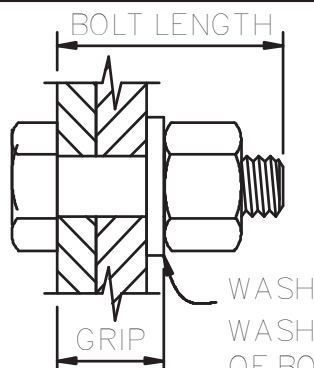
**Foundation Design** –The Metal Building Manufacturer is not responsible for the design, materials and workmanship of the foundation. Anchor rod plans prepared by the manufacturer are intended to show only location, diameter and projection of the anchor rods required to attach the Metal Building System to the foundation. It is the responsibility of the end customer to ensure that adequate provisions are made for specifying rod embedment, bearing values, tie rods and or other associated items embedded in the concrete foundation, as well as foundation design for the loads imposed by the Metal Building System, other imposed loads, and the bearing capacity of the soil and other conditions of the building site. (MBMA MBMS Chapter 4 Section 3.2.2 and Section A3)



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Descargue los manuales de instalación del panel desde:  
www.cornerstonebuildingbrands.com/installationmanuals/

1/2" Ø A325 BOLT GRIP TABLE (UNLESS NOTED)	
GRIP	LENGTH
0 TO 9/16"	1 1/4" F.T.
Over 9/16" TO 1 1/16"	1 3/4" F.T.
Over 1 1/16" TO 1 5/16"	2"
Over 1 5/16" TO 1 9/16"	2 1/4"
Over 1 9/16" TO 1 13/16"	2 1/2"
Over 1 13/16" TO 2 1/16"	2 3/4"



NOTE:  
FULL THREAD ENGAGEMENT IS DEEMED TO HAVE BEEN MET WHEN THE END OF THE BOLT IS FLUSH WITH THE FACE OF THE NUT.

WASHER REQUIRED ONLY WHEN SPECIFIED.  
WASHER MAY BE LOCATED UNDER HEAD OF BOLT, UNDER NUT, OR AT BOTH LOCATIONS NOTED ON ERECTION DRAWINGS.  
ADD 5/32" FOR EACH WASHER TO MATERIAL THICKNESS TO DETERMINE GRIP.

LOCATIONS OF BOLTS LONGER THAN 2 3/4" NOTED ON ERECTION DRAWINGS  
F.T. DENOTES FULLY THREADED

Rev. 11/15/2021

## PROJECT NOTES

Material properties of steel bar, plate, and sheet used in the fabrication of built-up structural framing members conform to ASTM A529, ASTM A572, or ASTM A1011 with 55 ksi min. yield, except flanges wider than 12" and thicker than 3/8", all flanges thicker than 1", and all webs thicker than 3/8" are 50 ksi min. yield. Rod X-bracing conforms to ASTM A529 or ASTM A572 with 50 ksi min. yield. Cable X-bracing conforms to ASTM A475 7 Strand Extra High-Strength grade. Hot rolled structural shapes conform to ASTM A992, ASTM A529, or ASTM A572 with 50 ksi min. yield. Hot rolled angles, other than flange braces, conform to ASTM A36 minimum. Round and rectangular HSS conforms to ASTM A500 Grade B. Cold-formed steel secondary framing Members conform to ASTM A1011 or ASTM A653 Grade 55 with 55 ksi min. yield.

The manufacturer does not assume any responsibility for the erection nor field supervision of the structure and or any special inspections that may be required by the local building authority during erection (including inspection of the high strength bolts or field welds) as required during erection. The coordination and the costs associated for setting up and Special Inspections are the responsibility of the Erector, Owner, Architect, or Engineer of Record.

Design is based upon the more severe loading of either the roof snow load or the roof live load.

Loads, as noted, are given within order documents and are applied in general accordance with the applicable provisions of the model code and/or specification indicated. Neither the manufacturer nor the certifying engineer declares or attests that the loads as designated are proper for the local provisions that may apply or for site specific parameters. The manufacturer's Engineer's certification is limited to design loads supplied by an Architect and/or engineer of record for the overall construction project.

This project is designed using manufacturer's standard serviceability standards. Generally this means that all stresses and deflections are within typical performance limits for normal occupancy and standard metal building products. If special requirements for deflections and vibrations must be adhered to, then they must be clearly stated in the contract documents.

This metal building system is designed as enclosed. All exterior components (i.e. doors, windows, vents, etc.) must be designed to withstand the specified wind loading for the design of components and cladding in accordance with the specified building code. Doors are to be closed when a maximum of 50% of design wind velocity is reached.

Unless otherwise noted, special inspection of fabricated items is not required. Per IBC section 1704.2.5.1. The fabricator is approved to perform such work without special inspection through maintenance of IAS AC 472 certification MB-136

## DEFLECTION CRITERIA

The material supplied by the manufacturer has been designed with the following minimum deflection criteria. The actual deflection may be less depending on actual load and actual member length.

BUILDING DEFLECTION LIMITS...: Building A

Roof Limits	Rafters	Purlins	Columns
Live L/	180	180	60
Snow L/	180	180	60
Wind L/	180	180	60
Total Gravity L/	180	180	60
Frame Limits	Sideway	Portal Frame	Sideway
Live H/	60		
Snow H/	60		
Wind H/	60		
Seismic H/	195		
Crane H/	100		
Total Gravity H/	60		
Total Wind H/	60		60
Total Seismic H/	195		195
Wall Limits	Limit		
Total Wind Panels L/	60		
Total Wind Girts L/	90		
Total Wind EW Columns L/	120		

The Service Seismic limit as shown here is at service level loads.

## ENGINEERING DESIGN CRITERIA

Building Code.....	CBC 19
Building Risk Category.....	II - Normal
Roof Dead Load	
Superimposed.....	2,500 psf
Collateral.....	6 psf (Total)
(0,00 psf Ceiling 6 psf Other)	
Roof Live Load.....	20,00 psf No reduction
Snow	
Ground Snow Load (Pg).....	0,00 psf
Snow Load Importance Factor (Is).....	1,00
Snow Exposure Factor (Ce).....	1,00
Thermal Factor (Ct).....	1,00
Flat Roof Snow Load (Pf).....	0 psf
Minimum Roof Snow Load (Prn).....	0,00 psf
Wind	
Ultimate Wind Speed (Vult).....	92 mph
Nominal Wind Speed (Vnom).....	71 mph
Serviceability Wind Speed.....	64 mph
Ground Elevation Factor.....	1,00 (53.64 ASL)
Wind Exposure Category.....	C
Internal Pressure Coefficient (GCp) 0,18 / ±0,18	
Loads for components not provided by building manufacturer.	
Wall Edge Zones (within 5,00' of corner)	
	19,12 psf pressure
-25,61 psf suction	
Other Wall Zones	19,12 psf pressure
-20,75 psf suction	

These values are the maximum values required based on a 10 square foot area.

Components with larger areas may have lower wind loads.

Zones per ASCE 7-16; FIG. 30.3-1

Zones pressures shown are Un-Factored

Seismic	
Seismic Importance Factor (Ie).....	1,00
Seismic Design Category.....	D
Soil Site Class.....	D
Ss.....	1,500 g
Sd1.....	0,600 g
Sd1.....	1,000 g
Sd1.....	0,680 g
Analysis Procedure.....	Equivalent Lateral Force

Location... Int RF Front SW Back SW Left EW Right EW						
System.....	C4	C4	C4	C4	C4	C4
R.....	3,5	3,5	3,5	3,5	3,5	3,5
Cs.....	0,286	0,286	0,286	0,286	0,286	0,286

Design Base Shear in kips (V) Transverse 12,74  
Design Base Shear in kips (V) Longitudinal 12,76

System –Basic Force Resisting System  
H –Steel System not Specifically Detailed for Seismic Resistance  
C4 –Steel Ordinary Moment Frames  
R3 –Steel Ordinary Concentric Braced Frames  
C2 –Steel Ordinary Cantilevered Column Systems  
R –Response Modification Coefficient  
Cs –Seismic Response Coefficient  
Transverse –Direction Parallel to the Rigid Frames  
Longitudinal –Direction Perpendicular to the Rigid Frames

Building Descriptions				
Building ID	Width(ft)	Length(ft)	Height(ft)	Slope
Building A	50	70	14	4.0:12

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

# HERITAGE BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #385  
NORTH LITTLE ROCK, AR 72116-7606  
1-800-643-5555

PROJECT:		OWNER:	
CUSTOMER:		LOCATION:	
CAD	DATE	SCALE	PHASE
	4/12/22	N.T.S.	1
BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
A		C1	0

Drawing Index	
Page	Description
C1	COVER SHEET
F1	ANCHOR BOLT PLAN
F2	ANCHOR BOLT REACTIONS
F3	ANCHOR BOLT DETAILS
E1	ROOF FRAMING PLAN
E2	ROOF SHEETING PLAN
E3	FRONT SIDEWALL
E4	BACK SIDEWALL
E5	LEFT ENDWALL
E6	RIGHT ENDWALL
E7	FRAME CROSS SECTION
DET 1-10	ANCHOR DETAILS
R1	INSTALLATION SHEETS

## DRAWING STATUS

FOR APPROVAL

These drawings, being For Approval, are by definition not final, and are for conceptual representation only. Their purpose is to confirm proper interpretation of the project documents. Only drawings issued "For Erector Installation" can be considered as complete.

FOR CONSTRUCTION PERMIT

These drawings, being For Permit, are by definition not final. Only drawings issued "For Erector Installation" can be considered as complete.

FOR ERECTOR INSTALLATION

Final drawings for construction.

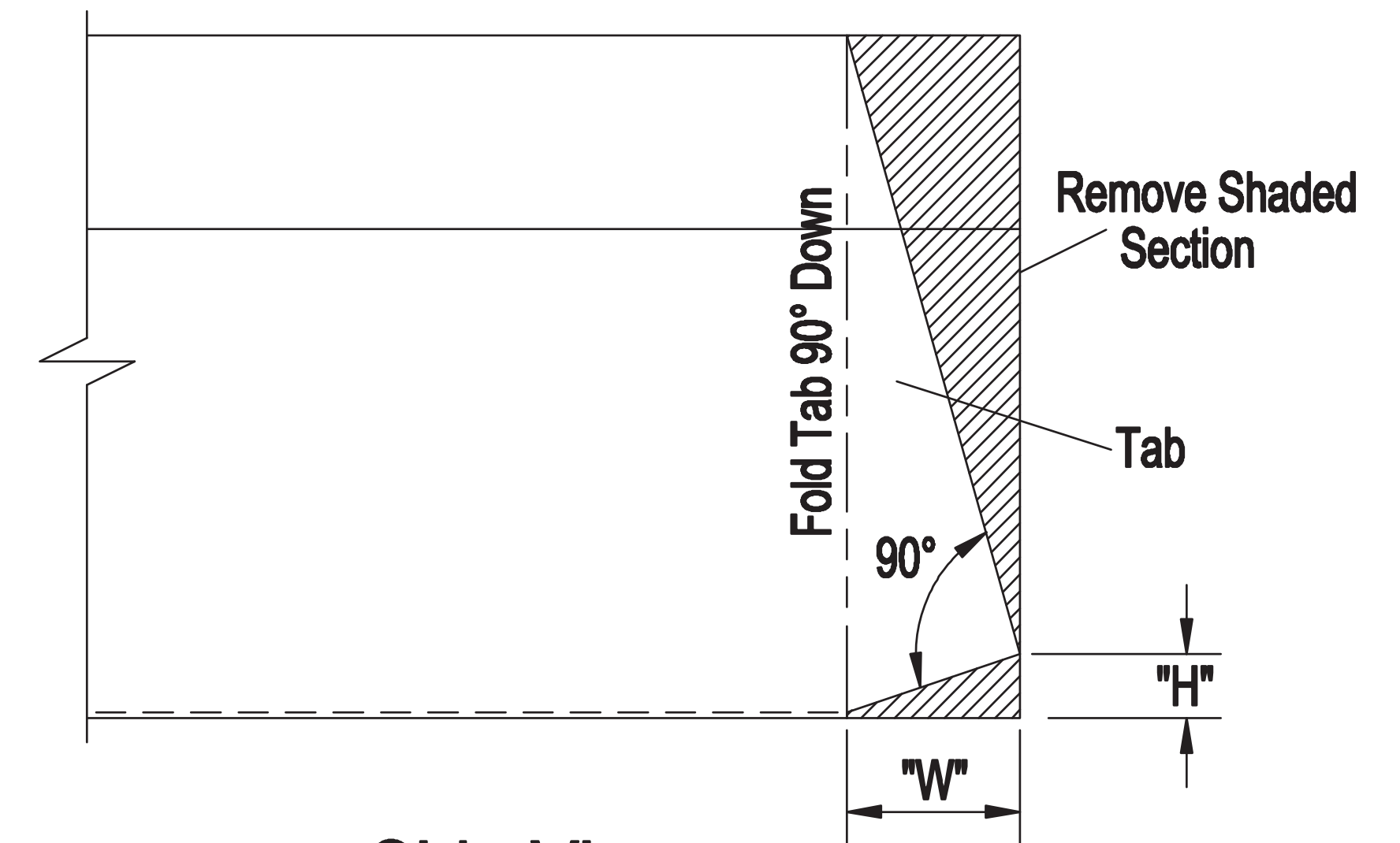
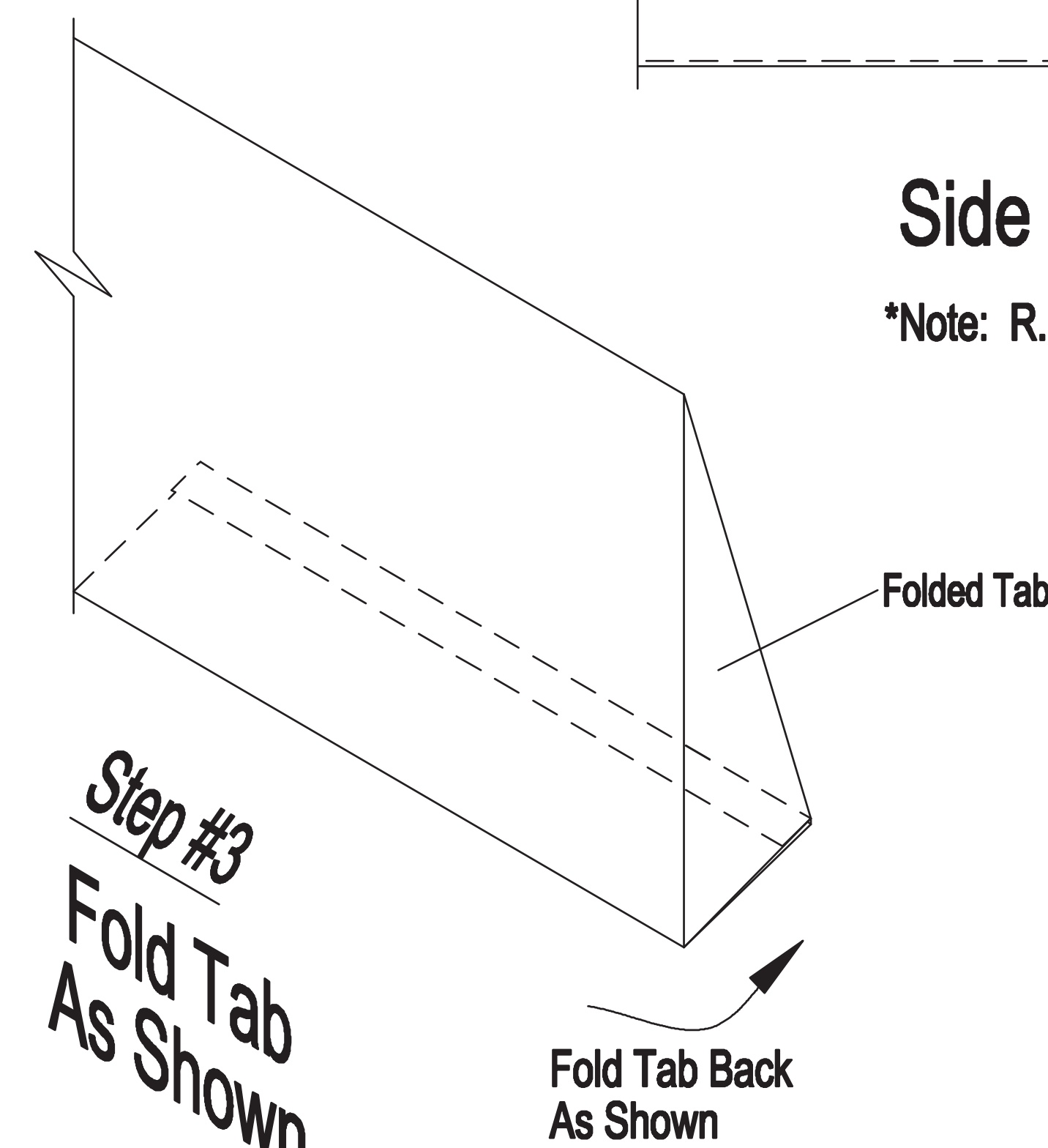
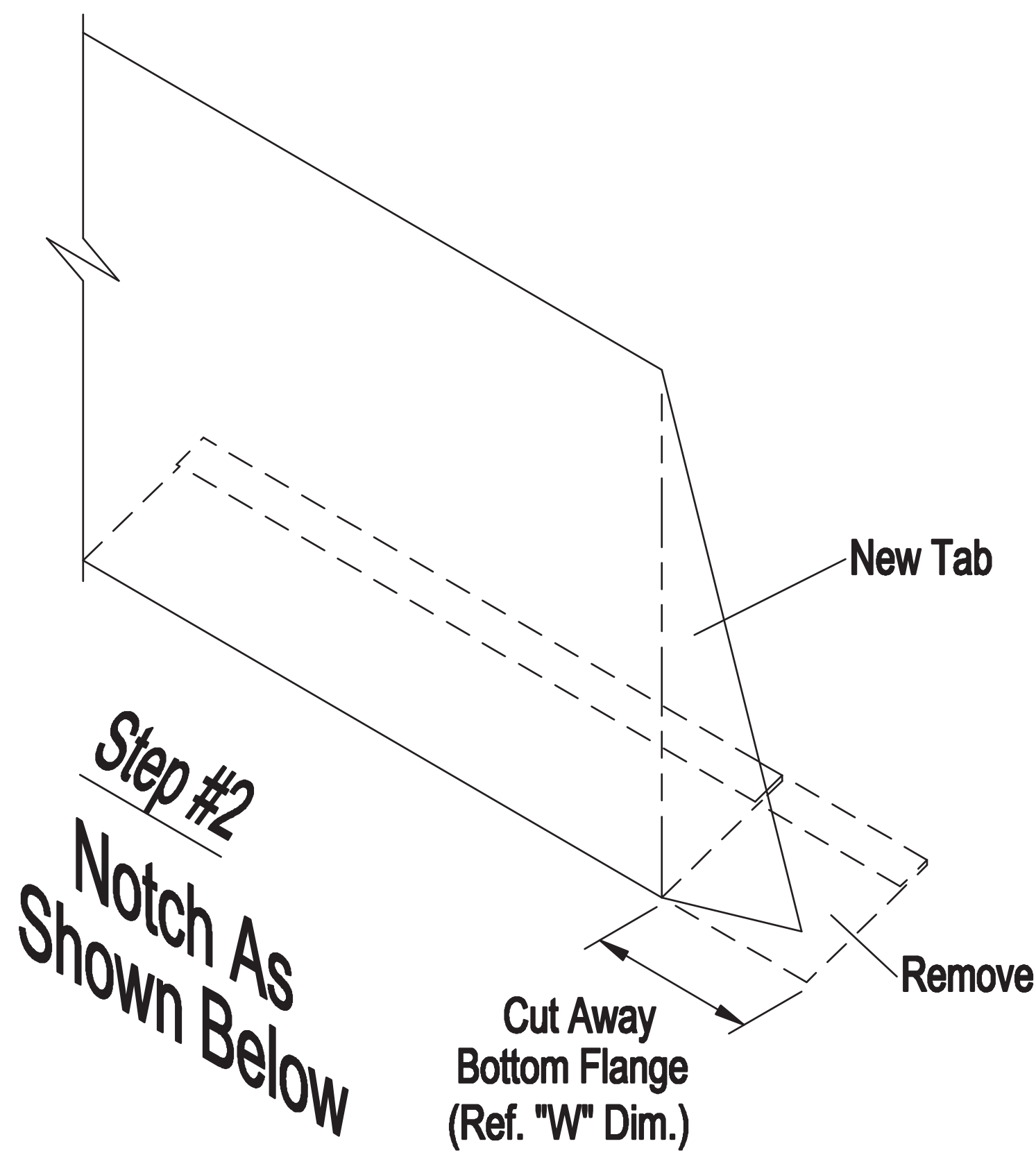
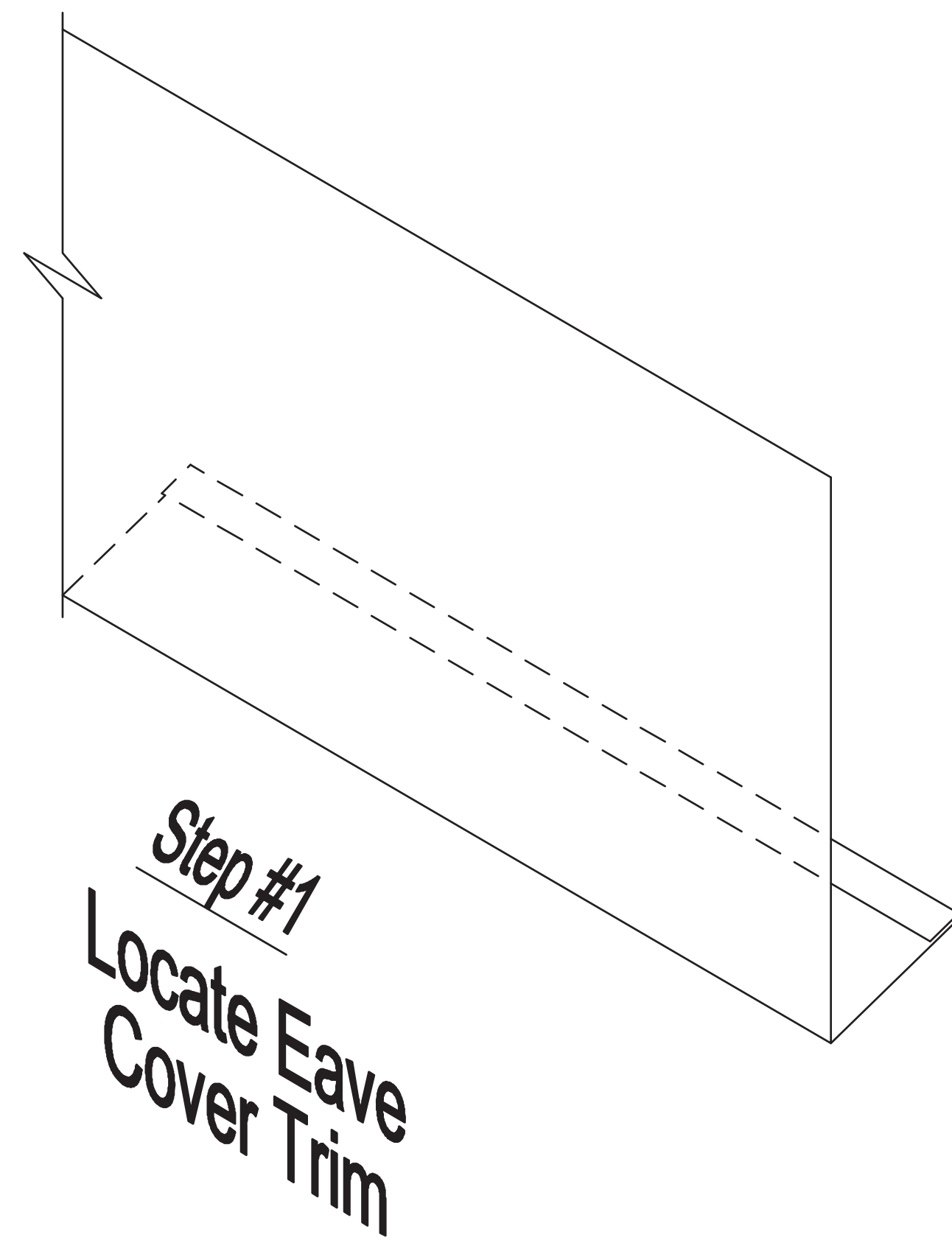
For questions or assistance  
Concerning Erection call or Email:  
**1-844-840-4603**  
Monday-Friday 7:30am to 5:00pm  
FIELD.SERVICE@CORNERSTONE-BB.COM

## ENGINEERING SEAL

The engineer whose seal appears hereon is an employee for the manufacturer for the materials described herein. Said seal or certification is limited to the products designed and manufactured by manufacturer only. The undersigned engineer is not the overall engineer of record for this project.

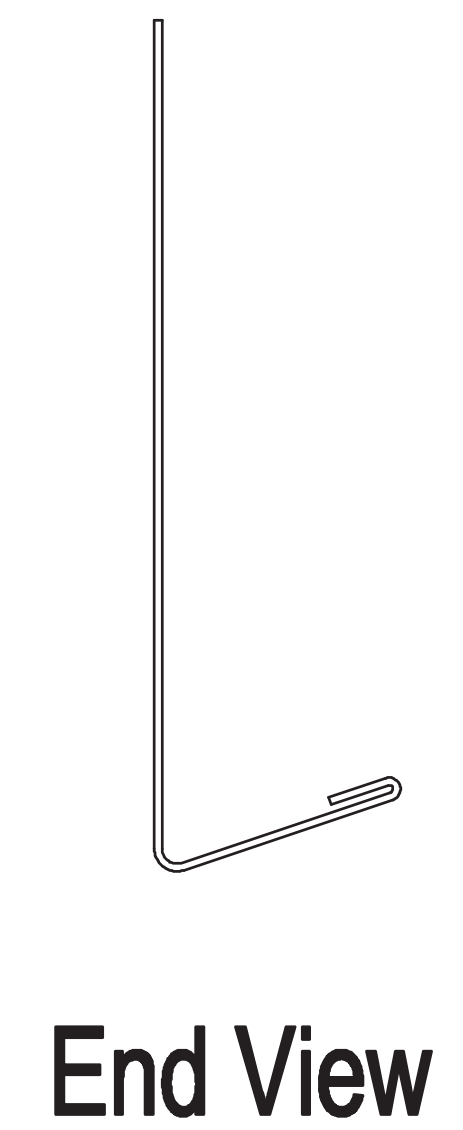
# GN-2

Cut To Roof Pitch		
Roof Pitch	"H" =	"W" =
1:12	--	9/16"
2:12	3/16"	1 1/8"
3:12	7/16"	1 11/16"
4:12	11/16"	2 1/8"
5:12	1 1/16"	2 1/2"
6:12	1 7/16"	2 7/8"

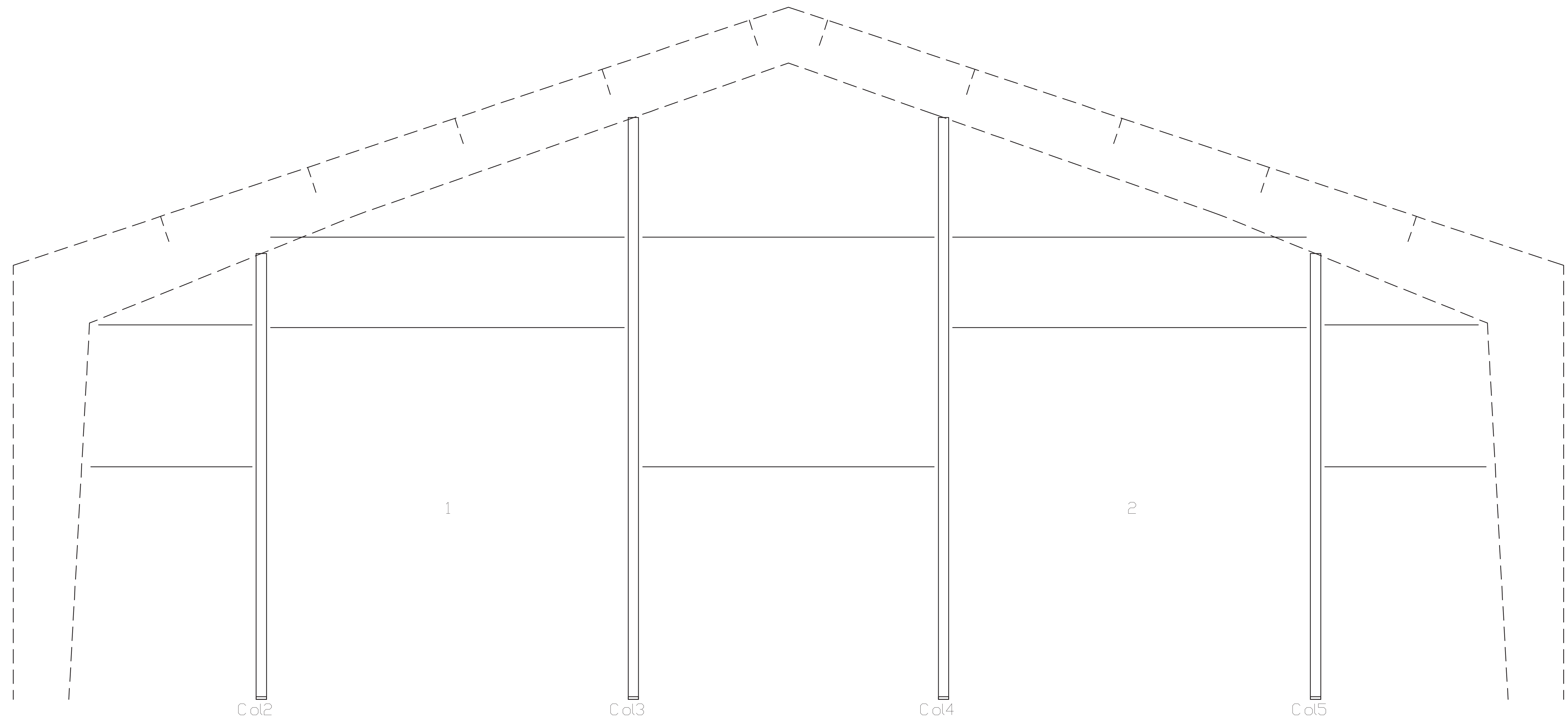


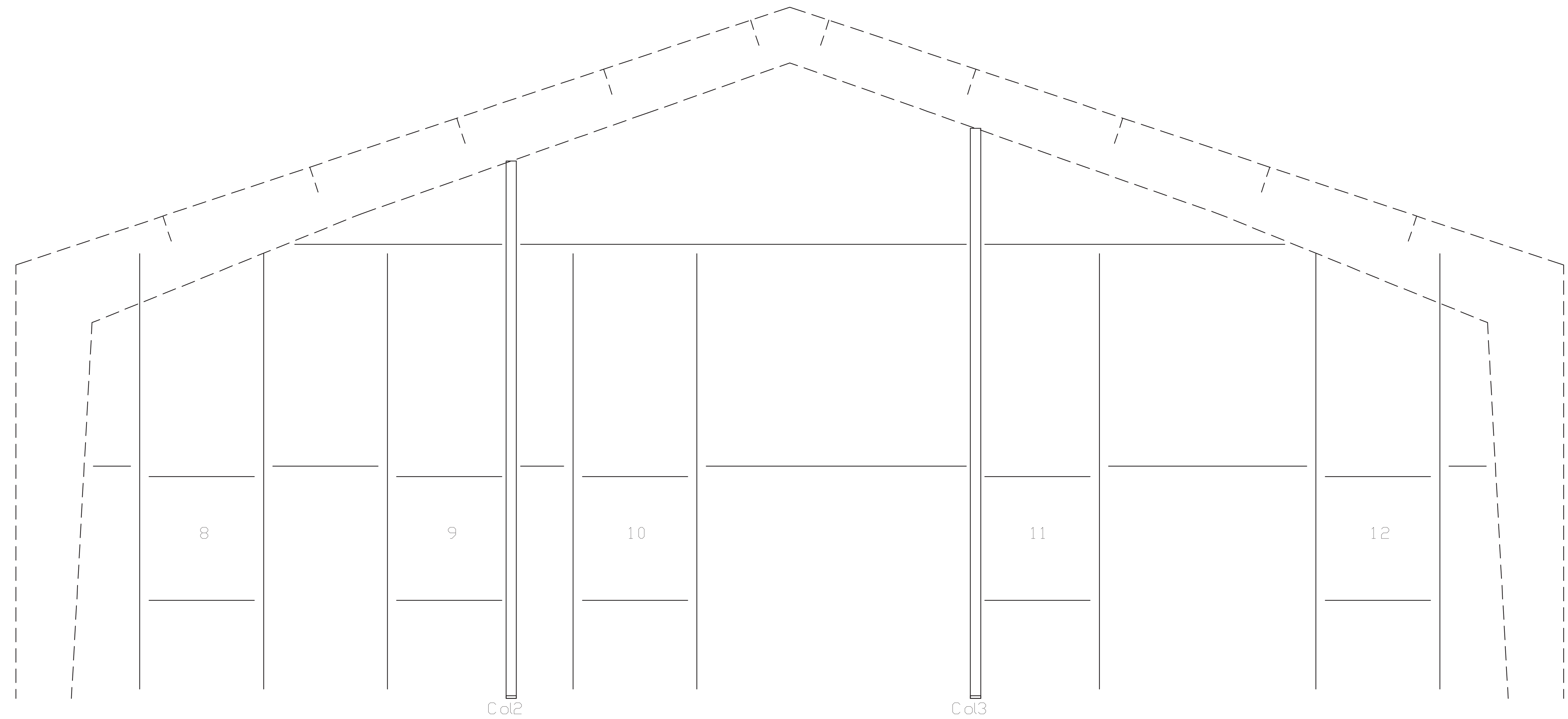
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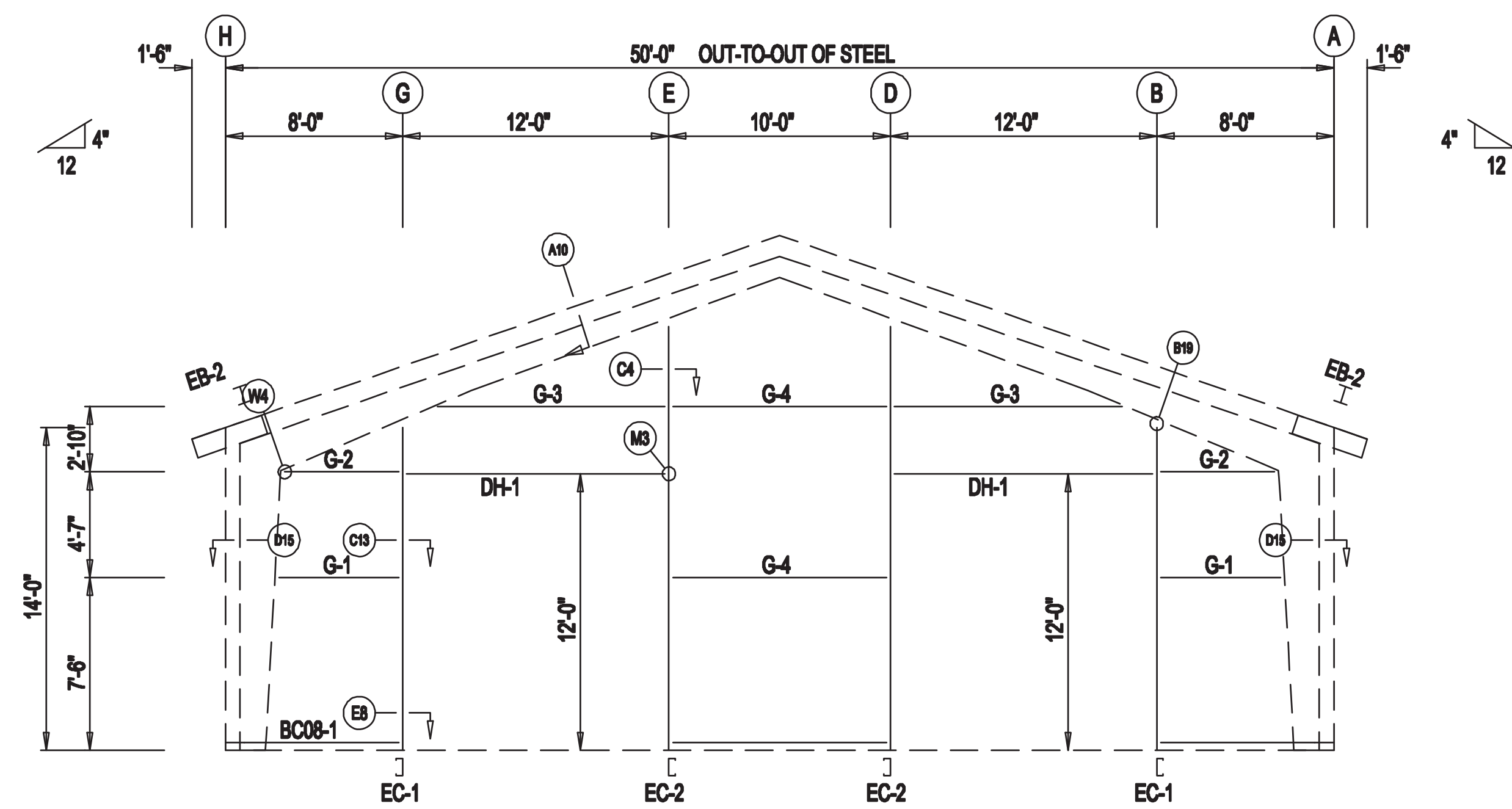
\*Note: R.H. Shown  
L.H. Opposite



Instructions: Field Trimed Tab For Extension Cover Trim



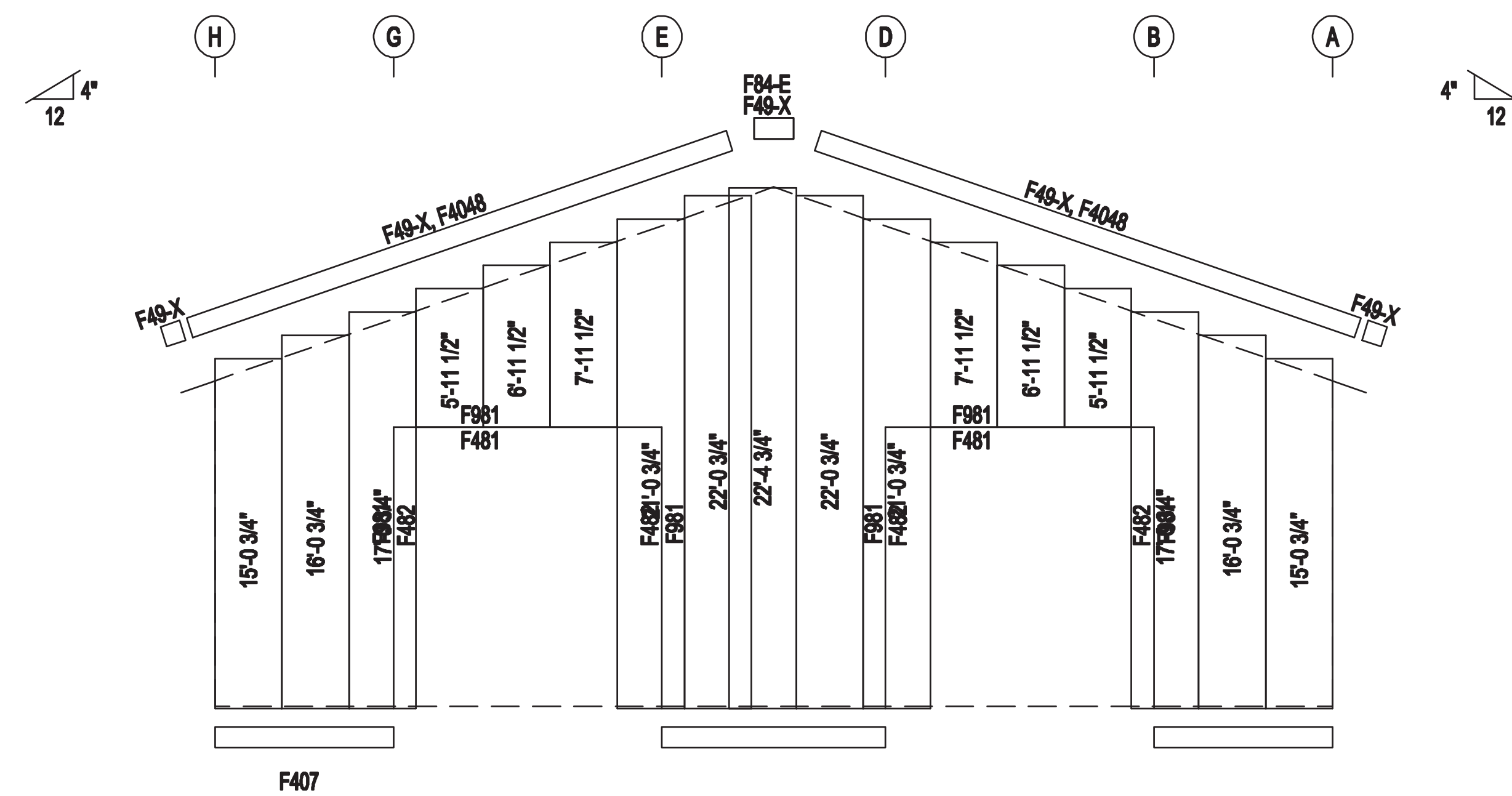




**BEARING FRAME ONLY!**  
 WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE				
FRAME LINE 1				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"

ENDWALL FRAMING: FRAME LINE 1



ENDWALL SHEETING & TRIM: FRAME LINE 1

PANELS: 26 Gauge PBR - Light Stone

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

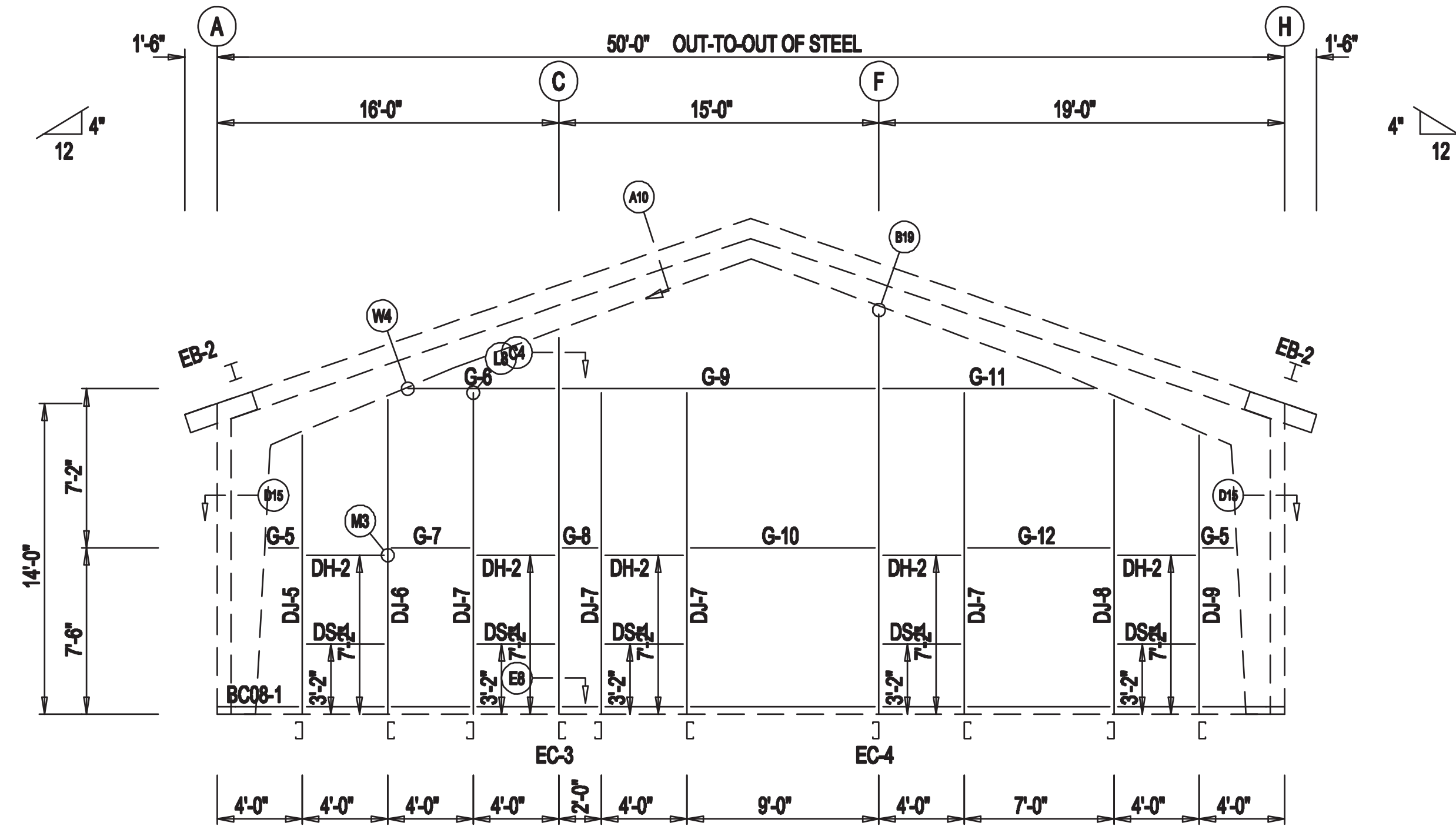
**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
 NORTH LITTLE ROCK, AR 72116-7606  
 1-800-643-8555

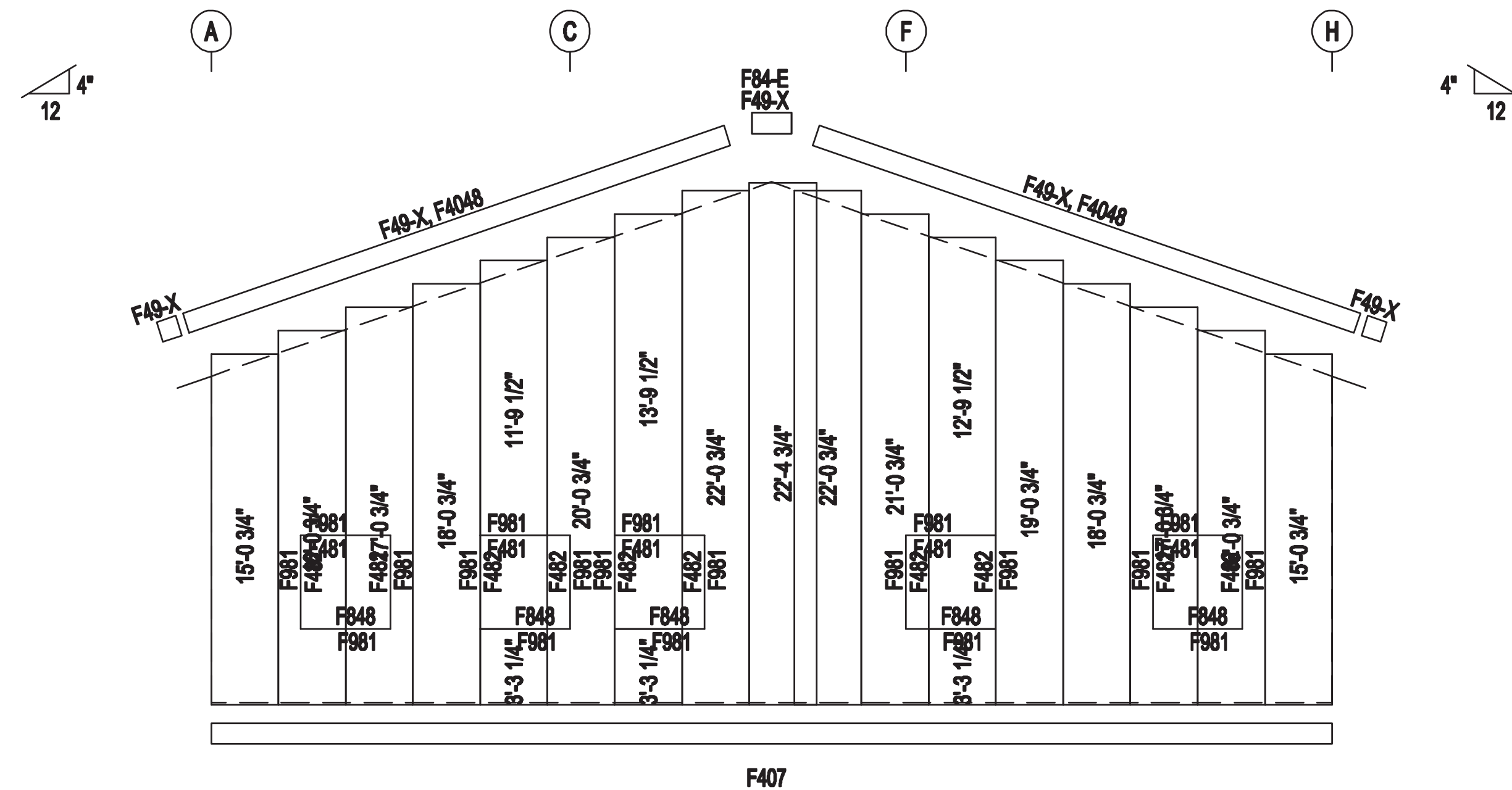
S-23

**GENERAL NOTES:**  
 1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.  
 2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.  
 3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.  
 4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

PROJECT:		CUSTOMER:		OWNER:			
LOCATION:							
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
	4/12/22	N.T.S.	1	A		E5	0



ENDWALL FRAMING: FRAME LINE 4



ENDWALL SHEETING & TRIM: FRAME LINE 4

PANELS: 26 Gauge PBR - Light Stone

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
NORTH LITTLE ROCK, AR 72116-7606  
1-800-643-6555

PROJECT:							
CUSTOMER:			OWNER:				
LOCATION:							
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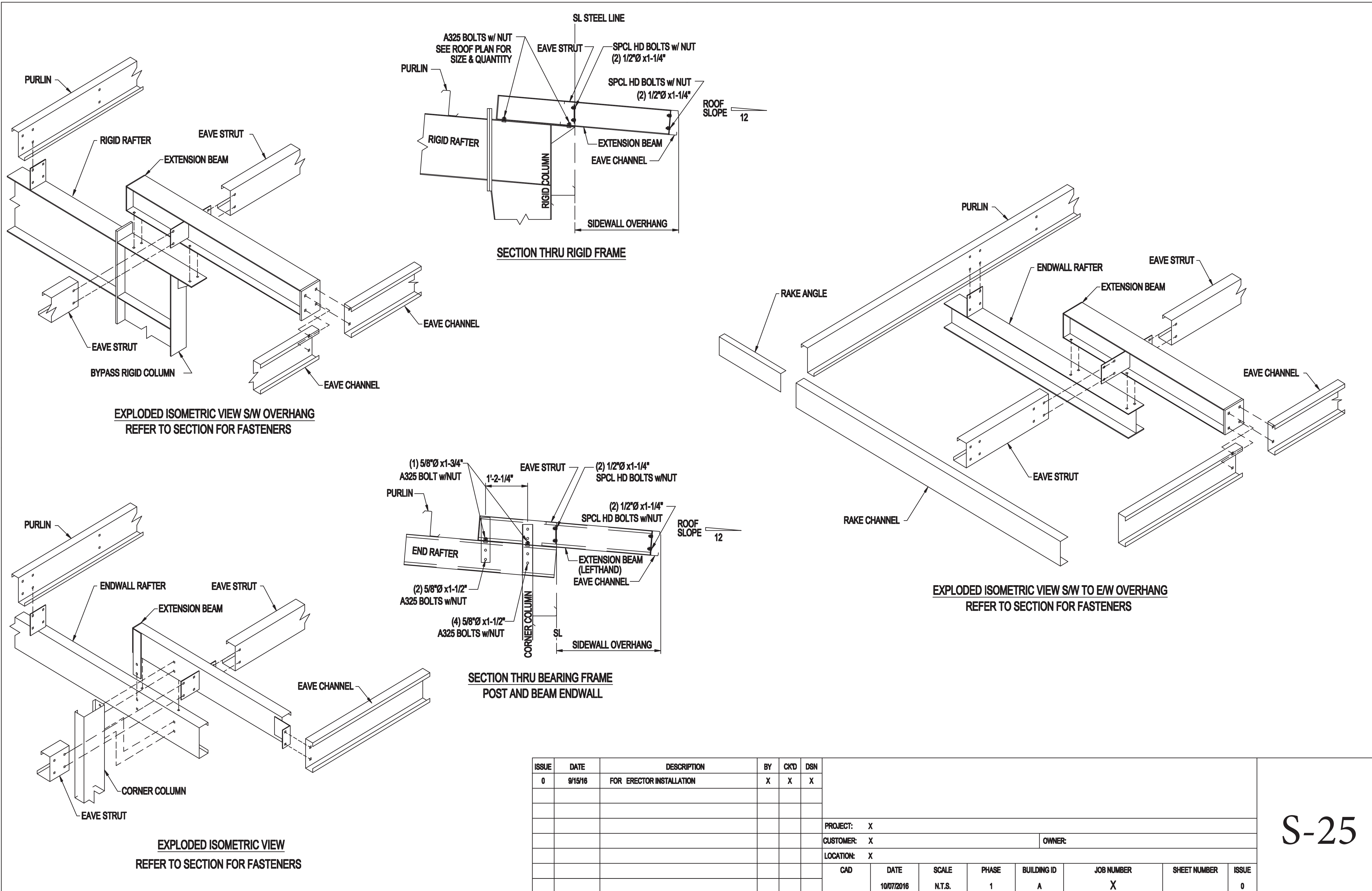
**BEARING FRAME ONLY!**  
WASHER TO BE USED AT ENDWALL COLUMN TO ENDWALL RAFTER CONNECTION. USE ONE WASHER ON COLUMN SIDE. WASHER NOT NEEDED ON CLIP SIDE.

BOLT TABLE				
FRAME LINE 4				
LOCATION	QUAN	TYPE	DIA	LENGTH
Columns/Raf	2	A325	1/2"	1 1/4"

**GENERAL NOTES:**

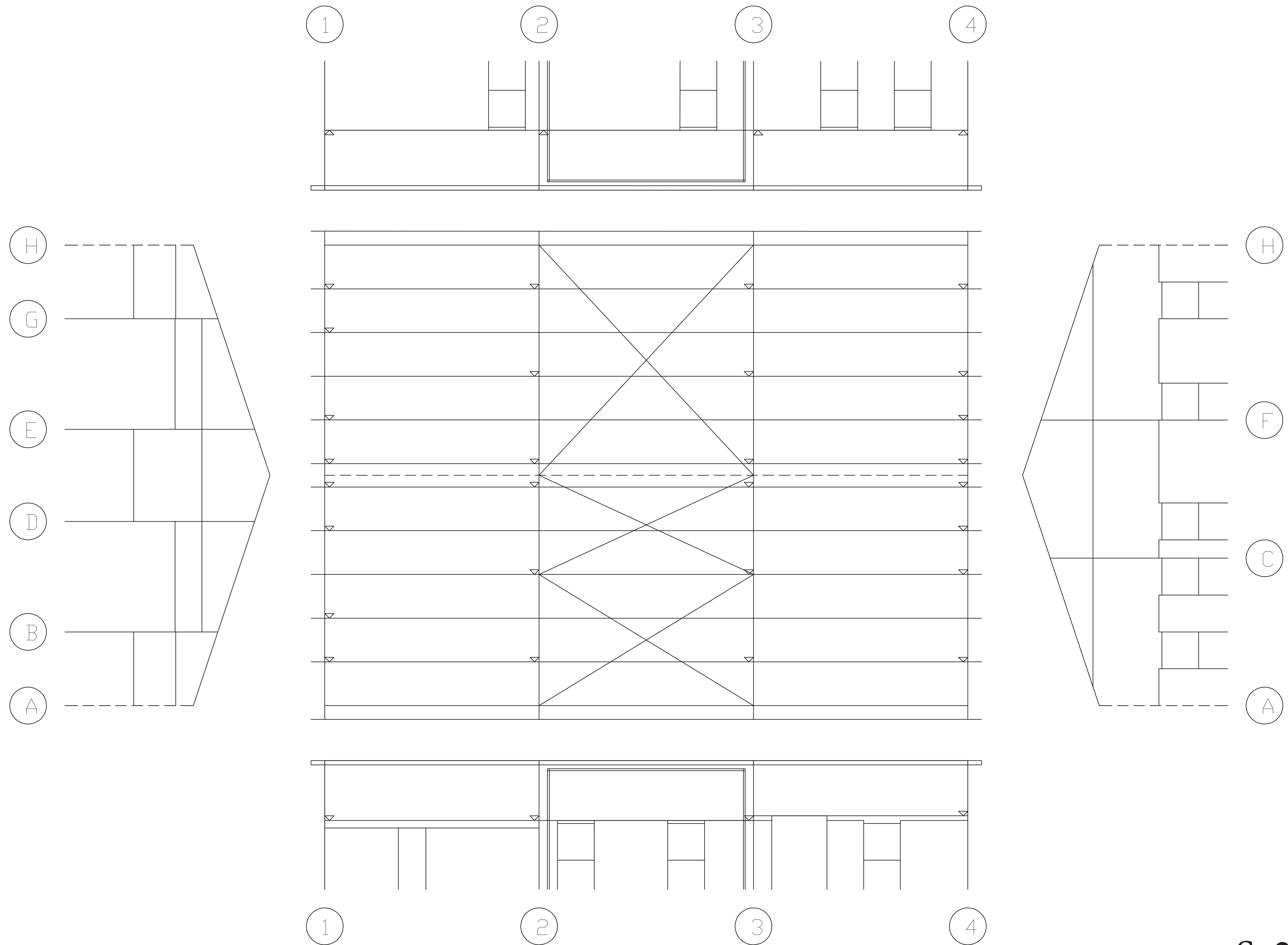
1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.
2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT. DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.
4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

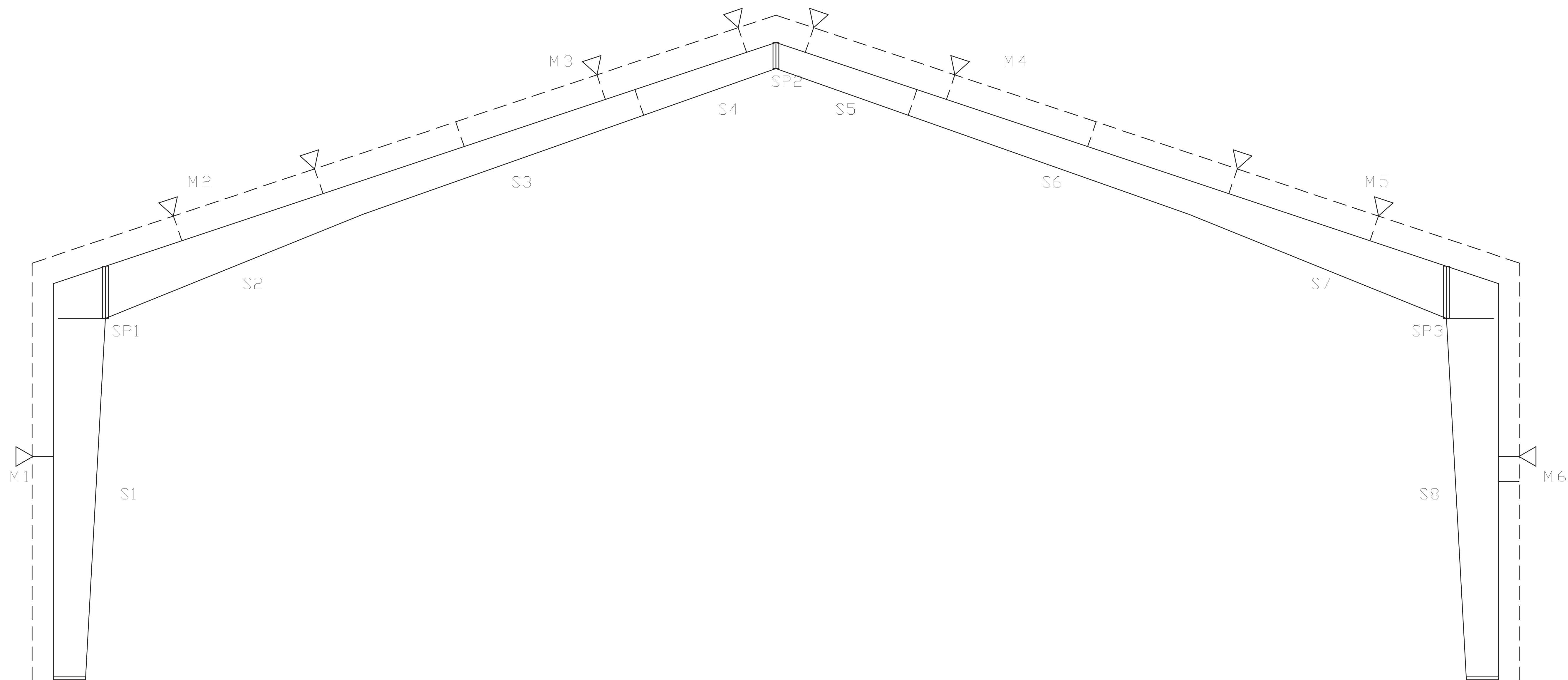
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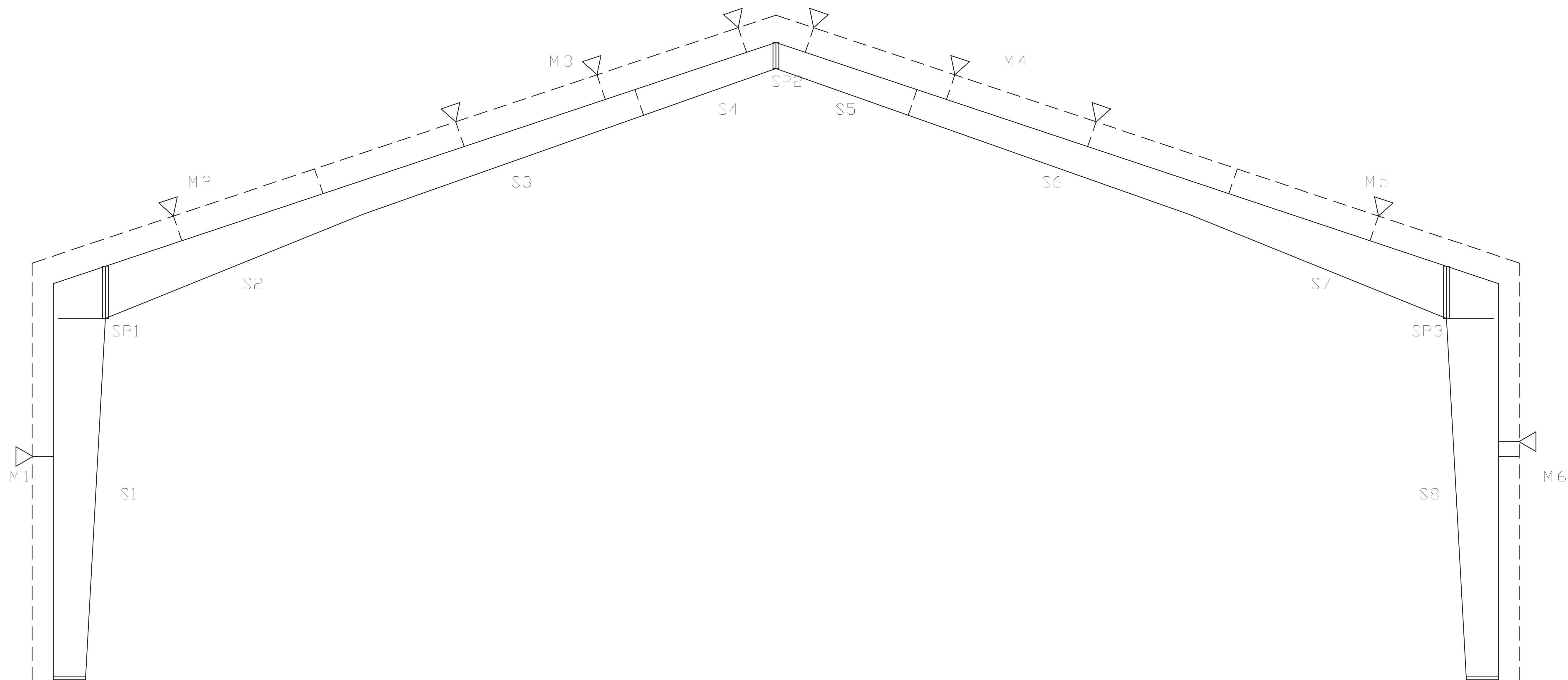


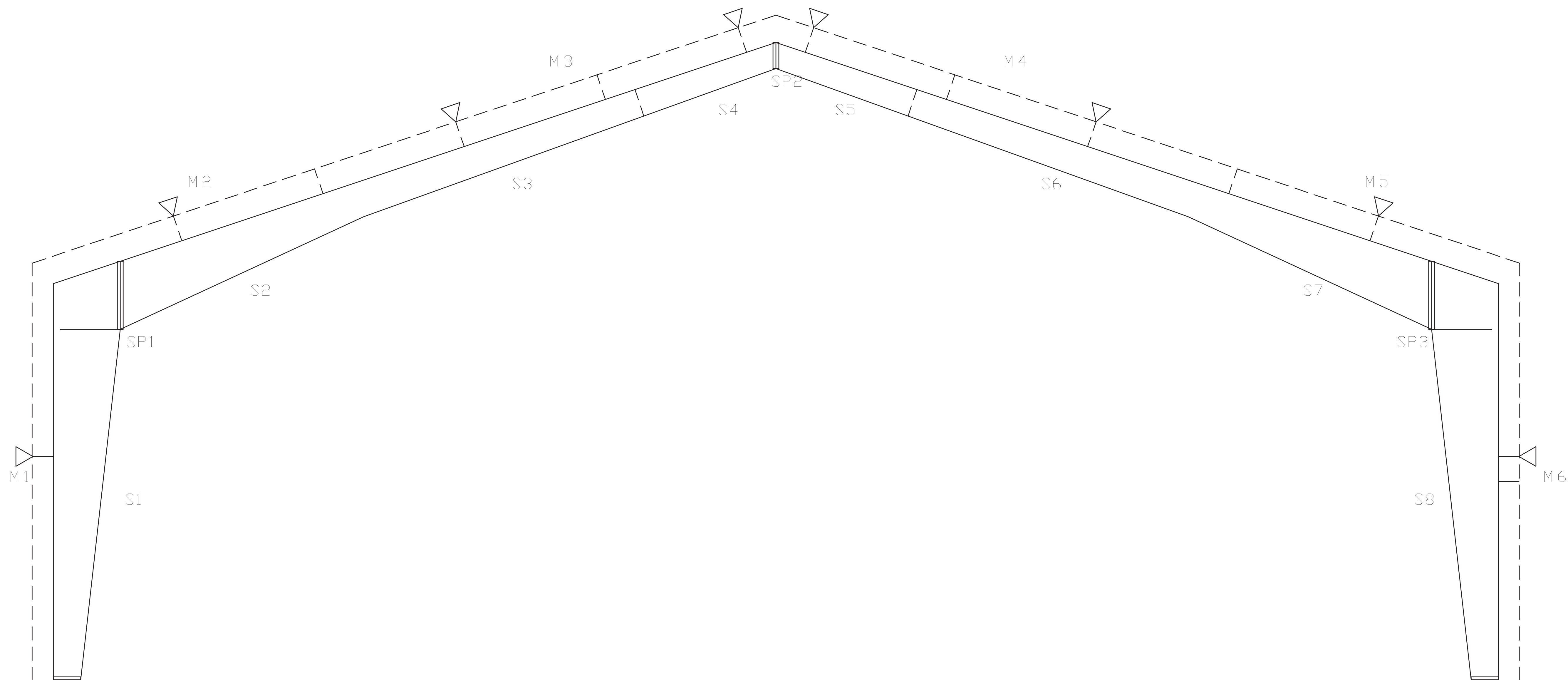
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						LOCATION:	X						
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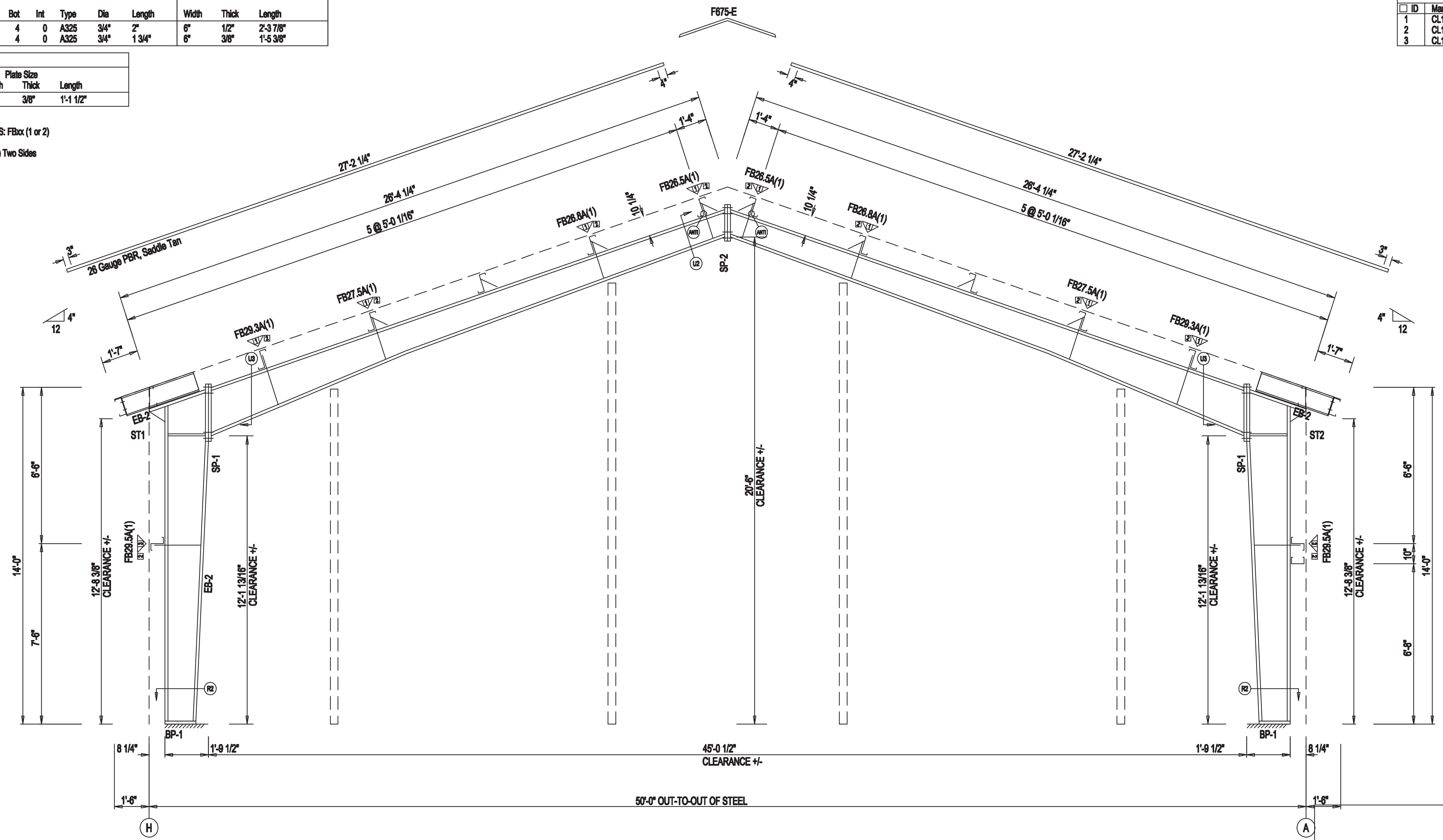
S-29

SPLICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0		A325	3/4"	2"	6"	1/2"	2'-3 7/8"
SP-2	4	4	0		A325	3/4"	1 3/4"	6"	3/8"	1'-5 3/8"

BASE PLATE TABLE			
Col	Width	Thick	Length
BP-1	6"	3/8"	1'-1 1/2"

FLANGE BRACES: FBxx (1 or 2)  
 xx=length(in)  
 (1) One Side; (2) Two Sides  
 A - L2X2X14G

CONNECTION PLATES	
ID	Mark/Part
1	CL197
2	CL198
3	CL198



RIGID FRAME ELEVATION: FRAME LINE 1

**GENERAL NOTES:**

- BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
- ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
 NORTH LITTLE ROCK, AR 72116-7606  
 1-800-443-5555

S-30

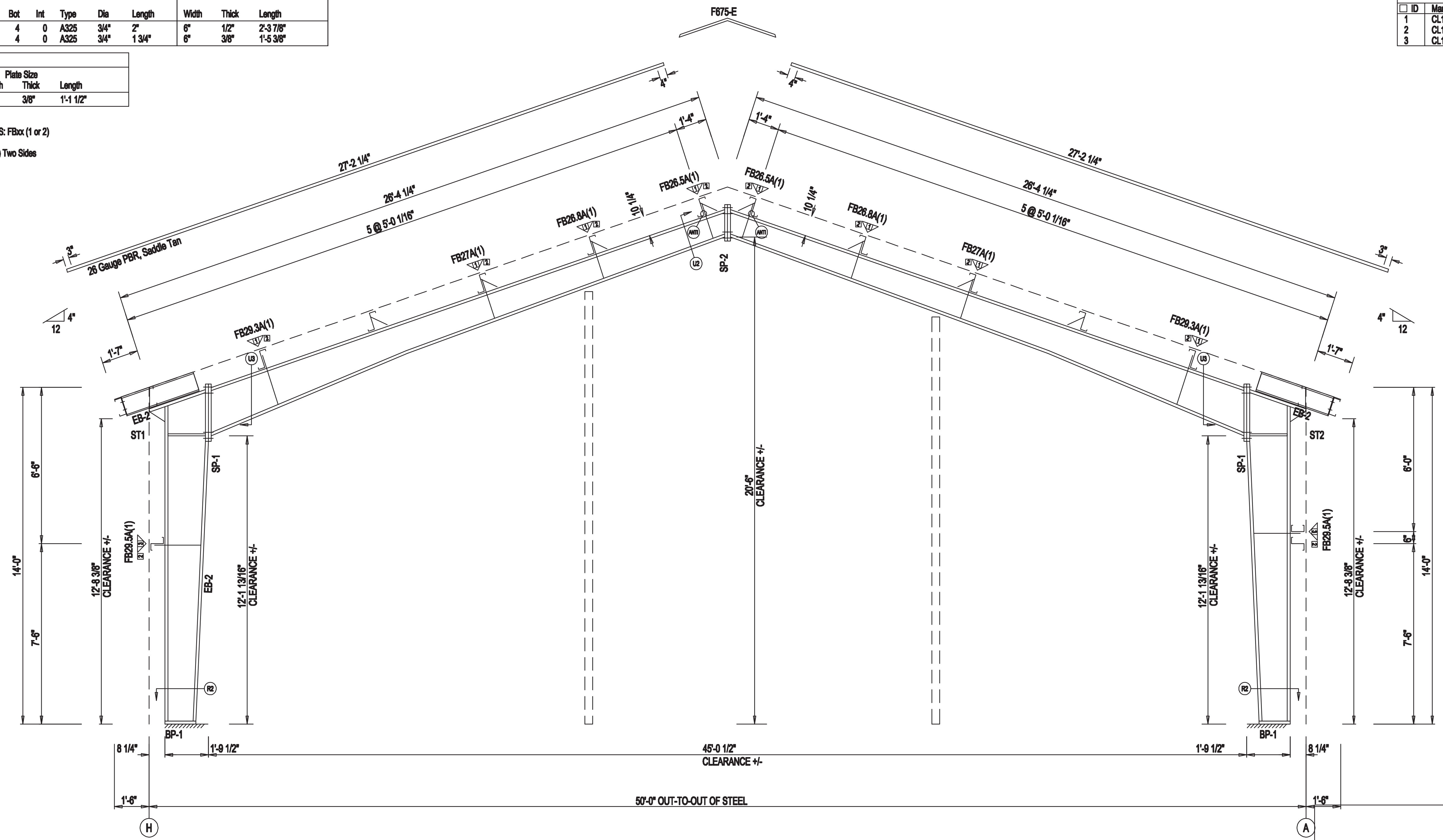
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LOCATION:		CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
			4/12/22	N.T.S.	1	A		E7	0

SPLICE PLATE & BOLT TABLE										
Mark	Qty	Top	Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	0		A325	3/4"	2"	6"	1/2"	2'-3 7/8"
SP-2	4	4	0		A325	3/4"	1 3/4"	6"	3/8"	1'-5 3/8"

BASE PLATE TABLE			
Col	Width	Thick	Length
BP-1	6"	3/8"	1'-1 1/2"

FLANGE BRACES: FBxx (1 or 2)  
 xx=length(in)  
 (1) One Side; (2) Two Sides  
 A - L2X2X14G

CONNECTION PLATES	
ID	Mark/Part
1	CL197
2	CL198
3	CL198



RIGID FRAME ELEVATION: FRAME LINE 4

**GENERAL NOTES:**

- BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
- ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

ISSUE	DATE	DESCRIPTION	BY	CKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
 NORTH LITTLE ROCK, AR 72116-7605  
 1-800-643-8555

PROJECT:		CUSTOMER:		OWNER:					
LOCATION:		CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE
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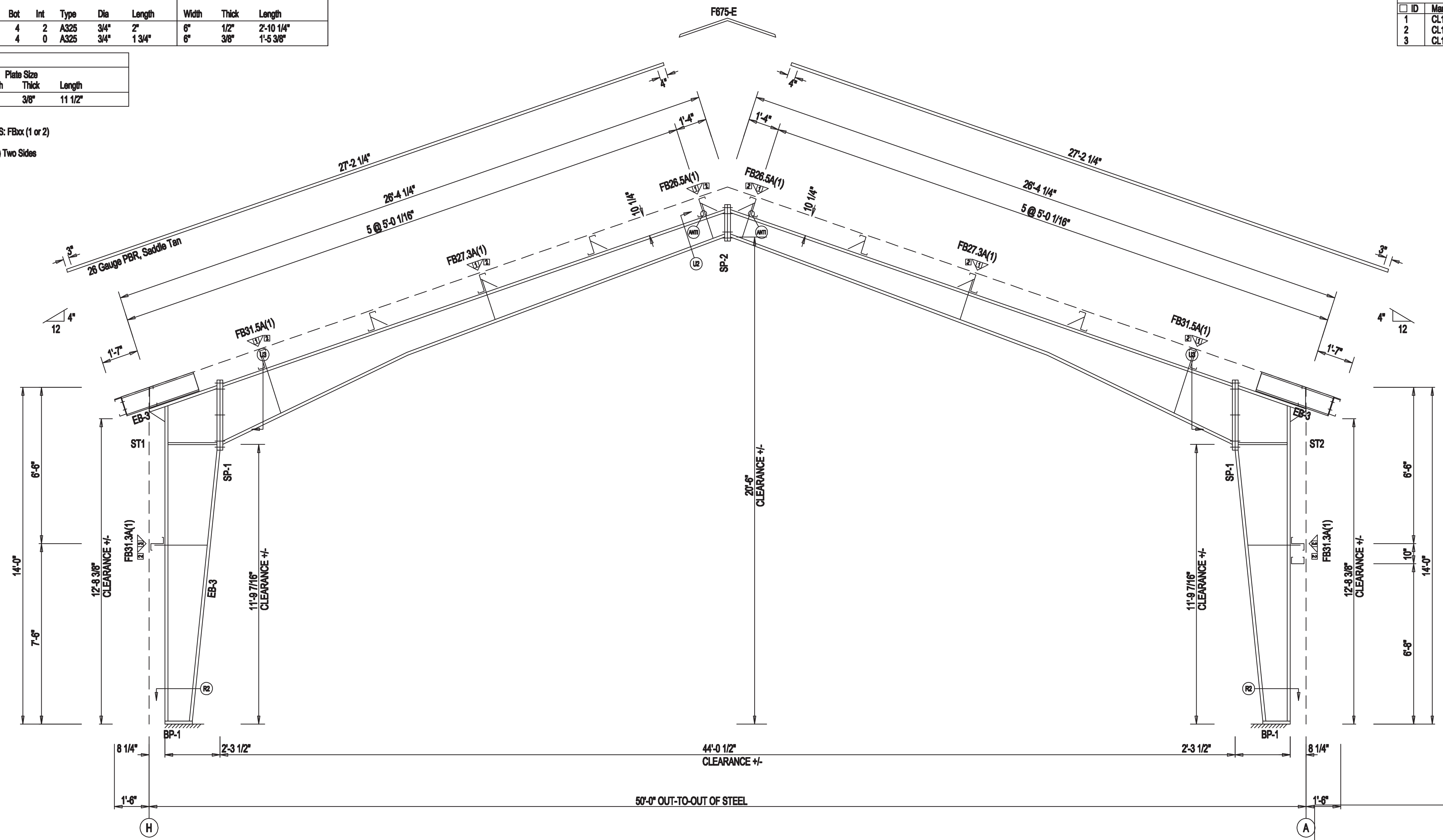
S-31

SPLICE PLATE & BOLT TABLE									
Mark	Qty Top	Qty Bot	Int	Type	Dia	Length	Width	Thick	Length
SP-1	4	4	2	A325	3/4"	2"	6"	1/2"	2'-10 1/4"
SP-2	4	4	0	A325	3/4"	1 3/4"	6"	3/8"	1'-5 3/8"

BASE PLATE TABLE			
Col Mark	Width	Plate Size Thick	Length
BP-1	6"	3/8"	11 1/2"

FLANGE BRACES: FBxx (1 or 2)  
 xx=length(in)  
 (1) One Side; (2) Two Sides  
 A - L2X2X14G

CONNECTION PLATES	
ID	Mark/Part
1	CL197
2	CL198
3	CL198



RIGID FRAME ELEVATION: FRAME LINE 2 3

GENERAL NOTES:

- BOLT TIGHTENING - ALL BOLTED JOINTS WITH A325 TYPE 1 BOLTS ARE SPECIFIED AS SNUG-TIGHTENED JOINTS IN ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. ACCORDANCE WITH THE MOST RECENT EDITION OF THE RCSC SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A325 OR A490 BOLTS. PRE-TENSIONING METHODS, INCLUDING TURN-OF-NUT, CALIBRATED WRENCH, TWIST-OFF-TYPE TENSION-CONTROL BOLTS OR DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS DIRECT-TENSION-INDICATOR ARE NOT REQUIRED. INSTALLATION INSPECTION REQUIREMENTS FOR SNUG TIGHT BOLTS (SPECIFICATION FOR STRUCTURAL JOINTS SECTION 9.1) IS SUGGESTED.
- ALL FIELD CONNECTIONS OF SECONDARY FRAMING SHALL BE BOLTED WITH A325 BOLTS.
- INSTALL ALL FLANGE BRACES ON COLUMN AND RAFTER AS SHOWN.

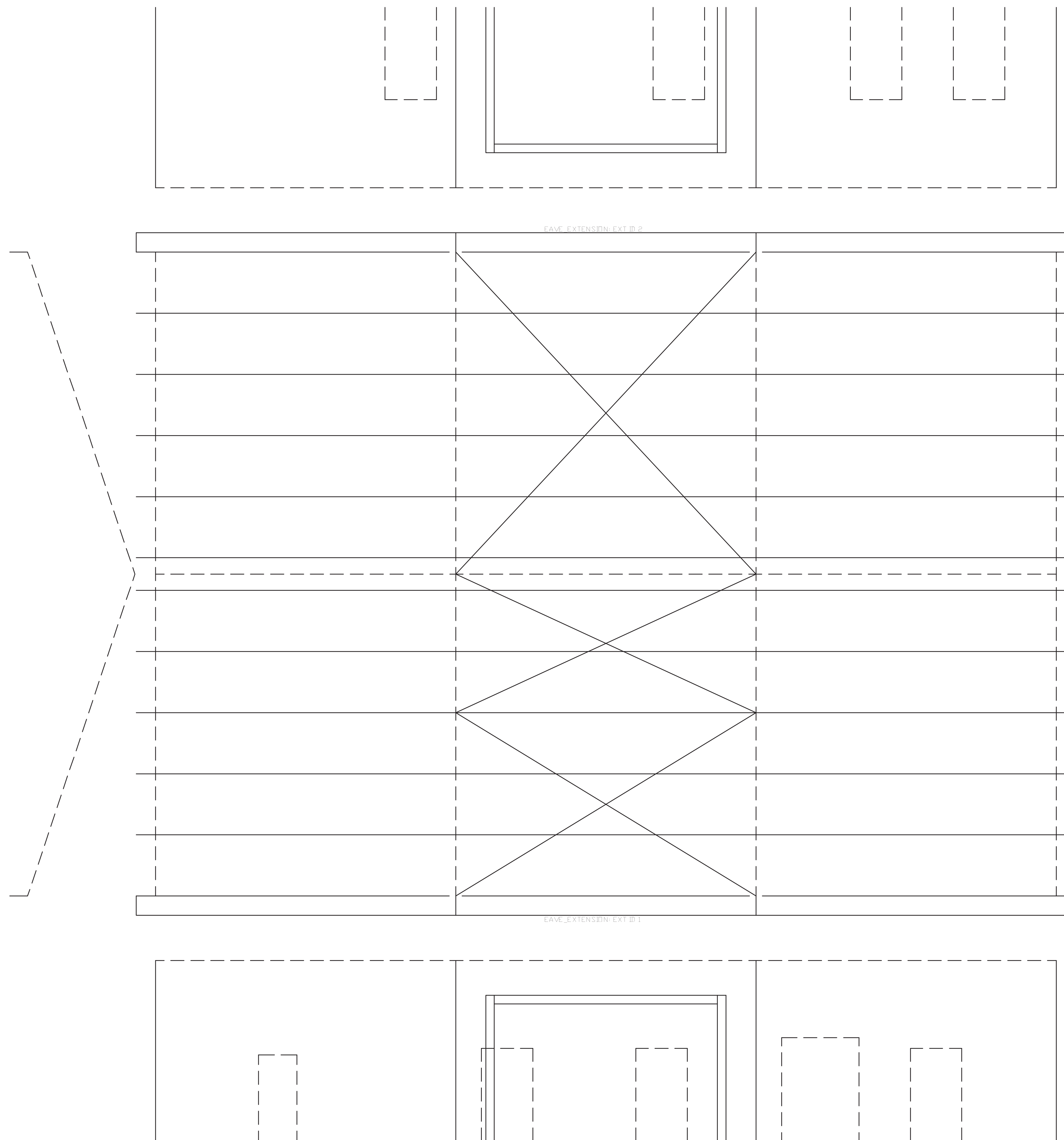
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0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #365  
 NORTH LITTLE ROCK, AR 72116-7606  
 1-800-643-8555

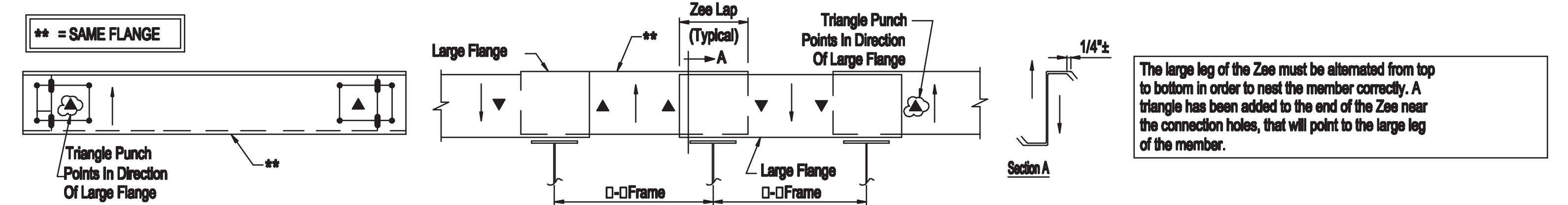
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CUSTOMER:	OWNER:						
LOCATION:							
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S-32

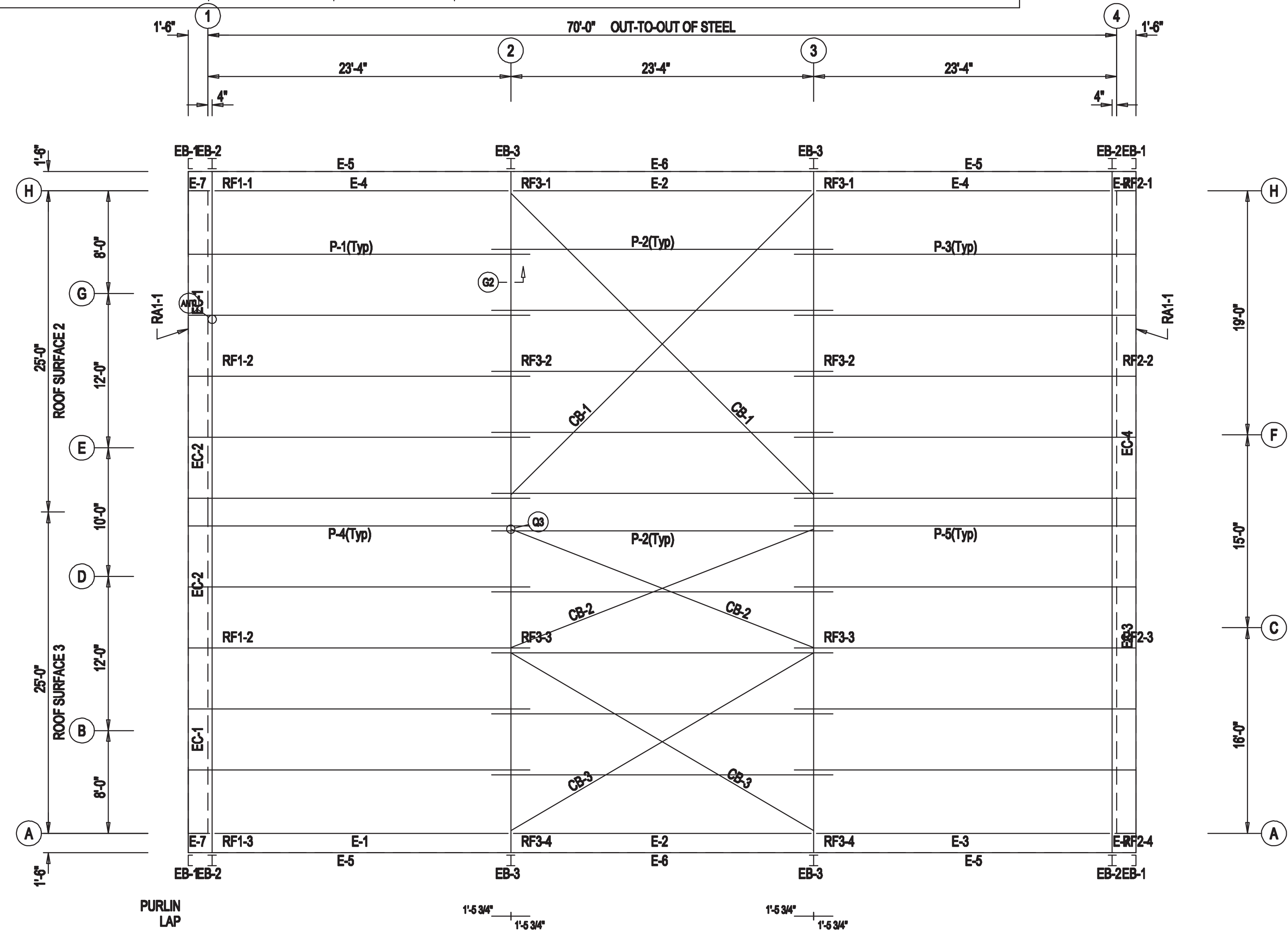


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EXTENSION/CANOPY BOLTS				
ROOF PLAN				
MARK	QUAN	TYPE	DIA	LENGTH
EB-2	4	A325	1/2"	1 1/4"
EB-3	4	A325	1/2"	1 1/4"



ROOF FRAMING PLAN

- GENERAL NOTES:**
1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
  2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
  3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
  4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
  5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
  6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CHKD	DSN
0	4/12/22	FOR QUOTE			

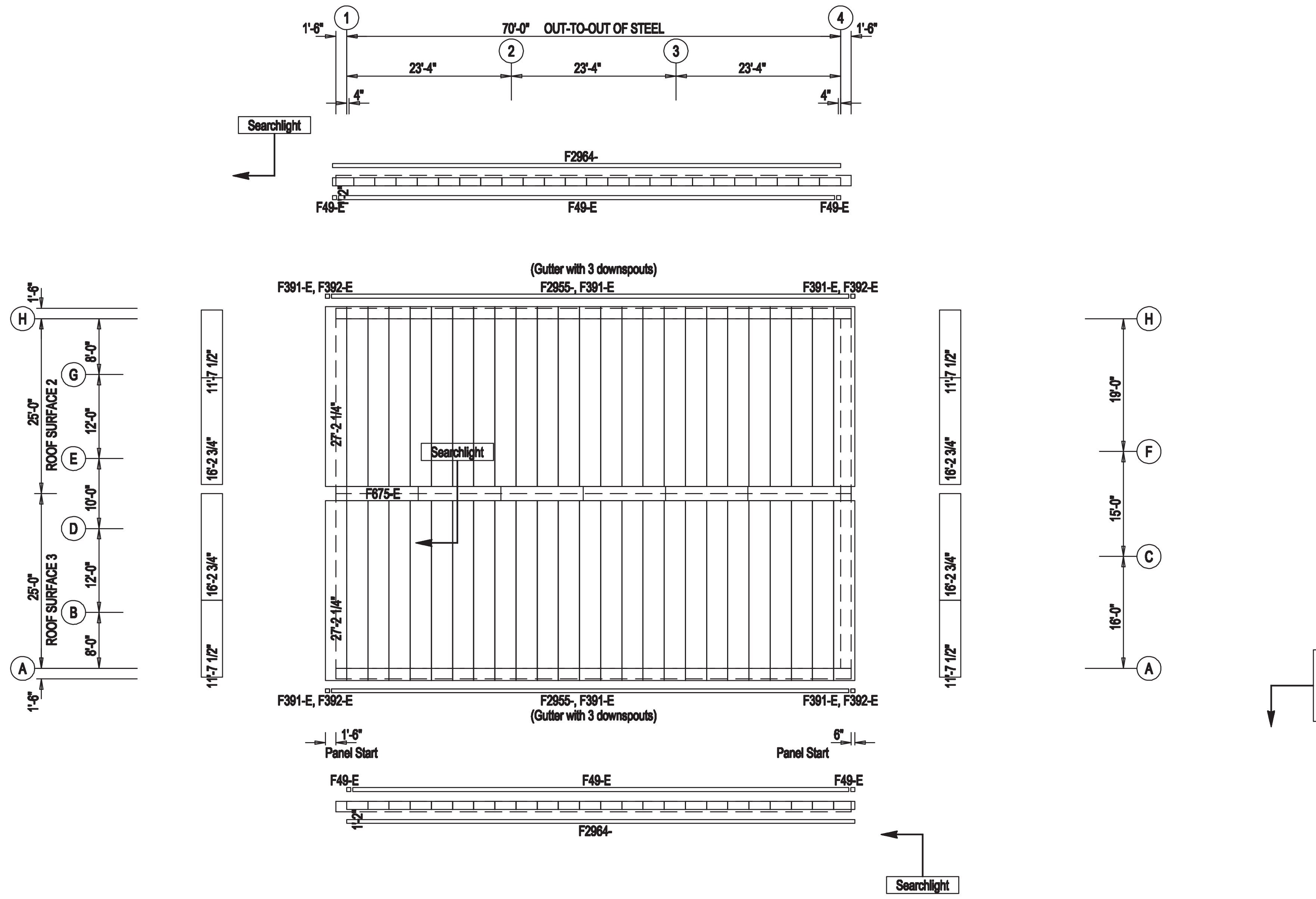
**HERITAGE**  
BUILDING SYSTEMS

2513 MCCAIN BLVD, STE 2 #385  
NORTH LITTLE ROCK, AR 72116-7606  
1-800-443-8555

PROJECT:		CUSTOMER:		OWNER:	
LOCATION:		CAD	DATE	SCALE	PHASE
			4/12/22	N.T.S.	1
BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE		
A		E1	0		

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**PBR ROOF SHEETING NOTE:**  
PBR ROOF PANELS ARE TO BE FIELD CUT IF THE PANELS EXTEND  
OUTSIDE OF THE ROOF PLANE, PANELS ARE NOT TO BE BACK LAPPED.



**ROOF SHEETING PLAN**  
PANELS: 26 Gauge PBR - Saddle Tan

- GENERAL NOTES:**
1. INSTALL ALL PURLIN AND FLANGE BRACES (FB) AS SHOWN.
  2. ROOF PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.
  3. STRUT PURLINS, IF PROVIDED, MUST BE INSTALLED AND FASTENED TO ROOF SHEETING PER "PBR" PANEL ROOF DETAIL.
  4. DO NOT ADD ANY ADDITIONAL ROOF OPENINGS WITHOUT BUILDING MANUFACTURER APPROVAL OR PROFESSIONAL ENGINEER APPROVAL.
  5. DO NOT STACK SHEET BUNDLES ON ROOF. ONLY RAISE INDIVIDUAL SHEETS AS NEEDED.
  6. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

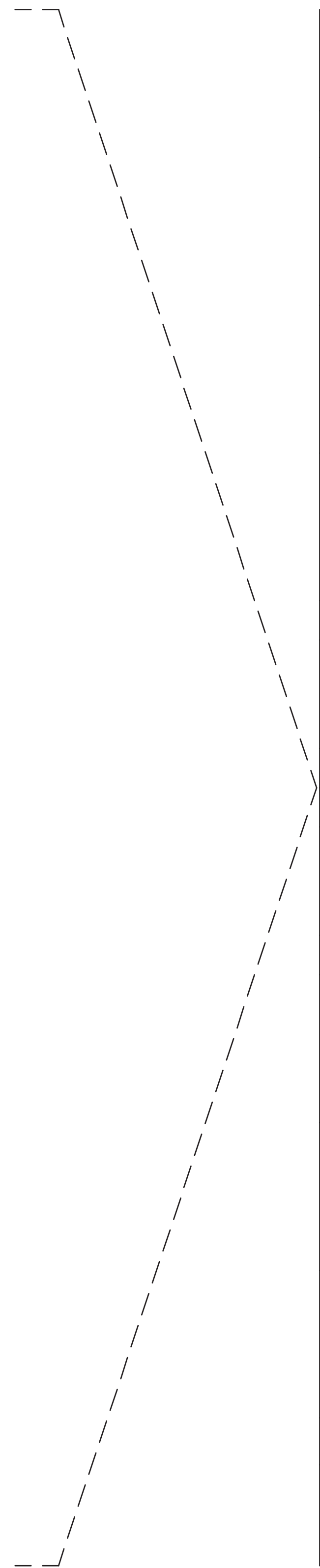
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0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

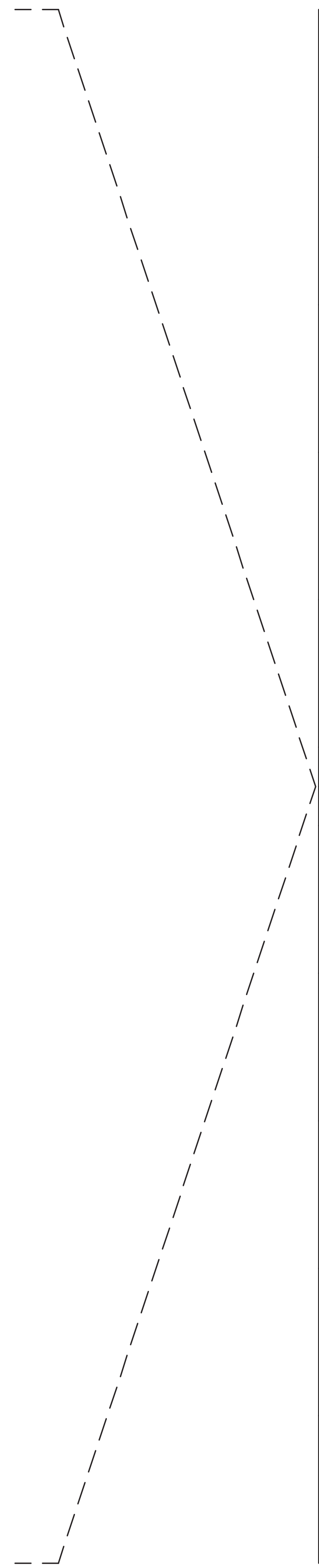
2513 MCCAIN BLVD., STE 2 #385  
NORTH LITTLE ROCK, AR 72116-7606  
1-800-643-5555

PROJECT:							
CUSTOMER:			OWNER:				
LOCATION:							
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	4/12/22	N.T.S.	1	A		E2	0

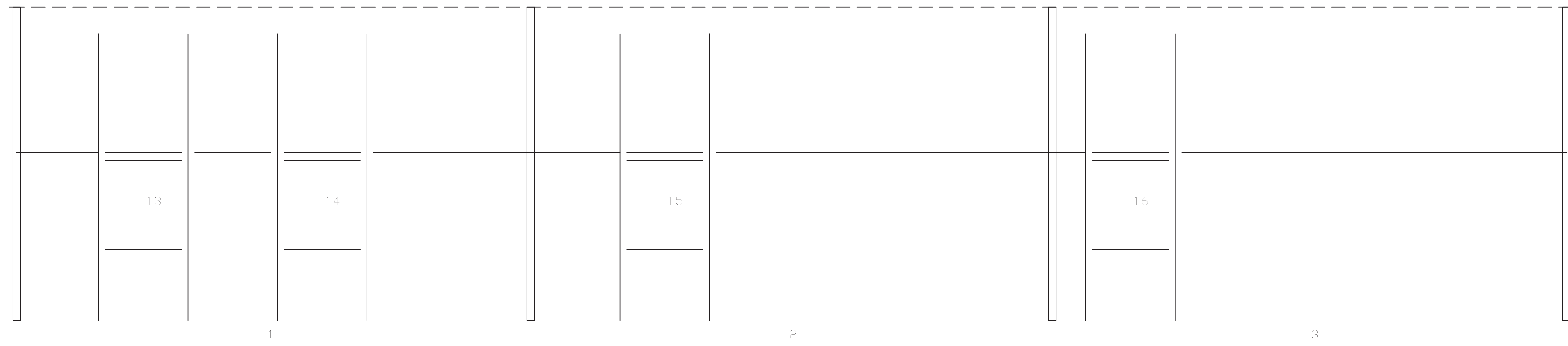
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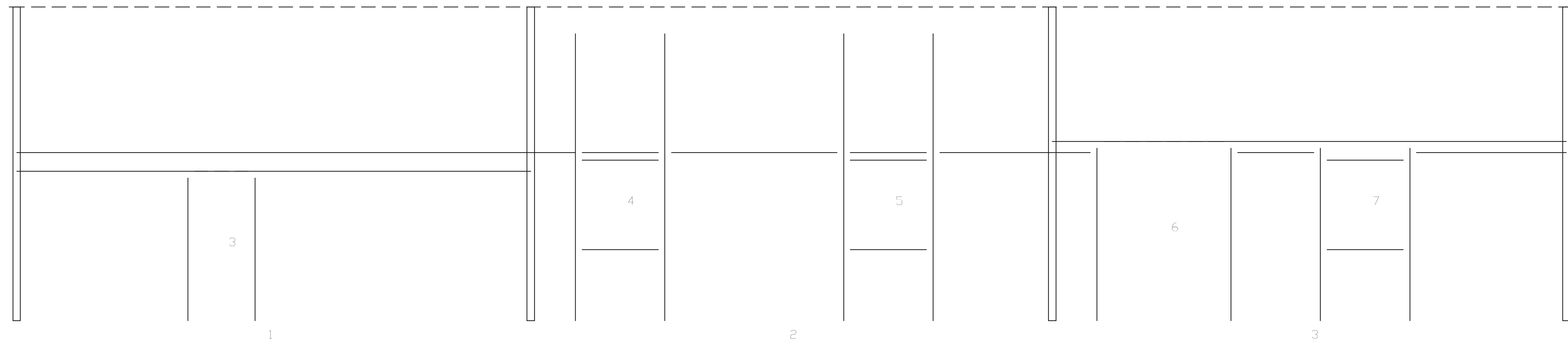


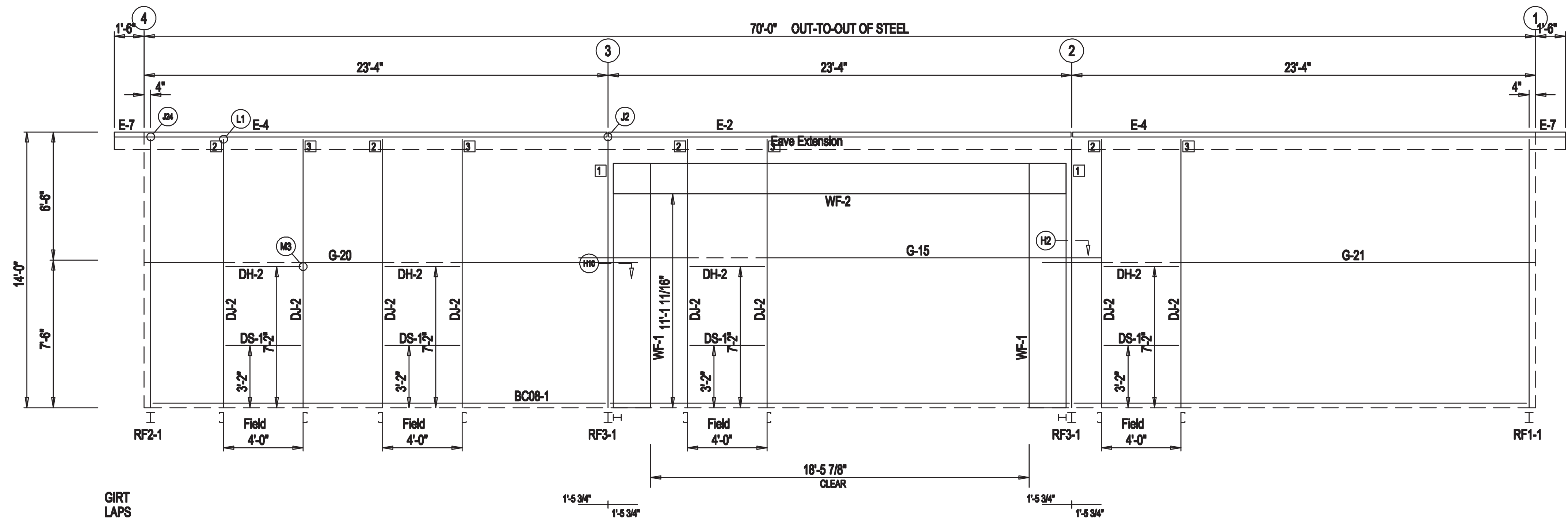
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	3							5	
16				1					19
18	10			6				9	21
17	7			4				8	20
16	3			1				5	19
18									21



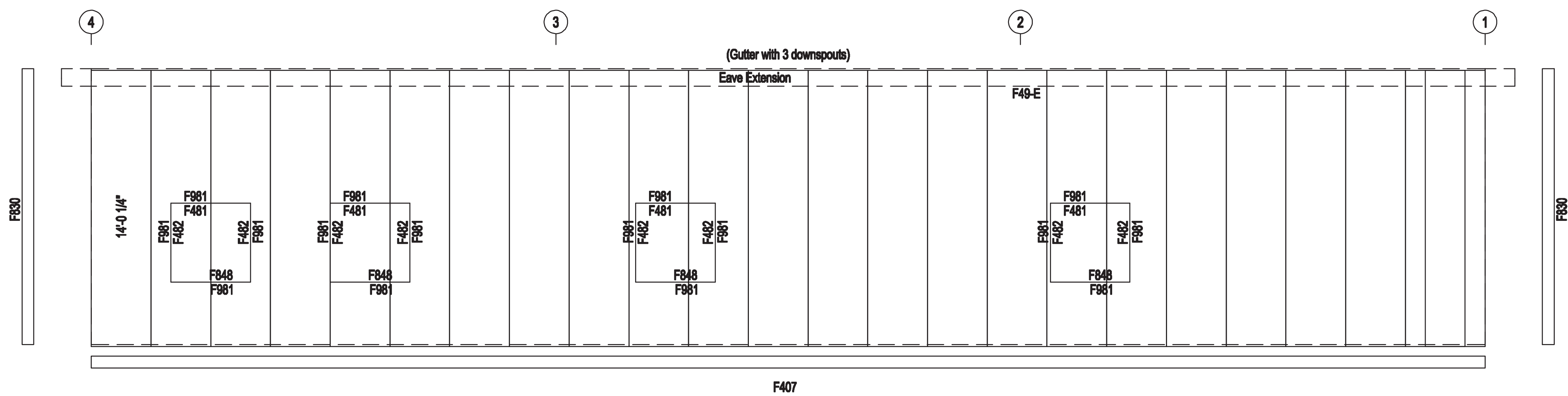
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16		1	19







SIDEWALL FRAMING: FRAME LINE H



SIDEWALL SHEETING & TRIM: FRAME LINE H

PANELS: 26 Gauge PBR - Light Stone

BOLT TABLE				
FRAME LINE H				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RF3-1	8	A325	3/4"	1 1/2"

CONNECTION PLATES	
FRAME LINE H	
ID	MARK/PART
1	SC479
2	SC587_L
3	SC587_R

**GENERAL NOTES:**  
 1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.  
 2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.  
 3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.  
 4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

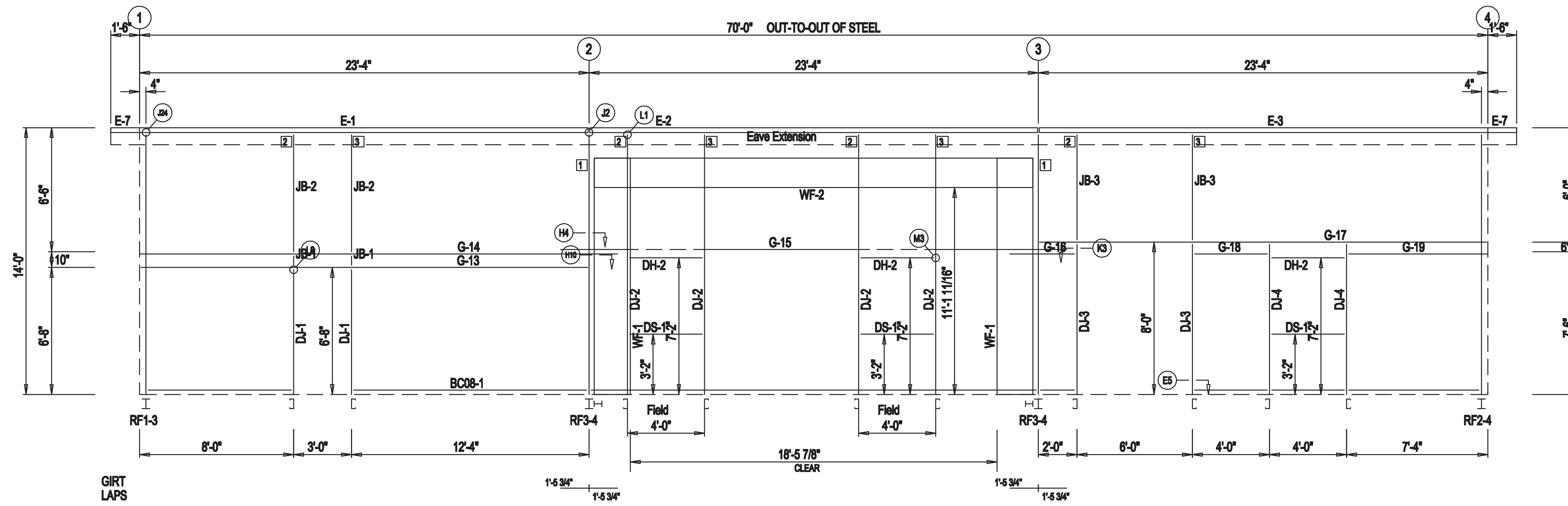
ISSUE	DATE	DESCRIPTION	BY	CHKD	DSN
0	4/12/22	FOR QUOTE			

**HERITAGE**  
BUILDING SYSTEMS

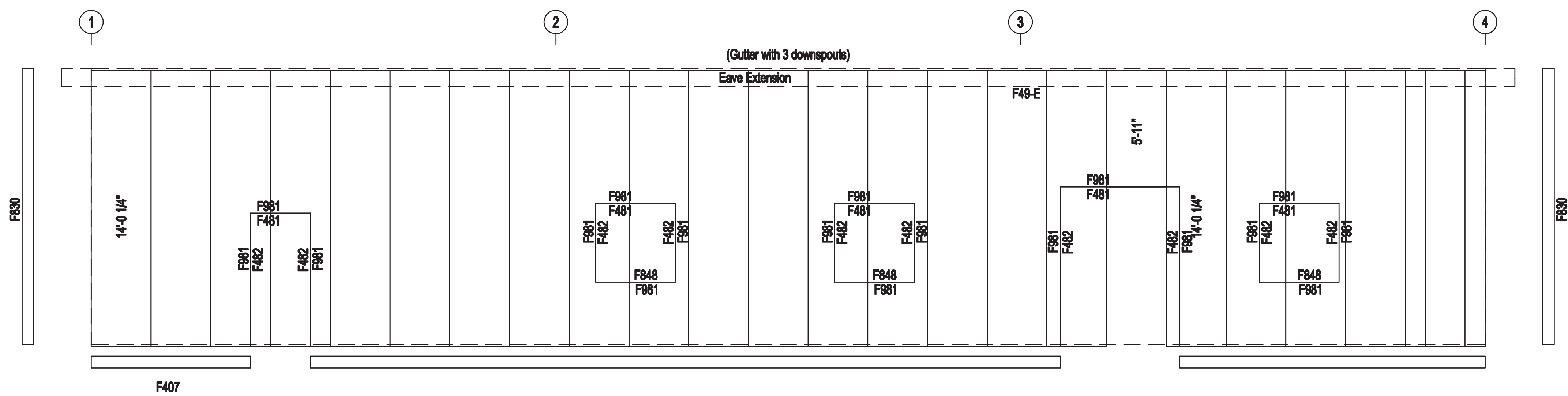
2513 MCCAIN BLVD, STE 2 #385  
 NORTH LITTLE ROCK, AR 72116-7606  
 1-800-443-6555

PROJECT:								
CUSTOMER:			OWNER:					
LOCATION:								
CAD	DATE	SCALE	PHASE	BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE	
	4/12/22	N.T.S.	1	A		E4	0	

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SIDEWALL FRAMING: FRAME LINE A



SIDEWALL SHEETING & TRIM: FRAME LINE A

PANELS: 26 Gauge PBR - Light Stone

BOLT TABLE				
FRAME LINE A				
LOCATION	QUAN	TYPE	DIA	LENGTH
WF-1 - WF-2	8	A325	3/4"	2"
WF-1 - RF3-4	8	A325	3/4"	1 1/2"

CONNECTION PLATES	
FRAME LINE A	
ID	MARK/PART
1	SC475
2	SC587_L
3	SC587_R

**DOWNSPOUT SPACING LOCATIONS**  
 DOWNSPOUTS ARE TO BE PLACED AT A SPACING NOT TO EXCEED ?? FT. WITH A DOWNSPOUT WITHIN ?? FT. OF EACH END OF THE GUTTER RUN. GUTTER STRAPS TO BE 2'-0" ON CENTER.

**GENERAL NOTES:**  
 1. INSTALL ALL GIRTS AND FLANGE BRACES (FB) AS SHOWN.  
 2. WALL PANEL PROVIDES STRUCTURAL STABILITY TO THE BUILDING.  
 3. OTHER THAN FOR WALK DOORS AND WINDOWS SHOWN ON THE CONTRACT, DO NOT ADD ADDITIONAL WALL OPENINGS WITHOUT APPROVAL OF BUILDING MANUFACTURER OR PROFESSIONAL ENGINEER.  
 4. AFTER INSTALLATION, WIPE ALL PANELS CLEAN OF METAL SHAVINGS CAUSED BY DRILLING.

ISSUE	DATE	DESCRIPTION	BY	CHKD	DSN
0	4/12/22	FOR QUOTE			

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			4/12/22	N.T.S.	1
BUILDING ID	JOB NUMBER	SHEET NUMBER	ISSUE		
A		E3	0		

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