### STATEMENT OF SPECIAL INSPECTIONS

- THE OWNER SHALL EMPLOY ONE OR MORE SPECIAL INSPECTORS TO PROVIDE INSPECTION DURING CONSTRUCTION. THE SPECIAL INSPECTOR(S) SHALL BE QUALIFIED TO THE SATISFACTION OF THE BUILDING OFFICIAL TO INSPECT THE KIND OF CONSTRUCTION BEING EMPLOYED IN THIS PROJECT. THE SPECIAL INSPECTOR SHALL SUBMIT REPORTS INDICATING RESULTS AND OBSERVATIONS OF TESTS AND INSPECTIONS AND STATING COMPLIANCE OR NONCOMPLIANCE WITH CONTRACT DOCUMENTS TO STRUCTURAL ENGINEER AND TO GOVERNING CODE AUTHORITY.
- TESTING LABORATORY SHALL PROVIDE SPECIAL INSPECTION, COMPLYING WITH LABC SECTION 1701 (UNLESS OTHERWISE NOTED). FOR THE FOLLOWING:
- EPOXY ANCHORS
- BOLTS INSTALLED IN CONCRETE CONCRETE STRENGTH f'c > 2,500 PSI
- SHEATHED SHEAR WALL WHEN SHEAR EXCEEDS 350 POUNDS PER LINEAR FOOT WHERE THE FASTENER SPACING OF THE SHEATHING IS 4 INCHES ON CENTER OR LESS.
- CONTRACTORS RESPONSIBLE FOR THE CONSTRUCTION OF A WIND OR SEISMIC FORCE RESISTING SYSTEM/COMPONENT LISTED IN THE "STATEMENT OF SPECIAL INSPECTIONS" SHALL SUBMIT A WRITTEN STATEMENT OF RESPONSIBILITY TO THE LADBS INSPECTORS AND THE OWNER PRIOR TO THE COMMENCEMENT OF WORK ON SUCH SYSTEM OR COMPONENT PER SECTION 1704.4.
- CONTINUOUS SPECIAL INSPECTOR BY A REGISTERED DEPUTY INSPECTOR IS REQUIRED FOR FIELD WELDING, POST-INSTALLED ADHESIVE ANCHORS INSTALLED HORIZONTALLY OR UPWARDLY INCLINED TO RESIST SUSTAINED TENSION LOADS, SHOTCRETE PLACEMENT, CONCRETE STRENGTH f'c > 2,500 PSI, HIGH STRENGTH BOLTING, SPRAYED-ON FIREPROOFING, ENGINEERED MASONRY, HIGH-LIFT GROUTING, PRE-STRESSED CONCRETE, HIGH LOAD DIAPHRAGMS, SPECIAL MOMENT-RESISTING CONCRETE FRAMES, AND HELICAL PILE FOUNDATIONS.
- PERIODIC SPECIAL INSPECTION IS REQUIRED FOR WOOD SHEAR WALLS, SHEAR PANELS, AND DIAPHRAGMS, INCLUDING NAILING, BOLTING, ANCHORING, AND OTHER FASTENING TO COMPONENTS OF THE SEISMIC FORCE RESISTING SYSTEM. SPECIAL INSPECTION BY A DEPUTY INSPECTOR IS REQUIRED WHERE THE FASTENER SPACING OF THE SHEATHING IS 4 INCHES ON CENTER OR LESS.

#### STRUCTURAL OBSERVATIONS

- STRUCTURAL OBSERVATION IS REQUIRED FOR THE STRUCRURAL SYSTEM IN ACCORDANCE WITH CITY OF SAN JOSE ORDINANCES. STRUCTURAL OBSERVATION IS THE VISUAL OBSERVATION OF THE ELEMENTS AND CONNECTIONS OF THE STRUCTURAL SYSTEM AT SIGNIFICANT CONSTRUCTION STAGES AND THE COMPLETED STRUCTURE FOR GENERAL CONFORMANCE TO THE APPROVED PLANS AND SPECIFICATIONS. STRUCTURAL OBSERVATION DOES NOT WAIVE THE RESPONSIBILITY FOR THE INSPECTIONS REQUIRED OF THE BUILDING INSPECTOR OR THE DEPUTY INSPECTOR.
- THE OWNER SHALL EMPLOY A CIVIL OR STRUCTURAL ENGINEER OR ARCHITECT TO PERFORM THE STRUCTURAL OBSERVATION. THE ENGINEER OR ARCHITECT SHALL BE REGISTERED OR LICENSED IN THE STATE OF CALIFORNIA. THE DEPARTMENT OF BUILDING & SAFETY RECOMMENDS THE USE OF THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN WHEN THEY ARE INDEPENDENT OF THE CONTACTOR.
- THE STRUCTURAL OBSERVER SHALL PROVIDE EVIDENCE OF EMPLOYMENT BY THE OWNER. A LETTER FROM THE OWNER OR A COPY OF THE AGREEMENT FOR SERVICES SHALL BE SENT TO THE BUILDING INSPECTOR BEFORE THE FIRST SITE VISIT. THE STRUCTURAL OBSERVER SHALL ALSO INFORM THE OWNER OF THE REQUIREMENTS FOR A PRE-CONSTRUCTION MEETING AND SHALL PRESIDE OVER THAT MEETING.
- . THE OWNER OR OWNER'S REPRESENTATIVE SHALL COORDINATE AND CALL FOR A MEETING BETWEEN THE ENGINEER OR ARCHITECT RESPONSIBLE FOR THE STRUCTURAL DESIGN, STRUCTURAL OBSERVER, CONTRACTOR, AFFECTED SUBCONTRACTORS AND DEPUTY INSPECTORS. THE PURPOSE OF THE MEETING SHALL BE TO IDENTIFY THE MAJOR STRUCTURAL ELEMENTS AND CONNECTIONS THAT AFFECT THE VERTICAL AND LATERAL LOAD SYSTEMS OF THE STRUCTURE AND TO REVIEW SCHEDULING OF THE REQUIRED OBSERVATIONS. A RECORD OF THE MEETING SHALL BE INCLUDED IN THE FIRST OBSERVATION REPORT AND SUBMITTED TO THE BUILDING INSPECTOR.
- 5. THE STRUCTURAL OBSERVER SHALL PERFORM SITE VISITS AT THOSE STEPS IN THE PROGRESS OF THE WORK THAT ALLOW FOR CORRECTION OF DEFICIENCIES WITHOUT SUBSTANTIAL EFFORT OR UNCOVERING OF THE WORK INVOLVED. AT A MINIMUM. THE FOLLOWING SIGNIFICANT CONSTRUCTION STAGES REQUIRE A SITE VISIT AND AN OBSERVATION REPORT FROM THE STRUCTURAL ENGINEER.

STRUCTURAL OBSERVATION & DESIGNATION OF THE				
STRUCTURAL OBSERVER				
PROJECT ADDRESS: 10818 CROTHERS F	ROAD, SAN JOSE, CA S	95127 PE	RMIT APPL. NO.:	
DESCRIPTION OF WORK: NEW ADU				
OWNER: LARSON RESIDENCE	ARCHITECT: ECO	-STRUCTION EN	IGINEER: THANG LE, SE	
(ON	STRUCTURAL OE			
FIRM OR INDIVIDUAL TO BE RESPONSIBL	E FOR THE STRUCTUR	AL OBSERVATIONS: THANG	LE, S.E.	
NAME: <u>THANG LE</u> F	PHONE: 626-731-153	9 CALIF.	REGISTRATION: S4978	
FOUNDATION	WALL	FRAME	DIAPHRAGM	
FTG., STEM WALLS, PIERS	CONCRETE	STL. MMNT. FRM.	CONCRETE	
MAT FOUNDATION	MASONRY	STL. BRACED FRM.	STEEL DECK	
CAISSON, PILES, GRD. BMS.	WOOD SHEAR	CONC. MMNT. FRM.	V WOOD	
STEPPED FTG./RETAINING FND. HILLSIDE SPECIAL ANCHORS	WALL GREATER THAN 350 PLF	MAS. WALL FRM.	OTHERS	
OTHERS:	RASTRA	OTHERS:		
BUILDING INSPECTOR'S OFFICE AND SHALL BE SIGNED AND SEALED (WET STAMPED) BY THE RESPONSIBLE STRUCTURAL OBSERVER. ONE COPY OF THE OBSERVATION REPORT SHALL BE ATTACHED TO THE APPROVED PLANS. COPIES OF THE REPORT SHALL ALSO BE GIVEN TO THE OWNER, CONTRACTOR AND DEPUTY INSPECTOR.				
7. A FINAL OBSERVATION REPORT MUST BE SUBMITTED WHICH SHOWS THAT ALL OBSERVED DEFICIENCIES WERE RESOLVED AND THE STRUCTURAL SYSTEM GENERALLY CONFORMS WITH THE APPROVED PLANS AND SPECIFICATIONS. THE DEPARTMENT OF BUILDING AND SAFETY WILL <u>NOT</u> ACCEPT STRUCTURAL WORK WITHOUT THIS FINAL OBSERVATION REPORT AND THE CORRECTION OF SPECIFIC DEFICIENCIES NOTED DURING NORMAL BUILDING AND DEPUTY INSPECTION.				
<ul> <li>8. WHEN THE OWNER ELECTS TO CHANGE THE STRUCTURAL OBSERVER OF RECORD, THE OWNER SHALL:</li> <li>a) NOTIFY THE BUILDING INSPECTOR IN WRITING BEFORE THE NEXT INSPECTION;</li> <li>b) CALL AN ADDITIONAL PRE-CONSTRUCTION MEETING AND</li> <li>c) FURNISH THE REPLACEMENT STRUCTURAL OBSERVER WITH A COPY OF ALL PREVIOUS OBSERVATION REPORTS.</li> </ul>				
THE REPLACEMENT STRUCTUR/ THE ORIGINAL OBSERVED DEF CHECK SUPERVISION. THE PO ANY PROPERLY NOTED DEFICI	ICIENCIES UNLESS LICY OF THE DEP/	OTHERWISE APPROVE ARTMENT SHALL BE 1	ED BY PLAN TO CORRECT	
9. THE ENGINEER OR ARCHITECT TO THE STRUCTURAL SYSTEMS APPROVE ALL CHANGES TO TH	S. THE BUILDING [	DEPARTMENT SHALL F	REVIEW AND	

E. ROUGH CARPENTRY

- CONTENT 19%.
- 2. MINIMUM LUMBER GRADES (UNLESS OTHERWISE NOTED): a. STUDS, SILLS, AND PLATES ...... b. JOISTS AND RAFTERS ..... c. BEAMS AND POSTS ..
- 3. ALL FOUNDATION PLATES OR SILLS AND SLEEPERS ON A CONCRETE OR MASONRY, SLAB, FOUNDATIONS SHALL BE PRESSURE TREATED DOUGHLAS FIR.
- 4. WOOD SILL PLATES SHALL BE PRESSURE TREATED, UNLESS OTHERWISE NOTED, WITH THE PLATE PER CBC 2308.3.2.
- SHEARWALLS AND WHERE NOTED OTHERWISE.
- 6. INTERIOR NON-BEARING WALL SILLS MAY BE CONNECTED WITH ICC-ES APPROVED IN CONCRETE CURBS.
- OVERSIZED.
- SECTIONS 2308.4.2.4, 2308.5.9, 2308.5.10, 2308.6.7.2 AND 2308.7.4.
- ON THESE DRAWINGS.
- RAFTERS, AND 8 FEET ON CENTER FOR FLOOR JOISTS.
- 11. UNDER WALLS PARALLEL TO JOISTS PROVIDE DOUBLE JOISTS. UNDER WALLS PERPENDICULAR TO JOISTS PROVIDE SOLID BLOCKING.
- 12. PROVIDE FULL HEIGHT STUDS FROM FLOOR TO ROOF UNLESS OTHERWISE NOTED.
- 13. GLUED LAMINATED MEMBERS
- THE ICC-ES APPROVAL NUMBER ON THE PLANS. e. COMBINATION 24F-V8 DF/DF.
- WRITTEN APPROVAL OF THANG LE. S.E.
- 14. A LADBS CERTIFICATE OF INSPECTION FOR ALL GLUED LAMINATED TIMBER SHALL BE
- 15. A LADBS LICENSED FABRICATOR IS REQUIRED FOR GLU-LAM.
- 17. METAL FRAMING ACCESSORIES: STEEL JOIST HANGERS, FRAMING ANCHORS AND FASTENERS AND OTHER SUCH CONNECTION DEVICES SHALL BE OF STANDARD BE GALVANIZED. "SIMPSON" PART NUMBERS ARE SHOWN ON THE DRAWINGS.
- 18. WOOD WALL STUDS: b. MAXIMUM HEIGHT OF 2x4 STUD WALLS IS 14'-0". c. PROVIDE 2x6 STUDS AT 16"o.c. FOR WALLS 14'-0" TO 18'-0" TALL. d. PROVIDE 2x8 STUDS AT 16"o.c. FOR WALLS 18'-0" TO 22'-0" TALL.
- 19. BEAMS BUILT UP FROM MULTIPLE 2x MEMBERS: a. 2–2x BEAMS – 16d FACE NAIL STAGGERED AT 9"oc. b. 3-2x BEAMS - 5/8" DIAMETER BOLTS STAGGERED AT 18" oc.
- USING "CLIPPED HEAD" OR SINKER NAILS ARE NOT ACCEPTABLE.
- BE OF HOT DIPPED ZINC COATED GALVANIZED STEEL OR STAINLESS STEEL.

SHEATHING F.

- SPAN RATED 24/0, PRODUCT STANDARD DOC PS-1, DOUGLAS FIR-LARCH, STRUCTURAL I (OR CDX).
- FIR-LARCH, STRUCTURAL I (OR CDX).
- 3. WALL SHEATHING SHALL BE APA RATED AS FOLLOWS: a. 15/32" APA RATED SHEATHING, EXPOSURE 1, SPAN RATED 32/16.
- b. 19/32" APA RATED SHEATHING, EXPOSURE 1, SPAN RATED 40/20.
- BLOCKED AND NAILED.

## STRUCTURAL GENERAL NOTES

I. ALL LUMBER SHALL BE GRADE MARKED DOUGLAS FIR - LARCH (DF-L). MAXIMUM MOISTURE

DF-L #2 DF-L #2 DF-L #1

WHICH IS DIRECT CONTACT WITH EARTH, AND SILLS THAT REST ON CONCRETE OR MASONRY

5/8" DIAMETER ANCHOR BOLTS BY 12" EMBEDMENT AT 4'-0"o.c. WITH 3"x3"x0.229" PLATE WASHERS (MINIMUM 2 ANCHOR BOLTS PER PIECE) LOCATED NOT MORE THAN 12" OR LESS THAN 7 DIAMETERS FROM EACH END OF THE PIECE. A PROPERLY SIZED NUT AND 3"x3"x0.229" THICK WASHER SHALL BE TIGHTENED ON EACH BOLT TO

5. PROVIDE WASHERS UNDER HEADS AND NUTS OF BOLTS AND LAG SCREWS BEARING ON WOOD. NUTS ON ALL BOLTS SHALL BE TIGHTENED BEFORE CLOSING IN AND/OR ON COMPLETION OF THE JOB. CUT WASHERS MAY BE USED EXCEPT FOR SILL BOLTS AT

HILTI "X-U" (ESR-2269) WITH 1-1/4" MINIMUM EMBEDMENT POWER DRIVEN FASTENERS. WITH CADMIUM WASHERS, AT 32" ON CENTER. DO NOT USE POWER DRIVEN FASTENERS

7. BOLTS SHALL CONFORM TO ASTM A307. ALL BOLT HOLES SHALL BE DRILLED 1/32 TO 1/16"

8. NO STRUCTURAL MEMBER INCLUDING STUDS AND PLATES SHALL BE CUT OR NOTCHED FOR PIPES, ETC... UNLESS SPECIFICALLY SHOWN, NOTED OR ACCEPTABLE TO THE ARCHITECT OR ENGINEER. FOR REQUIREMENT OF BORED HOLES, CONFORM TO CBC

9. NAILING SHALL CONFORM TO CBC TABLE 2304.10.1 IN ADDITION TO NAILING SPECIFIED IN THESE DRAWINGS. USE COMMON NAILS UNLESS SPECIFICALLY OTHERWISE NOTED

10. PROVIDE 2x FULL HEIGHT BLOCKING AT EACH SUPPORT, 10 FEET ON CENTER FOR ROOF

a. THE LAM BEAMS ARE TO BE FABRICATED IN THE SHOP OF A LICENSED FABRICATOR. b. THE MANUFACTURER'S LOGO IS TO BE IMPRINTED ON THE SIDE OF THE LAM BEAM. c. THE LAM BEAMS ARE TO BE LOAD TESTED BY THE MANUFACTURER AND THE TEST RESULTS SUBMITTED SUBMITTED TO THE BUILDING INSPECTOR. d. SPECIFY THE NAME OF THE MANUFACTURER OF THE LAM BEAMS AND SHOW

f. COMBINATION 24F-V4 DF/DF MAY BE USED ON SIMPLE SPAN MEMBERS WITH THE

SUBMITTED TO A BUILDING AND SAFETY DIVISION INSPECTOR PRIOR TO ERECTION.

16. GLUE-LAM BEAMS MUST BE FABRICATED IN A LADBS LICENSED SHOP. IDENTIFY GRADE SYMBOL AND LAMINATION SPECIES PER T 5-A, 2018 NDS SUPP.

MANUFACTURER OF THE TYPE REQUIRED BY THESE DRAWINGS. NAILS SHALL BE THOSE FURNISHED BY THE MANUFACTURER FOR THIS SPECIFIC USE. DEVICES SHALL

a. PROVIDE FULL HEIGHT STUDS FROM FLOOR TO ROOF UNLESS OTHERWISE NOTED.

20. ONLY COMMON NAILS SHALL BE USED FOR ALL PLYWOOD SHEAR WALLS AND NAIL GUNS

21. FASTENERS IN PRESERVATIVE TREATED WOOD OR FIRE RETARDANT TREATED WOOD SHALL

ROOF SHEATHING SHALL BE 15/32" APA RATED PLYWOOD SHEATHING, EXPOSURE 1,

2. FLOOR PANELS SHALL BE 23/32" APA RATED PLYWOOD STURDI-I-FLOOR, TONGUE AND GROOVE, EXPOSURE 1, SPAN RATED 24"o.c. PRODUCT STANDARD DOC PS-1, DOUGLAS

4. ALL SHEATHING SHALL BE 2'-0" IN THE LEAST DIMENSION UNLESS ALL EDGES ARE

B. FOUNDATION

 PERFORM FOUNDATION WORK COMPLYING WITH REPORT AND ADDENDA. GEOTECHNICAL REPORT AND ADDENDA HEREBY BECOME PART OF THESE CONTRACT DOCUMENTS AND SHALL BE KEPT ON JOB SITE AT ALL TIMES.

FOUNDATION DESIGN IS BASED ON RECOMMENDATIONS OF BUTANO GEOTECHNICAL ENGINEERING, INC. 231 GREEN VALLEY ROAD, SUITE E FREEDOM, CALIFORNIA 95019 REPORT NO. 19-150-SCL DATED MAY 8, 2020

ALLOWABLE SOIL BEARING = 1,500 PSF MAXIMUM SOIL BEARING = 3,000 PSF MINIMUM FOOTING DEPTH = 24 INCHES MINIMUM FOOTING WIDTH = 12 INCHES FOR CONTINUOUS FOOTINGS = 24 INCHES FOR PAD FOOTINGS

- 2. FOUNDATION EXCAVATIONS ARE TO BE OBSERVED BY AND ACCEPTABLE TO A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE PRIOR TO PLACEMENT OF FILL, REINFORCING STEEL, OR CONCRETE.
- PERFORM FILLING, BACKFILLING, COMPACTION, ETC... AS INDICATED IN GEOTECHNICAL REPORT AND ONLY UNDER SUPERVISION OF A GEOTECHNICAL ENGINEER OR HIS REPRESENTATIVE.
- 5. DO NOT PLACE BACKFILL BEHIND RETAINING WALLS PRIOR TO COMPLETION AND INSPECTION OF WATERPROOFING. ADEQUATELY SHORE RETAINING WALLS DURING BACKFILL OPERATION, UNLESS ADEQUATELY SHORED, DO NOT PLACE BACKFILL BEHIND BUILDING STRUCTURE RETAINING WALLS, EXCLUDING SITE RETAINING WALLS, UNTIL CONCRETE AT ELEVATED FLOOR LEVELS ADJACENT TO WALLS ARE COMPLETELY POURED AND HAVE CURED FOR AT LEAST 7 DAYS.
- 6. THE APPROVED SOILS REPORT SHALL BE A PART OF THE PLANS AND SHALL BE KEPT AT THE JOB SITE AT ALL TIMES.
- C. REINFORCING STEEL
- REINFORCING STEEL COMPLYING WITH ASTM A615. GRADE 60 DEFORMED BARS. EXCEPT #3 BAR CAN BE GRADE 40 OR STRONGER.
- 2. WELDED REINFORCING STEEL COMPLYING WITH ASTM A706, GRACE 60 DEFORMED BARS.
- 3. SMOOTH WELDED WIRE FABRIC COMPLYING WITH ASTM A185. LAP FABRIC 1-1/2 SPACES (12" MINIMUM). PROVIDE DEFORMED WIRE STIRRUPS, SIZE D4 AND LARGER ONLY. COMPLYING WITH ASTM 497.
- 2. SPLICE REINFORCING STEEL WHERE INDICATED. IF SPLICE LOCATIONS ARE NOT SPECIFICALLY SHOWN OR INDICATED, VERIFY SPLICE LOCATIONS WITH ARCHITECT/ ENGINEER PRIOR TO DEVELOPING REINFORCING STEEL SHOP DRAWINGS.
- 3. LAP REINFORCING STEEL AT SPLICES TO THE FOLLOWING MINIMUM LENGTHS, UNLESS OTHERWISE NOTED, (APPLICABLE TO 3,000 PSI OR HIGHER, NORMAL WEIGHT CONCRETE ONLY):

BAR BIZE	TOP BARS 1'-9"	OTHER BARS 1'-4"	BAR SIZE #8	TOP BARS 6'-10"	OTHER BARS 5'–3"
4 5	2'-4" 2'-11"	1'-10" 2'-3"	#8 #9 #10	8'-8" 11'-0"	6'-8" 8'-6"
6 7	3'-10" 5'-3"	2'-11" 4'-0"	#11	13'-6"	10'-6"

TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW REBAR.

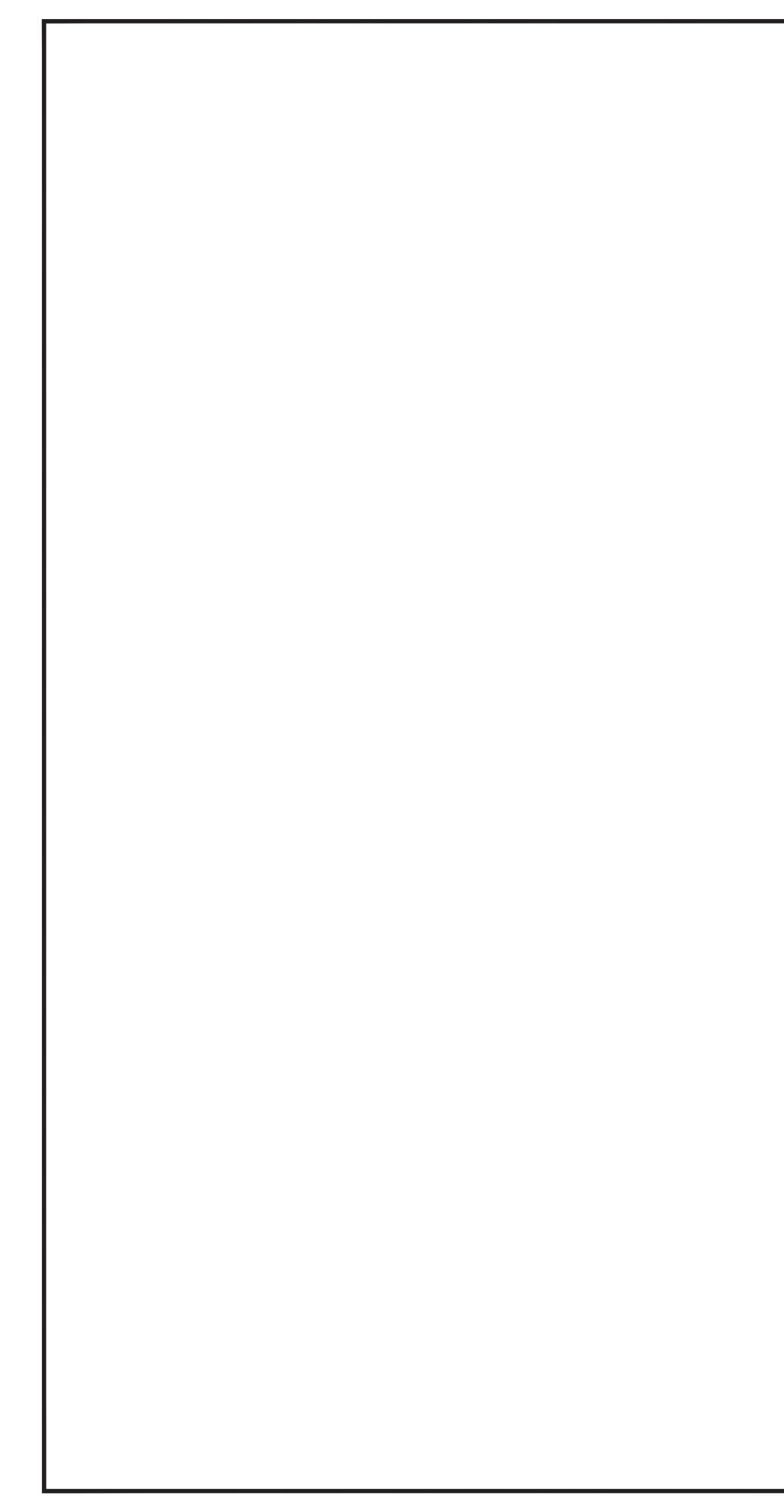
OTHER BARS ARE HORIZONTAL BARS WITH LESS THAN 12 INCHES OF CONCRETE CAST BELOW BARS AND ALL VERTICAL BARS.

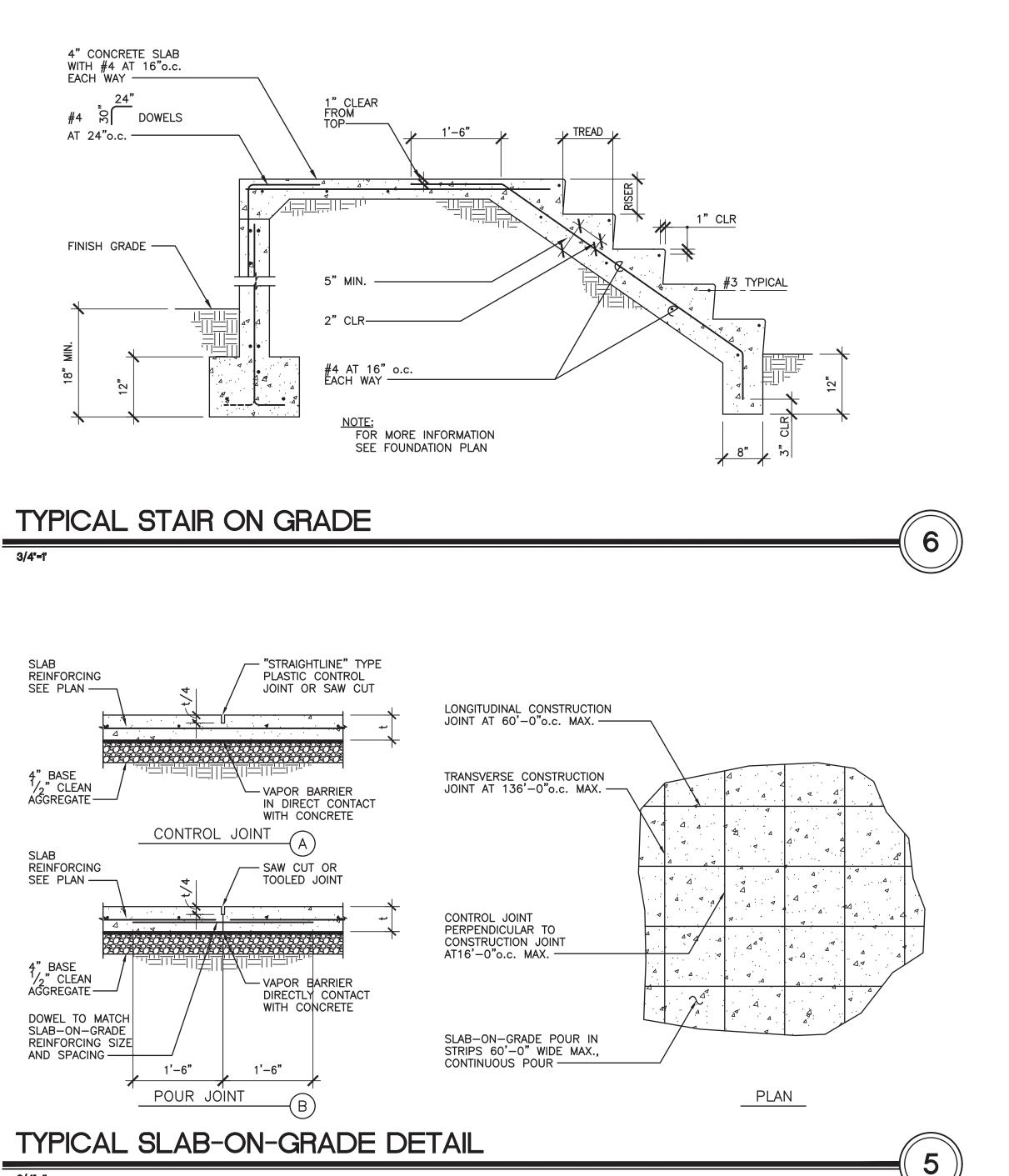
- 4. MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL, INCLUDING SPLICED REINFORCING STEEL, SHALL BE 1 INCH OR 1 BAR DIAMETER, WHICHEVER IS GREATER, FOR BUNDLED BARS, MINIMUM CLEAR DISTANCES BETWEEN UNITS OF BUNDLED BARS SHALL BE SAME AS SINGLE BARS EXCEPT BAR DIAMETER IS DERIVED FROM EQUIVALENT TOTAL AREA OF BUNDLE.
- MAINTAIN THE FOLLOWING MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL AND FACE OF CONCRETE UNLESS OTHERWISE NOTED:
- C/L OF SLAB a. SLAB-ON-GRADE b. CONCRETE BELOW GRADE, FORMED 2 INCHES
- 3 INCHES c. CONCRETE BELOW GRADE, UNFORMED d. WALLS ABOVE GRADE, EXPOSED TO WEATHER 2 INCHES
- e. WALLS ABOVE GRADE. NOTE EXPOSED TO WEATHER ...... .. 1 INCHES
- f. COLUMNS, CLEAR TO FACE OF TIES .....  $\dots 1-1/2$  INCHES a. BEAMS, CLEAR TO FACE OF TIES .....  $\dots$  1-1/2 INCHES
- 6. BEND REINFORCING STEEL COLD UNLESS OTHERWISE ACCEPTED BY ARCHITECT OR ENGINEER.
- 7. CHAIRS OR SPACERS FOR REINFORCING SHALL BE PLASTIC OR PLASTIC COATED WHEN RESTING ON EXPOSED SURFACES.
- 8. WELD REINFORCING STEEL COMPLYING WITH AWS D1.4. DO NOT WELD REINFORCING STEEL OTHER THAN THOSE CONFORMING TO ASTM A706.
- 9. SECURELY TIE ANCHOR BOLTS, REINFORCING STEEL, INSERTS, ETC... IN PLACE PRIOR TO PLACING CONCRETE OR GROUT.
- 10. SUBMIT REINFORCING STEEL SHOP DRAWINGS INDICATING REINFORCING PLACEMENT. INCLUDING SPLICE LOCATIONS AND LENGTHS, TO ARCHITECT/ENGINEER FOR REVIEW AND ACCEPTANCE.
- D. CAST-IN-PLACE CONCRETE
- . NORMAL WEIGHT AGGREGATES OF NATURAL SAND AND ROCK COMPLYING WITH ASTM C33 AND UBC STANDARD 26-2.
- 2. PORTLAND CEMENT CONFORMING TO ASTM C150, TYPE II, AND UBC STANDARD 26-1, PART I.
- 3. NORMAL WEIGHT CONCRETE (145 PCF), WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.05%, ATTAINING MINIMUM COMPRESSIVE STRENGTHS (f'c) AT 28 DAYS AS FOLLOWS:

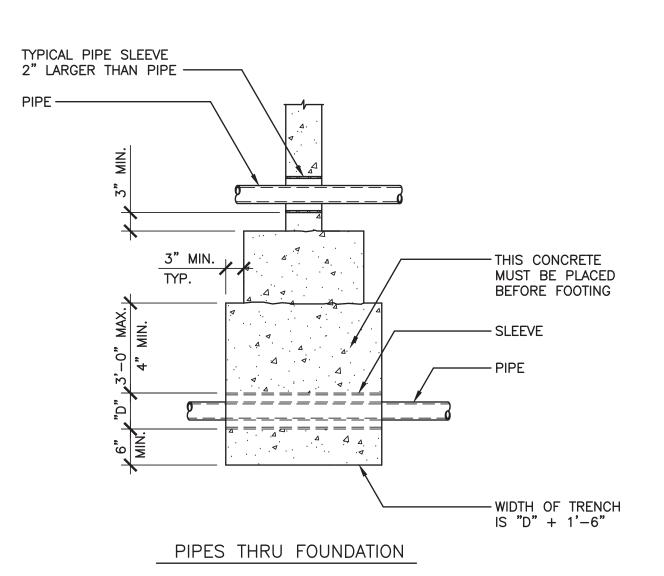
FOUNDATIONS	3,000	PSI
CONCRETE WALLS	3,000	PSI
SLAB-ON-GRADE	2,500	PSI
UNLESS OTHERWISE NOTED	3,000	PSI

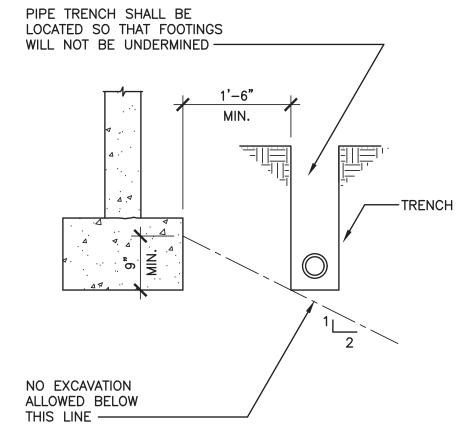
- 4. SLUMP NOT TO EXCEED 4 INCHES.
- 5. DO NOT USE CONCRETE OR GROUT CONTAINING CHLORIDES
- 6. DO NOT EMBED CONDUITS, PIPES, OR SLEEVES OTHER THAN ELECTRICAL CONDUITS 1 INCH DIAMETER AND SMALLER IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY DETAILED OR ACCEPTED BY ARCHITECT OR ENGINEER.
- 7. FORM EXPOSED CORNERS OF COLUMNS, BEAMS, WALLS, ETC... WITH 3/4 INCH CHAMFERS UNLESS OTHERWISE DETAILED.
- 8. PROVIDE KEYS IN CONSTRUCTION JOINTS UNLESS OTHERWISE DETAILED.
- 9. ROUGHED CONCRETE SURFACE TO FULL AMPLITUDE OF 1/16 INCH WHERE MASONRY WALLS INTERSECT CONCRETE.

		ITTAL
1.	ALL MATERIALS AND WORKMANSHIP SHALL CONFORM TO THE CALIFORNIA BUILDING CODE 2019 EDITION (CBC 2019), REFERENCED STANDARDS OF CHAPTER 35 AND ALL APPLICABLE CODES AND ORDINANCES.	T SUBMITTAL
	BASIS OF DESIGN: a. SEISMIC LOADS + IMPORTANCE FACTOR, le = 1.0	PARTMENT
	+ Ss = $2.356g$ + S1 = $0.911g$ + SITE CLASS: C	DEP
	+ Sds = $1.885g$ + Sd1 = $0.850g$ + Rho = $1.3$ (REDUNDANCY FACTOR)	BUILDING
	+ SEISMIC DESIGN CATEGORY: E + BASIC SEISMIC-FORCE-RESISTING SYSTEM: SHEATHED SHEAR BEARING WALLS + SEISMIC RESPONSE COEFFICIENT, Cs = 0.290 (STRENGTH) = 0.207 (SERVICE)	
	+ RESPONSE MODIFICATION FACTOR, R = 6.5 + ANALYSIS PROCEDURE USED: EQUIVALENT LATERAL FORCE PROCEDURE b. WIND LOAD	DATE 05-2022
	+ BASIC WIND SPEED = 110 MPH (ULTIMATE) + EXPOSURE C + IMPORTANCE FACTOR, Iw=1.0	DA 07-05
	+ INTERNAL PRESSURE COEFFICIENT = 0.18 + DESIGN WIND PRESSURE = 27.5 PSF + COMPONENTS AND CLADDING WIND PRESSURE = 39 PSF c. LIVE LOADS	<u>ż</u> ddddd
	+ $ROOF = 20 PSF$ + $FLOOR = 40 PSF$	
	+ DECK/BALCONY = 60 PSF d. DEAD LOADS + ROOF = 18 PSF + FLOOR = 18 PSF	ISSUED 5-2022 N BY KED BY
2.	AISC – SPECIFICATIONS FOR THE DESIGN, FABRICATION AND ERECTION OF STRUCTURAL STEEL FOR BUILDING.	DATE ISS 07-05-2 DRAWN B ML CHECKED TL
3.	ACI-318 - BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE.	
4. 5	ALL ASTM SPECIFICATIONS NOTED ON THESE DRAWINGS SHALL BE OF THE LATEST REVISION.	
5. 6.	WRITTEN INFORMATION AND DIMENSIONS SHALL TAKE PRECEDENCE OVER GRAPHIC INFORMATION. DO NOT SCALE DRAWINGS. ALL DIMENSIONS ARE TO TAKE PRECEDENCE OVER SCALE SHOWN ON	L BLVD., SUIT L BLVD., SUIT L BLVD., SUIT FORNIA 9100 6) 731-1539
o. 7.	ALL DIMENSIONS ARE TO TAKE PRECEDENCE OVER SCALE SHOWN ON PLANS, ELEVATIONS, SECTIONS, AND DETAILS. ANY DISCREPANCIES ON THE PLANS OR ANY DEVIATIONS FROM THE PLANS	ENG LFORN 6) 73
, .	WHICH ARE NECESSITATED BY FIELD CONDITIONS OR ANY CONDITION DIFFERENT FROM THOSE INDICATES ON THE PLANS, SHALL BE CALLED TO THE ATTENTION OF THANG LE, S.E. PRIOR TO CONTINUING CONSTRUCTION. ALL WORK IS TO BE COORDINATED SO THAT COOPERATION BETWEEN THE TRADES WHERE REQUIRED, IS ACCOMPLISHED.	ANG LE & A JCTURAL EN JCTURAL EN E FOOTHILL E ARCADIA, CALIFC PHONE: (626)
8.	SEE ARCHITECTURAL DRAWING FOR KINDS OF FLOOR FINISH, DEPRESSION IN SLAB, OPENINGS IN WALLS AND ROOF REQUIRED BY DOOR, WINDOWS, DUCTS, VENTS, HATCHES, PLUMBING, ETC; ALL TYPE OF FLASHING, INSERTS, ANCHORS, HANGERS, ETC EMBEDDED OR ATTACHED TO CONCRETE STRUCTURE; PAVING, WALKS, STAIRS, RAMPS, CURBS, PARAPETS, TERRACES, ETC; EXTERIOR GRADES; ROOF SLABS, CRICKETS AND DRAINS.	<b>THANG LE</b> <b>STRUCTURAL</b> 319 E. FOOTH ARCADIA, C/ PHONE: (6
9.	THE CONTRACTOR SHALL COMPARE THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL DRAWINGS AS TO LAYOUT DIMENSIONS AND ELEVATIONS. ALL DISCREPANCIES SHALL BE REPORTED TO THANG LE, S.E. AND THE OWNER FOR PROPER ADJUSTMENT BEFORE PROCEEDING WITH THE WORK.	CONTROPESSION
10.	IN THE EVENT THAT CERTAIN FEATURES OF THE CONSTRUCTION ARE NOT FULLY SHOWN ON THE DRAWINGS OR CALLED FOR IN THE THE GENERAL NOTES, THEN THEIR CONSTRUCTION SHALL BE OF THE SAME CHARACTER AS FOR CONDITIONS THAT ARE SHOWN OR CALLED FOR.	THOTOMAL A
11.	THE BUILDER SHALL TAKE FULL AND FINAL RESPONSIBILITY FOR CONSTRUCTING A FINAL PRODUCT OF APPROPRIATE QUALITY AND SERVICEABILITY CONSISTENT WITH THE INFORMATION AND REQUIREMENTS CONTAINED IN THE CONSTRUCTION DOCUMENTS OR REASONABLY INFERABLE THEREFROM, AND/OR CONTAINED IN THE REQUIREMENTS OF ANY GOVERNMENTAL ENTITY WITH JURISDICTION OVER THE PROJECT.	NCE NCE
	THE BUILDER SHALL TAKE FULL RESPONSIBILITY FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES INCLUDING WITHOUT LIMITATION DEMOLITION, EXCAVATION AND ERECTION PROCEDURES.	SIDE AD A 95127
13.	STRUCTURAL OBSERVATION VISITS TO SITE BY REPRESENTATIVES OF THANG LE, S.E. DO NOT INCLUDE INSPECTIONS OF CONSTRUCTION MEANS AND METHODS. OBSERVATIONS PERFORMED BY ENGINEER DURING CONSTRUCTION ARE NOT CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE PERFORMED BY OTHERS. OBSERVATIONS PERFORMED BY ENGINEER ARE PERFORMED SOLELY FOR THE PURPOSE OF DETERMINING IF THE CONTRACTOR UNDERSTAND DESIGN INTENT CONVEYED IN CONTRACT DOCUMENTS. OBSERVATIONS DO NOT GUARANTEE CONTRACTOR'S PERFORMANCE AND ARE NOT TO BE CONSTRUED AS SUPERVISION OF CONSTRUCTION.	ON RE ROTHERS RO
14.	MODIFICATIONS OR SUBSTITUTIONS: DESIGN, MATERIALS, EQUIPMENT AND PRODUCTS OTHER THAN THOSE INDICATED OR SPECIFIED MAY BE CONSIDERED FOR USE PROVIDED A WRITTEN REQUEST, SUBJECT TO REVIEW, IS SUBMITTED TO OWNER, ARCHITECT, ENGINEER AND GOVERNING CODE AUTHORITY PRIOR TO ITS USE OR INCLUSION ON ANY SHOP DRAWING.	LARS 10818 CF SAN JOSE
15.	BRACE PIPING AND DUCTS COMPLYING WITH LATEST ADDITION OF GUIDELINES FOR SEISMIC RESTRAINTS OF MECHANICAL SYSTEMS BY THE SHEET METAL AND AIR CONDITIONING CONTRACTORS NATIONAL ASSOCIATION.	
16.	INSTALL AND ANCHOR MECHANICAL AND ELECTRICAL EQUIPMENT TO STRUCTURE COMPLYING ASCE/SEI 7-05, CHAPTER 13, AS MODIFIED BY CBC 1614.1.11 THROUGH 1614.1.16. ISOLATORS, FASTENERS AND ANY OTHER ELEMENT PROVIDING STABILITY FOR EQUIPMENT SHALL BE APPROVED BY ICC-ES OR EQUIVALENT TESTING PROCEDURE. PROVIDE SUSPENDED EQUIPMENT WITH APPROVED LATERAL OR SWAY BRACING.	TES
		NOT
		GENERAL
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		<b>S1.1</b>
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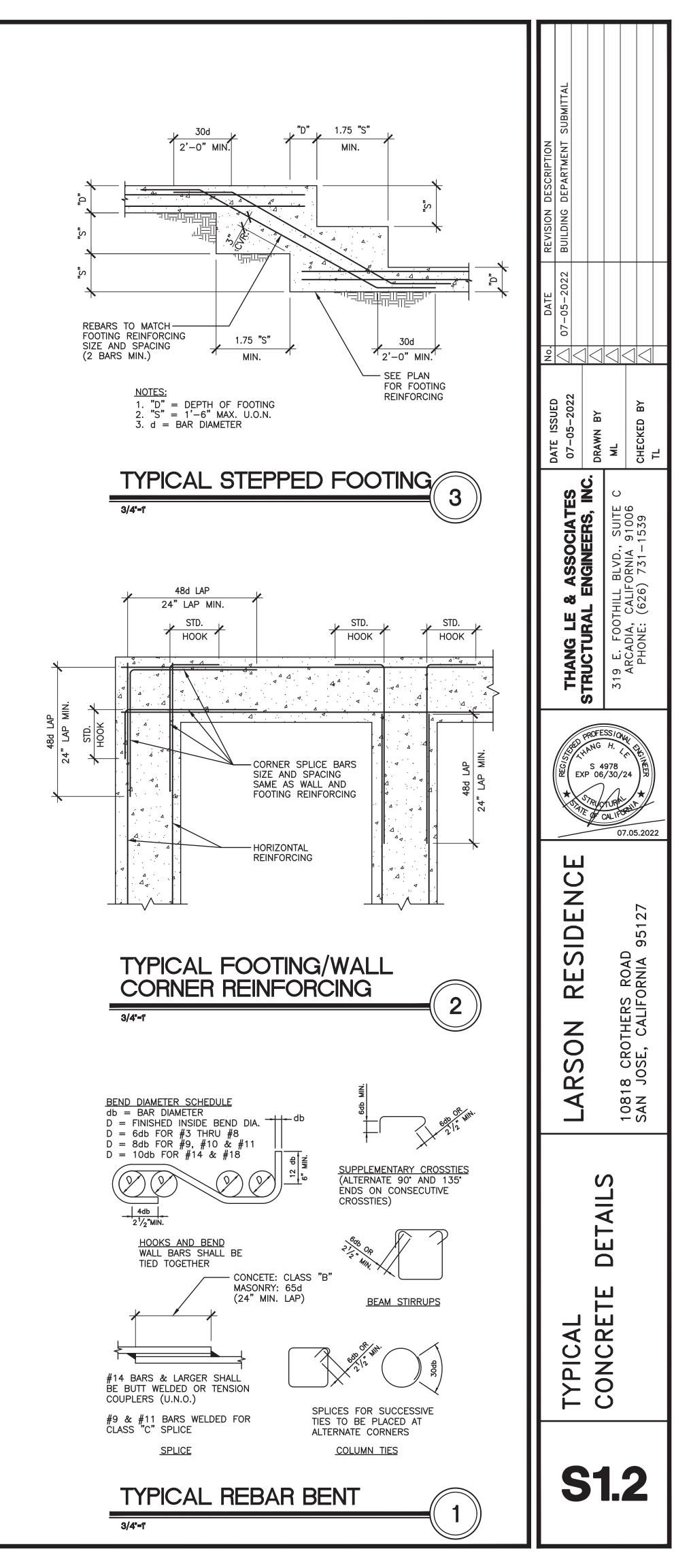


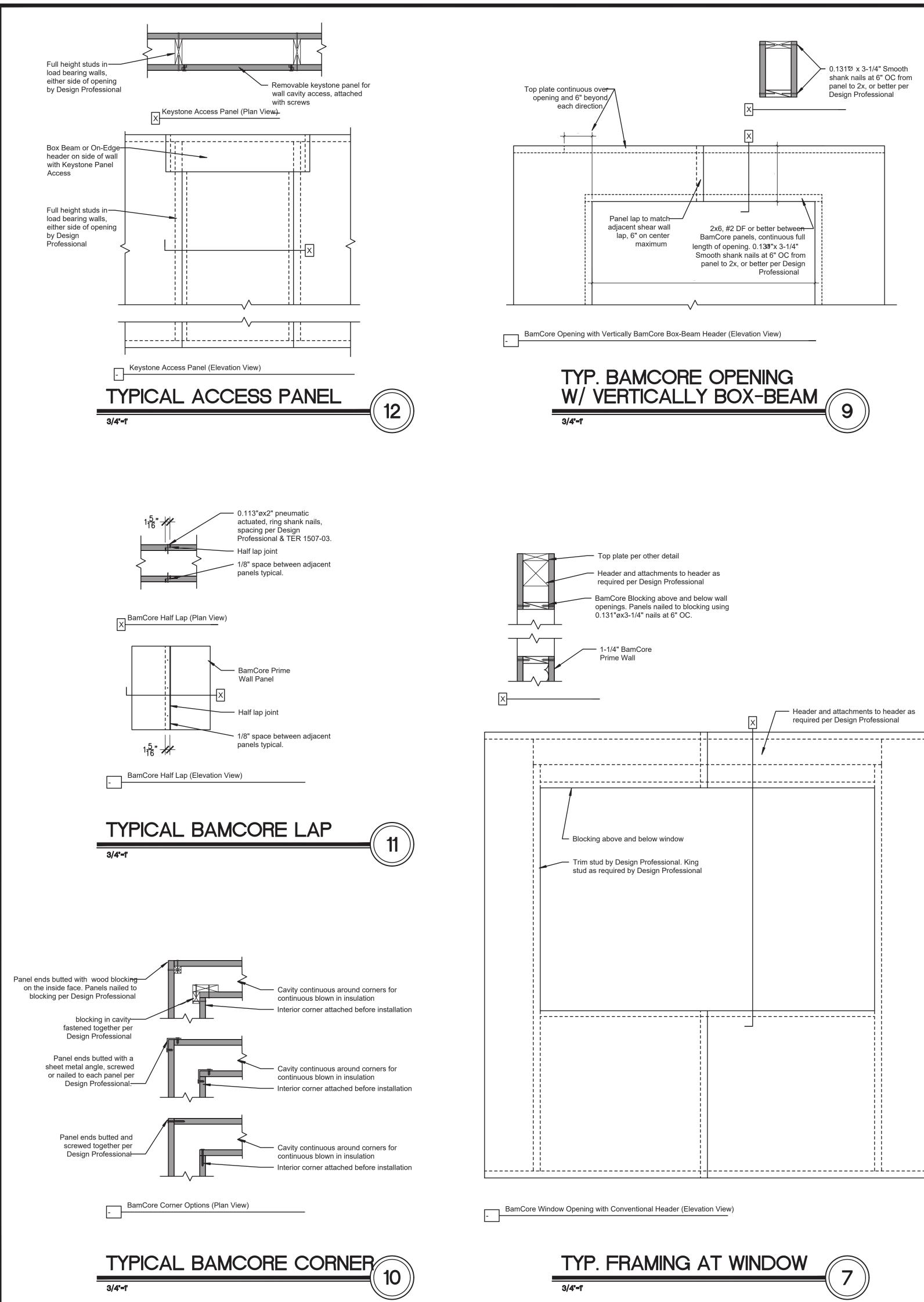
PIPE AND TRENCH AT FOUNDATION

# TYPICAL PIPE AT FOOTING

3/4**'-1**'

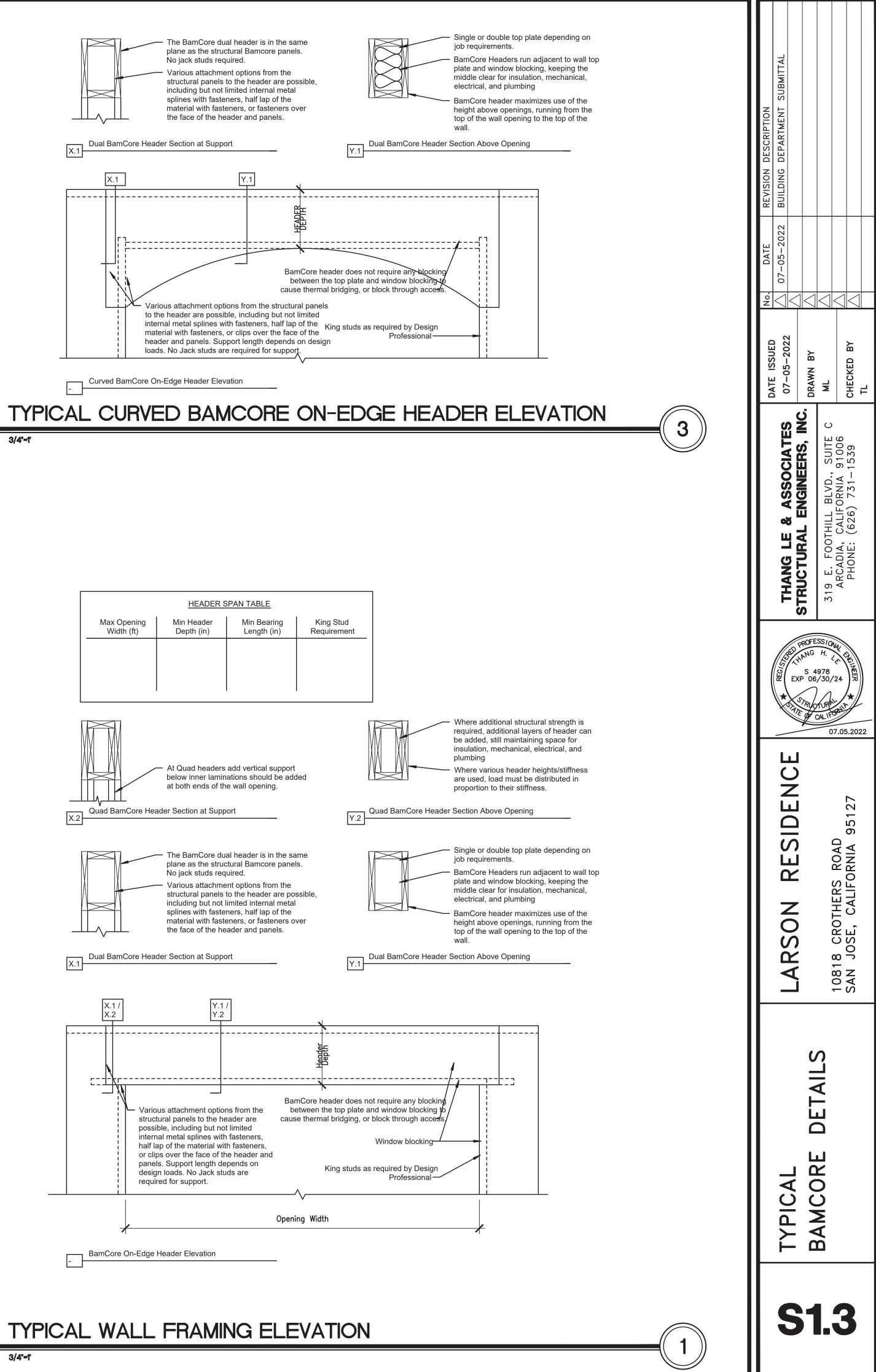
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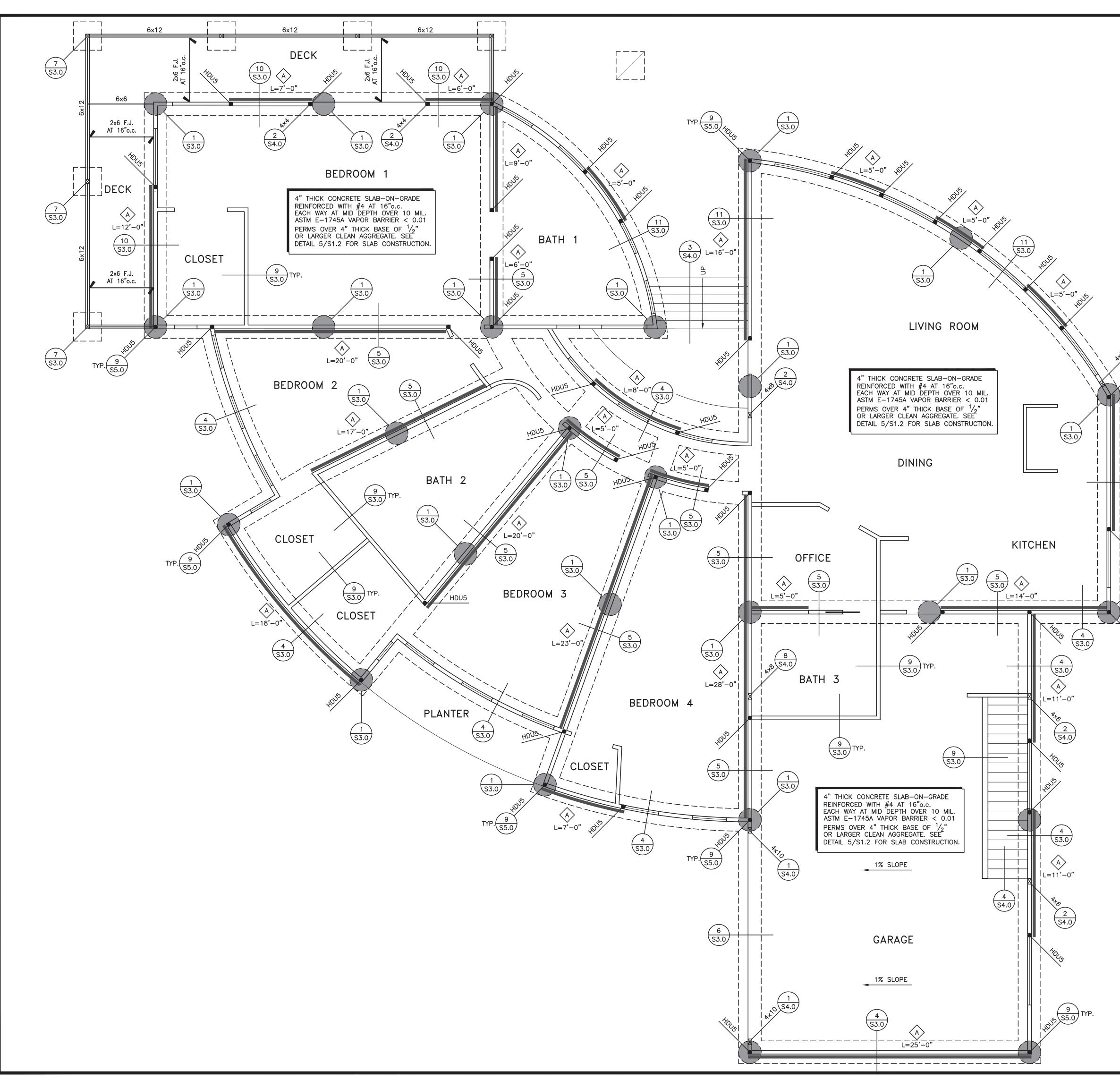


X.1 Dual BamCore Header Section at Support X.1

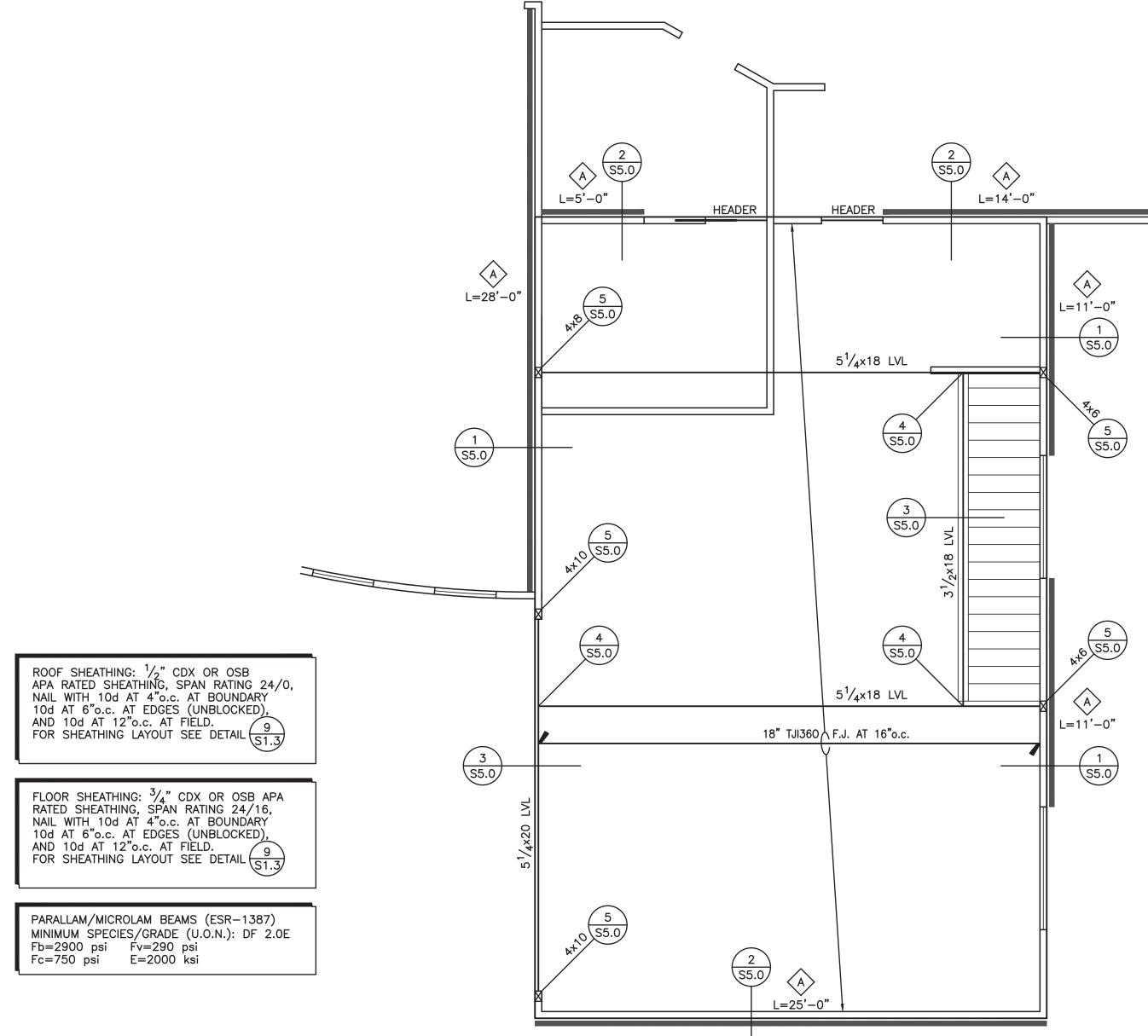
3/**4'-**1'



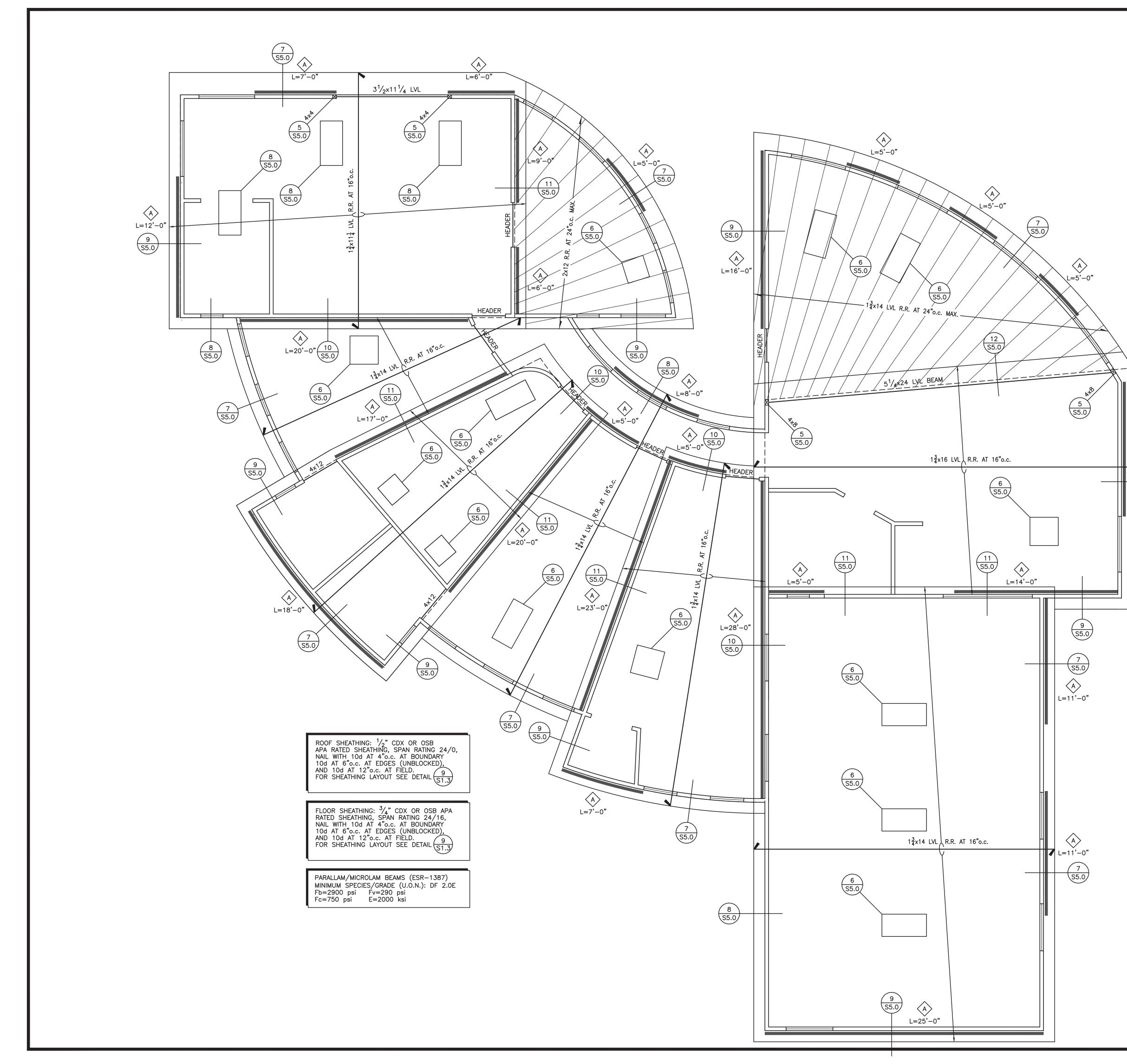
3/4**'=**1'



<ul> <li>SCALE 1/4-1-0'</li> <li>NOTES:         <ol> <li>ALL COLUMN FOOTINGS ARE CENTERED ON GRIDS U.O.N. WALL FOOTINGS ARE CENTERED BENEATH WALLS U.O.N.</li> <li>SEE SHEET S1.1 FOR GENERAL STRUCTURAL NOTES.</li> <li>SEE SHEET S1.2 FOR TYPICAL CONCRETE DETAILS.</li> <li>SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.</li> <li>SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.</li> </ol> </li> <li>VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL (15.2)</li> <li>VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL &amp; ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.</li> </ul> <li>FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE (2)/(S1.2)</li> <li>DESIGNATES BAMCORE PRIME SHEAR WALL PANEL. NAILING SCHEDULE SEE SHEET S6.0.</li>	ASSOCIATES ASSOCIATESDATE ISSUED No.DATEREVISION DESCRIPTASSOCIATES BUILDINGDATE ISSUED O7-05-2022DATEREVISION DESCRIPTASSOCIATES ENGINEERS, INC.DATE ISSUED O7-05-2022DATEREVISION DESCRIPTASSOCIATES ENGINEERS, INC.DATE ISSUED DRAWN BYDATEREVISION DESCRIPTL BLVD., SUITE C IFORNIA 91006MLDATEREVISION DESCRIPTC BLVD., SUITE C IFORNIA 91006MLDATEDATEC BLVD., SUITE C ITMLDATEDATEC BLVD., SUITE C ITMLDATE
<ol> <li>ALL COLUMN FOOTINGS ARE CENTERED ON COLUMNS AND COLUMNS ARE CENTERED ON GRIDS U.O.N. WALL FOOTINGS ARE CENTERED BENEATH WALLS U.O.N.</li> <li>SEE SHEET S1.1 FOR GENERAL STRUCTURAL NOTES.</li> <li>SEE SHEET S1.2 FOR TYPICAL CONCRETE DETAILS.</li> <li>SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.</li> <li>SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.</li> <li>VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL 4 (S1.2)</li> <li>VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL &amp; ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.</li> <li>FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE 2 (S1.2)</li> <li>DESIGNATES BAMCORE PRIME SHEAR WALL PANEL. NAILING SCHEDULE SEE SHEET S6.0.</li> </ol>	Clates     Date issued     No.     Date     Revision       Clates     Date issued     07-05-2022     Buildidididididididididididididididididid
<ol> <li>SEE SHEET S1.1 FOR GENERAL STRUCTURAL NOTES.</li> <li>SEE SHEET S1.2 FOR TYPICAL CONCRETE DETAILS.</li> <li>SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.</li> <li>VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL 4 S1.2</li> <li>VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL &amp; ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.</li> <li>FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE 2 S1.2</li> <li>DESIGNATES BAMCORE PRIME SHEAR WALL PANEL. NAILING SCHEDULE SEE SHEET S6.0.</li> </ol>	CIATES CIATES CIATES CIATES CIATES 07-05-202 07-05-202 07-05-202 DRAWN BY CIATES DRAWN BY ML SUITE C ML SUITE C ML TL
<ul> <li>4. SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.</li> <li>5. VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL (4) S1.2</li> <li>6. VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL &amp; ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.</li> <li>7. FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE (2) S1.2</li> <li>8. DESIGNATES BAMCORE PRIME SHEAR WALL PANEL. NAILING SCHEDULE SEE SHEET S6.0.</li> </ul>	CIATES CIATES CIATES EERS, INC. EERS, INC. SUITE C 91006 1539 TL TL
<ul> <li>5. VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL (51.2)</li> <li>6. VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL &amp; ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.</li> <li>7. FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE (2) S1.2)</li> <li>8. DESIGNATES BAMCORE PRIME SHEAR WALL PANEL. NAILING SCHEDULE SEE SHEET S6.0.</li> </ul>	CIATES CLATES EERS, INC. SUITE C 91006 1539 TL TL
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	D., SL D., SL 1-153
9. DESIGNATES SIMPSON HOLDOWN TO NEW FOOTING SEE	
S6.0 10. — — — DESIGNATES GRADE BEAM — — — SEE PLAN FOR REINFORCEMENT	
11. DESIGNATES CONCRETE PILE SEE PLAN FOR REINFORCEMENT	THANG LE STRUCTURAL 319 E. FOOTH ARCADIA, C PHONE: (
-0" 12. MINIMUM ANCHOR BOLT SIZE AND SPACING SHALL BE <sup>5</sup> / <sub>8</sub> " DIA. A.B. AT 48"o.c., WITH 7" EMBEDMENT, AND 3"x3"x0.229" STAINLESS STEEL PLATE WASHERS. ANCHOR BOLTS SHALL BE LOCATED A MAXIMUM OF 12" AND 7" MINIMUM FROM THE END OF THE PLATE AND SPACED NOT MORE THAN 4 FEET APART.	
13. PLATE WASHERS ARE REQUIRED FOR ALL HOLD-DOWNS.	RED PROFESSION
14. HOLD-DOWN CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE APPROVED PLATE WASHERS. HOLD-DOWNS SHALL BE TIGHTENED JUST PRIOR TO COVERING THE WALL FRAMING. CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE STEEL PLATE WASHERS ON THE POST ON THE OPPOSITE SIDE OF THE ANCHORAGE DEVICE. PLATE SIZE SHALL BE A MINIMUM OF 0.299 INCH BY 3 INCHES BY 3 INCHES.	54978 54978 EXP 06/30/24 *0.37 Protune *0.47 CALIFORM 07.05.2022
15. ALL ANCHOR BOLTS (INCLUDING HOLD-DOWN BOLT ANCHORS) SHALL HAVE MINIMUM EDGE DISTANCE OF $1\frac{3}{4}$ ".	ш
16. FASTENERS IN PRESERVATIVE TREATED WOOD OR FIRE RETARDANT TREATED WOOD SHALL BE OF HOT DIP ZINC COATED GALVANIZED STEEL OR STAINLESS STEEL.	ENC 127
RETARDANT TREATED WOOD SHALL BE OF HOT DIP ZINC COATED GALVANIZED STEEL OR STAINLESS STEEL. 17. FOUNDATION SILLS SHALL BE NATURALLY DURABLE OR PRESERVATIVE—TREATED WOOD.	951 951
18. ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING. FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS.	ON RES ROTHERS ROAD E, CALIFORNIA
19. ALL DIAPHRAGM AND SHEAR WALL NAILING SHALL UTILIZE COMMON NAILS OR GALVANIZED BOX.	LIFOR R
20. ALL BOLT HOLES SHALL BE DRILLED 1/32 TO 1/16" OVERSIZED.	CAHE O N
21. HOLD-DOWN HARDWARE MUST BE SECURED IN PLACE PRIOR TO FOUNDATION INSPECTION.	လ ပ လ
22. NUTS OF THE PRIMARY AND SECONDARY ANCHORS FASTENERS SHALL BE WRENCH TIGHTENED PRIOR TO INSPECTION AND COVERING.	LAR 10818 SAN JC
23. POWER-DRIVEN FASTENERS SHALL NOT BE USED TO ANCHOR SILL PLATES EXCEPT AT INTERIOR NONBEARING WALLS NOT DESIGNED AS SHEAR WALLS.	
24. PROVIDE LEAD HOLE 40%-70% OF THREADED SHANK DIAMETER AND FULL DIAMETER FOR SMOOTH SHANK PORTION.	
25. IF ADVERSE SOIL CONDITIONS ARE ENCOUNTERED, A SOILS INVESTIGATION REPORT MAY BE REQUIRED.	LAN
26. ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING. FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS. FLOOR SHALL HAVE TONGUE AND GROOVE OR BLOCKED PANEL EDGES. PLYWOOD SPAN SHALL CONFORM WITH TABLE 2304.8.	TION PI
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	S2.1

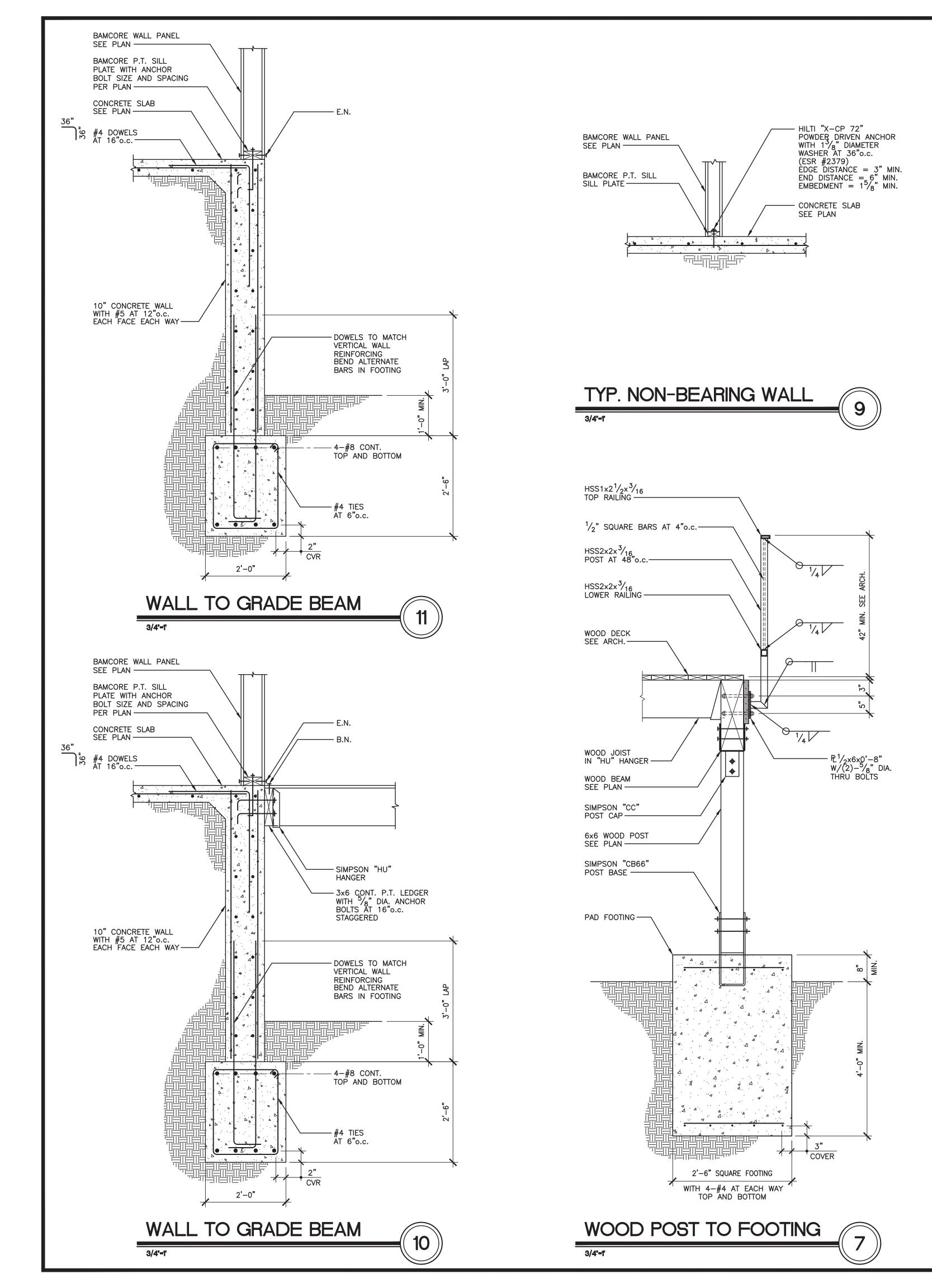


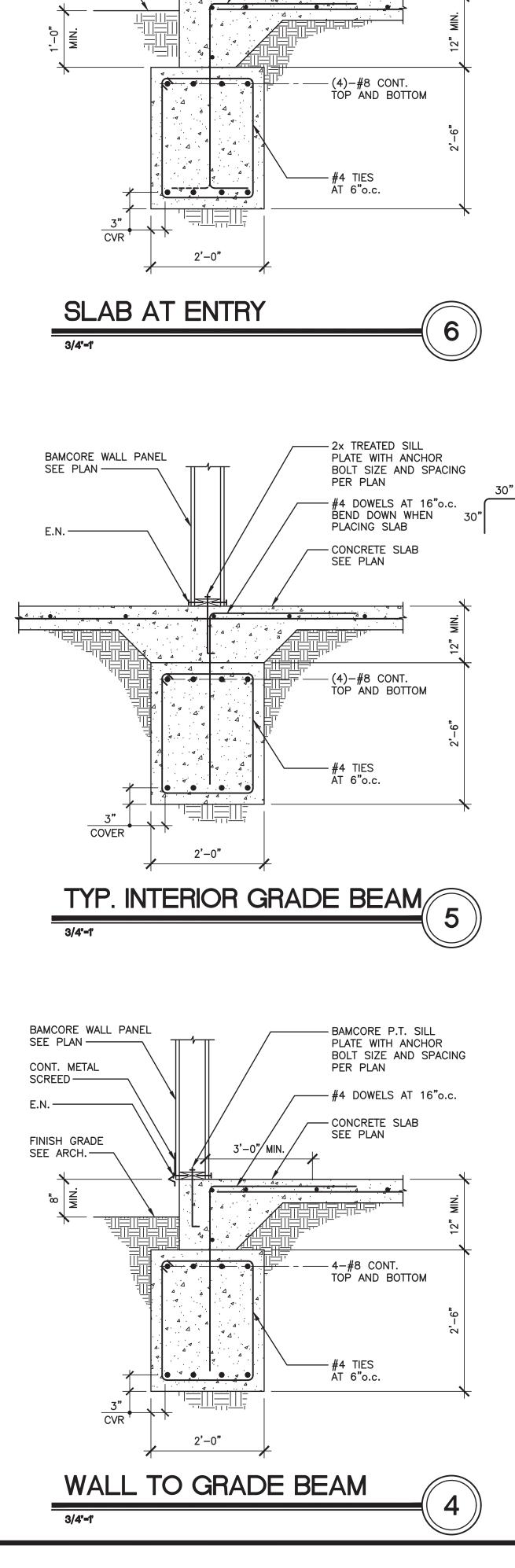
	EZZANINE FRAMING PLAN	DESCRIPTION DEPARTMENT SUBMITTAL
<u>NC</u>	TES: ALL COLUMN FOOTINGS ARE CENTERED ON	BUILDING
	COLUMNS AND COLUMNS ARE CENTERED ON GRIDS U.O.N. WALL FOOTINGS ARE CENTERED BENEATH WALLS U.O.N.	
2.	SEE SHEET S1.1 FOR GENERAL STRUCTURAL NOTES.	DATE 05-20
3. 4.	SEE SHEET S1.2 FOR TYPICAL CONCRETE DETAILS. SEE SHEET S1.3 FOR TYPICAL WOOD DETAILS.	02-0
5.	VERIFY SIZE, LOCATION AND DEPTH OF UTILITIES AND SLEEVES WITH OTHER TRADES. FOR MECH LINES BELOW FOUNDATION, STEP AND THICKEN FOOTING AS INDICATED IN DETAIL (51.2)	
6.	VERIFY SHOWN DIMENSIONS WITH ARCHITECTURAL, MECHANICAL & ELECTRICAL DRAWINGS. NOTIFY ARCHITECT OF DISCREPANCIES PRIOR TO CONSTRUCTION.	DATE ISSUED 07-05-2022 DRAWN BY ML CHECKED BY
7.	FOR TYPICAL FOOTING/WALL CORNER BAR REINFORCING SEE $2$ $\langle ? \rangle$	
8.	DESIGNATES BAMCORE PRIME SHEAR WALL PANEL. NAILING SCHEDULE SEE SHEET S6.0.	
9.	DESIGNATES SIMPSON HOLDOWN TO NEW FOOTING SEE	ASSO ASSO AGINE DRNIA 731-
10	——— SEE PLAN FOR REINFORCEMENT	ALE CALIF (626)
11	I SEE PLAN FOR REINFORCEMENT	THANG LI IRUCTUR/ 319 E. FOO ARCADIA, PHONE:
12	MINIMUM ANCHOR BOLT SIZE AND SPACING SHALL BE $\frac{9}{8}$ " DIA. A.B. AT 48"o.c., WITH 7" EMBEDMENT, AND 3"x3"x0.229" STAINLESS STEEL PLATE WASHERS. ANCHOR BOLTS SHALL BE LOCATED A MAXIMUM OF 12" AND 7" MINIMUM FROM THE END OF THE PLATE AND SPACED NOT MORE THAN 4 FEET APART.	STE STE
13	PLATE WASHERS ARE REQUIRED FOR ALL HOLD-DOWNS.	Stell PROFESSIONAL
14	HOLD-DOWN CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE APPROVED PLATE WASHERS. HOLD-DOWNS SHALL BE TIGHTENED JUST PRIOR TO COVERING THE WALL FRAMING. CONNECTOR BOLTS INTO WOOD FRAMING REQUIRE STEEL PLATE WASHERS ON THE POST ON THE OPPOSITE SIDE OF THE ANCHORAGE DEVICE. PLATE SIZE SHALL BE A MINIMUM OF 0.299 INCH BY 3 INCHES BY 3 INCHES.	S 4978 EXP 06/30/24
15	ALL ANCHOR BOLTS (INCLUDING HOLD-DOWN BOLT ANCHORS) SHALL HAVE MINIMUM EDGE DISTANCE OF $1\frac{3}{4}$ .	07.05.2
16	FASTENERS IN PRESERVATIVE TREATED WOOD OR FIRE RETARDANT TREATED WOOD SHALL BE OF HOT DIP ZINC COATED GALVANIZED STEEL OR STAINLESS STEEL.	SIDENC AD AD B5127
17	FOUNDATION SILLS SHALL BE NATURALLY DURABLE OR PRESERVATIVE—TREATED WOOD.	951
18	ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING. FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS.	
19	ALL DIAPHRAGM AND SHEAR WALL NAILING SHALL UTILIZE COMMON NAILS OR GALVANIZED BOX.	ON RES ROTHERS ROAD
20	ALL BOLT HOLES SHALL BE DRILLED 1/32 TO 1/16" OVERSIZED.	
21	HOLD-DOWN HARDWARE MUST BE SECURED IN PLACE PRIOR TO FOUNDATION INSPECTION.	
22	NUTS OF THE PRIMARY AND SECONDARY ANCHORS FASTENERS SHALL BE WRENCH TIGHTENED PRIOR TO INSPECTION AND COVERING.	AR 100
23	POWER-DRIVEN FASTENERS SHALL NOT BE USED TO ANCHOR SILL PLATES EXCEPT AT INTERIOR NONBEARING WALLS NOT DESIGNED AS SHEAR WALLS.	
24	. PROVIDE LEAD HOLE 40%—70% OF THREADED SHANK DIAMETER AND FULL DIAMETER FOR SMOOTH SHANK PORTION.	
25	IF ADVERSE SOIL CONDITIONS ARE ENCOUNTERED, A SOILS INVESTIGATION REPORT MAY BE REQUIRED.	
26	ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING. FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS. FLOOR SHALL HAVE TONGUE AND GROOVE OR BLOCKED PANEL EDGES. PLYWOOD SPAN SHALL CONFORM WITH TABLE 2304.8.	LAN
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		NGN
		MEZZANIN FRAMING
		IEZ RA



	ROOF FRAMING PLAN	SCRIPTION PARTMENT SUBMITTAL
$\mathbf{I}_{12}^{A}$	PROOF FRAMMOR PLAN         SUBJECT         PAIL         PAIL <th>LARSON RESIDENCE       THANG LE &amp; ASSOCIATES       Date issued       No.       Date issued       No.       Date issued         10818 CROTHERS ROAD       95127       319 E. FOOTHILL BLVD., SUITE C       ML       ML       ML       ML       ML         10818 CROTHERS ROAD       95127       95127       95127       95127       1539       ML       ML</th>	LARSON RESIDENCE       THANG LE & ASSOCIATES       Date issued       No.       Date issued       No.       Date issued         10818 CROTHERS ROAD       95127       319 E. FOOTHILL BLVD., SUITE C       ML       ML       ML       ML       ML         10818 CROTHERS ROAD       95127       95127       95127       95127       1539       ML       ML
	<ol> <li>POWER-DRIVEN FASTENERS SHALL NOT BE USED TO ANCHOR SILL PLATES EXCEPT AT INTERIOR NONBEARING WALLS NOT DESIGNED AS SHEAR WALLS.</li> <li>PROVIDE LEAD HOLE 40%-70% OF THREADED SHANK DIAMETER AND FULL DIAMETER FOR SMOOTH SHANK PORTION.</li> <li>IF ADVERSE SOIL CONDITIONS ARE ENCOUNTERED, A SOILS INVESTIGATION REPORT MAY BE REQUIRED.</li> <li>ROOF DIAPHRAGM NAILING TO BE INSPECTED BEFORE COVERING. FACE GRAIN OF PLYWOOD SHALL BE PERPENDICULAR TO SUPPORTS. FLOOR SHALL HAVE TONGUE AND GROOVE OR BLOCKED PANEL EDGES. PLYWOOD SPAN SHALL CONFORM WITH TABLE 2304.8.</li> </ol>	ROOF FRAMING PLAN

**S2.3** 



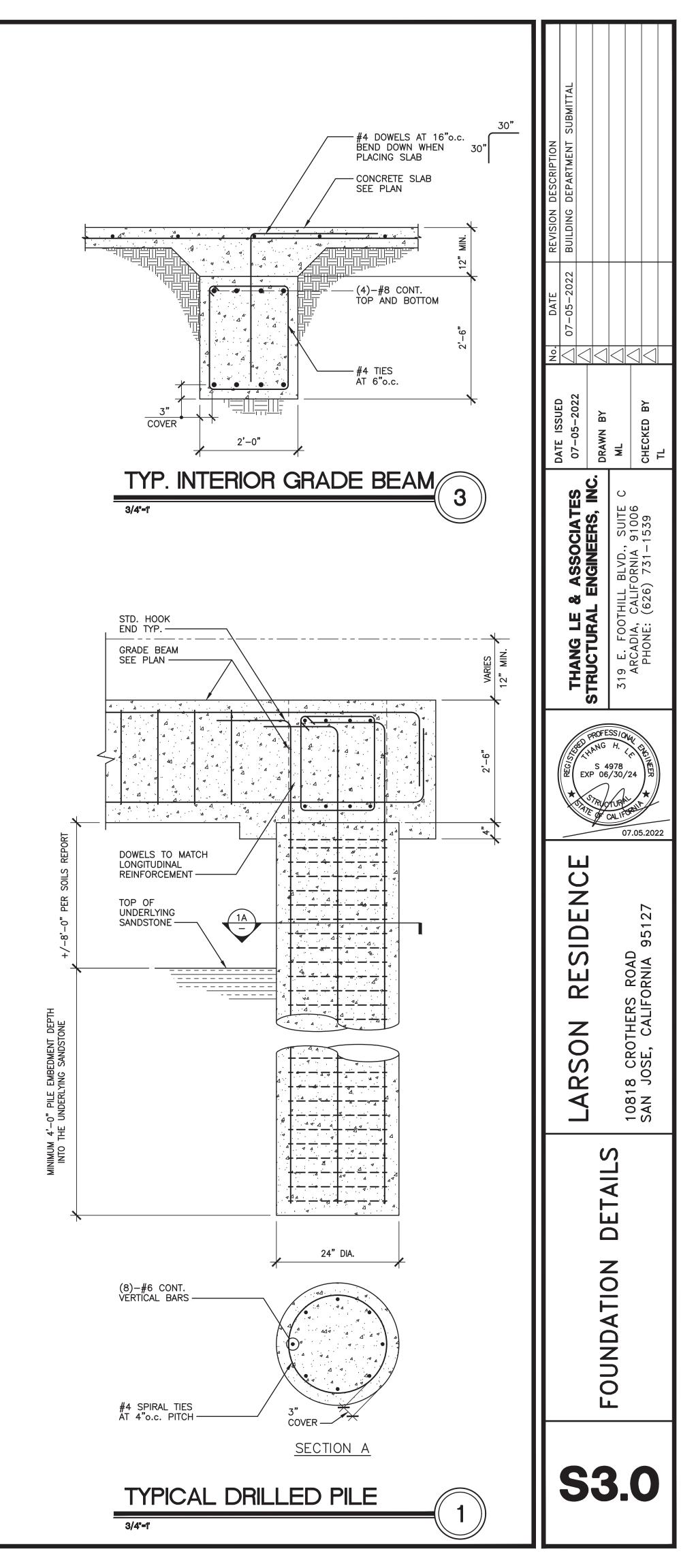


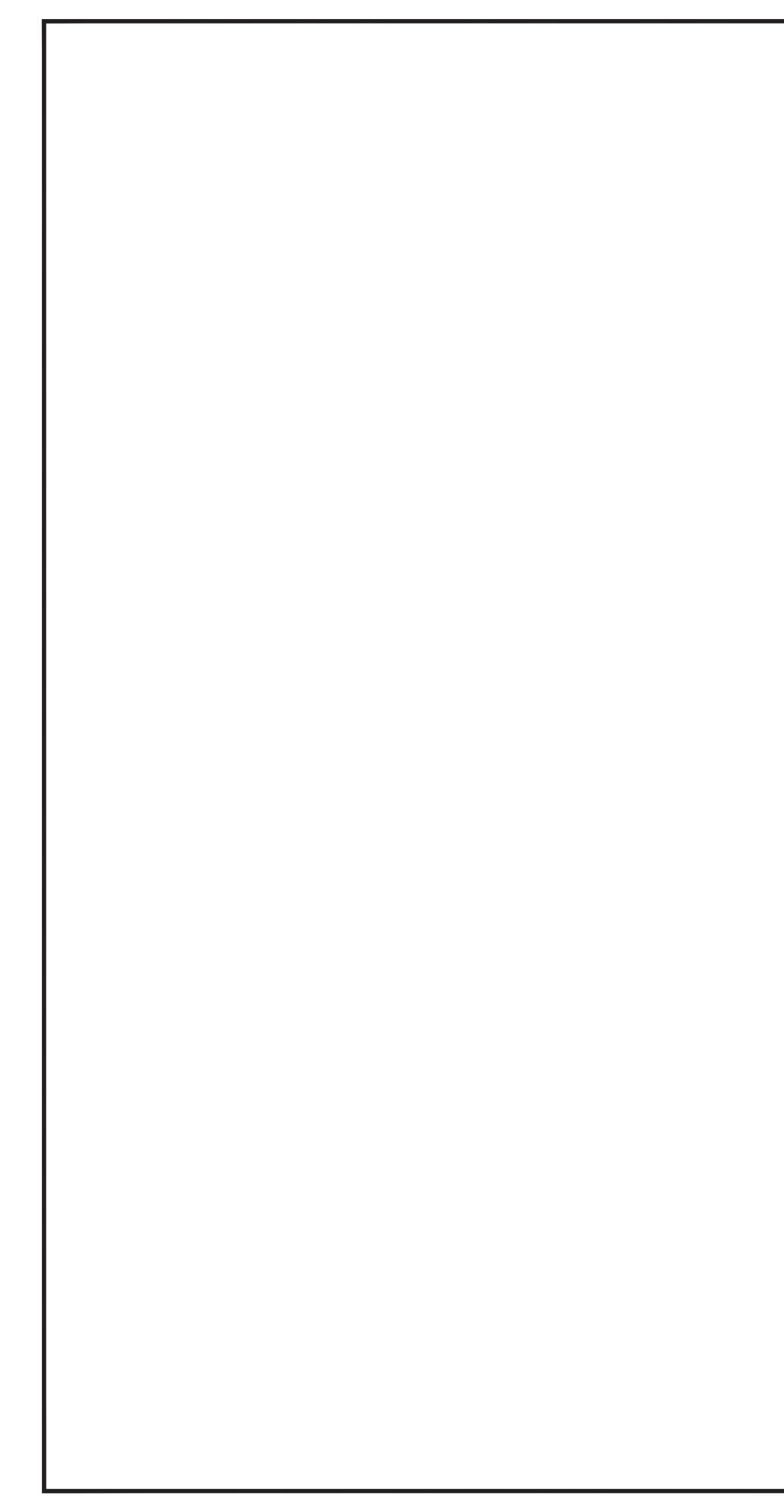
- #4 DOWELS AT 16"o.c.

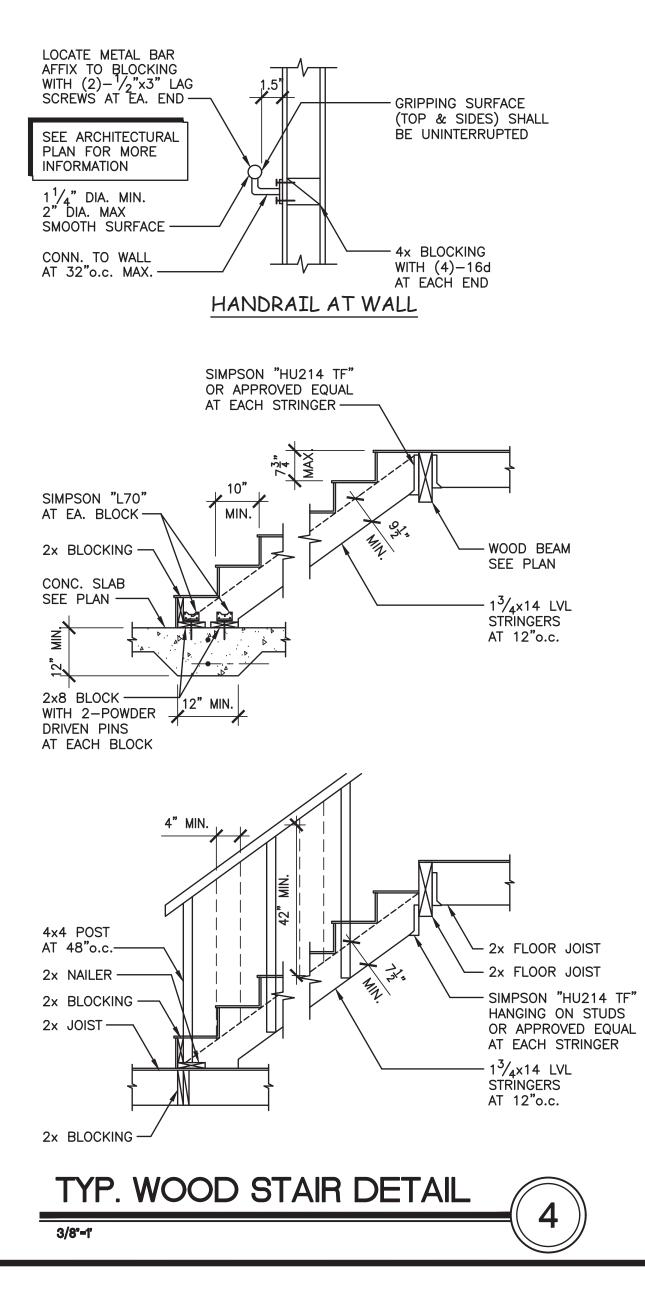
- CONCRETE SLAB SEE PLAN

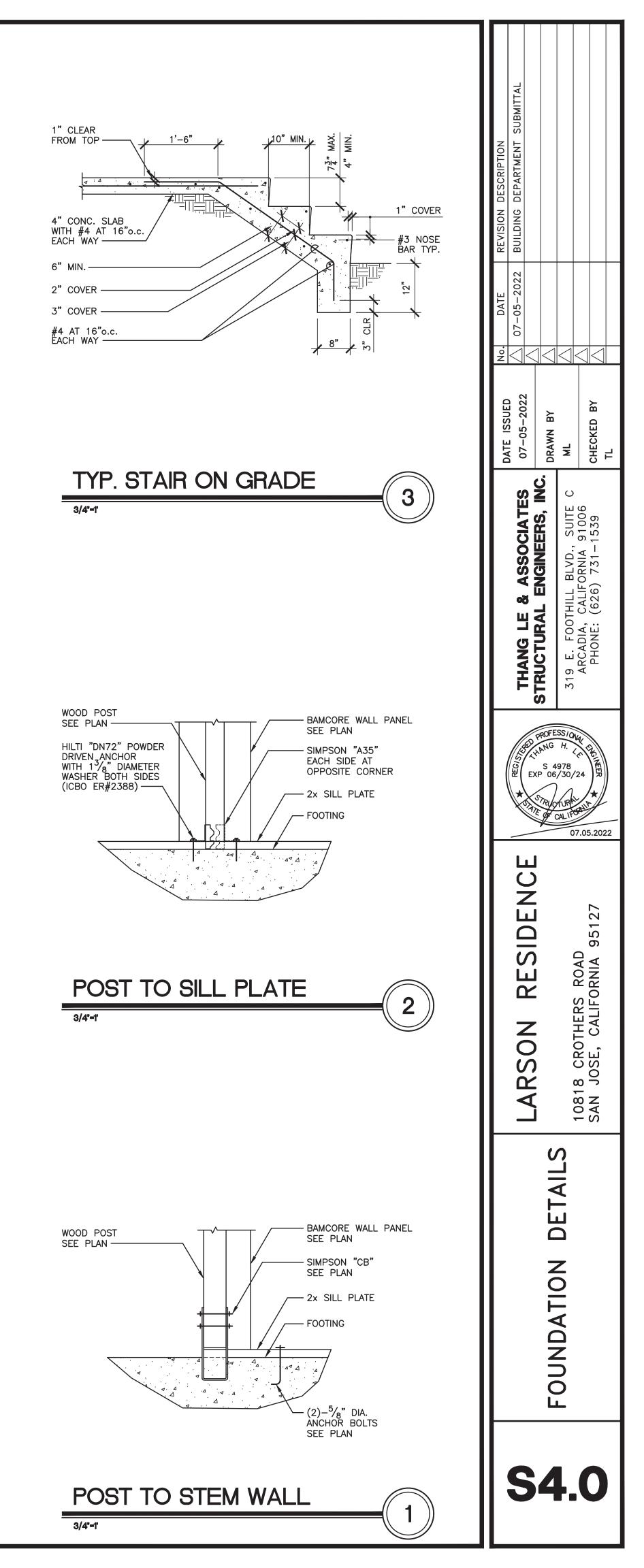
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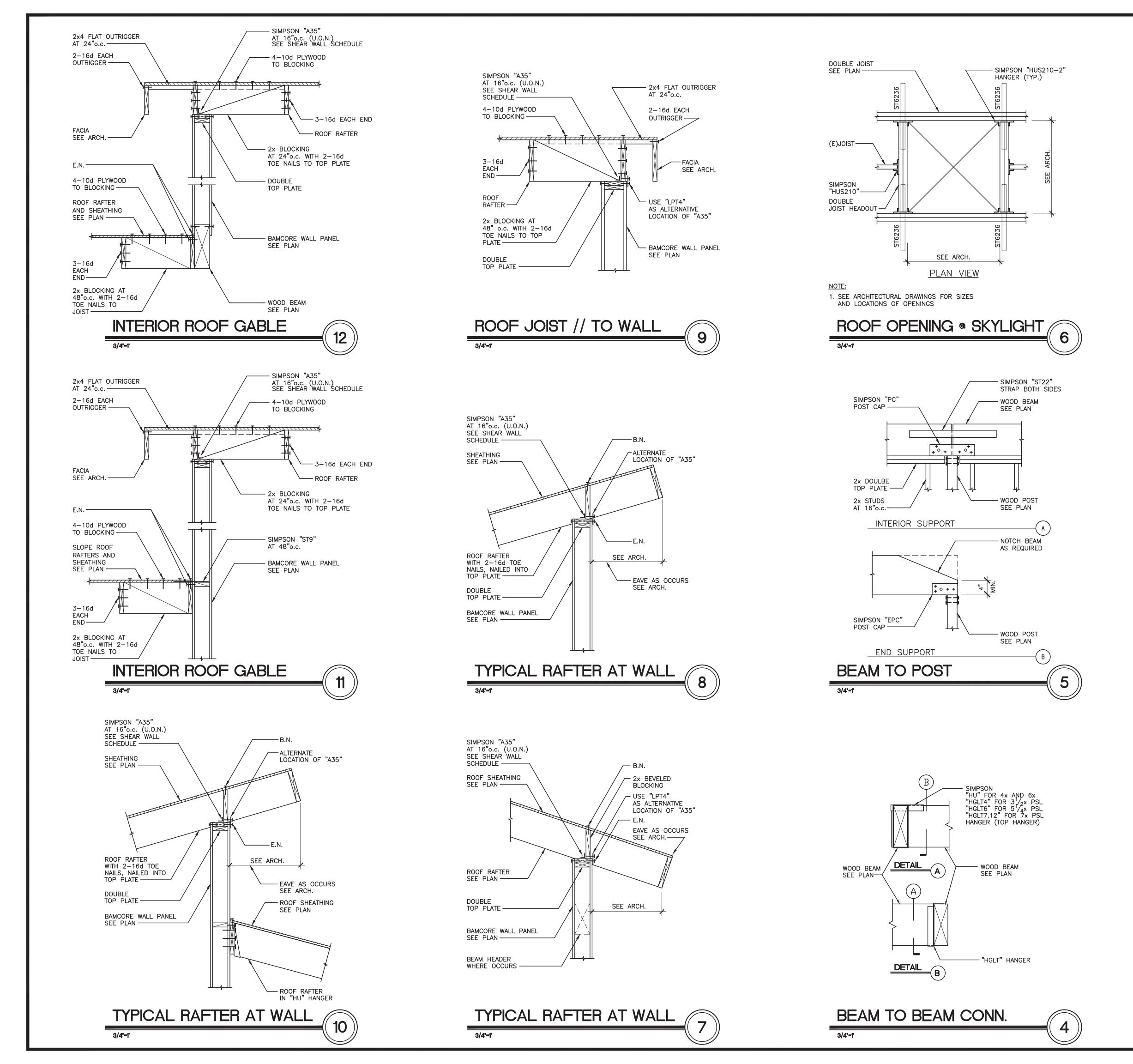
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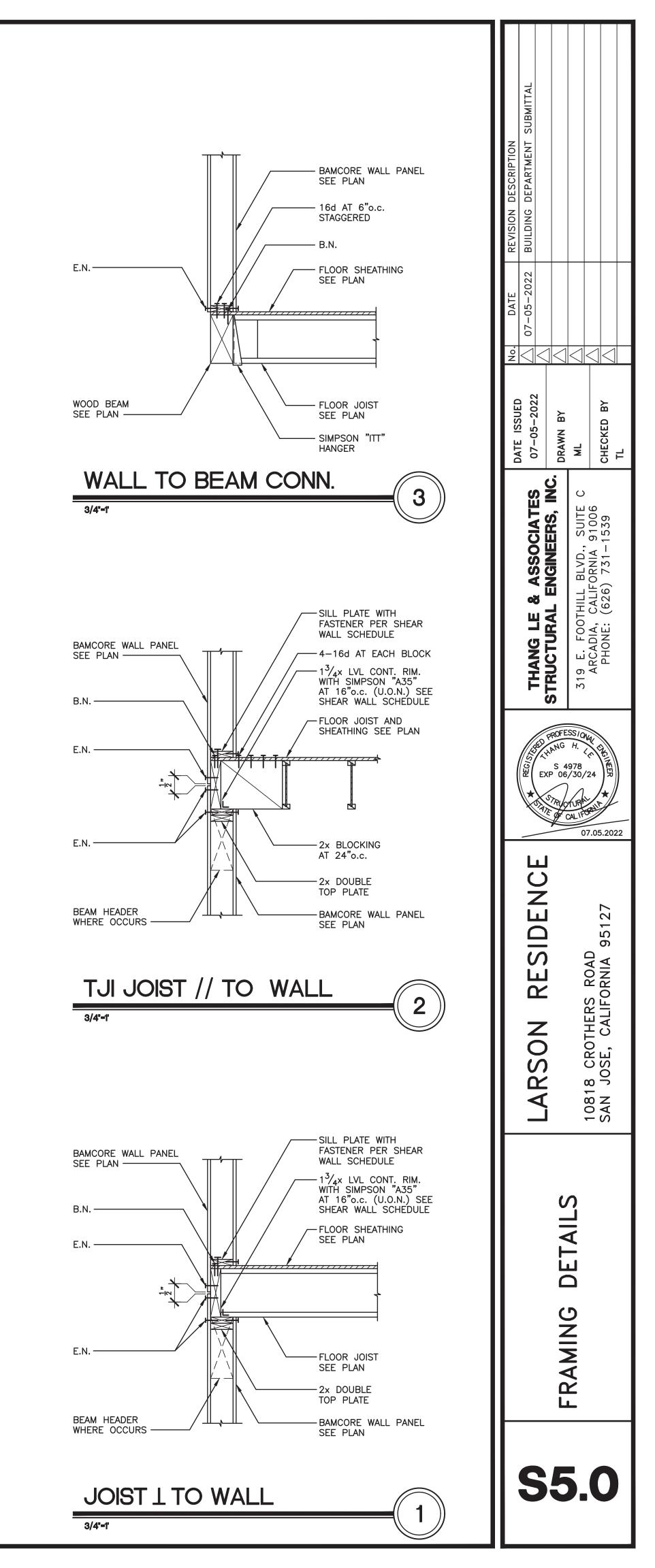


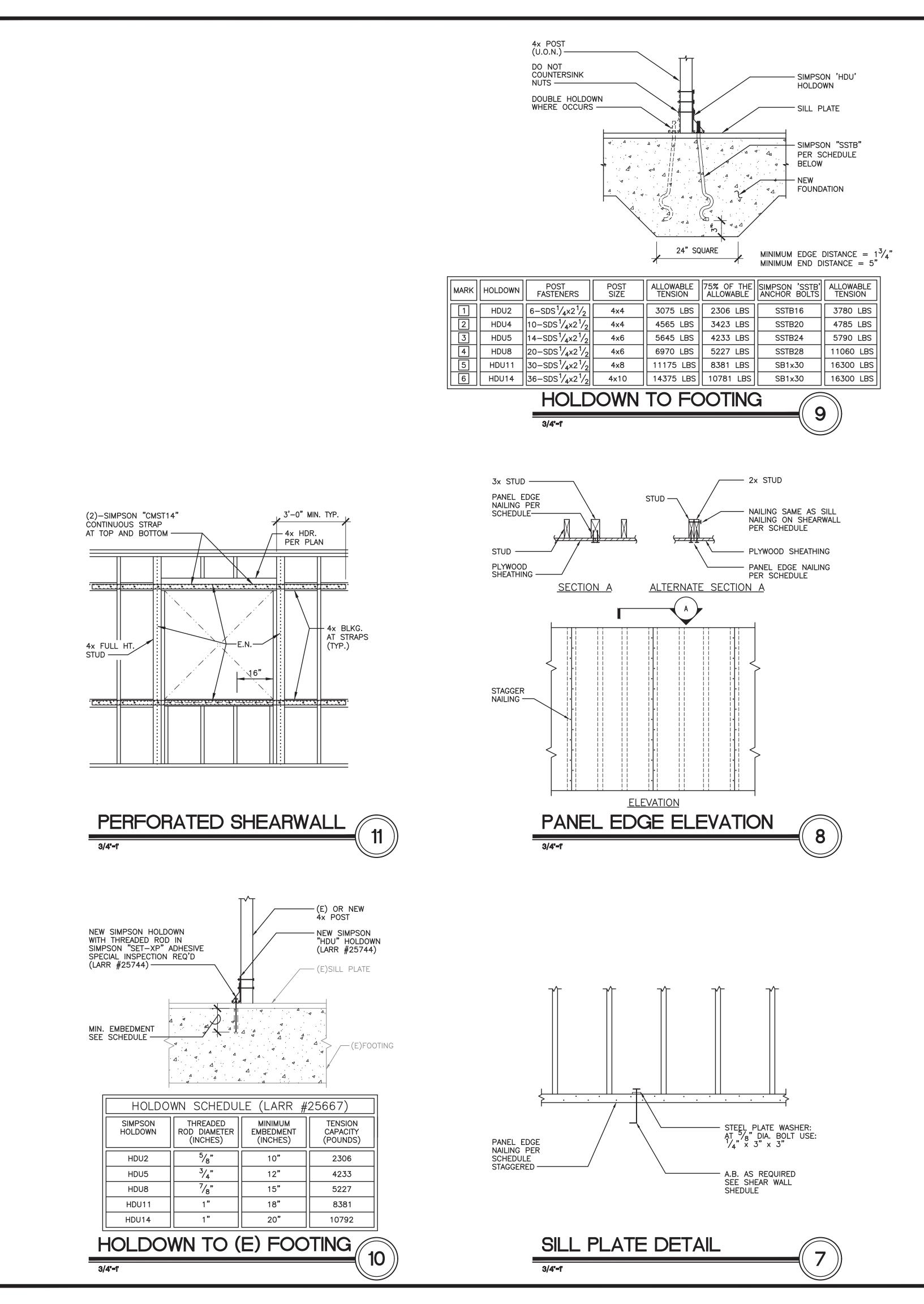


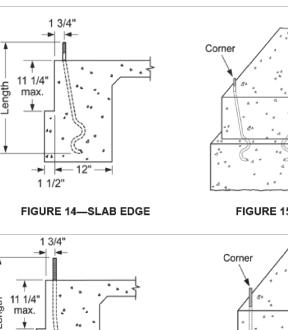












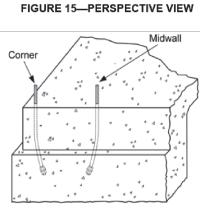


FIGURE 23—SLAB EDGE

1 1/2"

FIGURE 24—PERSPECTIVE VIEW

	SHEAR WALL SCHEDULE					
MARK	MATERIAL	NAILING (EDGE:FIELD)	UPPER FLOOR SILL I <u>P</u> NAILING	BLOCKING TO DOUBLE PL	ANCHOR BOLT SPACING	REMARKS
Â	15/32" STRUCT-I 4-PLY OR 5-PLY PLYWOOD 32/16	10d @ 6:12	16d @ 4"o.c.	SIMPSON A35 @ 16"o.c. ALT. SIMPSON LTP4 @ 16"o.c.	<sup>5</sup> ∕ <sub>8</sub> "ø @ 32"o.c.	CAPACITY:340 PLF
B	15/32" STRUCT-I 4-PLY OR 5-PLY PLYWOOD 32/16	10d @ 4:12	SDS <sup>1</sup> / <sub>4</sub> "ø x6" SCREWS @ 8"o.c. *	SIMPSON A35 @ 9"o.c. ALT. SIMPSON LTP4 @ 9"o.c.	<sup>5</sup> ∕ <sub>8</sub> "ø © 16"o.c.	CAPACITY:510 PLF
$\diamond$	15/32" STRUCT-I 4-PLY OR 5-PLY PLYWOOD 32/16	10d @ 3:12	SDS <sup>1</sup> / <sub>4</sub> "ø x6" SCREWS © 6"o.c. *	SIMPSON A35 @ 6"o.c. ALT. SIMPSON LTP4 @ 6"o.c.	<sup>5</sup> ∕ <sub>8</sub> "ø @ 16"o.c.	CAPACITY:665 PLF PRE-DRILL FOR LAG SCREW
	15/32" STRUCT-I 4-PLY OR 5-PLY PLYWOOD 32/16	10d @ 2:12	SDS <sup>1</sup> / <sub>4</sub> "ø x6" SCREWS © 6"o.c. *	SIMPSON A35 @ 12"o.c. EA. SIDE ALT. SIMPSON LTP4 @ 12"o.c. EA. SIDE	<sup>5</sup> / <sub>8</sub> "ø @ 16"o.c.	CAPACITY:870 PLF PRE-DRILL FOR LAG SCREW
BB	15/32" STRUCT–I 4–PLY OR 5–PLY PLYWOOD 32/16	10d @ 4:12	SDS <sup>1</sup> / <sub>4</sub> "ø x6" SCREWS © 6"o.c. *	SIMPSON A35 @ 9"o.c. EA. SIDE ALT. SIMPSON LTP4 @ 9"o.c. EA. SIDE	<sup>5</sup> / <sub>8</sub> "ø @ 16"o.c.	CAPACITY:1020 PLF PRE-DRILL FOR LAG SCREW
CB	15/32" STRUCT-I 4-PLY OR 5-PLY PLYWOOD 32/16	10d @ 3:12	SDS <sup>1</sup> / <sub>4</sub> "ø x6" SCREWS @ 4"o.c. *	SIMPSON A35 @ 6"o.c. EA. SIDE ALT. SIMPSON LTP4 @ 6"o.c. EA. SIDE	<sup>3</sup> ∕₄"ø @ 16"o.c.	CAPACITY:1330 PLF PRE-DRILL FOR LAG SCREW
DB	15/32" STRUCT–I 4–PLY OR 5–PLY PLYWOOD 32/16	10d @ 2:12	SDS <sup>1</sup> / <sub>4</sub> "ø x6" SCREWS @ 3"o.c. *	SIMPSON A35 @ 5"o.c. EA. SIDE ALT. SIMPSON LTP4 @ 5"o.c. EA. SIDE	<sup>3</sup> ∕₄"ø © 12"o.c.	CAPACITY:1740 PLF PRE-DRILL FOR LAG SCREW

\* INDICATES WITH  $4\frac{1}{4}$ " EMBEDMENT INTO MINIMUM  $2\frac{1}{2}$ x TIMBER STRAND BLOCKING OR RIM JOIST. MINIMUM EDGE DISTANCE SHALL BE  $\frac{3}{4}$ ". USE FULL BODY DIAMETER LAG SCREWS ONLY.

NOTES:

 FRAMING AT ADJOINING PANEL EDGES SHALL BE NOMINAL 3" OR WIDER. NAILS SHALL BE STAGGERED IN TWO ROWS ALONG PANEL EDGES.
 ALL NAILS SHALL BE COMMON NAILS. PROVIDE HOT DIPPED GALVANIZED NAILS AT ALL FIRE TREATED OR PRESSURE TREATED PLYWOOD AND STUDS. 3. WHERE PLYWOOD IS APPLIED ON BOTH FACES OF A WALL AND NAIL SPACING IS LESS THAN 6"o.c. ON EITHER SIDE, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS OR FRAMING SHALL BE 3" NOMINAL OR THICKER AND NAILS ON EACH SIDE BE STAGGERED. 3x FRAMING SHALL BE USED AT BOTTOM SILL PLATE AND ALL BLOCKING.

4. NAILS SHALL BE PLACED AT LEAST  $\frac{1}{2}$ " FROM PANEL EDGES AND AT LEAST  $\frac{3}{8}$ " FROM THE EDGE OF THE CONNECTING MEMBERS. 5. SIMPSON 'LTP4' FRAMING ANCHOR (LARR #25293) MAY BE APPLIED OVER  $\frac{1}{2}$ " SHEATHING WITH 8d COMMON NAILS IN LIEU OF 8dx1 $\frac{1}{2}$ " NAILS.

(B)- INDICATES SHEATHING OCCURS ON BOTH SIDES OF WALL. 6.

7. SHEATHING NAILS OR OTHER APPROVED SHEATHING CONNECTORS SHALL BE DRIVEN SUCH THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING. OVER-DRIVEN NAILS WILL BE DEEMED UNSATISFACTORY. 8. PROVIDE A SINGLE 3x NOMINAL OR WIDER FRAMING MEMBER AT BOTTOM SILL PLATE AND BEHIND VERTICAL AND HORIZONTAL EDGES, MINIMUM  $\frac{1}{2}$ " EDGE NAILING DISTANCE AT PANEL ENDS AND EDGES, AND STRUCTURAL OBSERVATION PER GENERAL NOTES.

9. SPECIAL INSPECTION IS REQUIRED FOR SHEAR WALL TYPES B, C, D, E, BB, CB, DB.

10. THE FOLLOWING APPLIES TO ALL SHEAR WALLS WITH A SHEAR VALUES USING ALLOWABLE STRESS DESIGN (ASD) EXCEED 350 PLF. THESE WALLS SHALL

PROVIDE WITH THE FOLLOWINGS: a. 3x STUDS AND BLOCKINGS FOR ALL FRAMING MEMBERS RECEIVING EDGE NAILING FROM ABUTTING PANELS. 12" EDGE DISTANCE FROM THE PANEL EDGES AND 37" FROM THE EDGE OF THE CONNECTING MEMBERS. b.

c. ALL WOOD STRUCTURAL PANEL JOINT AND SILL PLATE NAILING SHALL BE STAGGERED AT ALL PANEL EDGES. SHEAR WALL SCHEDULE

3/4**'=**1'

SILL NAILING FOR UPPER FLOOR SHEAR WALL WITH 16d AT 3"o.c STAGGERED TYP. U.O.N		
JOIST SEE PLAN	14" 2"	
2x BLOCKING OR END JOIST		
SIMPSON "A35" OR TOE NAILING OF BLOCKING OR JOIST TO TOP PLATE SEE SHEAR WALL SCHEDULE -	28-8d 28-8d 14" 2"	
BLOCK ALL PANEL EDGE W/ 2 × BLOCKING TYPIC (3× WHERE REQUIRED S NOTE #5 AND #6 ON SHEAR WALL SCHEDULE)	CAL EE	· · · · · · · · · · · · · · · · · · ·
SILL PLATE BOLTING AT FOOTING SEE SHEAR WALL SCHEDULE	+	
EDGE NAILING AT ALL KING STUDS AND HOLDOWN POSTS —		
TYPICAL SH	HEAR WAL	L CONS

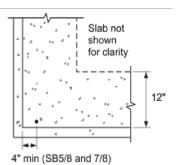
3/4**'=1**'



SSTB ANCHOR BOLTS

SB ANCHOR BOLTS

FIGURE 16—PLAN VIEW



3" min (SB1)

FIGURE 25—PLAN VIEW

 $\overline{\phantom{a}}$ <u>سطا\_\_\_\_\_لمعدما\_\_\_\_</u> 

HOLDOWN WHERE OCCURS SEE PLAN \_\_\_\_\_\_\_\_\_ - DOUBLE TOP PLATE - END POST SEE PLAN - NEW SIMPSON "CS16 COILED STRAP" WITH AND SHEAR WALL MINIMUM PANEL/WIDTH SHALL BE 2'.-0" TYPICAL: -NEW 2×4 FLAT SCHEDULE 8d AT 1"d.c. - FIELD NAILING AT 12"o.c. SEE SHEAR WALL SCHEDULE - EDGE NAILING - STUDS AT 16" o.c. SEE NOTE #5 AND #6 ON MAXIMUM - NEW 2×4 FLAT SHEAR WALL SCHEDULE-|| ₹|\ - SILL PLATE SEE NOTE #4 ON SHEAR WALL SCHEDULE ചഗ - HOLDOWN WHERE OCCURS SEE PLAN CONSTRUCTION

MAX

- WALL ABOVE

WHERE OCCURS

