# Structural Engineering Calculations 

## 1715 WESTBROOK AVE <br> RETAINING WALL DESIGN



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## KEY PLANS



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Adj Ftg Load = 817.\#
Ecc. $=-2.45$ in from CL


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Description :

Scope :

| Criteria |  |  |
| :--- | :--- | :---: |
| Retained Height | $=$ | 3.29 ft |
| Wall height above soil | $=$ | 0.50 ft |
| Slope Behind Wal | $=$ | $2.50: 1$ |
| Height of Soil over Toe | $=$ | 0.00 in |
| Soil Density | $=$ | 110.00 pct |
|  |  | 0.0 psf |

## $\begin{array}{ll}\text { Surcharge Loads } \\ \text { Surcharge Over Heel } & =0.0 \mathrm{psf} \\ \text { NOT Used To Resist Sliding \& Overturning } \\ \text { Surcharge Over Toe }= & =.0 \mathrm{psf} \\ \text { Used for Sliding \& Overturning }\end{array}$

## Axial Load Applied to Stem

| Axial Dead Load | $=$ | 0.0 lbs |
| :--- | :--- | :--- |
| Axial Live Load | $=$ | 0.0 lbs |
| Axial Load Eccentricity | $=$ | 0.0 in |


| Design Summary |  |  |
| :--- | :--- | :--- |
| Total Bearing Load | $=$ | 950 lbs |
| ...resultant ecc. | $=$ | 2.66 in |
| Soil Pressure @ Toe | $=$ | 145 psf OK |
| Soil Pressure @ Heel | $=$ | 262 psf OK |
| $\quad$ Allowable | $=$ | $1,500 \mathrm{psf}$ | Soil Pressure Less Than Allowabs ACI Factored @ Toe = 247 psf ACI Factored @ Heel = 445 psf

Footing Shear @ Toe = 6.4 psi OK
Footing Shear @ Heel = 0.0 psi OK
Allowable $=93.1 \mathrm{psi}$
Wall Stability Ratios
Overturning $=5.65 \mathrm{OK}$
Sliding $=\quad 0.97$ UNSTABLE

Sliding Calcs Slab Resists All Sliding!
Lateral Sliding Force $=369.2 \mathrm{lbs}$


| Soil Data |  |
| :--- | :--- | :--- |
| Allow Soil Bearing | $=1,500.0 \mathrm{psf}$ |
| Equivalent Fluid Pressure Method   <br> Heel Active Pressure $=$ 45.0 <br> Toe Active Pressure $=$ 45.0 <br> Passive Pressure $=$ 250.0 <br> Water height over heel $=$ 0.0 ft <br> Footing\||Soil Frictior $=$ 0.300 <br> Soil height to ignore <br> for passive pressure $=$ 0.00 in |  |


| Lateral Load Applied to Stem |  |  |
| :--- | :--- | :--- |
| Lateral Load | $=$ | $0.0 \mathrm{\# fft}$ |
| $\ldots$ Height to Top | $=$ | 0.00 ft |
| $\ldots$ Height to Bottor | $=$ | 0.00 ft |


| Adjacent Footing Load |  |  |
| :--- | :--- | ---: |
| Adjacent Footing Load | $=$ | 817.0 lbs |
| Footing Width | $=$ | 1.00 ft |
| Eccentricity | $=$ | 2.45 in |
| Wall to Ftg CL Dist | $=$ | 5.80 ft |
| Footing Type |  | Line Load |
| Base Above/Below Soil |  | -2.0 ft |


| Stem Construction Top Stem |  |  |
| :---: | :---: | :---: |
|  | Design height $\mathrm{ft}=$ | $\mathrm{ft}=\begin{array}{r} \text { Stem OK } \\ 0.00 \end{array}$ |
|  | Wall Material Above "Ht" = | $=$ Concrete |
|  | Thickness = | - 8.00 |
|  | Rebar Size = | = $\# 4$ |
|  | Rebar Spacing = | 12.00 |
|  | Rebar Placed at = | Edge |
|  | Design Data $\mathrm{fb} / \mathrm{FB}+\mathrm{fa} / \mathrm{Fa}$ | $=0.084$ |
|  | Total Force @ Section lbs = | $\mathrm{lbs}=\quad 414.0$ |
|  | Moment....Actual ft-\# = | $\mathrm{ft}-\mathrm{=}=454.0$ |
|  | Moment.....Allowable = | $=5,412.6$ |
|  | Shear.....Actual psi = | psi $=\quad 5.5$ |
|  | Shear.....Allowable psi = | $\mathrm{psi}=\quad 85.0$ |
| E! | ! Bar Develop ABOVE Ht. in = | in $=18.72$ |
|  | Bar Lap/Hook BELOW Ht. in = | in $=6.00$ |
|  | Wall Weight = | 96.7 |
|  | Rebar Depth 'd' in = | in $=\quad 6.25$ |
|  | $\underset{\text { Masonry Data }}{ }$ |  |
|  | f'm $\mathrm{psi}=$ <br> Fs psi | psi = |
|  | Solid Grouting | = |
|  | Special Inspection | $=$ |
|  | Modular Ratio ' n ' | = |
|  | Short Term Factor | = |
|  | Equiv. Solid Thick. = | $=$ |
|  | Masonry Block Type = Medium | dium Weight |
|  | Concrete Data f'c $\qquad$ | psi $=2,500.0$ |
|  | Fy psi = | $\mathrm{psi}=60,000.0$ |
|  | Other Acceptable Sizes \& Spacings <br> Toe: Not req'd, $\mathrm{Mu}<\mathrm{S}$ * Fr <br> Heel: Not req'd, Mu < S * Fr <br> Key: No key defined |  |

## Summary of Overturning \& Resisting Forces \& Moments



## THE END

