15225 OAK GLEN AVE - STORAGE SHED

15225 OAK GLEN AVENUE, MORGAN HILL, CA 95037

CUSTOM COLD-FORMED STEEL SHED

GENERAL NOTES

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S0.1	GENERAL NOTES, SHEET INDEX & ABBREVIATIONS
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S1.2	TYP FRAMING DETAILS
S1.3	TYP FRAMING DETAILS
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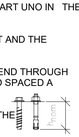
LIGHT GAUGE STEEL (CONT.)

10. ALL CALCULATED STUD PROPERTIES PER AISI SPECIFICATION ARE BASED ON THE FOLLOWING THICKNESS:
    - A. 10GA. (118 MIL) 0.1242"
    - B. 12GA. (97 MIL) 0.1017"
    - C. 14GA. (68 MIL) 0.0713"
    - D. 16GA. (54 MIL) 0.0566"
    - E. 18GA. (43 MIL) 0.0451"
    - F. 20GA. (33 MIL) 0.0346"
11. LATERAL BRIDGING FOR STEEL STUDS IS REQUIRED WHEN WALL BOARD, INSTALLED IN ACCORD WITH BUILDING CODE REQUIREMENTS, DOES NOT CONTINUE FULL HEIGHT ON BOTH SIDES, UNLESS NOTED OTHERWISE. BRIDGING SHALL BE INSTALLED IN ACCORD WITH OUR TYPICAL DETAILS. WALL STUD BRIDGING SHALL BE INSTALLED IN A MANNER TO PROVIDE RESISTANCE TO BOTH MINOR AXIS BENDING AND ROTATION.
12. TRACK SHALL BE UNPUNCHED WITH GAUGE TO MATCH STUD FRAMING UNLESS NOTED OTHERWISE.
13. UTILITY PUNCH HOLES IN STUDS SHALL BE LOCATED AWAY FROM CONNECTIONS.
14. THE MINIMUM CLEAR DISTANCE FROM THE UTILITY PUNCH HOLE TO END OF MEMBER SHALL BE 10", UNLESS NOTED OTHERWISE.
15. AXIAL LOAD BEARING STUDS MUST BE FULLY SEATED INTO THE WALL TRACKS, (1/16" MAXIMUM GAP BETWEEN THE STUDS AND THE TRACK WEBS).
16. OPENINGS IN STUD WEBS OTHER THAN STANDARD HOLES PUNCHED BY THE MANUFACTURER ARE PROHIBITED UNLESS SPECIFICALLY DETAILED.
17. ALL STEEL STUDS AND TRACKS SHALL BE MANUFACTURED BY A MANUFACTURER WITH A MINIMUM OF TEN YEARS EXPERIENCE. STEEL STUDS AND TRACKS MUST, AT A MINIMUM, MEET THE PROPERTIES LISTED IN THE SFIA PRODUCT TECHNICAL INFORMATION GUIDE AND THE STUD AND TRACK PROPERTIES LISTED ON THIS PAGE. ALL STUDS AND TRACKS SHALL BE LABELED WITH GAUGE, YIELD STRENGTH AND SIZE CLEARLY VISIBLE.
18. INSTALL LOAD BEARING SHIMS OR GROUT BETWEEN THE UNDERSIDE OF WALL BOTTOM TRACK AND THE TOP OF FOUNDATION WALL OR SLAB AT STUD TO ENSURE A UNIFORM BEARING SURFACE ON SUPPORTING CONCRETE CONSTRUCTION.
19. FABRICATE COLD-FORMED METAL FRAMING AND ACCESSORIES PLUMB, SQUARE, AND TRUE TO LINE, AND WITH CONNECTIONS SECURELY FASTENED, ACCORDING TO REFERENCED AISI'S SPECIFICATIONS AND STANDARDS, MANUFACTURER'S WRITTEN INSTRUCTIONS.
    - A. FABRICATE FRAMING ASSEMBLIES USING JIGS OR TEMPLATES.
    - B. CUT FRAMING MEMBERS BY SAWING OR SHEARING; DO NOT TORCH CUT.
    - C. FASTEN COLD-FORMED METAL FRAMING MEMBERS BY WELDING, SCREW FASTENING.
    - D. COMPLY WITH AWS D1.3 REQUIREMENTS AND PROCEDURES FOR WELDING, APPEARANCE AND QUALITY OF WELDS, AND METHODS USED IN CORRECTING WELDING WORK.
20. INSTALL SUPPLEMENTARY FRAMING, BLOCKING, AND BRACING IN STUD FRAMING INDICATED TO SUPPORT FIXTURES, EQUIPMENT, SERVICES, CASEWORK, HEAVY TRIM, FURNISHINGS, AND SIMILAR WORK REQUIRING ATTACHMENT TO FRAMING.

FASTERNERS & CONNECTERS				
CONNECTOR TYPE	SUBSTRATE	DESCRIPTION	PRODUCT	NOTED ON PLANS AS
SCREWS	METAL TRACK	#10-16 PAN HEAD	PROTWIST SCREWS PER ESR-1048 OR APPROVED EQUAL	SMS
	STUD-TO-STUD	#10-16 HEX HEAD OR PAN HEAD	PROTWIST SCREWS PER ESR-1408 OR APPROVED EQUAL	SMS
LVF'S (LOW VELOCITY FASTENERS)	CONCRETE OR GROUTED CMU	0.157" DIA x 1 1/4" EMBED	HILTI X-U PER ESR-2269 OR APPROVED EQUAL	LVF
LVF'S (LOW VELOCITY FASTENERS)	STRUCTURAL STEEL	0.157" DIA	HILTI X-U PER ESR-2269 OR APPROVED EQUAL	LVF
MECHANICAL ANCHORS	CONCRETE	1/2" DIA x 3" EMBED UNO	HILTI KWIK HUS-EZ PER ESR-3027 OR APPROVED EQUAL	SCREW ANCHOR

FASTENERS AND CONNECTOR NOTES

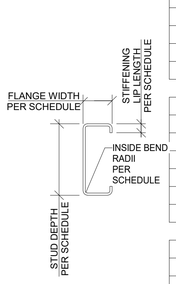
1. ALL FASTENERS SHALL BE THE MIN. SIZES AND EMBEDMENTS OF THE ABOVE CHART UNO IN THE PLANS.
2. ALL FASTENERS SHALL BE INSTALLED IN ACCORD WITH THE NOTED ESR REPORT AND THE REQUIREMENTS OF THE GOVERNING AUTHORITY.
3. SCREWS LISTED IN THE ABOVE CHART SHALL BE SUFFICIENT IN LENGTH TO EXTEND THROUGH THE STEEL CONNECTION WITH A MINIMUM OF THREE (3) EXPOSES THREADS AND SPACED A MINIMUM OF 3 FULL DIAMETERS.
4. FOR MECHANICAL ANCHORS, THE EMBEDMENT LISTED IN THE ABOVE CHART IS THE NOMINAL EMBEDMENT, h_{nom} SEE THE DIAGRAM TO THE RIGHT.



LIGHT GAUGE STEEL

1. ALL WORK SHALL MEET THE REQUIREMENTS OF THE FOLLOWING STANDARDS:
    - A. AMERICAN IRON AND STEEL INSTITUTE (AISI) DESIGN OF COLD FORMED STEEL STRUCTURAL MEMBERS.
    - B. AMERICAN WELDING SOCIETY (AWS) D1.1 AND D1.3 SPECIFICATION FOR WELDING SHEET STEEL IN STRUCTURE.
    - C. AMERICAN SOCIETY FOR TESTING AND MATERIALS (ASTM).
2. ALL STUD AND TRACK MATERIAL TO CONFORM TO THE FOLLOWING:
    - A. 16GA. AND HEAVIER: 50 KSI MIN. YIELD, 65 KSI MIN. TENSILE STRENGTH ASTM A1003 STRUCTURAL GRADE 50 TYPE H (ST50H)
    - B. 18 GA. AND LIGHTER: 33 KSI MIN. YIELD, 45 KSI MIN. TENSILE STRENGTH ASTM A1003 STRUCTURAL GRADE 33 TYPE H (ST33H)
    - C. ALL STUDS, TRACKS, AND MISC PIECES TO BE MIN G60 GALVANIZED
3. MISCELLANEOUS STEEL TO CONFORM TO THE FOLLOWING:
    - A. 20GA. - 18GA. 33 KSI MIN. YIELD, 45 KSI MIN. TENSILE
    - B. 16GA. - 10GA. 50 KSI MIN. YIELD, 65 KSI MIN. TENSILE
    - C. 3/16" AND HEAVIER ASTM A36
4. ALL WELDING TO BE PERFORMED BY CERTIFIED LIGHT GAUGE WELDERS CERTIFIED FOR ALL APPROPRIATE DIRECTION COMPLYING WITH AWS D1.3. WELDING RODS TO CONFORM TO THE FOLLOWING:
    - A. 43 MIL AND LIGHTER E60XX
    - B. 54 MIL AND HEAVIER E70XX OR E6013
    - C. LT. GAUGE TO STRUCT'L STL. E70XX LOW HYDROGEN
5. NOMINAL WELD SIZES FOR WELDING LIGHT GAUGE MATERIAL SHALL BE AS FOLLOWS:
    - A. 20GA. 1/16"
    - B. 18GA. 3/32"
    - C. 16GA. AND HEAVIER 1/8"
6. ALL FRAMING COMPONENTS SHALL BE CUT SQUARELY OR ON AN ANGLE SUCH AS BRACING TO SQUARELY FIT AGAINST ABUTTING MEMBERS. MEMBERS SHALL BE HELD FIRMLY IN POSITION UNTIL PROPERLY FASTENED.
7. ALL STUDS SHALL BE ATTACHED BY SCREWS OR WELDS UNLESS NOTED OTHERWISE. WIRE TYING OF FRAMING COMPONENTS IS NOT PERMITTED.
8. SPLICES IN TOP AND BOTTOM TRACKS ARE REQUIRED WHERE TRACKS ARE NOT ATTACHED TO A COMMON CONTINUOUS STRUCTURAL MEMBER AND SHALL BE ACCOMPLISHED WITH A NESTED STUD OF THE SAME GAGE AS TRACK WITH A 10" LENGTH AND (2) #1O S.M.S. EACH SIDE, EACH TRACK U.N.O. ON PLANS.(8) #10 S.M.S. TOTAL
9. BUTT WELDS OR SPLICE SHALL BE USED AT ALL JOINTS IN TRACK SPLICES IN AXIAL LOADED STUDS OR BRACES ARE NOT PERMITTED. ALL WELDS SHALL BE PLUG, BUTT, OR SEAM WELDS. WHERE STUDS ARE BURNED THROUGH BY WELDING, PROVIDE SUITABLE STITCH PLATE OF THE SAME GAUGE.

STUD PROPERTIES

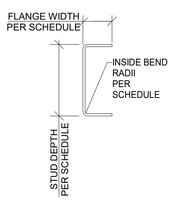


MINIMUM REQUIRED STIFFENING LIP LENGTH	
FLANGE WIDTH	MIN. STIFFENING LIP LENGTH (in.)
1 1/4"	0.188"
1 3/8"	0.375"
1 5/8"	0.500"
2"	0.625"
2 1/2"	0.625"
3"	0.625"
3 1/2"	1.000"

STUD / TRACK DEPTH
1 5/8"	1.625"
2 1/2"	2.500"
3 5/8"	3.625"
4"	4.000"
6"	6.000"
8"	8.000"

INSIDE BEND RADII PER MATERIAL THICKNESS
33 MIL	0.0764"
43 MIL	0.0712"
54MIL	0.0849"
68 MIL	0.1069"
97 MIL	0.1525"
118 MIL	0.1863"

TRACK PROPERTIES



TRACK FLANGE WIDTH
1"	1.000"
1 1/4"	1.250"
1 1/2"	1.500"
2"	2.000"
2.5"	2.500"
3"	3.000"

INSIDE BEND RADII PER MATERIAL THICKNESS
33 MIL	0.0764"
43 MIL	0.0712"
54MIL	0.0849"
68 MIL	0.1069"
97 MIL	0.1525"
118 MIL	0.1863"

STANDARD STUD IDENTIFICATION (SFIA NOMENCLATURE)

STUD IDENTIFICATION SHALL BE AS SHOWN:

1. MEMBER DEPTH:
   (EXAMPLE: 6"=600/100 INCHES)
   ALL MEMBER DEPTHS ARE TAKEN IN 1/100 INCHES. FOR ALL "T" SECTIONS MEMBER DEPTH IS THE INSIDE TO INSIDE DIMENSION.
2. STYLE:
   (EXAMPLE: STUD OR JOIST SECTIONS=S)
   THE FOUR ALPHA CHARACTERS UTILIZED BY THE DESIGNATOR SYSTEM ARE:
   S = STUD
   T = TRACK
   U = CHANNEL SECTIONS
   F = FURRING CHANNEL SECTIONS
3. FLANGE WIDTH:
   (EXAMPLE: 1 5/8"=1.625"=162x1/100 INCHES)
   ALL FLANGE WIDTHS ARE TAKEN IN 1/100 INCHES.
4. MATERIAL THICKNESS:
   (EXAMPLE: 0.054IN. = 54MILS; 1 MIL. = 1/1000 IN.)
   MATERIAL THICKNESS IS THE MINIMUM BASE METAL THICKNESS IN MILS. MINIMUM BASE METAL THICKNESS REPRESENTS 95% OF THE DESIGN THICKNESS.

1	2	3		4
600	S	162	-	54

GENERAL

1. THE CONTRACTOR SHALL PERFORM CONSTRUCTION AND WORKMANSHIP IN COMPLIANCE WITH THE DRAWINGS, SPECIFICATIONS AND THE 2022 CALIFORNIA BUILDING CODE. THE PROJECT SPECIFICATIONS FORM A PART OF THE GENERAL NOTES.
2. GOVERNING CODE AUTHORITY: CITY OF MORGAN HILL BUILDING DEPARTMENT
3. GENERAL NOTES AND TYPICAL DETAILS APPLY TO THE DRAWINGS UNLESS NOTED OR DETAILED OTHERWISE. WHERE DISCREPANCIES BETWEEN GENERAL NOTES AND DRAWINGS OCCUR, DRAWINGS TAKE PRECEDENCE.
4. DRAWINGS INDICATE GENERAL AND TYPICAL DETAILS OF CONSTRUCTION. WHERE CONDITIONS ARE NOT SPECIFICALLY INDICATED BUT ARE OF SIMILAR CHARACTER TO DETAILS SHOWN, USE SIMILAR DETAILS OF CONSTRUCTION SUBJECT TO REVIEW BY ARCHITECT.
5. ARCHITECTURAL, MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS ARE CONSIDERED A PART OF THE STRUCTURAL DRAWINGS AND ARE TO BE USED TO DEFINE CONFIGURATIONS INCLUDING, BUT NOT LIMITED TO, RELATIVE LOCATION OF MEMBERS, ELEVATIONS, LOCATION OF OPENINGS, LOCATION OF CURBS, AND SLAB DEPRESSIONS, HOUSEKEEPING PADS, ETC.
6. REFER TO MECHANICAL, PLUMBING AND ELECTRICAL DRAWINGS FOR SIZE AND LOCATION OF OPENINGS, PENETRATIONS AND EMBEDMENTS FOR DUCTS, PIPING, VENTS, CONDUITS AND OTHER ITEMS TO BE INCORPORATED IN STRUCTURAL WORK. FOR SPECIFIC OPENING, PENETRATION AND EMBEDMENT REQUIREMENTS IN STRUCTURAL WORK, SEE SPECIFICATIONS AND MATERIAL SECTIONS OF THESE GENERAL NOTES BELOW.
7. FIRMLY ATTACH MECHANICAL AND ELECTRICAL EQUIPMENT TO THE STRUCTURE. ISOLATORS, FASTENERS AND ANY OTHER ELEMENT PROVIDING STABILITY FOR EQUIPMENT SHALL BE APPROVED BY ICBO OR EQUIVALENT TESTING PROCEDURE AND BE CAPABLE OF TRANSMITTING CODE REQUIRED LOADS BUT IN NO EVENT LESS THAN A SHEAR LOAD EQUIVALENT TO AT LEAST 0.3 X OPERATING WEIGHT OF THE EQUIPMENT.
8. COORDINATE THE WORK OF ALL TRADES AND CHECK ALL DIMENSIONS. BRING DISCREPANCIES TO THE ATTENTION OF THE ARCHITECT TO BE RESOLVED BEFORE PROCEEDING WITH THE WORK.
9. CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. UNLESS OTHERWISE SHOWN, THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION. CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES ARE THE SOLE RESPONSIBILITY OF THE CONTRACTOR. PROVIDE ADEQUATE SHORING, BRACING AND GUYS COMPLYING WITH NATIONAL, STATE AND LOCAL SAFETY ORDINANCES. ERECTION PROCEDURES SHALL COMPLY WITH OSHA STANDARDS.
10. THE CONTRACTOR IS SOLELY RESPONSIBLE FOR ALL EXCAVATION PROCEDURES INCLUDING LAGGING, SHORING AND PROTECTION OF ADJACENT PROPERTY, STRUCTURES, STREETS AND UTILITIES COMPLYING WITH NATIONAL, STATE, AND LOCAL SAFETY ORDINANCES. THE CONTRACTOR SHALL INVESTIGATE SITE FOR FILLED EXCAVATIONS OR BURIED STRUCTURES SUCH AS CESSPOOLS, CISTERNS, FOUNDATIONS, ETC. IF ANY SUCH STRUCTURES ARE FOUND, ARCHITECT SHALL BE IMMEDIATELY NOTIFIED.
11. OBSERVATION VISITS TO THE SITE BY FIELD REPRESENTATIVES OF THE ARCHITECT DO NOT INCLUDE INSPECTIONS OF THE PROTECTIVE MEASURES, OR INSPECTIONS OF THE METHODS OF CONSTRUCTION. SUPPORT SERVICES PERFORMED BY THE ARCHITECT DURING CONSTRUCTION SHALL BE DISTINGUISHED FROM CONTINUOUS AND DETAILED INSPECTION SERVICES WHICH ARE PERFORMED BY OTHERS.
12. MODIFICATIONS OR SUBSTITUTIONS: DESIGN, MATERIALS, EQUIPMENT AND PRODUCTS OTHER THAN THOSE INDICATED OR SPECIFIED MAY BE CONSIDERED FOR USE PROVIDED PRIOR APPROVAL IS OBTAINED FROM THE OWNER, ARCHITECT, AND THE GOVERNING CODE AUTHORITY.
13. REVIEW AND STAMP SHOP DRAWINGS PRIOR TO SUBMISSION TO ARCHITECT. REVIEW SHOP DRAWINGS FOR COMPLETENESS AND COMPLIANCE WITH THE DRAWINGS AND SPECIFICATIONS. SUBMIT WRITTEN REQUEST TO THE ARCHITECT FOR APPROVAL OF ANY MODIFICATION OR SUBSTITUTION. MODIFICATIONS OR SUBSTITUTIONS MUST BE APPROVED BEFORE SHOP DRAWINGS ARE SUBMITTED.

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NOTE:
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CUSTOM COLD-FORMED STEEL SHED
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REGISTERED PROFESSIONAL ENGINEER
THOMAS A. OSTLER
No. 4083
STRUCTURAL
STATE OF CALIFORNIA

REV	DATE	DESCRIPTION

DRAWN BY RDB

1ST ISSUE DATE 12/17/2024

SHEET TITLE

GENERAL NOTES, SHEET INDEX & ABBREVIATIONS

DOCUMENT REVIEW	
DESIGN ENGINEER	PROJECT DRAFTSMAN
JSK	RDB

PROJECT NO.

B24-239

SHEET NO.

S0.1

GENERAL NOTES

ROUGH CARPENTRY

1. PROVIDE DOUGLAS FIR STRUCTURAL LUMBER COMPLYING WITH STANDARD GRADING RULES OF THE WEST COAST LUMBER INSPECTION BUREAU (1995 EDITION) AND CBC SECTION 2303. PROVIDE AIR DRY LUMBER WITH A 19% MAXIMUM MOISTURE CONTENT.

2. PROVIDE PLYWOOD OR OSB COMPLYING WITH U.S. PRODUCT STANDARD PS 1.95 OR OSB MANUFACTURERED IN CONFORMANCE WITH THE US DEPARTMENT OF COMMERCE VOLUNTARY PRODUCT STANDARD PS 2 AND CLASSIFIED AS EXPOSURE 1 EACH SHEET OF PLYWOOD OR OSB SHALL BE IDENTIFIED WITH THE APPROPRIATE TRADEMARK OF THE AMERICAN PLYWOOD ASSOCIATION. PLYWOOD TYPES:

   A. WALL SHEATHING.....STRUCTURAL I,5 T+G PLYWOOD OR OSB
      1/2" OR 15/32".....SPAN RATING 32/16
   B. ROOF SHEATHING.....STRUCTURAL I,5 T+G PLYWOOD OR OSB
      1/2 OR 15/32".....SPAN RATING 32/16

TEST AND INSPECTIONS

1. PROVIDE ALL STRUCTURAL MATERIALS FROM TESTED STOCK. FURNISH COPIES OF TEST REPORTS TO ARCHITECT AND THE GOVERNING CODE AUTHORITY UPON REQUEST.

2. SEE SPECIFICATIONS FOR ADDITIONAL TEST AND INSPECTION REQUIREMENTS.

3. THE USE OF ROLLED STEEL SECTIONS, BOLTS AND OR REBAR MANUFACTURED OUTSIDE THE U.S. WILL REQUIRE VERIFICATION THAT THE PRODUCTS COMPLY WITH APPLICABLE ASTM STANDARDS. MILL CERTIFICATES WILL BE REQUIRED FOR ALL STEEL. ALL FOREIGN BOLTS MUST BE APPROVED BY CALAVERAS COUNTY BUILDING DEPARTMENT PRIOR TO THEIR USE.

   A. BASE METAL THICKER THAN 1 1/2 INCHES, WHEN JOINED BY FULL OR PARTIAL PENETRATION GROOVE WELDS, SHALL BE ULTRASONICALLY INSPECTED FOR DISCONTINUITIES DIRECTLY BEHIND SUCH WELDS AFTER JOINT COMPLETION. DISCONTINUITIES SHALL BE ACCEPTED OF REJECTED ON THE BASIS OF THE DEFECT RATING IN ACCORDANCE WITH THE (LARGER REFLECTOR) CRITERIA.

4. TESTING LABORATORY TO PROVIDE CONTINUOUS INSPECTION, COMPLYING WITH CHAPTER 17 OF THE CBC, FOR THE FOLLOWING:

   A. FIELD WELDING.
   B. CONCRETE AND REINFORCING STEEL WHERE SPECIFIED CONCRETE COMPRESSIVE STRENGTH GREATER THAN 2500 PSI.
   C. BOLTS INSTALLED IN CONCRETE.
   D. INSTALLATION OF EXPANSION TYPE AND ADHESIVE TYP ANCHORS.

STRUCTURAL STEEL

1. COPROVIDE STRUCTURAL STEEL COMPLYING WITH THE FOLLOWING ASTM STANDARD SPECIFICATIONS, UNLESS NOTED OTHERWISE:

   ANCHOR BOLTS OR UNFINISHED MACHINE BOLTS.....ASTM F1554 GR. 36
   THREADED ROUND STOCK.....ASTM F1554 GR. 36

2. GALVANIZE ALL STRUCTURAL STEEL AND CONNECTIONS PERMANENTLY EXPOSED TO WEATHER.

REINFORCING STEEL

1. PROVIDE REINFORCING STEEL COMPLYING WITH ASTM A615 GRADE 60. PROVIDE REINFORCING STEEL TO BE WELDED COMPLYING WITH ASTM A706, GRADE 60.

2. LAP REINFORCING STEEL AT SPLICES TO THE FOLLOWING MINIMUM LENGTHS UNLESS NOTED OTHERWISE:

#3 AND #4.....	2'-0"	#8.....	5'-3"
#5.....	2'-1"	#9.....	6'-8"
#6.....	3'-0"	#10.....	8'-5"
#7.....	4'-1"	#11.....	10'-5"

3. SPLICE REINFORCING STEEL WHERE INDICATED. WHERE SPLICES LOCATIONS ARE NOT SPECIFICALLY INDICATED, SPLICES SHALL BE WELL STAGGERED.

4. MINIMUM CLEAR DISTANCES BETWEEN REINFORCING STEEL, INCLUDING SPLICED AREAS, SHALL BE 1" OR 1 BAR DIAMETER, WHICHEVER IS GREATER. MINIMUM CLEAR DISTANCE AT COLUMNS SHALL BE 1-1/2" OR 1-1/2 BAR DIAMETERS, WHICHEVER IS GREATER.

5. DOWELS FOR WALLS OR COLUMNS SHALL BE THE SAME SIZE AND SPACING AS WALL OR COLUMN REINFORCING STEEL AND SHALL LAP WITH WALL OR COLUMN REINFORCING STEEL AS NOTED ABOVE, UNLESS NOTED OTHERWISE.

6. ALL REINFORCING BAR BENDS SHALL BE MADE COLD.

CAST-IN-PLACE CONCRETE

1. SCHEDULE OF STRUCTURAL CONCRETE 28-DAY STRENGTHS & TYPES:

LOCATION IN STRUCTURE	STRENGTH PSI	TYPE
FOUNDATIONS	2500	HARDROCK
SLABS ON GRADE	2500	HARDROCK

2. PROVIDE PORTLAND CEMENT CONFORMING TO ASTM C150, TYPE II/V, LOW ALKALI, AND CBC STANDARD 19-1.

3. MAXIMUM WATER / CEMENT = 0.50

4. PROVIDE SILICEOUS, NORMAL WEIGHT AGGREGATES OF NATURAL SAND AND ROCK CONSISTING OF SILICA OR COMPOUNDS OTHER THAN CALCIUM OR MAGNESIUM CARBONATE. AGGREGATES TO COMPLY WITH ASTM C33 WITH PROVEN SHRINKAGE CHARACTERISTICS OF LESS THAN 0.05%.

5. SECURELY TIE ANCHOR BOLTS, REINFORCING STEEL, INSERTS, ETC. IN PLACE PRIOR TO POURING CONCRETE OR GROUT.

6. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT PLACED IN CAST-IN-PLACE CONCRETE:
   A. CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH .....3"
   B. FORMED CONCRETE EXPOSED TO EARTH OR WEATHER:
      #6 THROUGH #18 BARS..... 2"
      #5 BAR. W31 OR D31 WIRE, AND SMALLER.....1 1/2"
   C. CONCRETE NOT EXPOSED TO WEATHER OR IN CONTACT WITH GROUND:
      SLABS, WALLS AND JOISTS:
      #14 AND #18 BARS..... 1 1/2"
      #11 BAR AND SMALLER..... 1"
      BEAMS, COLUMNS AND WALL JAMBS PRIMARY REINFORCEMENT, TIES, STIRRUPS, AND SPIRALS:
      #3 THROUGH #11 BARS..... 1 1/2"
      #14 AND #18 BARS..... 2 1/2"

7. PLACE CONCRETE IN COMPLIANCE WITH ACI 301.

8. PROVIDE KEYS IN CONSTRUCTION JOINTS UNLESS DETAILED OTHERWISE. THOROUGHLY CLEAN, REMOVE ALL LAITANCE AND THOROUGHLY WET AND REMOVE STANDING WATER IN CONSTRUCTION JOINTS BEFORE PLACING NEW CONCRETE. AT VERTICAL JOINTS, SLUSH WITH A COAT OF NEAT CEMENT BEFORE PLACING NEW CONCRETE.

9. MAINTAIN CONCRETE ABOVE 50 DEGREES FAHRENHEIT AND IN A MOIST CONDITION FOR A MINIMUM OF 7 DAYS AFTER PLACEMENT UNLESS OTHERWISE APPROVED BY THE ARCHITECT.

10. SLUMP IN FLATWORK NOT TO EXCEED 4 INCHES.

11. DO NOT EMBED CONDUITS, PIPES AND SLEEVES OTHER THAN ELECTRICAL CONDUITS 1" AND SMALLER IN STRUCTURAL CONCRETE EXCEPT WHERE SPECIFICALLY DETAILED OR APPROVED BY THE ARCHITECT. LOCATE ELECTRICAL CONDUIT 3" APART MINIMUM.

12. FORM EXPOSED CORNERS OF COLUMNS, BEAMS, WALLS, ETC. WITH 3/4 INCH CHAMFERS UNLESS DETAILED OTHERWISE.

DESIGN CRITERIA

1. DESIGN IS IN ACCORDANCE WITH THE FOLLOWING CRITERIA:

   A. DEAD LOADS:
      ROOF.....10 PSF

   B. LIVE LOADS:
      ROOF.....20 PSF (REDUCIBLE)

   C. LATERAL LOADS:

      WIND LOADS:
      BASIC WIND SPEEDS.....92 MPH
      IMPORTANCE FACTOR.....1.0
      EXPOSURE.....C
      INTERNAL PRESSURE COFFICIENT.....0.18

      SEISMIC LOADS:
      IMPORTANCE FACTOR.....1.0
      SS.....1.5
      S1.....0.6
      SITE CLASS.....D - DEFAULT
      SDS.....1.2
      DESIGN CATEGORY.....D

      SYSTEM.....LIGHT FRAMED WALL
      BASE SHEAR.....4.8K (ULT)
      RESPONSE COEFFICIENT.....0.185W (ULT)
      .....0.130W (ASD)
      RESPONSE MOD FACT.....6.5
      ANALYSIS.....LINEAR STATIC

FOUNDATIONS

1. THE FOUNDATION DESIGN IS BASED ON CBC TABLE 1806.2.

2. FOUNDATION DESIGN IS BASED ON AN IMPOSE DEAD PLUS LIVE LOAD BEARING CAPACITY OF 1500 PSF AND A ONE-THIRD INCREASE FOR LOAD COMBINATIONS INCLUDING WIND OR SEISMIC TO 2000 PSF.

3. FOUND FOOTINGS INTO COMPETENT SOIL WITH SIMILAR PHYSICAL CHARACTERISTICS AND DISPOSITION. DOES NOT INCLUDE MUD, ORGANIC SILT, ORGANIC CLAYS, PEAT OR UNPREPARED FILL.

4. ALL ABANDONED FOOTINGS, UTILITIES, ETC., THAT INTERFERE WITH NEW CONSTRUCTION SHALL BE REMOVED.

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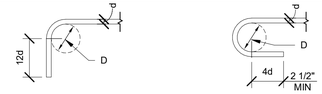
GENERAL NOTES, SHEET INDEX & ABBREVIATIONS

DOCUMENT REVIEW

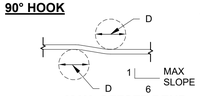
DESIGN ENGINEER	PROJECT DRAFTSMAN
JSK	RDB

PROJECT NO. B24-239

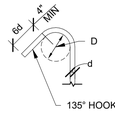
SHEET NO. S0.2



90° HOOK 180° HOOK



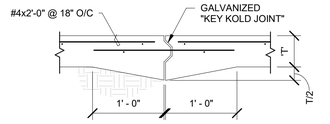
MAX. OFFSET BEND PRINCIPAL REINFORCING



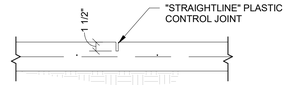
NOTES:
1. SEE ACI-318 FOR BEND DIAMETER "D".
2. ALL BENDS SHALL BE MADE COLD.

TIE OR STIRRUP

TYPICAL BAR BENDS DETAIL	1" = 1'-0"	8

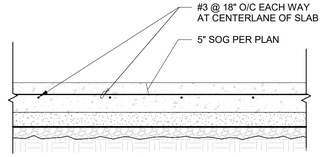


(A) POUR JOINT

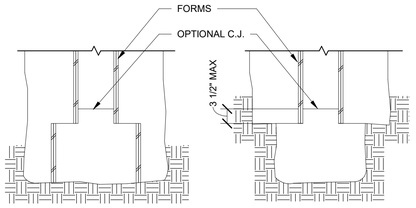


(B) CONTROL JOINT

TYPICAL JOINTS IN SLAB-ON-GRADE	1" = 1'-0"	4



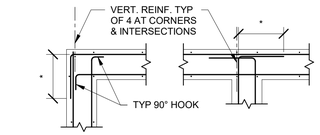
TYPICAL SLAB-ON-GRADE	1" = 1'-0"	11



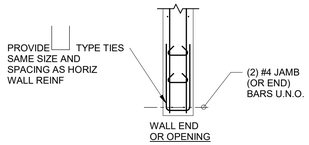
FORMED FTG. FTG POURED AGAINST UNDISTURBED SOIL

NOTE:
FOOTING AND FOUNDATION WALLS MAY BE POURED MONOLITHICALLY PROVIDED THE SIDES OF THE FOOTING ARE FORMED AND THE EXCAVATION IS LARGE ENOUGH TO PROPERLY CLEAN THE EXCAVATION AND TIE THE STEEL. A CONSTRUCTION JOINT IS MANDATORY IF THE SIDES OF THE FOOTING ARE NOT FORMED.

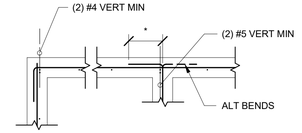
TYPICAL FOOTING DETAIL	1" = 1'-0"	10



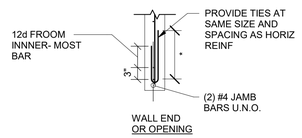
CORNER INTERSECTION



DOUBLE LAYER
(PLAN VIEW)



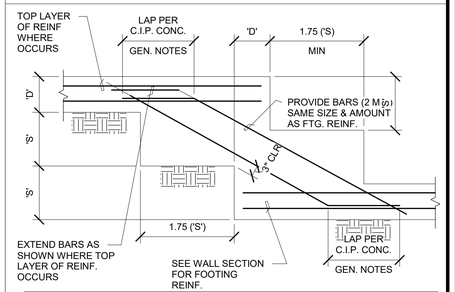
CORNER INTERSECTION



SINGLE LAYER
(PLAN VIEW)

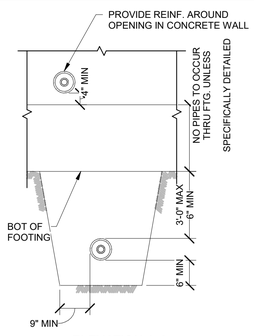
NOTE:
* LAP 48d U.O.N.

TYP CONC WALL REINF AT INTERSECTION AND CORNER	1" = 1'-0"	6

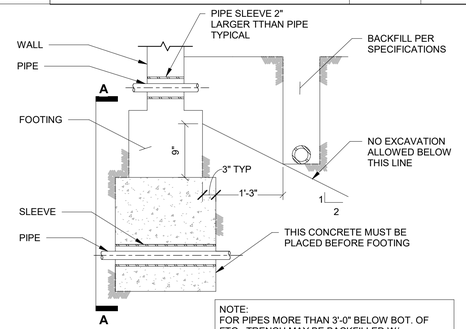


NOTES:
1. S INDICATES STEPPED FTG. SYMBOL, SEE PLAN FOR LOCATION
2. DIMENSION 'S' SHALL NOT EXCEED 2'-0"
3. 'D' INDICATES DEPTH OF FOOTING

TYP STEPPED FOOTING DETAIL	1" = 1'-0"	2

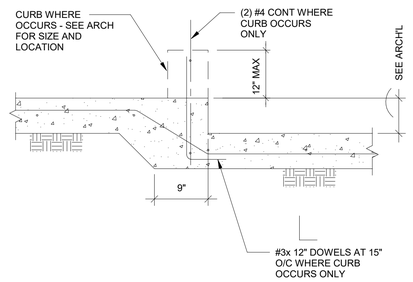


ELEVATION A-A

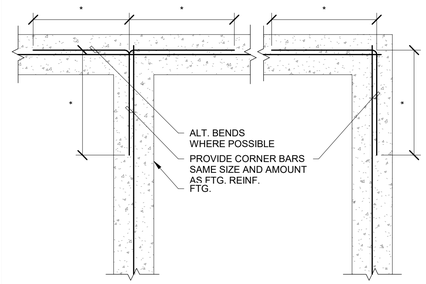


NOTE:
FOR PIPES MORE THAN 3'-0" BELOW BOT. OF FTG., TRENCH MAY BE BACKFILLED W/ COMPACTED FILL PER SPECIFICATIONS - DO NOT TRENCH BELOW FTG. AFTER FTG. IS POURED.

TYPICAL PIPE THRU FOUNDATION AND TRENCH DETAIL	1" = 1'-0"	9



TYP STEPPED SLAB	3/4" = 1'-0"	5



PLAN VIEW

NOTE:
* LAP 48d

TYP FTG REINF AT INTERSECTIONS	1" = 1'-0"	1

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NOTE:
DRAWINGS ARE NOT TO SCALE IF PRINTED OUT AT A SIZE OTHER 24X36

15225 OAK GLEN AVE - STORAGE SHED

CUSTOM COLD-FORMED STEEL SHED

15225 OAK GLEN AVENUE, MORGAN HILL, CA 95037

REGISTERED PROFESSIONAL ENGINEER THOMAS A. CASTLE No. 4093 STRUCTURAL STATE OF CALIFORNIA

REV	DATE	DESCRIPTION

DRAWN BY RDB

1ST ISSUE DATE 12/17/2024

SHEET TITLE

TYP CONCRETE DETAILS

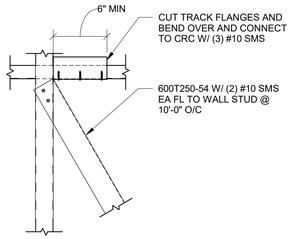
DOCUMENT REVIEW	
DESIGN ENGINEER	PROJECT DRAFTSMAN
JSK	RDB

PROJECT NO.

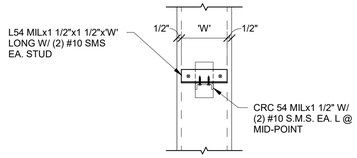
B24-239

SHEET NO.

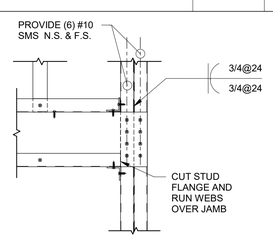
S1.1



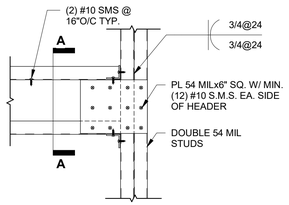
A **BRIDGING BRACE AT AXIALLY LOADED STUDS**



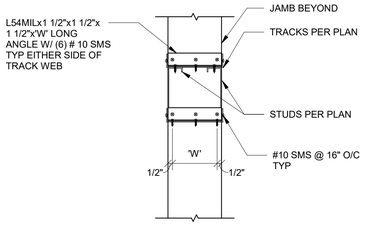
TYPICAL BRIDGING DETAIL	1 1/2" = 1'-0"	19



ALTERNATE

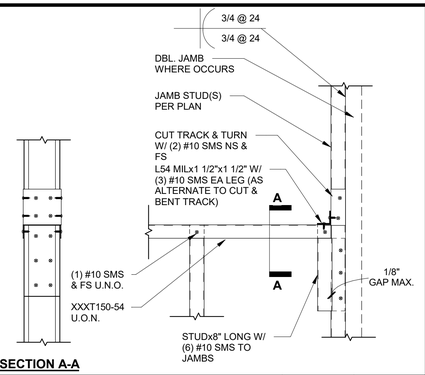


ELEV. VIEW



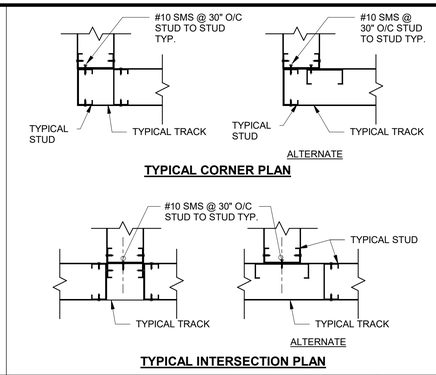
SECTION A-A

BOX HEADER DETAIL	1 1/2" = 1'-0"	17



SECTION A-A

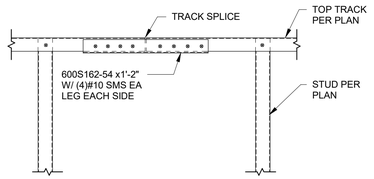
TRACK SILL DETAIL	1 1/2" = 1'-0"	16



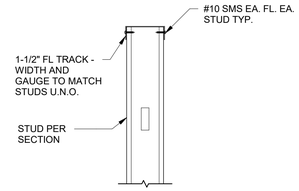
TYPICAL CORNER PLAN

TYPICAL INTERSECTION PLAN

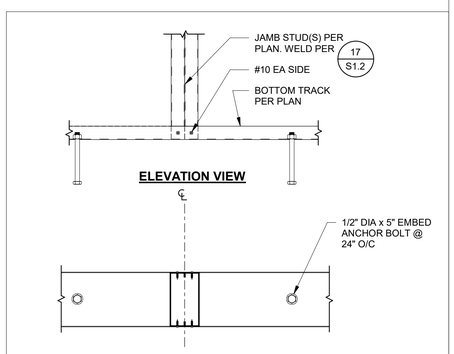
CORNER & INTERSECTION FRAMING	1 1/2" = 1'-0"	12



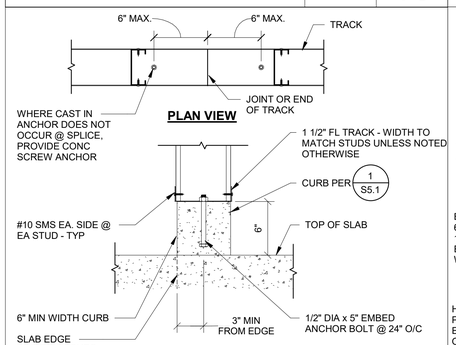
TRACK SPLICE DETAIL	1 1/2" = 1'-0"	8



TRACK CLOSURE DETAIL	1 1/2" = 1'-0"	4

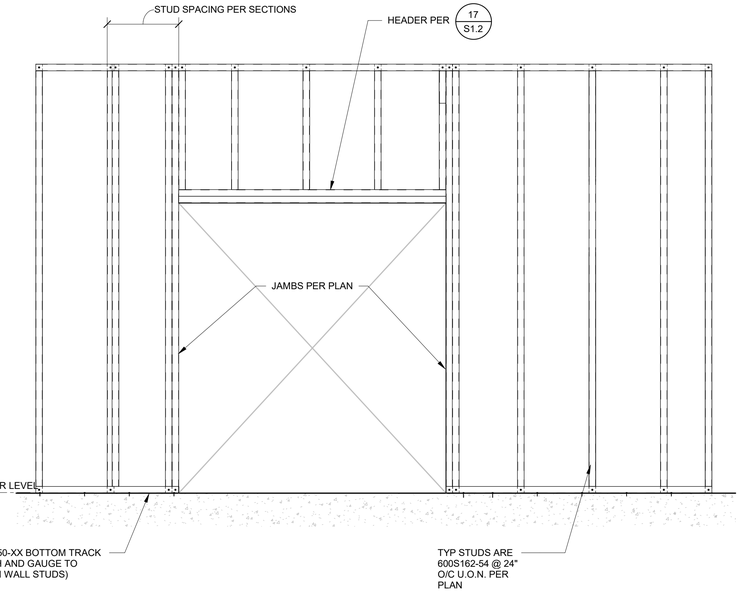


JAMB CONN AT SLAB	1 1/2" = 1'-0"	10

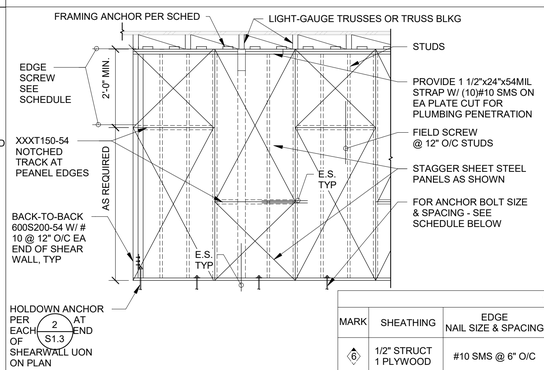


WALL ON SLAB OR CURB 6" OR WIDER

EXTERIOR STUD WALL BASE DETAIL	1 1/2" = 1'-0"	9



TYP BEARING AND EXTERIOR WALL FRAMING ELEVATION	3/4" = 1'-0"	2



NOTES:
1. ALL PANEL EDGES SHALL BE BLOCKED
2. ALL SCREWS SHALL BE #10 SHEET METAL SCREWS
3. MIN CONC EDGE DISTANCE FOR SCREW ANCHORS AT SHEAR WALLS TO BE 2 1/2".

SHEARWALL SCHEDULE

MARK	SHEATHING	EDGE NAIL SIZE & SPACING	SHEAR TRANSFER	SCREW ANCHOR SIZE & SPACING	ALLOWABLE SHEAR LOAD
6	1/2" STRUCT 1 PLYWOOD	#10 SMS @ 6" O/C	#10 @ 16" O/C TO BLOCKING	1/2"ø x 5" EMBED ANCHOR BOLT OR CONC SCREW ANCHOR @ 24" O/C	356 PLF

TYP SHEAR WALL SCHEDULE	3/4" = 1'-0"	1

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TYP FRAMING DETAILS

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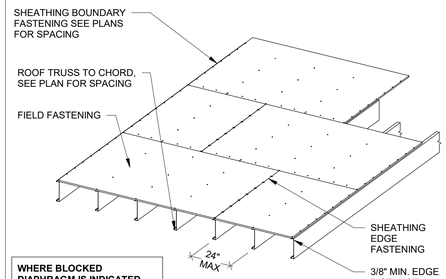


WHERE BLOCKED DIAPHRAGM IS INDICATED, USE 4" WIDE x 43MIL FLAT STRAP OR 400T150-43 NOTCHED TRACK BLOCKING W/ (2) #10 SMS TO EA JOIST

NOTE:
1. SHEATHING FASTENER SHALL BE #10 SMS SEE PLAN FOR SPACING

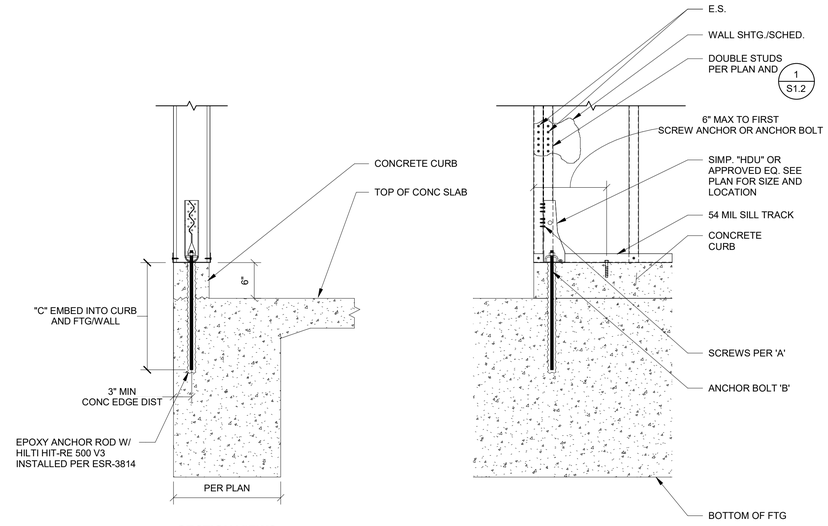
TYPICAL ROOF SHEATHING DETAIL	3/8" = 1'-0"	8

CONNECTION:	SCREWING: 1, 2, 3
1. JOIST TO TRACK	(2) #10
2. BRIDGING TO JOIST, CLIPS EA. END	(3) #10
3. 1" x 6" SUBFLOOR OR LESS TO EACH JOIST, FACE SCREW	(2) #10
4. WIDER THAN 1" x 6" SUBFLOOR TO EACH JOIST, FACE SCREW	(3) #10
5. 2" SUBFLOOR TO JOIST OR BEAM, BLIND AND FACE SCREW	(2) #10
6. BOTTOM TRACK TO JOIST OR BLOCKING, FACE SCREW	#10 @ 16" O/C
7. TOP & BOT TRACK TO EA STUD	#10 EA SIDE
8. DOUBLE STUDS, FACE SCREW	(2) ROWS #10 @ 12" O/C 12" O.C.
9. TOP TRACK @ INTERSECTIONS, FACE SCREW	(4) #10
10. TOP TRACK @ LAPS, FACE SCREW	(6) #10
11. CEILING JOISTS TO PLATE	(2) #10
12. CEILING JOISTS, LAPS OVER PARTITIONS, FACE SCREWS	(2) #10
13. CEILING JOISTS TO PARALLEL RAFTERS, FACE SCREWS	(2) #10
14. RAFTER TO TRACK	(2) #10
15. 1" x 8" SHEATHING OR LESS TO EACH BEARING, FACE SCREW	(2) #10
16. WIDER THAN 1" x 8" SHEATHING TO EACH BEARING, FACE SCREW	(3) #10
17. BUILT UP CORNER STUDS	#10 @ 12" O.C.
18. BUILT UP BEAMS	#10 @ 12" O.C. @ T&B, EA SIDE

FOOTNOTES:
1. THIS SCHEDULE SHALL BE FOLLOWED UON ON PLAN
2. IT IS ALWAYS ACCEPTABLE TO USE LARGER SCREWS THAN SPECIFIED
3. SEE S0.1 FOR FASTENER DESCRIPTIONS & INFORMATION

LIGHT GAUGE METAL SCREWING SCHEDULE	3/4" = 1'-0"	4

HOLDOWN SCHEDULE			
HOLDOWN	"A" STUD BOLTS	"B" ANCHOR ø	"C" EMBED
S/HDU6	(12) #14 SCREWS	5/8" ø ASTM F1554 GR36 THREADED ROD	18"



SECTION VIEWS

SHEAR WALL HOLDOWN	NTS	2

BACKING PLATE SCHEDULE		
TYPE	LOAD DIAGRAM / SECTION	PLAN DETAIL
1	150# / 500#	STUD WEB; (5) #10 SMS EA STUD; 33 MIL STUD MIN. (3 5/8" MIN. DEPTH AND 1 5/8" MIN. FLANGE); BACKING PLATE 600T150-54
2	10# / 100#	STUD WEB; (3) #10 SMS EA STUD; 33 MIL STUD MIN.; SHEET METAL BACKING PLATE 54 MILx6"

NOTES:
1. FOR LOCATION AND TYPE OF BACKING PLATE, SEE ARCH'L DRAWINGS.
2. EXTEND BACKING PLATE TO NEXT STUD BEYOND SIDE OF FIXTURE OR ACCESORY.

BACKING PLATE DETAIL	1 1/2" = 1'-0"	1

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TYP FRAMING DETAILS

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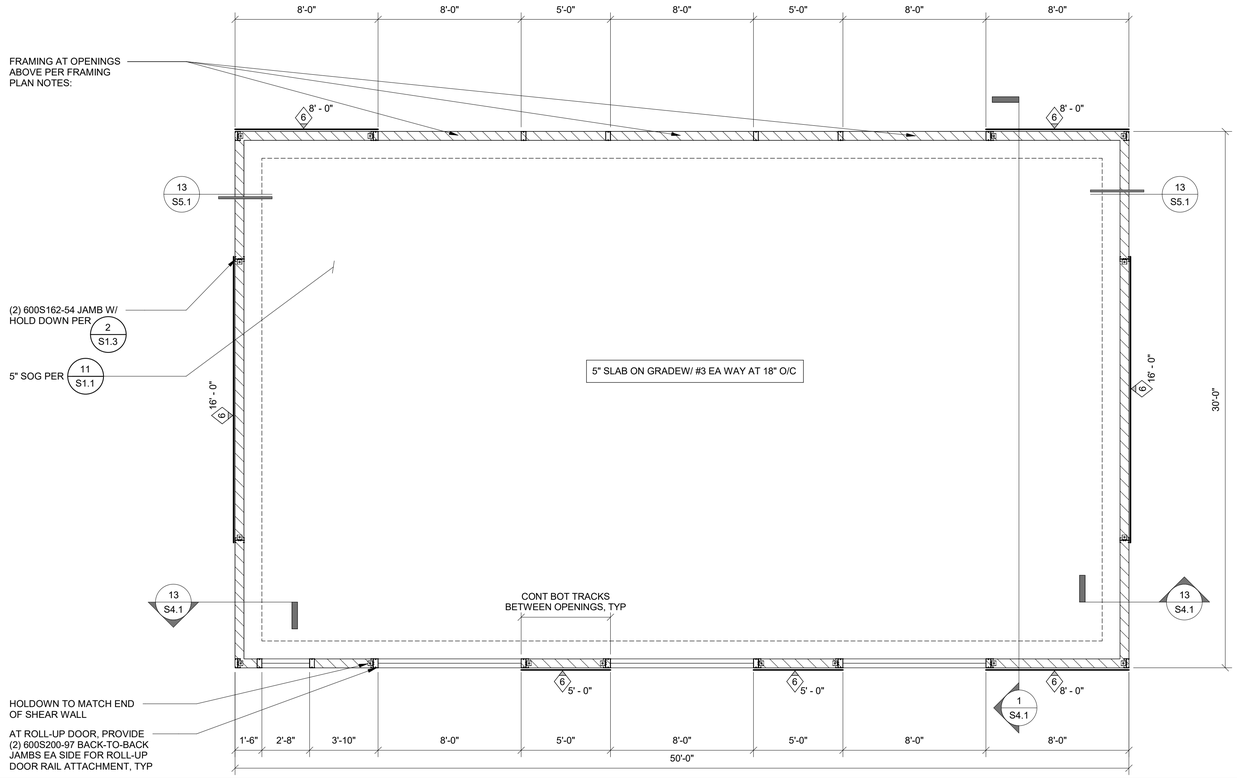
JSK	RDB

PROJECT NO.

B24-239

SHEET NO.

S1.3



FOUNDATION AND FLOOR FRAMING PLAN

1/4" = 1'-0"

1

FRAMING PLAN NOTES:

1. X INDICATES SHEAR WALL PER 1 S1.2 W/ HOLD DOWN AT EACH END AT FOUNDATION PER 2 S1.3
2. ALL EXTERIOR WALLS TO BE 600S162-54 @ 16" O/C U.O.N. W/ BRIDGING @ MID-HEIGHT PER 19 S1.2
3. INDICATES BEARING OR SHEAR WALL BELOW
4. INDICATES BEARING OR SHEAR WALL
5. PROVIDE ANCHOR BOLT OR SCREW ANCOR @ 24" O/C MIN. @ EXTERIOR WALLS, PER 9 S1.2
6. PROVIDE A.B. PER SCHEDULE 1 S1.2 AT INDICATED SHEAR WALLS
7. FOR FOUNDATION NOTES, REFER TO THIS SHEET.
8. INDICATES JOIST EXTENT
   INDICATES JOIST DIRECTION
9. TYPICAL HEADER TO BE (2) 800S300-54 + (2) 600T150-54 PER 17 S1.2 UON
10. TYPICAL 600S162-54 10 S1.2 JAMB AT WINDOWS/ DOORS AND SUPPORTING BEAMS TO BE (2) PER UON
11. TYPICAL SILL TO BE 600T150-54 PER 16 S1.2 UON
12. FIRST PUNCHOUT IS TYPICALLY 18" FROM PERPENDICULAR BEARING WALL.

FOUNDATION PLAN NOTES

1. REFER TO GENERAL NOTES AND TYPICAL DETAILS ON S1 SERIES SHEETS.
2. INTERIOR NONBEARING WALLS CONN TO FLOOR PER 9 S1.2
3. EXTERIOR BEARING WALLS SHALL HAVE ANCHOR BOLT OR SCREW ANCHORS @ 16" O/C U.N.O. REFER TO DETAIL 9 S1.2
4. SEE ARCHITECTURAL DRAWINGS PLANS FOR SLAB ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS and CURBS, DRAINS, TRENCHES, SLAB EDGE LOCATIONS, ETC. AND FOR ALL WALL OVERALL DIMENSIONS, LOCATION OF OPENINGS, ETC.
5. X INDICATES SHEARWALL WITH A.B. PER DETAIL 1 S1.2
6. SEE WALL FRAMING NOTES ON THIS SHEET.

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FOUNDATION PLAN

DOCUMENT REVIEW	
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JSK	RDB

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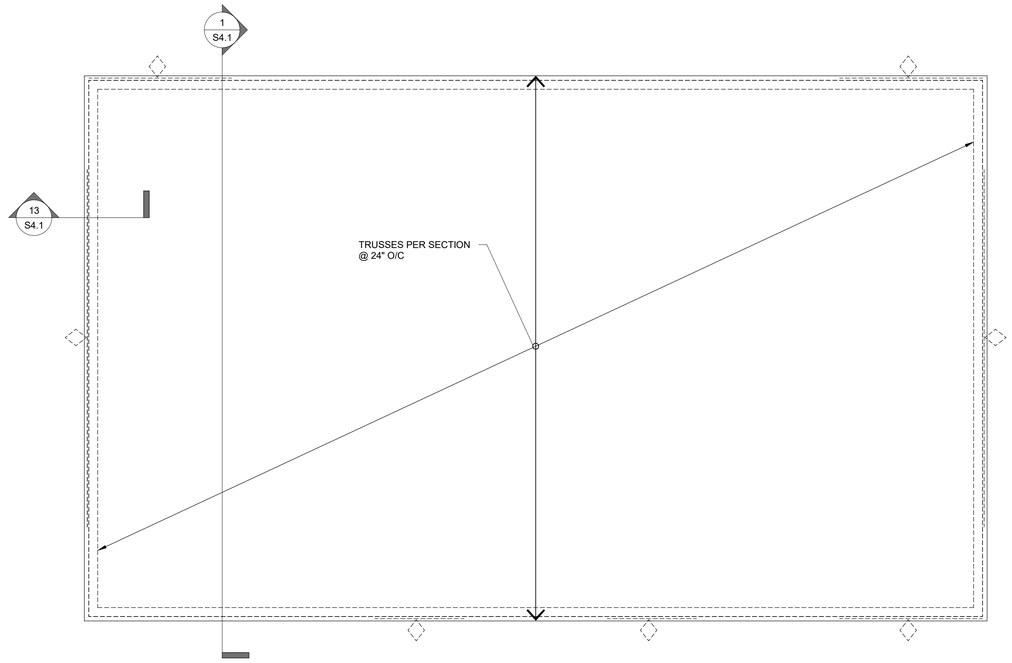
SHEET NO.

S2.1



ROOF PLAN

1/4" = 1'-0"

1

ROOF FRAMING NOTES:

1. REFER TO (1/S2.1) FOR FOUNDATION AND FRAMING NOTES.
2. ROOF DIAPHRAGM TO BE 5/8" THICK PLYWOOD W/ #10 SMS @ 6" O/C EDGE AND 12" O/C FIELD PER (8/S1.3).
3. SHEATHING TO RUN PERPENDICULAR TO TRUSSES.

FICCADENTI WAGGONER and CASTLE
Structural Engineers
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Telephone: (925) 280-0098
www.fwcse.com

NOTE:
DRAWINGS ARE NOT TO SCALE IF PRINTED OUT AT A SIZE OTHER 24X36

15225 OAK GLEN AVE - STORAGE SHED

CUSTOM COLD-FORMED STEEL SHED

15225 OAK GLEN AVENUE, MORGAN HILL, CA 95037

REGISTERED PROFESSIONAL ENGINEER
THOMAS A. CASTLE
No. 4083
STRUCTURAL
STATE OF CALIFORNIA

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ROOF PLAN

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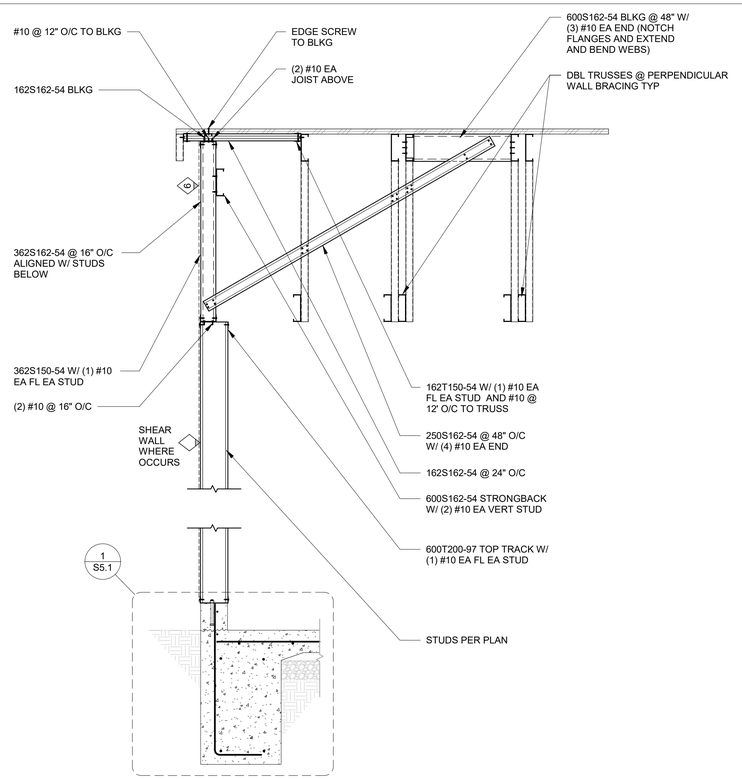
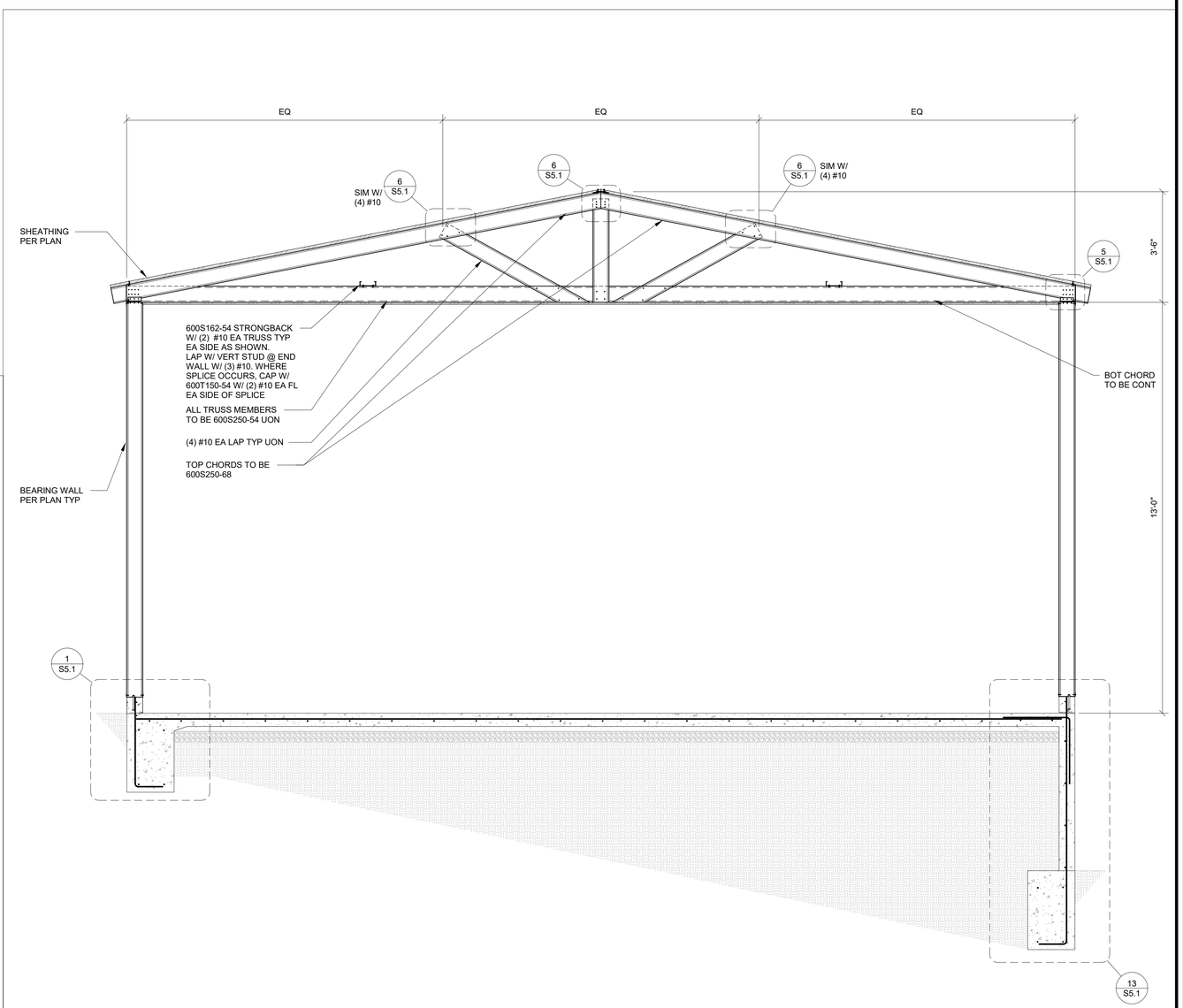
S2.2



SECTION AT END WALL OF GARAGE | 3/4" = 1'-0" | 13



MAIN SECTION THROUGH HOUSE | 1/2" = 1'-0" | 1

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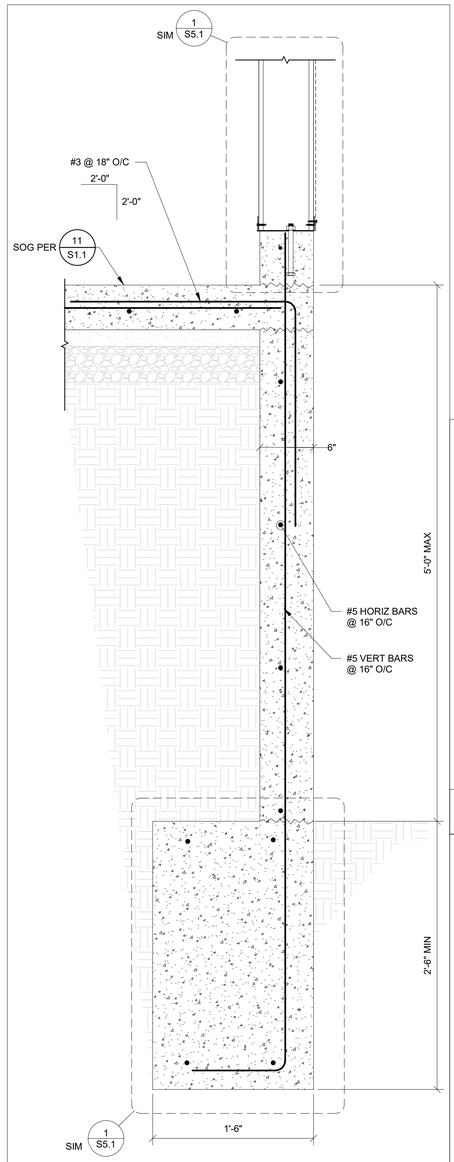
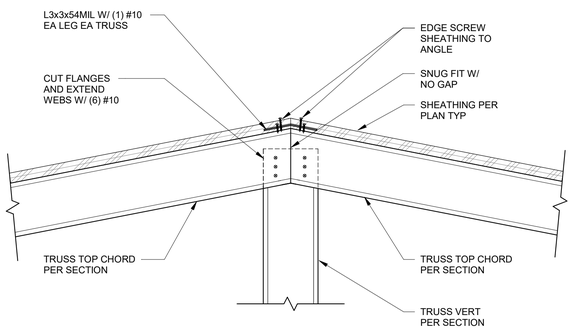
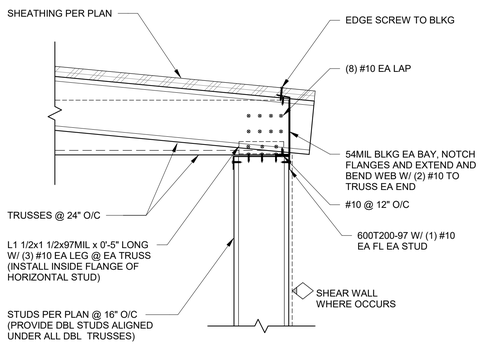
S4.1



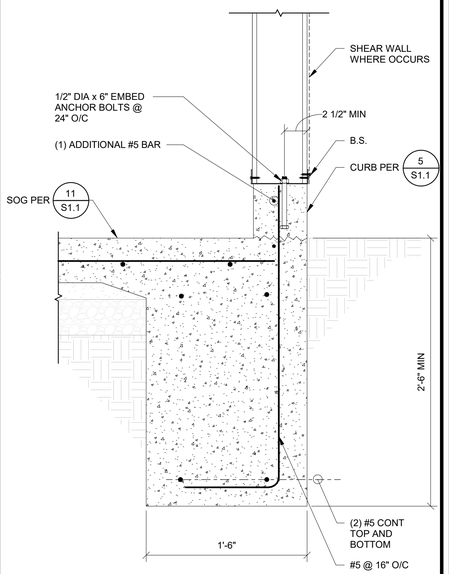
TYPICAL FOUNDATION | 1 1/2" = 1'-0" | 13



TRUSS JOINT DETAIL @ RIDGE | 1 1/2" = 1'-0" | 6



TYP EAVE DETAIL | 1 1/2" = 1'-0" | 5



TYPICAL FOUNDATION | 1 1/2" = 1'-0" | 1

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DETAILS

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SHEET NO.

S5.1